

Report Information Form

Engineer:

Email:

Date:

Project Details

Report Type:

Product:

Job Name:

Job Location:

Project No.:

Distributor P.O.:

Customer Details

Name:

Company:

Address:

GRE Name:

GRE Company:



RisiStone[®]
retaining wall systems

Report on Proposed
Segmental Retaining Wall

Project No.

Distribution

Risi Stone Inc.

10-480 Harry Walker Pkwy S.

Newmarket ON Canada L3Y 0B3

P 905.868.9255 | **F** 905.868.9254

E

www.risistone.com



RisiStone
retaining wall systems

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5. Design Drawings
6. Specifications
7. Retaining Wall Budget & Design

The Solid Choice.



RisiStone®

retaining wall systems

Risi Stone Inc.

10-480 Harry Walker Pkwy S.
Newmarket ON Canada L3Y 0B3

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www.risistone.com

Attn:

Re:

Proposed SRW:

Project No.:

Please find enclosed the Wall Design for the above noted project. Information on the design is provided in the Drawings.

A qualified Professional Engineer must be retained to provide Geotechnical Inspection of the Wall and General Review of Construction in accordance with Division C – Part 1, Section 1.2.2 of the Ontario building Code. Risi Stone Inc. does not provide these services. Refer to the Specification for further explanation of these requirements.

Included in this report are:

- 1 VESPA Report:** Contains wall layout information and quantity calculations for wall face area, geogrid requirements, infill quantities (Estimate based on infill required within the reinforced zone. Does not account for other infill that may be required beyond the infill zone), base quantities, drainage quantities, coping quantities, etc. Contractor must review the layout information provided and ensure the wall dimensions (lengths, TW/BW elevations) match the most recent grading and site information available. (all quantities can be found on the first page of the Vespa Report)
- 2 Construction Review & Inspection Guidelines:** Guidelines provided by Risi Stone Inc. to aid in the Geotechnical Inspection, General Review, and Contractor Quality Assurance of the Wall(s).
- 3 Design Drawings & Specifications:** These must be sealed by a Professional Engineer to be used for Construction. If these drawings are not sealed, they are Preliminary only and can not be used for Construction.

Please advise this office if further design services are required.

Sincerely,

Letter of Intent for General Review Engineer

Project Name:
Project Number:
Date:

General Review Engineer:
Company Name:

Has been retained to provide the General Review of the wall(s) in accordance with the Design, Notes, and Specifications contained with this.

In addition, I undertake to ensure that the overall Global Stability of the proposed wall/slope configuration will be addressed by this firm or the Site Geotechnical Engineer (if they are not the same) prior to construction.

Signature

Date

*Please provide a completed copy of this letter of intent to the Contractor, Site Civil Engineer, and Risi Stone Inc.
Please send to Risi Stone Inc. via email to:*



RisiStone[®]
retaining wall systems

Vespa Output

Client	February 2026		
Name	Mixed Use Residential Development	Number	202602029
Site	1493 Sixth Line, Oakville, ON	Designer	CYK
Revision	2	Created	3/2/2026
Standard	National Concrete Masonry Association 3rd Edition		
Modified	3/2/2026		

Note †: Total Facing quantity is based on using full-sized units only on bottom course and an even mix of defined facing sizes, as identified elsewhere in this report, on remaining courses of each Section. The use of corners, tapered or cut units is not reflected in this quantity.

Note ‡: Infill values are calculated based on the average geogrid length in each Section. They do not account for anything beyond the reinforced zone (end of the geogrids). Actual infill values may be significantly higher.

Note : Face drain values do not include the drainage stone within block. Drainage stone within block is calculated based on the percentage hollow core of the wall unit selected. If the percentage hollow core is not defined then the drainage stone within block will not be calculated.

Quantities

Wall	Facing	Wall/Cap Length [m]	Corner Quantity [#]	TOW Steps [#]	BOW Steps [#]	Facing Area [m ²]	Total Wall Area [m ²]
Wall 1	Sonoma Stone 375	70	0	4	6	44	57
Wall 2	Sonoma Stone 375	26	0	1	15	42	47
Wall 3	Sonoma Stone 375	37	0	2	15	63	70
Wall 4	Sonoma Stone 375	34	2	0	1	12	18
		168	2	7	37	160	191

Wall Unit	Wall 1	Wall 2	Wall 3	Wall 4
Sonoma Stone 375	199	189	283	53

Wall	Leveling Pad [m ³]	Reinforced Fill [m ³]	Drainage Fill [m ³]	Core Fill [m ³]
Wall 1	10.9	44.5	0.0	0.0
Wall 2	4.1	75.9	0.0	0.0
Wall 3	5.8	110.8	0.0	0.0
Wall 4	5.3	0.0	1.6	0.0
Totals:	26.0	231.1	1.5	0.0

Reinforcements

Wall	SGU60 [m ²]	Geogrid Connectors
Wall 1	168.3	0
Wall 2	184.1	0
Wall 3	276.5	0
Wall 4	0.0	0
Totals:	628.9	0

NOTE: THESE CALCULATIONS, QUANTITIES, AND LAYOUTS ARE FOR PRELIMINARY DESIGN ONLY AND SHOULD NOT BE USED FOR CONSTRUCTION WITHOUT REVIEW BY A QUALIFIED ENGINEER

Project: 202602029 - Mixed Use Residential Development

Site: 1493 Sixth Line, Oakville, ON

Date: 3/2/2026

Wall: Wall 1

Project Information

Client February 2026

Name Mixed Use Residential Development

Number 202602029

Site 1493 Sixth Line, Oakville, ON

Designer CYK

Revision 2 **Created** 3/2/2026

Modified 3/2/2026

Standard National Concrete Masonry Association 3rd Edition

Seismic As 0.17 Default Deflection of 50.80 mm

Comments

Revision Drawings provided on March 2, 2026

Note

Selected Facing Unit

Licenser/Product Line: Risi Stone Systems
Name: Sonoma Stone 375

NOTE: THESE CALCULATIONS, QUANTITIES, AND LAYOUTS ARE FOR PRELIMINARY DESIGN ONLY
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Project Summary

Geogroups

Geogroup	Layer	Length (m)
A	All	1.20
B	All	1.20

Quantities

Wall Length	70.20 m
Steps in Top of Wall	4
Steps in Bottom of Wall	6
Total Wall Area	57.1 m ²
Exposed Area (includes cap)	43.5 m ²
Embedded Area	13.6 m ²
Tallest Panel Height	1.11 m
Longest reinforcement length	1.20 m
Base soil volume	10.9 m ³
Infill soil volume ‡	44.5 m ³
Gravity Face Drain	0.0 m ³

Reinforcement

SGU60 - SGU60	168.3 m ²
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Note †: Total Facing Unit quantity is based on using full-sized units only on bottom course and an even mix of defined facing sizes, as identified elsewhere in this report, on remaining courses of each Section. The use of corners, tapered or cut units is not reflected in this quantity.

