WINSTON CHURCHILL BLVD

DEER RUN AVE.

BUILDING C
WAREHOUSING
30,082.31 m2
323,803 sf
F.F.E = 96.00
FFE = 96.00

BUILDING A
WAREHOUSING
16,205.91 m2
174,439 sf
F.F.E = 94.90
FFE = 94.90

568 WCB
ASSUMED FFE
95.00

OFFICE
B.F. ENTRANCE

ROOF DRAINS TO BE ZURN Z-105-5-ERC WITH A TOTAL OF 33 NOTCHES DISTRIBUTED BETWEEN THE DRAINS. EACH NOTCH TO BE RATED AT 104.5 l/min. TOTAL ROOF FLOW = 57.5 l/s

ROOF DRAINS TO BE ZURN Z-105-5-ERC WITH A TOTAL OF 18 NOTCHES DISTRIBUTED BETWEEN THE DRAINS. EACH NOTCH TO BE RATED AT 104.5 l/min. TOTAL ROOF FLOW = 31.4 l/s

REFUSE STORAGE BINS TO BE STORED INTERNALLY AND REMOVED OFF-SITE BY PRIVATE CONTRACTORS.

EXISTING DRIVEWAYS TO BE RESTORED TO EXISTING CONDITION OR BETTER (TYP.)

MATCH EXISTING GRADES AT PROPERTY LINE

SEDIMENT FOREBAY

ACCESS ROAD

STORMWATER MANAGEMENT FACILITY
POND BOTTOM=90.00m
PERMANENT HWL=91.10m
PERMANENT VOLUME=3,045.1m³
EROSION CONTROL HWL=91.90
EROSION CONTROL VOLUME=3,146.2m³
100yr HWL=92.68m
100yr STORAGE VOLUME=7,211.0m³
REGIONAL HWL=93.14m

SEE DWG. G-4

DRIVE-IN RAMP (6.8m X 18.3m TYP.)

20.0m WIDE X 0.15m DEPTH CURB CUT-IN SCREEN WALL

4.0m HIGH SCREEN WALL

0.5m HIGH CURB T/C 94.30

0.70% SWALE WITH T/C BOTTOM, 0.50% DEPTH AND 3:1 SIDE SLOPES

10.5m WIDE X 0.30m DEPTH CURB CUT-IN T/C 94.30

REFER TO PLAN C-1 FOR SECTIONS.

ALL MH 1200Ø UNLESS OTHERWISE NOTED.

OVERLAND SPILLWAY – Tributary Area = 11.31ha

$Q_{100YR} = CAIN$
$= (0.90)(11.31ha)(200.8mm/hr)2.778$
$= 5,678.1\ l/s$

$Q_{SYR} = CAIN$
$= (0.90)(11.31ha)(114.2mm/hr)2.778$
$= 3,229.5\ l/s$

$Q_{REMAINING} = (Q_{100YR} - Q_{5YR})$
$= (5,678.1\ l/s - 3,229.5\ l/s)$
$= 2,448.6\ l/s$

$Q_{WEIR} = CLH^{3/2}$
$= (1.7)(10.5m\ WIDE)(0.30m)^{3/2}$
$= 2,933.1\ l/s$

OVERLAND SPILLWAY CURB CUT DETAIL
SCALE: 1:100

SECTION A–A
SCALE 1:150

THE PAVEMENT STRUCTURE SHALL BE CONSTRUCTED OF THE FOLLOWING MATERIALS, AS STATED IN THE TOWN OF OAKVILLE STANDARD DRAWING 7-2 AND 7-20:

INDUSTRIAL AND COLLECTOR ROADS OR BUS ROUTES
40mm HL3
80mm HL8
150mm OF GRANULAR 'A' (x)
350mm OF GRANULAR 'B' (RW3)

A. GRANULAR 'A' TO MEET OR EXCEED OPSS 1010 AND SHALL BE QUARRIED BEDROCK OR RECYCLED CONCRETE MATERIAL. 30% RAP MAY BE PRESENT IN GRANULAR 'A' MATERIAL. STEEL SLAG NOT PERMITTED IN GRANULAR 'A'.