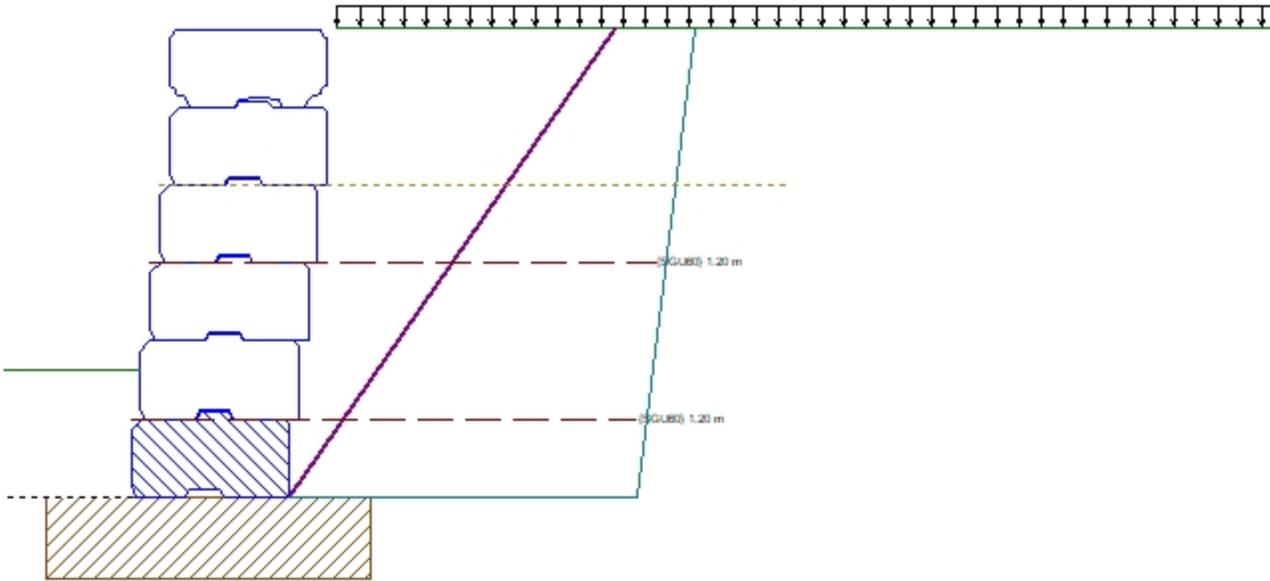
Note ‡: Reinforced fill values are calculated based on the average geogrid length in each Section. They do not account for anything beyond the reinforced zone (end of the geogrids). Actual infill values may be significantly higher.

Note : Drainage fill does not include the drainage stone within block. Core fill are calculated based on the percentage hollow core of the wall unit selected. If the percentage hollow core is not defined then the Core fill value within block will not be calculated.

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Tallest Section

Section Height 1.11 m



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Project Design Inputs

Design Standard National Concrete Masonry Association 3rd Edition

Minimum Factors of Safety

Conventional

External		Value	Internal		Value	Facing	Value
FSsl	Base Sliding	1.50	FSsl	Internal Sliding	1.50		
FSbc	Bearing Capacity	2.00	FSsc	Shear Capacity	1.50		
FSot	Overturning	1.50					

MultiDepth

External		Value	Internal		Value	Facing	Value
FSsl	Base Sliding	1.50					
FSbc	Bearing Capacity	2.00					
FSsh	Interface Shear	1.50					
FSot	Overturning	1.50					

No Fines

External		Value	Internal		Value	Facing	Value
FSsl	Base Sliding	1.50					
FSbc	Bearing Capacity	2.00					
FSot	Overturning	1.50					

Reinforced

External		Value	Internal		Value	Facing		Value
FSsl	Base Sliding	1.50	FSsl	Internal Sliding	1.50	FScs	Connection Strength	1.50
FSbc	Bearing Capacity	2.00	FSpO	Pullout	1.50	FSsc	Facing Shear	1.50
FSct	Crest Toppling	1.50	FSto	Tensile Overstress	1.50			
FSot	Overturning	2.00						

Seismic

Conventional

External		Value	Internal		Value	Facing	Value
FSsl	Base Sliding	1.10	FSsl	Internal Sliding	1.10		
FSbc	Bearing Capacity	1.10	FSsc	Shear Capacity	1.10		
FSot	Overturning	1.10					

MultiDepth

External		Value	Internal		Value	Facing	Value
FSsl	Base Sliding	1.10					
FSbc	Bearing Capacity	1.10					
FSsh	Interface Shear	1.10					
FSot	Overturning	1.10					

No Fines

External		Value	Internal		Value	Facing	Value
FSsl	Base Sliding	1.10					
FSbc	Bearing Capacity	1.50					
FSot	Overturning	1.10					

Reinforced

External		Value	Internal		Value	Facing		Value
FSsl	Base Sliding	1.10	FSsl	Internal Sliding	1.10	FScs	Connection Strength	1.10
FSbc	Bearing Capacity	1.50	FSpO	Pullout	1.10	FSsc	Facing Shear	1.10
FSct	Crest Toppling	1.10	FSto	Tensile Overstress	1.10			
FSot	Overturning	1.50						

Design Factors

Term	Description	Minimum (as appl.)	Maximum (as appl.)
RC	Reinforced coverage ratio	1.00	0.00

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Selected Facing Unit

Licenser/Product Line:	Risi Stone Systems	
Name:	Sonoma Stone 375	
Facing Height	Hu	0.19 m
Facing Width	Lu	1.20 m
Facing Depth	Wu	0.38 m
Facing Weight	Xu	21.8 kN/m ³
Center of Gravity	Gu	0.19 m
Setback	u	0.02 m
Batter		7.12 °
Cap Height	Hcu	0.19 m
Initial Shear Capacity	au	64.00 kN/m
Apparent Shear Angle	u	46.00 °
Maximum Shear Capacity	Vu(max)	125.00 kN/m

Selected Reinforcement Types**Reinforcements**

SGU60 - SGU60	Supplier: Strata Systems, Inc., Product Line: Strata SGU SERIES, Fill Type: Sands						
Tult	60.00 kN/m	RFcr	1.47	RFd	1.10	LTDS	33.73 kN/m
RFid	1.10	Cds	0.80	Ci	0.80		

Connection/Shear Properties

cs1	7.30 kN/m	IP-1	26.90 kN/m	cs2	21.00 kN/m	IP-2	57.00 kN/m
cs max	24.80 kN/m	au	25.00 kN/m	u	48.00 kN/m	Vu(max)	88.00 kN/m

Selected Soil Types

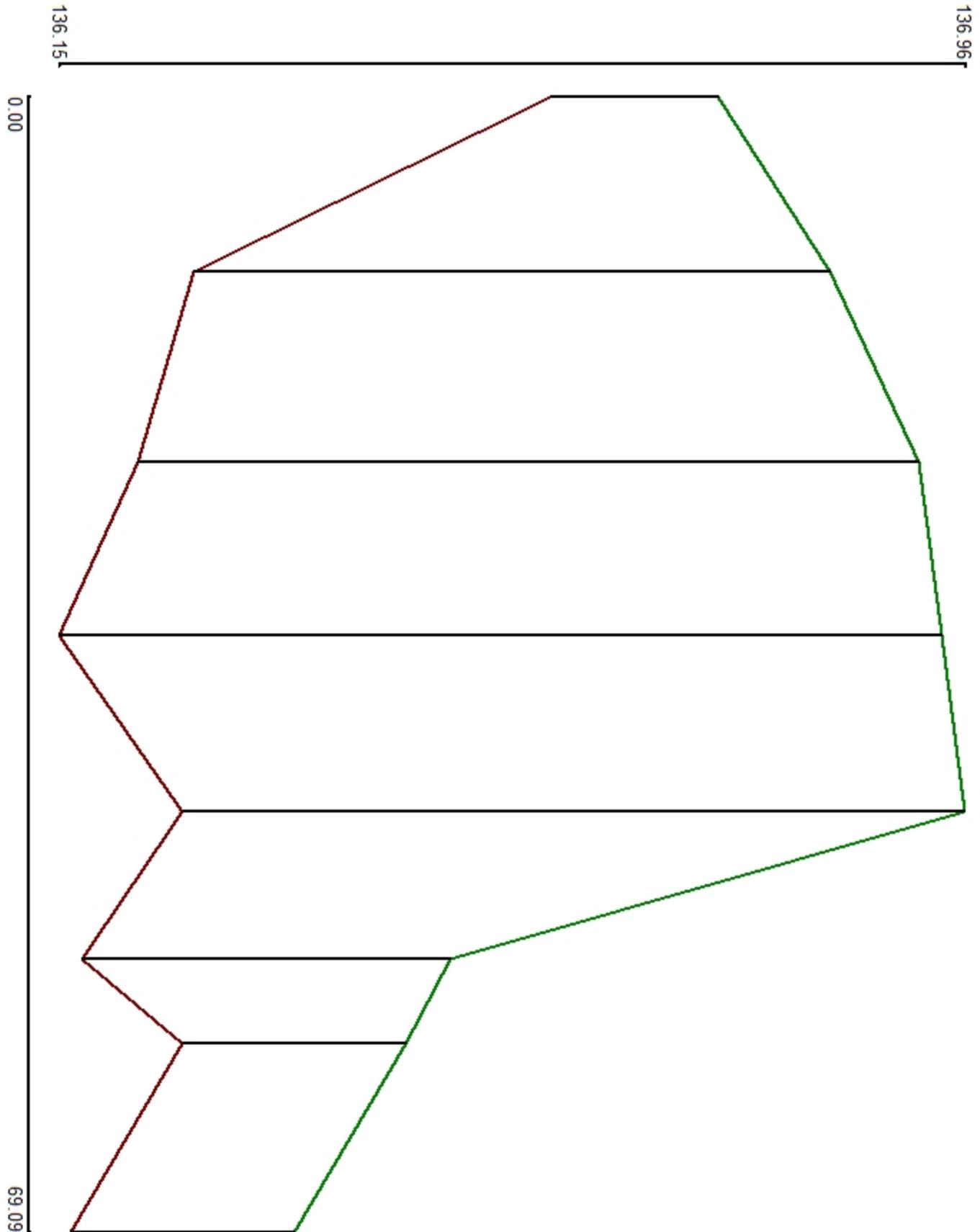
Soil Zone	Soil Type	In Situ		
		Friction Angle	Density [kN/m ³]	Cohesion Cf [kN/m ²]
Infill (i)	GW	33°	21.00	n/a
Retained (r)	CL	28°	19.95	n/a
Foundation (f)	CL	28°	19.95	0.00
Base (b)	GW	37°	22.00	n/a
Drainage (d)	GP	38°	19.64	n/a

Soil Glossary

CH:	Inorganic clays, high plasticity
CL:	Inorganic clays, low to medium plasticity, gravelly, sandy, silty, lean clays
GC:	Clayey gravels, poorly graded gravel-sand-clay mixtures
GM:	Silty gravels, poorly graded gravel-sand-silt mixtures
GP:	1/2"-3/4" clean crushed stone or crushed gravel
GW:	Well-graded gravels, gravel-sand. Little or no fines.
MH:	Inorganic clayey silts, elastic silts
ML:	Inorganic silts, very fine sands, silty or clayey, slight plasticity
SC:	Clayey sands, poorly graded sand-clay mixtures
SM:	Silty sands, poorly graded sand-silt mixtures
SP:	Poorly-graded sands, gravelly sands. Little or no fines.
SW:	Well-graded sands, gravelly sands. Little or no fines.

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Station Detail



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Note: Station Layout is the face view of the wall, looking at it from left to right

Station Layout

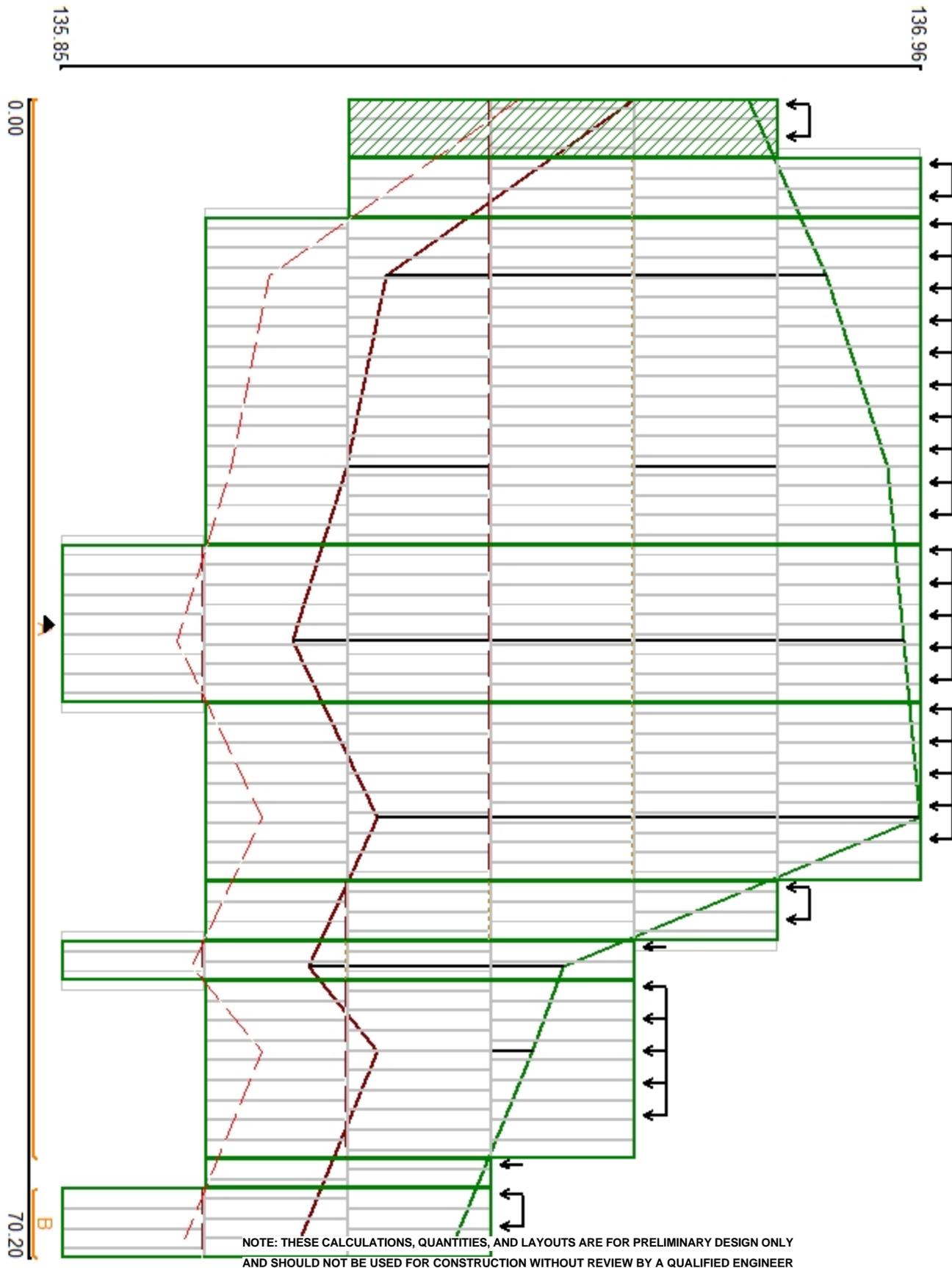
No.	Station [m]	Top [m]	Bottom [m]	Height [m]
1	0.00	136.74	136.59	0.15
2	10.67	136.84	136.27	0.57
3	22.28	136.92	136.22	0.70
4	32.80	136.94	136.15	0.79
5	43.50	136.96	136.26	0.70
6	52.51	136.50	136.17	0.33
7	57.64	136.46	136.26	0.20
8	69.09	136.36	136.16	0.20

Station Wall Length	69.09 m
Minimum Height	0.15 m
Maximum Height	0.79 m

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Section Geometry

Section Drawing



Section Extents

Section	Top Elevation [m]	Base Elevation [m]	Left Side [m]	Right Side [m]	Bottom Grade Elevation [m]
1	136.78	136.22	0.00	3.60	136.48
2	136.96	136.22	3.60	7.20	136.37
3	136.96	136.04	7.20	27.00	136.19
4	136.96	135.85	27.00	36.60	136.15
5	136.96	136.04	36.60	47.40	136.19
6	136.78	136.04	47.40	51.00	136.19
7	136.59	135.85	51.00	53.40	136.17
8	136.59	136.04	53.40	64.20	136.19
9	136.41	136.04	64.20	66.00	136.19
10	136.41	135.85	66.00	70.20	136.16