B. GRANULAR 'B' TYPE II SHALL BE 100% QUARRIED BEDROCK MEETING OR EXCEEDING OPSS 1010. GRANULAR 'B' TYPE I COMPRISED OF 100% RECYCLED CONCRETE MEETING OR EXCEEDING OPSS 1010 MAY BE USED IN LIEU OF GRANULAR 'B' TYPE II, NEITHER FURNACE SLAG NOR NICKEL SLAG IS PERMITTED FOR USE IN GRANULAR 'B' TYPE II OR TYPE I MATERIAL.

C. AN EXTRA 150mm THICKNESS SHALL BE PLACED WITHIN ARTERIAL AND INDUSTRIAL ROAD INTERSECTIONS. THE EXTRA THICKNESS SHALL EXTEND FOR A MINIMUM OF 15m BEYOND THE PROPERTY LINE OF THE INTERSECTING STREET.

OR AS SPECIFIED IN THE GEOTECHNICAL REPORT PREPARED BY A SOILS ENGINEER, WHICHEVER IS GREATER.

SECTION C1–C1: CLEARVIEW CREEK CHANNEL EASEMENT (TYPICAL)
SCALE 1:75

GENERAL NOTES:
1. THE ELEVATION AT STREET LIE FOR LOT DRAINAGE SHALL BE 150mm HIGHER THAN THE FINISHED ROAD CROWN.
2. ALL MATERIALS SHALL MEET OR EXCEED ONTARIO PROVINCIAL STANDARD AND TOWN STANDARD SPECIFICATIONS.
3. WATERMAINS AND SANITARY SEWERS TO CONFORM TO LATEST REGIONAL MUNICIPALITY OF HALTON SPECIFICATIONS AND REQUIREMENTS.

DRIVEWAYS
4. RESIDENTIAL DRIVEWAY APRONS BETWEEN THE SIDEWALK AND THE ROAD CURB SHALL BE CONSTRUCTED WITH 180mm CONCRETE AND 75mm OF 19mm CRUSHER RUN LIMESTONE.
5. ALL DRIVEWAYS BETWEEN CURB AND SIDEWALK PLAIN CONCRETE.

STREET SECTION DETAILS
6. MAXIMUM ROADWAY PROFILE GRADIENT SHALL BE 6% MINIMUM ROADWAY PROFILE GRADIENT SHALL BE 0.5%
7. SUB-GRADE SHALL HAVE A CROSS-FALL OF -3% FROM CENTRELINE OF ROAD.
8. ALL GRANULARS TO ASPHALT LAYERS SHALL HAVE A CROSS-FALL OF -3%.
9. TOP COURSE ASPHALTIC CONCRETE SHALL BE PLACED ONLY AFTER ADJACENT BUILDINGS (HOMES, INDUSTRIAL, COMMERCIAL, ETC.) HAVE BEEN CONSTRUCTED AND ONLY WITH THE APPROVAL OF THE DIRECTOR OF ENGINEERING AND CONSTRUCTION.
10. FULL LENGTH SUBDRAINS ARE TO BE INSTALLED ON BOTH SIDES OF THE ROAD.
11. CONCRETE CURB AND GUTTER SHALL BE OPSD 600.040 FOR LOCAL ROADS (STD. 7-21A AND 7-22B (SANDWAYS).
12. CONCRETE CURBS SHALL BE OPSD 600.040 FOR ALL STANDARD STREET SECTIONS EXCEPT STD. 7-21A AND 7-22B (SANDWAYS).
13. EASTBEND BOULEVARD AREAS SHALL BE OVERLAIN WITH 200mm TOPSOIL AND NO. 1 NURSEY SOD PER PARKS AND OPEN SPACE DEPARTMENT SPECIFICATIONS.
14. SIDEWALKS SHALL CONFORM TO OPSD. 310.030
15. SIDEWALK RAMPS AT INTERSECTIONS AND MID-BLOCK CROSSING SHALL CONFORM TO OPSD 310.030 WITH THE REQUIREMENT THAT THE RAMP GRADIENT SHALL NOT EXCEED 5%