Section Measurements

Section	Height [m]	Design Height [m]	Width [m]	Face Area [m ²]	Embedment [m]	Infill Volume [m ³]
1	0.56	0.56	3.60	2.0	0.26	1.6
2	0.74	0.74	3.60	2.7	0.15	1.7
3	0.93	0.93	19.80	18.3	0.15	14.6
4	1.11	1.11	9.60	10.7	0.30	8.7
5	0.93	0.93	10.80	10.0	0.15	8.2
6	0.74	0.74	3.60	2.7	0.15	2.2
7	0.74	0.74	2.40	1.8	0.32	1.4
8	0.56	0.56	10.80	6.0	0.15	4.1
9	0.37	0.37	1.80	0.7	0.15	0.0
10	0.56	0.56	4.20	2.3	0.31	1.9

Section Slopes

Section	Crest Slope [°]	Crest Offset [m]	Toe Slope [°]	Toe Offset [m]
1	0.00	0.00	0.00	0.00
2	0.00	0.00	0.00	0.00
3	0.00	0.00	0.00	0.00
4	0.00	0.00	0.00	0.00
5	0.00	0.00	0.00	0.00
6	0.00	0.00	0.00	0.00
7	0.00	0.00	0.00	0.00
8	0.00	0.00	0.00	0.00
9	0.00	0.00	0.00	0.00
10	0.00	0.00	0.00	0.00

Section Loads

Section	Live Load [kN/m ²]	Live Offset [m]	Dead Load [kN/m ²]	Dead Offset [m]
1	6.0	0.00	0.0	0.00
2	6.0	0.00	0.0	0.00
3	6.0	0.00	0.0	0.00
4	6.0	0.00	0.0	0.00
5	6.0	0.00	0.0	0.00
6	6.0	0.00	0.0	0.00
7	6.0	0.00	0.0	0.00
8	6.0	0.00	0.0	0.00
9	6.0	0.00	0.0	0.00
10	6.0	0.00	0.0	0.00

Reinforcement Details

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Section	Course	Length [m]	Area [m²]	Reinforcement
1	1	1.20	4.32	SGU60 - SGU60
2	1	1.20	4.32	SGU60 - SGU60
3	2	1.20	23.76	SGU60 - SGU60
4	3	1.20	11.52	SGU60 - SGU60
	1	1.20	11.52	SGU60 - SGU60
5	2	1.20	12.96	SGU60 - SGU60
6	1	1.20	4.32	SGU60 - SGU60
7	1	1.20	2.88	SGU60 - SGU60
8	1	1.20	12.96	SGU60 - SGU60
10	1	1.20	5.04	SGU60 - SGU60

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Project: 202602029 - Mixed Use Residential Development

Site: 1493 Sixth Line, Oakville, ON

Date: 3/2/2026

Wall: Wall 2

Project Information

Client	February 2026		
Name	Mixed Use Residential Development	Number	202602029
Site	1493 Sixth Line, Oakville, ON	Designer	CYK
Revision	2	Created	3/2/2026
Standard	National Concrete Masonry Association 3rd Edition	Modified	3/2/2026
Seismic As	0.17	Default Deflection of 50.80 mm	
Comments			
Revision	Drawings provided on March 2, 2026		
Note			

Selected Facing Unit

Licenser/Product Line:	Risi Stone Systems
Name:	Sonoma Stone 375

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Project Summary

Geogroups

Geogroup	Layer	Length (m)
A	All	2.70
B	All	2.40
C	All	2.10
D	All	1.80
E	All	1.50
F	All	1.20

Quantities

Wall Length	26.40 m
Steps in Top of Wall	1
Steps in Bottom of Wall	15
Total Wall Area	46.7 m ²
Exposed Area (includes cap)	40.5 m ²
Embedded Area	6.2 m ²
Tallest Panel Height	3.33 m
Longest reinforcement length	2.70 m
Base soil volume	4.1 m ³
Infill soil volume ‡	75.9 m ³
Gravity Face Drain	0.0 m ³

Reinforcement

SGU60 - SGU60	184.1 m ²
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Note †: Total Facing Unit quantity is based on using full-sized units only on bottom course and an even mix of defined facing sizes, as identified elsewhere in this report, on remaining courses of each Section. The use of corners, tapered or cut units is not reflected in this quantity.

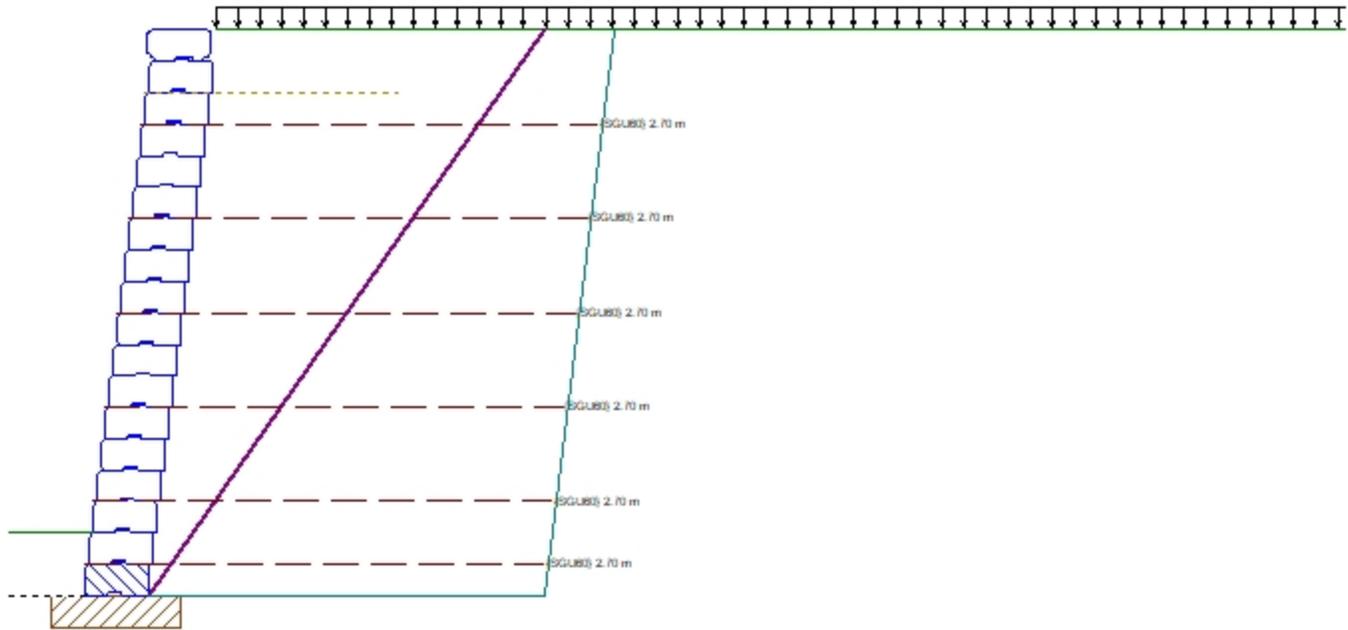
Note ‡: Reinforced fill values are calculated based on the average geogrid length in each Section. They do not account for anything beyond the reinforced zone (end of the geogrids). Actual infill values may be significantly higher.

Note : Drainage fill does not include the drainage stone within block. Core fill are calculated based on the percentage hollow core of the wall unit selected. If the percentage hollow core is not defined then the Core fill value within block will not be calculated.

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Tallest Section

Section Height 3.33 m



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Project Design Inputs

Design Standard National Concrete Masonry Association 3rd Edition

Minimum Factors of Safety

Conventional

External		Value	Internal		Value	Facing	Value
FSsl	Base Sliding	1.50	FSsl	Internal Sliding	1.50		
FSbc	Bearing Capacity	2.00	FSsc	Shear Capacity	1.50		
FSot	Overturning	1.50					

MultiDepth

External		Value	Internal		Value	Facing	Value
FSsl	Base Sliding	1.50					
FSbc	Bearing Capacity	2.00					
FSsh	Interface Shear	1.50					
FSot	Overturning	1.50					

No Fines

External		Value	Internal		Value	Facing	Value
FSsl	Base Sliding	1.50					
FSbc	Bearing Capacity	2.00					
FSot	Overturning	1.50					

Reinforced

External		Value	Internal		Value	Facing		Value
FSsl	Base Sliding	1.50	FSsl	Internal Sliding	1.50	FScs	Connection Strength	1.50
FSbc	Bearing Capacity	2.00	FSpO	Pullout	1.50	FSsc	Facing Shear	1.50
FSct	Crest Toppling	1.50	FSto	Tensile Overstress	1.50			
FSot	Overturning	2.00						

Seismic

Conventional

External		Value	Internal		Value	Facing	Value
FSsl	Base Sliding	1.10	FSsl	Internal Sliding	1.10		
FSbc	Bearing Capacity	1.10	FSsc	Shear Capacity	1.10		
FSot	Overturning	1.10					

MultiDepth

External		Value	Internal		Value	Facing	Value
FSsl	Base Sliding	1.10					
FSbc	Bearing Capacity	1.10					
FSsh	Interface Shear	1.10					
FSot	Overturning	1.10					

No Fines

External		Value	Internal		Value	Facing	Value
FSsl	Base Sliding	1.10					
FSbc	Bearing Capacity	1.50					
FSot	Overturning	1.10					

Reinforced

External		Value	Internal		Value	Facing		Value
FSsl	Base Sliding	1.10	FSsl	Internal Sliding	1.10	FScs	Connection Strength	1.10
FSbc	Bearing Capacity	1.50	FSpO	Pullout	1.10	FSsc	Facing Shear	1.10
FSct	Crest Toppling	1.10	FSto	Tensile Overstress	1.10			
FSot	Overturning	1.50						

Design Factors

Term	Description	Minimum (as appl.)	Maximum (as appl.)
RC	Reinforced coverage ratio	1.00	0.00

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Selected Facing Unit

Licenser/Product Line:	Risi Stone Systems	
Name:	Sonoma Stone 375	
Facing Height	Hu	0.19 m
Facing Width	Lu	1.20 m
Facing Depth	Wu	0.38 m
Facing Weight	Xu	21.8 kN/m ³
Center of Gravity	Gu	0.19 m
Setback	u	0.02 m
Batter		7.12 °
Cap Height	Hcu	0.19 m
Initial Shear Capacity	au	64.00 kN/m
Apparent Shear Angle	u	46.00 °
Maximum Shear Capacity	Vu(max)	125.00 kN/m

Selected Reinforcement Types**Reinforcements**

SGU60 - SGU60	Supplier: Strata Systems, Inc., Product Line: Strata SGU SERIES, Fill Type: Sands						
Tult	60.00 kN/m	RFcr	1.47	RFd	1.10	LTDS	33.73 kN/m
RFid	1.10	Cds	0.80	Ci	0.80		

Connection/Shear Properties

cs1	7.30 kN/m	IP-1	26.90 kN/m	cs2	21.00 kN/m	IP-2	57.00 kN/m
cs max	24.80 kN/m	au	25.00 kN/m	u	48.00 kN/m	Vu(max)	88.00 kN/m

Selected Soil Types

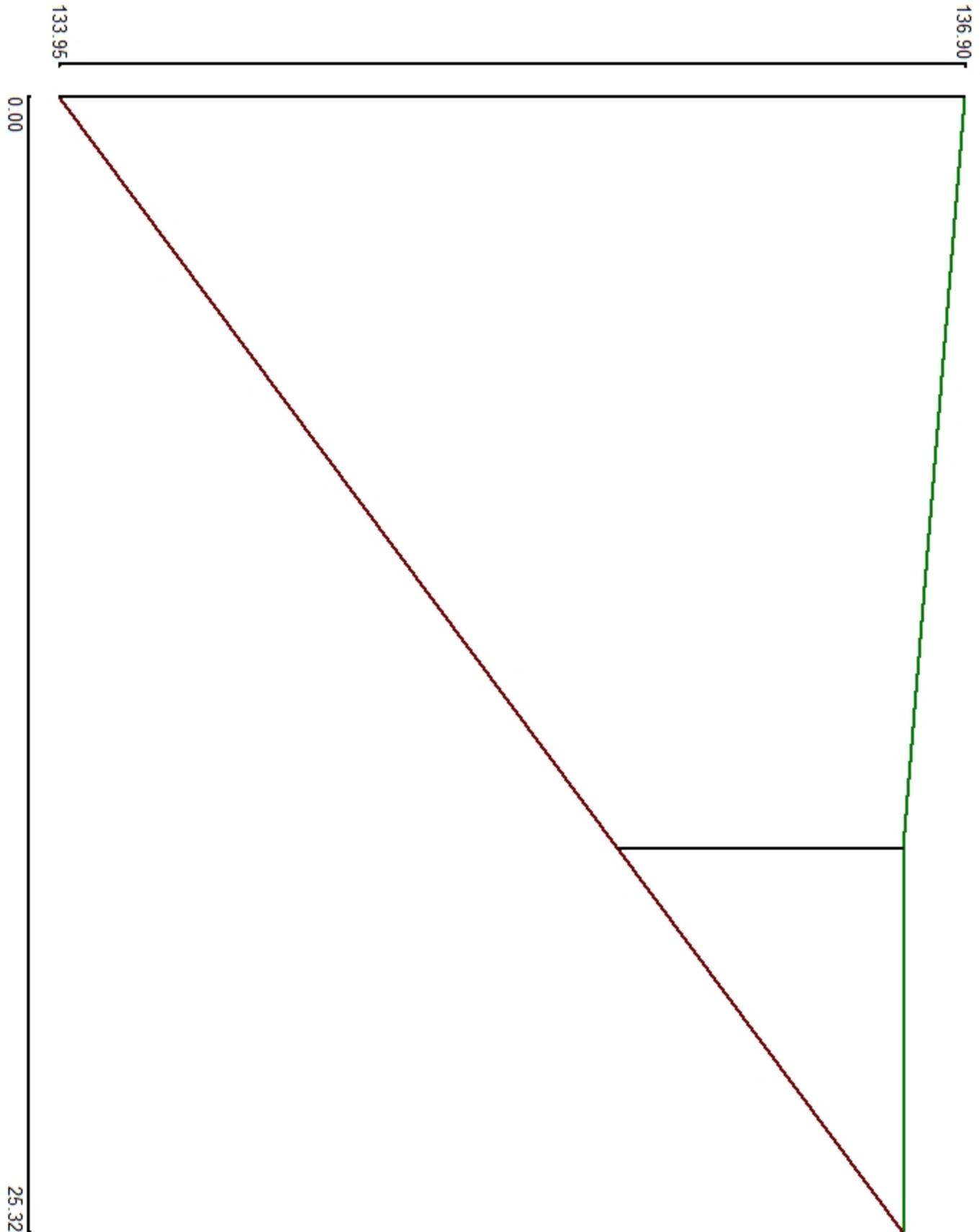
Soil Zone	Soil Type	In Situ		
		Friction Angle	Density [kN/m ³]	Cohesion Cf [kN/m ²]
Infill (i)	GW	33°	21.00	n/a
Retained (r)	CL	28°	19.95	n/a
Foundation (f)	CL	28°	19.95	0.00
Base (b)	GW	37°	22.00	n/a
Drainage (d)	GP	38°	19.64	n/a

Soil Glossary

CH:	Inorganic clays, high plasticity
CL:	Inorganic clays, low to medium plasticity, gravelly, sandy, silty, lean clays
GC:	Clayey gravels, poorly graded gravel-sand-clay mixtures
GM:	Silty gravels, poorly graded gravel-sand-silt mixtures
GP:	1/2"-3/4" clean crushed stone or crushed gravel
GW:	Well-graded gravels, gravel-sand. Little or no fines.
MH:	Inorganic clayey silts, elastic silts
ML:	Inorganic silts, very fine sands, silty or clayey, slight plasticity
SC:	Clayey sands, poorly graded sand-clay mixtures
SM:	Silty sands, poorly graded sand-silt mixtures
SP:	Poorly-graded sands, gravelly sands. Little or no fines.
SW:	Well-graded sands, gravelly sands. Little or no fines.