ASPHALT AND GRANULAR REQUIREMENTS:
A. LANEWAYS AND PRIVATE ROADS (NOT SUPPORTING TRANSIT)
- 40mm HL3 (PGAC 58–28) ***
- 50mm HL8 (PGAC 58–28) *
- 150mm GRANULAR 'A' *** (x)
- 350mm GRANULAR 'B' *** (x)

B. LOCAL EMPLOYMENT ROADS (NOT SUPPORTING TRANSIT)
- 40mm HL3 (PGAC 58–28) *
- 50mm HL8 (PGAC 58–28) *
- 150mm GRANULAR 'A' *** (x)
- 50mm GRANULAR 'B' *** (x) (x)

C. CONNECTOR ROADS AND LOCAL ROADS (SUPPORTING TRANSIT)
- 40mm HL3 HS (PGAC 64–22) *
- 50mm HDBC (PGAC 64–22) *
- 150mm GRANULAR 'A' *** (x)
- 200mm GRANULAR 'B' *** (x)(x)

D. AVENUES AND MINOR ARTERIAL ROADS
- 40mm HL3 HS (PGAC 64–22) *
- 50mm HDBC (PGAC 64–22) *
- 150mm GRANULAR 'A' *** (x)
- 350mm GRANULAR 'B' *** (x)(x)

* – ALL HOT LAID ASPHALTIC CONCRETE SHALL MEET OR EXCEED OPSS 1150.
** – WHERE THE PROJECT OR DEVELOPMENT GEOTECHNICAL REPORT SPECIFIES A GREATER THICKNESS, THAT SPECIFIED THICKNESS SHALL BE THE REQUIREMENT.
*** – ALL GRANULAR A AND GRANULAR B MATERIALS – SCC TESTING SHALL BE UNDERTAKEN BY THE CONTRACTOR COMPLETING THE WORKS AS PER RELEVANT OPS SPECIFICATIONS. ALL PROJECT ADMINISTRATORS AND DEVELOPERS ARE REQUIRED TO PERFORM QA TESTING PER RELEVANT OPS SPECIFICATIONS AND THE RESULTS OF THE QA TESTING WILL DETERMINE THE ACCEPTANCE OR REJECTION OF PLACED MATERIALS.

(a) – AN EXTRA 150mm THICKNESS SHALL BE PLACED WITHIN ARTERIAL AND INDUSTRIAL ROAD INTERSECTIONS. THE EXTRA THICKNESS SHALL EXTEND FOR A MINIMUM OF 15m BEYOND THE PROPERTY LINE OF THE INTERSECTING STREET.
(b) – GRANULAR 'A' TO MEET OR EXCEED OPS 1010 AND SHALL BE QUARRIED BEDROCK OR RECYCLED CONCRETE MATERIAL. 30% RAP MAY BE PRESENT IN GRANULAR 'A' MATERIAL. STEEL SLAG NOT PERMITTED IN GRANULAR 'A'.
(c) – GRANULAR 'B' TYPE II SHALL BE 100% QUARRIED BEDROCK MEETING OR EXCEEDING OPSS 1010. GRANULAR 'B' TYPE I COMPRISED OF 100% RECYCLED CONCRETE MEETING OR EXCEEDING OPSS 1010 MAY BE USED IN LIEU OF GRANULAR 'B' TYPE II. NEITHER FURNACE SLAG NOR NICKEL SLAG IS PERMITTED FOR USE IN GRANULAR 'B' TYPE II OR TYPE I MATERIAL.