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Station Detail



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Note: Station Layout is the face view of the wall, looking at it from left to right

Station Layout

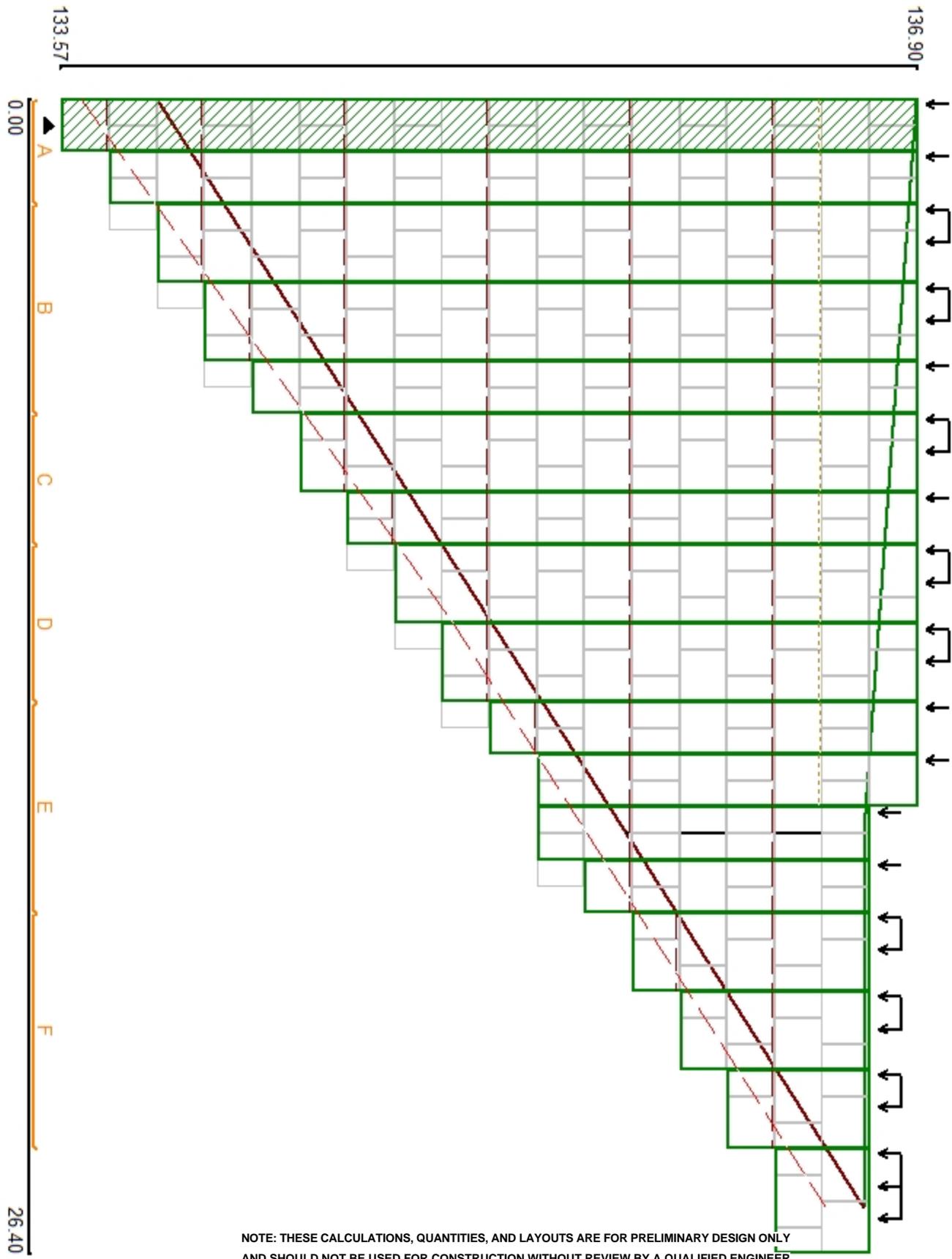
No.	Station [m]	Top [m]	Bottom [m]	Height [m]
1	0.00	136.90	133.95	2.95
2	16.78	136.70	135.77	0.93
3	25.32	136.70	136.70	0.00

Station Wall Length	25.32 m
Minimum Height	0.00 m
Maximum Height	2.95 m

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Section Geometry

Section Drawing



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Section Extents

Section	Top Elevation [m]	Base Elevation [m]	Left Side [m]	Right Side [m]	Bottom Grade Elevation [m]
1	136.90	133.57	0.00	1.20	133.95
2	136.90	133.76	1.20	2.40	134.08
3	136.90	133.94	2.40	4.20	134.21
4	136.90	134.13	4.20	6.00	134.41
5	136.90	134.31	6.00	7.20	134.60
6	136.90	134.50	7.20	9.00	134.73
7	136.90	134.68	9.00	10.20	134.93
8	136.90	134.87	10.20	12.00	135.06
9	136.90	135.05	12.00	13.80	135.25
10	136.90	135.24	13.80	15.00	135.45
11	136.90	135.42	15.00	16.20	135.58
12	136.72	135.42	16.20	17.40	135.71
13	136.72	135.60	17.40	18.60	135.84
14	136.72	135.79	18.60	20.40	135.97
15	136.72	135.98	20.40	22.20	136.17
16	136.72	136.16	22.20	24.00	136.36
17	136.72	136.35	24.00	26.40	136.56

Section Measurements

Section	Height [m]	Design Height [m]	Width [m]	Face Area [m ²]	Embedment [m]	Infill Volume [m ³]
1	3.33	3.33	1.20	4.0	0.38	9.3
2	3.15	3.15	1.20	3.8	0.33	8.7
3	2.96	2.96	1.80	5.3	0.27	10.7
4	2.78	2.78	1.80	5.0	0.28	9.9
5	2.59	2.59	1.20	3.1	0.29	6.1
6	2.41	2.41	1.80	4.3	0.24	7.2
7	2.22	2.22	1.20	2.7	0.25	4.4
8	2.03	2.03	1.80	3.7	0.19	4.9
9	1.85	1.85	1.80	3.3	0.20	4.4
10	1.66	1.66	1.20	2.0	0.21	2.0
11	1.48	1.48	1.20	1.8	0.16	1.8
12	1.30	1.30	1.20	1.6	0.29	1.7
13	1.11	1.11	1.20	1.3	0.23	1.5
14	0.93	0.93	1.80	1.7	0.18	1.4
15	0.74	0.74	1.80	1.3	0.19	1.1
16	0.56	0.56	1.80	1.0	0.20	0.8
17	0.37	0.37	2.40	0.9	0.21	0.0