REGION OF HALTON NOTES

GENERAL NOTES
1. THE LOCATION OF ALL UNDERGROUND AND ABOVEGROUND UTILITIES AND STRUCTURES IS NOT NECESSARILY SHOWN ON THESE DRAWINGS, AND, WHERE SHOWN, THE ACCURACY OF THE LOCATION IS NOT GUARANTEED. THE CONTRACTOR SHALL BE RESPONSIBLE FOR DETERMINING EXACTLY THE CONTRACTOR SHALL DETERMINE THE EXACT LOCATION OF ALL SUCH UTILITIES AND STRUCTURES, AND SHALL ASSUME ALL LIABILITY FOR DAMAGE TO THEM.
2. ALL AREAS DISTURBED BY THE CONTRACTOR DURING THE CONSTRUCTION OF THE WORKS SHOWN HEREIN SHALL BE RESTORED TO ORIGINAL CONDITION OR BETTER AS DETERMINED BY PLANNING AND ECONOMIC DEVELOPMENT STAFF. ALL DISTURBED AREAS SHALL BE RESTORED TO GRADE BY PLACING TOPSOIL AND NO. 1 NURSERY SOD TO ESTABLISH A GRASS COVER TO THE SATISFACTION OF THE CITY/TOWN UNLESS NOTED OTHERWISE.
3. CONSTRUCTION, MATERIAL AND WORKMANSHIP STANDARDS DRAWINGS AND O.P.S.D. WITH REGIONAL SUPPLEMENTARY SPECIFICATIONS AND STANDARDS SHALL CONSTITUTE PART OF THE ENGINEERING DESIGN AND CONSTRUCTION CONTRACT.
4. ALTERNATIVE MATERIALS MAY BE ACCEPTABLE, PROVIDED APPROVAL HAS FIRST BEEN OBTAINED FROM THE TOWN ENGINEER AND/OR THE REGIONAL COMMISSIONER OF PLANNING AND PUBLIC WORKS, AS THE CASE MAY REQUIRE.
5. ANY AREAS WITHIN R.O.W. WHICH REQUIRE FILL IN EXCESS OF 0.30M ARE SUBJECT TO COMPACTION TESTS AND SUCH TESTS MUST SHOW A MIN. COMPACTION OF 95% S.P.D. AT ALL DEPTHS.
6. MANHOLE AND VALVE CHAMBER COVERS ARE TO BE SET FLUSH WITH BASE COURSE ASPHALT AND ADJUSTED TO FINAL GRADE PRIOR TO INSTALLING TOP LIFT OF ASPHALT.
7. ALL WORKS WITHIN THE REGIONAL RIGHT-OF-WAY ARE TO BE BACKFILLED IN ACCORDANCE WITH TOWN OF OAKVILLE REQUIREMENTS.

SANITARY SEWER NOTES
1. SANITARY MANHOLES AS PER O.P.S.D. 701.010 WITH FRAMES AND COVERS AS PER O.P.S.D. 401.01 TYPE 'A' UNLESS OTHERWISE NOTED ON THE DRAWINGS.
2. BENCHING IN MANHOLES TO BE AS PER O.P.S.D. 701.021 AS AMENDED BY THE REGION OF HALTON. BENCHING IN SANITARY MANHOLES TO BE TO THE CROWN OF THE PIPE.
3. DROP PIPES AS PER O.P.S.D. 701.021 SHALL BE USED ONLY IN MANHOLES WHERE DEPTH EXCEED 5.0M AS DIRECTED BY THE REGION AND AS INDICATED ON THE PROFILE DRAWINGS.
4. SANITARY SEWER PIPE SHALL BE PVC SDR35 CONFORMING TO A.S.T.M. D3034 UNLESS OTHERWISE NOTED.
5. ALL SANITARY SERVICE CONNECTIONS TO BE 125MM DIA FOR SINGLE RESIDENTIAL CONNECTIONS AND 150MM DIA FOR DUAL RESIDENTIAL AND SINGLE NON-RESIDENTIAL CONNECTIONS. SANITARY SERVICE CONNECTIONS TO BE:
6. MINIMUM 2% GRADE AND SHALL BE NON-WHITE IN COLOUR.
7. MINIMUM COVER 2.5M MAX. 7.0M. MAX. 2.75M DEEP AT PROPERTY LINE. RISERS SHALL BE USED WHERE NOTED AS PER OPSD 1006.01.
8. 7. CLASS 'B' BEDDING ON ALL SEWERS AND CONNECTIONS TO BE AS PER O.P.S.D. 1005.02 UNLESS NOTED OTHERWISE.
9. 8. GRANULAR BACKFILL AROUND MANHOLES SHALL BE COMPACTED BY MECHANICAL MEANS TO A MINIMUM OF 95% S.P.D.