Section Slopes

Section	Crest Slope [°]	Crest Offset [m]	Toe Slope [°]	Toe Offset [m]
1	0.00	0.00	0.00	0.00
2	0.00	0.00	0.00	0.00
3	0.00	0.00	0.00	0.00
4	0.00	0.00	0.00	0.00
5	0.00	0.00	0.00	0.00
6	0.00	0.00	0.00	0.00
7	0.00	0.00	0.00	0.00
8	0.00	0.00	0.00	0.00
9	0.00	0.00	0.00	0.00
10	0.00	0.00	0.00	0.00
11	0.00	0.00	0.00	0.00
12	0.00	0.00	0.00	0.00
13	0.00	0.00	0.00	0.00
14	0.00	0.00	0.00	0.00
15	0.00	0.00	0.00	0.00
16	0.00	0.00	0.00	0.00

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Section	Crest Slope [°]	Crest Offset [m]	Toe Slope [°]	Toe Offset [m]
17	0.00	0.00	0.00	0.00

Section Loads

Section	Live Load [kN/m ²]	Live Offset [m]	Dead Load [kN/m ²]	Dead Offset [m]
1	12.0	0.00	0.0	0.00
2	12.0	0.00	0.0	0.00
3	12.0	0.00	0.0	0.00
4	12.0	0.00	0.0	0.00
5	12.0	0.00	0.0	0.00
6	12.0	0.00	0.0	0.00
7	12.0	0.00	0.0	0.00
8	12.0	0.00	0.0	0.00
9	12.0	0.00	0.0	0.00
10	12.0	0.00	0.0	0.00
11	12.0	0.00	0.0	0.00
12	12.0	0.00	0.0	0.00
13	12.0	0.00	0.0	0.00
14	12.0	0.00	0.0	0.00
15	12.0	0.00	0.0	0.00
16	12.0	0.00	0.0	0.00
17	12.0	1.00	0.0	0.00

Reinforcement Details

Section	Course	Length [m]	Area [m ²]	Reinforcement
1	15	2.70	3.24	SGU60 - SGU60
	12	2.70	3.24	SGU60 - SGU60
	9	2.70	3.24	SGU60 - SGU60
	6	2.70	3.24	SGU60 - SGU60
	3	2.70	3.24	SGU60 - SGU60
	1	2.70	3.24	SGU60 - SGU60
2	14	2.70	3.24	SGU60 - SGU60
	11	2.70	3.24	SGU60 - SGU60
	8	2.70	3.24	SGU60 - SGU60
	5	2.70	3.24	SGU60 - SGU60
3	2	2.70	3.24	SGU60 - SGU60
	13	2.40	4.32	SGU60 - SGU60
	10	2.40	4.32	SGU60 - SGU60
	7	2.40	4.32	SGU60 - SGU60
	4	2.40	4.32	SGU60 - SGU60
4	1	2.40	4.32	SGU60 - SGU60
	12	2.40	4.32	SGU60 - SGU60
	9	2.40	4.32	SGU60 - SGU60
	6	2.40	4.32	SGU60 - SGU60
	3	2.40	4.32	SGU60 - SGU60
5	1	2.40	4.32	SGU60 - SGU60
	11	2.40	2.88	SGU60 - SGU60
	8	2.40	2.88	SGU60 - SGU60
	5	2.40	2.88	SGU60 - SGU60
6	2	2.40	2.88	SGU60 - SGU60
	10	2.10	3.78	SGU60 - SGU60
	7	2.10	3.78	SGU60 - SGU60
	4	2.10	3.78	SGU60 - SGU60
7	1	2.10	3.78	SGU60 - SGU60
	9	2.10	2.52	SGU60 - SGU60
	6	2.10	2.52	SGU60 - SGU60
	3	2.10	2.52	SGU60 - SGU60
8	1	2.10	2.52	SGU60 - SGU60
	8	1.80	3.24	SGU60 - SGU60
	5	1.80	3.24	SGU60 - SGU60

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Section	Course	Length [m]	Area [m ²]	Reinforcement
9	2	1.80	3.24	SGU60 - SGU60
	7	1.80	3.24	SGU60 - SGU60
	4	1.80	3.24	SGU60 - SGU60
10	1	1.80	3.24	SGU60 - SGU60
	6	1.50	1.80	SGU60 - SGU60
	3	1.50	1.80	SGU60 - SGU60
11	1	1.50	1.80	SGU60 - SGU60
	5	1.50	1.80	SGU60 - SGU60
	2	1.50	1.80	SGU60 - SGU60
12	5	1.50	1.80	SGU60 - SGU60
	2	1.50	1.80	SGU60 - SGU60
13	4	1.50	1.80	SGU60 - SGU60
	1	1.50	1.80	SGU60 - SGU60
14	3	1.20	2.16	SGU60 - SGU60
	1	1.20	2.16	SGU60 - SGU60
15	2	1.20	2.16	SGU60 - SGU60
16	1	1.20	2.16	SGU60 - SGU60

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Project: 202602029 - Mixed Use Residential Development

Site: 1493 Sixth Line, Oakville, ON

Date: 3/2/2026

Wall: Wall 3

Project Information

Client	February 2026		
Name	Mixed Use Residential Development	Number	202602029
Site	1493 Sixth Line, Oakville, ON	Designer	CYK
Revision	2	Created	3/2/2026
Standard	National Concrete Masonry Association 3rd Edition	Modified	3/2/2026
Seismic As	0.17	Default Deflection of 50.80 mm	
Comments			
Revision	Drawings provided on March 2, 2026		
Note			

Selected Facing Unit

Licenser/Product Line:	Risi Stone Systems
Name:	Sonoma Stone 375

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Project Summary

Geogroups

Geogroup	Layer	Length (m)
A	All	1.30
B	All	1.60
C	All	1.90
D	All	2.20
E	All	2.50
F	All	2.20
G	All	2.50

Quantities

Wall Length	37.20 m
Steps in Top of Wall	2
Steps in Bottom of Wall	15
Total Wall Area	69.6 m ²
Exposed Area (includes cap)	61.3 m ²
Embedded Area	8.3 m ²
Tallest Panel Height	3.15 m
Longest reinforcement length	2.50 m
Base soil volume	5.8 m ³
Infill soil volume ‡	110.8 m ³
Gravity Face Drain	0.0 m ³

Reinforcement

SGU60 - SGU60	276.5 m ²
---------------	----------------------

Note †: Total Facing Unit quantity is based on using full-sized units only on bottom course and an even mix of defined facing sizes, as identified elsewhere in this report, on remaining courses of each Section. The use of corners, tapered or cut units is not reflected in this quantity.

Note ‡: Reinforced fill values are calculated based on the average geogrid length in each Section. They do not account for anything beyond the reinforced zone (end of the geogrids). Actual infill values may be significantly higher.

Note : Drainage fill does not include the drainage stone within block. Core fill are calculated based on the percentage hollow core of the wall unit selected. If the percentage hollow core is not defined then the Core fill value within block will not be calculated.