WATERMAIN NOTES
1. WATERMAIN FROM 50MM TO 300MM DIAMETER TO BE P.V.C. CL150 (DR=18) WITH GASKETED JOINTS.
2. WATER SERVICE CONNECTIONS TO BE AS PER O.P.S.D. 1104.01. PIPE FOR ALL SERVICE CONNECTIONS UP TO 50MM DIA. SHALL BE TYPE K SOFT COPPER TUBING.
3. A MIN. HORIZONTAL SEPARATION OF 2.0M MUST BE MAINTAINED BETWEEN WATERMAINS AND SANITARY OR STORM SEWERS, INCLUDING SERVICE LATERALS.
4. A MIN. VERTICAL SEPARATION OF 0.5M BETWEEN WATERMAINS AND SEWERS MUST BE MAINTAINED IF WATERMAIN IS INSTALLED UNDER OR CLOSER THAN 0.5M ABOVE WATERMAIN.
5. WATERMAIN BEDDING TO BE SUITABLE GRANULAR BEDDING MATERIAL AS PER MUNICIPAL STANDARDS.
6. ALL HYDRANTS AS PER O.P.S.D. 1105.01 TO HAVE STEAMER CONNECTIONS. HYDRANTS TO BE SUPPLIED WITH:
   6.1 TWO (2) 63.5MM (2 ½") WITH CSA STANDARD THREAD, 63.5MM I.D., 79.4 O.D., 5 THREADS PER 25MM, 31.75MM SQUARE OPERATING NUT;
   AND
   6.2 ONE (1) 100MM (4") STORZ PUMPER CONNECTION AS PER CAN/CSA-B 1-S20, 31.75MM SQUARE OPERATING NUT, AND STORZ CAP PAINTED GLOSS BLACK.
7. HYDRANTS SHALL BE INSTALLED SUCH THAT THE 900 STEM LENGTH SHALL NOT EXCEED 1.7M MEASURED FROM THE BREAK-OFF FLANGE. IF HYDRANT BARREL LENGTH EXCEEDS 1.7M THEN A HYDRANT EXTENSION CAN BE RAISED FROM THE BOTTOM WITHOUT INCREASING ROD LENGTH IS TO BE USED.
8. ALL METALLIC WATERMAINS, FITTINGS, HYDRANTS AND RESTRAINERS TO HAVE CATHODIC PROTECTION AS PER REGIONAL STANDARD DRAWING W-CP-1 AND SPECIFICATIONS W-CP-1-S1 AND W-CP-1-S2.
9. ALL GASKROPAL ANODES SHALL CONFORM TO A.S.T.M.-B-418 TYPE II, AND SHALL BE MADE OF HIGH GRADE ELECTROLYTIC ZINC, 99.99% PURE.
10. ANODE INSTALLATION IS NOT REQUIRED WITHIN VALVE-CHAMBERS, DRAIN CHAMBERS OR AIR RELEASE CHAMBERS.
11. ALL WIRE CONNECTIONS TO BE COATED WITH 70 MASTIC OR APPROVED EQUIVALENT.
12. FOR ALL ANODES CONNECTED TO PIPE, FITTINGS OR TO EXISTING METALLIC WATERMAINS, A CADWELDER AND CA-15 OR EQUIVALENT CARTRIDGE SHALL BE USED. ANODE INSTALLATION SHALL BE IN ACCORDANCE WITH REGIONAL STANDARD DRAWING AND SPECIFICATIONS.
13. WHERE NEW PIPE IS TO BE CONNECTED TO EXISTING DUCTILE IRON OR CAST IRON PIPE A 14.5KG MAGNESIUM ANODE IS TO BE CONNECTED TO THE FIRST LENGTH OF EXISTING PIPE, AS PER REGION OF HALTON STANDARD DRAWING W-CP-2-S1.
14. ALL VALVES TO OPEN LEFT (COUNTER-CLOCKWISE) AND SHALL HAVE 50MM SQUARE STANDARD OPERATING NUTS.
15. ALL PLUGS, CAPS, TEES, AND BENDS SHALL BE MECHANICALLY RESTRAINED AS PER MANUFACTURERS SPECIFICATIONS. RESTRAINTS SHALL MEET UNI-B-13-92.
16. RECOMMENDED RESTRAINED LENGTH OF PVC PIPE AS FOLLOWS:

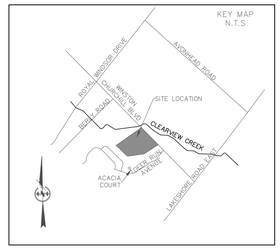
RECOMMENDED RESTRAINED LENGTH OF PVC PIPE INSTALLED IN SAND-SILT:

NOM. PIPE SIZE	90° BEND	45° BEND	22.5° BEND	11.25° BEND	45° VERT. BEND DOWN/UP	VALVE/END DEAD-END
150	3.7m	1.6m	0.8m	0.4m	4.6m/1.6m	11.1m
200	4.8m	2.0m	1.0m	0.5m	2.9m/1.0m	14.4m
300	6.8m	2.5m	1.4m	0.7m	8.6m/2.9m	20.7m

NOM. PIPE SIZE	REDUCING TEE	NOM. PIPE SIZE	STRAIGHT TEE
200x150	6.2m (BRANCH ONLY)	150	8.5m (BRANCH)/2.9m (MAIN)
300x150	3.7m (BRANCH ONLY)	200	12.1m (BRANCH)/5.5m (MAIN)
300x200	8.9m (BRANCH)/1.8m (MAIN)	300	18.9m (BRANCH)/13.3m (MAIN)

17. WHERE WATERMAIN IS PLACED IN FILL OR IN PREVIOUSLY DISTURBED GROUND ALL JOINTS TO BE MECHANICALLY RESTRAINED.
18. MINIMUM COVER TO TOP OF PIPE OVER WATERMAIN SHALL BE 1.70M MEASURED FROM THE ROAD CENTRELINE ELEVATION.
19. THE DEPTH OF WATER SERVICES AT PROPERTY LINE SHOULD BE A MINIMUM OF 1.7M AND A MAXIMUM OF 2.75M. THE DIFFERENCE BETWEEN THE GROUND ELEVATION AND THE TOP OF THE ROD SHOULD BE BETWEEN 0.9M AND 1.5M.
20. WATER SERVICES CROSSING THE STORM SEWER TO HAVE MIN. 1.70M OF COVER. WHERE THIS CANNOT BE ACHIEVED, WATER SERVICE IS TO CROSS UNDER STORM SEWER.
21. GATE VALVES CONFORMING TO A.W.W.A. C500 STANDARDS ARE REQUIRED ON WATERMAINS 300MM AND SMALLER. VALVES 350MM AND LARGER SHALL BE BUTTERFLY VALVES.
22. ALL WATERMAIN FITTINGS SHALL HAVE MECHANICAL JOINTS.
23. VERTICAL AND HORIZONTAL ALIGNMENT OF WATERMAIN TO BE ACHIEVED BY DEFLECTION OF JOINTS WHERE POSSIBLE. WHERE DEFLECTION IS NOT POSSIBLE, BENDS SHALL BE USED.
24. TRACER WIRE IS TO BE INSTALLED ON ALL NEW INSTALLATIONS (OPSD). WATERMAIN PIPE FOR LOCATING PURPOSES, A SOLID 12 GAUGE T.W.U. COPPER WIRE IS TO BE INSTALLED ALONG THE PIPE, STRAPPED TO THE PIPE AT 6 METRE INTERVALS. JOINTS IN THE WIRE BETWEEN VALVES ARE NOT PERMITTED.
25. THE CONTRACTOR MAY TEST THE TRACING WIRE FOR CONDUCTIVITY. IF THE TRACING WIRE IS NOT CONTINUOUS FROM VALVE TO VALVE, THE CONTRACTOR SHALL, AT HIS OWN EXPENSE, REPLACE OR REPAIR THE WIRE.
26. ALL WATERMAINS SUPPLIED BY A WATERMAIN TO BE SHUT DOWN SHALL BE NOTIFIED BY THE CONTRACTOR AT LEAST 24 HOURS IN ADVANCE OF THE SHUT DOWN AS PER REGION OF HALTON SPECIFICATIONS. NOTIFICATION SHALL TAKE PLACE UNDER THE ENGINEER'S DIRECTION.
27. OPERATION OF EXISTING WATERMAINS SHALL BE BY REGION OF HALTON STAFF ONLY.
28. FIRE HYDRANT CAPS SHALL BE COLOUR CODED AS PER NFPA STANDARDS.

KEY MAP N.T.S



LEGEND

- CATCHBASIN
- CATCHBASIN MANHOLE
- STORM MANHOLE
- SANITARY MANHOLE
- M MECHANICAL ROOM
- x118.02 EXISTING ELEVATION
- x120.00 EX EXISTING ELEVATION
- x120.00 PROPOSED ELEVATION
- x120.00 TC PROPOSED TOP OF CURB ELEVATION
- x120.00 TW PROPOSED TOP OF WALL ELEVATION
- x120.00 BW PROPOSED BOTTOM OF WALL ELEVATION
- x120.00 SW PROPOSED SWALE ELEVATION
- EXISTING REGIONAL FLOODLINE
- PROPOSED REGIONAL FLOODLINE
- 1981 SECTION NUMBER
- TREE PROTECTION

100 YEAR AND REGIONAL FLOODLINE INFORMATION TAKEN FROM PLAN PREPARED BY CVC FLOOD HAZARD MAP.

No.	Date	By	REVISIONS
6	NOV.25/21	J.M.N.	ISSUED FOR THIRD SUBMISSION
5	NOV.09/21	J.M.N.	ISSUED FOR COORDINATION
4	FEB.26/21	J.M.N.	ISSUED FOR SECOND SUBMISSION
3	JAN.25/21	J.M.N.	ISSUED FOR SECOND SUBMISSION
2	DEC.23/20	J.M.N.	ISSUED FOR COORDINATION
1	JUL.11/19	ADD	FIRST SUBMISSION

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560, 570 AND 580 WINSTON CHURCHILL BLVD.

TOWN OF OAKVILLE
REGION OF HALTON

GRADING
PLAN - NORTH

SCALE: 1:500	DATE: MAY 2019	PROJ No.	
DRAWN: A.D.D.	CHK'D: A.M.C.	PLAN No.	G-3
DESIGNED: A.M.C.	SHEET 1 OF 2		

FILENAME: G:\STRUCT\570 - MAST.dwg
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PLOTTED BY: dandrus