Project Design Inputs

Design Standard National Concrete Masonry Association 3rd Edition

Minimum Factors of Safety

Conventional

External		Value	Internal		Value	Facing	Value
FSsl	Base Sliding	1.50	FSsl	Internal Sliding	1.50		
FSbc	Bearing Capacity	2.00	FSsc	Shear Capacity	1.50		
FSot	Overturning	1.50					

MultiDepth

External		Value	Internal		Value	Facing	Value
FSsl	Base Sliding	1.50					
FSbc	Bearing Capacity	2.00					
FSsh	Interface Shear	1.50					
FSot	Overturning	1.50					

No Fines

External		Value	Internal		Value	Facing	Value
FSsl	Base Sliding	1.50					
FSbc	Bearing Capacity	2.00					
FSot	Overturning	1.50					

Reinforced

External		Value	Internal		Value	Facing		Value
FSsl	Base Sliding	1.50	FSsl	Internal Sliding	1.50	FScs	Connection Strength	1.50
FSbc	Bearing Capacity	2.00	FSpO	Pullout	1.50	FSsc	Facing Shear	1.50
FSct	Crest Toppling	1.50	FSto	Tensile Overstress	1.50			
FSot	Overturning	2.00						

Seismic

Conventional

External		Value	Internal		Value	Facing	Value
FSsl	Base Sliding	1.10	FSsl	Internal Sliding	1.10		
FSbc	Bearing Capacity	1.10	FSsc	Shear Capacity	1.10		
FSot	Overturning	1.10					

MultiDepth

External		Value	Internal		Value	Facing	Value
FSsl	Base Sliding	1.10					
FSbc	Bearing Capacity	1.10					
FSsh	Interface Shear	1.10					
FSot	Overturning	1.10					

No Fines

External		Value	Internal		Value	Facing	Value
FSsl	Base Sliding	1.10					
FSbc	Bearing Capacity	1.50					
FSot	Overturning	1.10					

Reinforced

External		Value	Internal		Value	Facing		Value
FSsl	Base Sliding	1.10	FSsl	Internal Sliding	1.10	FScs	Connection Strength	1.10
FSbc	Bearing Capacity	1.50	FSpO	Pullout	1.10	FSsc	Facing Shear	1.10
FSct	Crest Toppling	1.10	FSto	Tensile Overstress	1.10			
FSot	Overturning	1.50						

Design Factors

Term	Description	Minimum (as appl.)	Maximum (as appl.)
RC	Reinforced coverage ratio	1.00	0.00

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Selected Facing Unit

Licenser/Product Line:	Risi Stone Systems	
Name:	Sonoma Stone 375	
Facing Height	Hu	0.19 m
Facing Width	Lu	1.20 m
Facing Depth	Wu	0.38 m
Facing Weight	Xu	21.8 kN/m ³
Center of Gravity	Gu	0.19 m
Setback	u	0.02 m
Batter		7.12 °
Cap Height	Hcu	0.19 m
Initial Shear Capacity	au	64.00 kN/m
Apparent Shear Angle	u	46.00 °
Maximum Shear Capacity	Vu(max)	125.00 kN/m

Selected Reinforcement Types**Reinforcements**

SGU60 - SGU60	Supplier: Strata Systems, Inc., Product Line: Strata SGU SERIES, Fill Type: Sands						
Tult	60.00 kN/m	RFcr	1.47	RFd	1.10	LTDS	33.73 kN/m
RFid	1.10	Cds	0.80	Ci	0.80		

Connection/Shear Properties

cs1	7.30 kN/m	IP-1	26.90 kN/m	cs2	21.00 kN/m	IP-2	57.00 kN/m
cs max	24.80 kN/m	au	25.00 kN/m	u	48.00 kN/m	Vu(max)	88.00 kN/m

Selected Soil Types

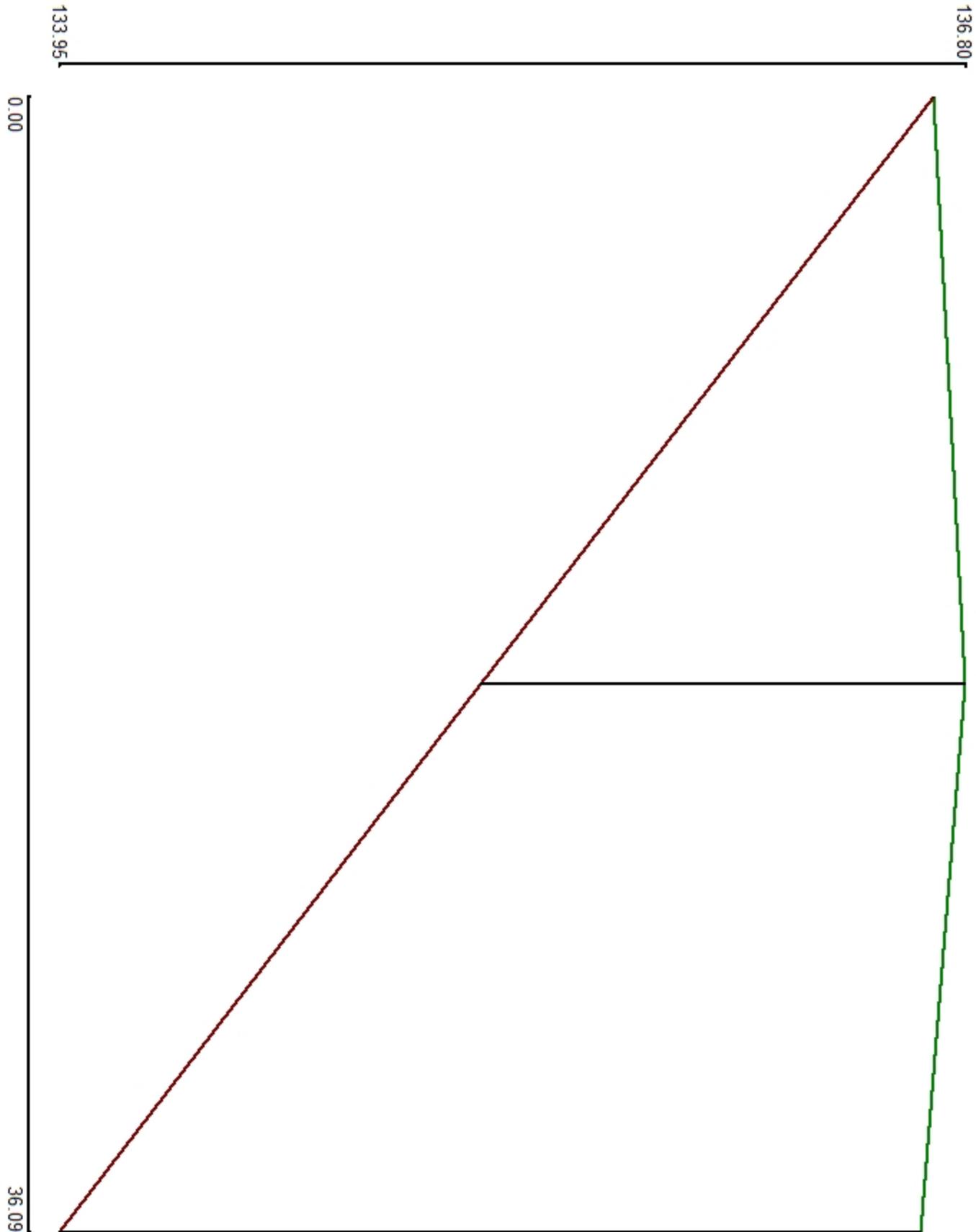
Soil Zone	Soil Type	In Situ		
		Friction Angle	Density [kN/m ³]	Cohesion Cf [kN/m ²]
Infill (i)	GW	33°	21.00	n/a
Retained (r)	CL	28°	19.95	n/a
Foundation (f)	CL	28°	19.95	0.00
Base (b)	GW	37°	22.00	n/a
Drainage (d)	GP	38°	19.64	n/a

Soil Glossary

CH:	Inorganic clays, high plasticity
CL:	Inorganic clays, low to medium plasticity, gravelly, sandy, silty, lean clays
GC:	Clayey gravels, poorly graded gravel-sand-clay mixtures
GM:	Silty gravels, poorly graded gravel-sand-silt mixtures
GP:	1/2"-3/4" clean crushed stone or crushed gravel
GW:	Well-graded gravels, gravel-sand. Little or no fines.
MH:	Inorganic clayey silts, elastic silts
ML:	Inorganic silts, very fine sands, silty or clayey, slight plasticity
SC:	Clayey sands, poorly graded sand-clay mixtures
SM:	Silty sands, poorly graded sand-silt mixtures
SP:	Poorly-graded sands, gravelly sands. Little or no fines.
SW:	Well-graded sands, gravelly sands. Little or no fines.

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Station Detail



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Note: Station Layout is the face view of the wall, looking at it from left to right

Station Layout

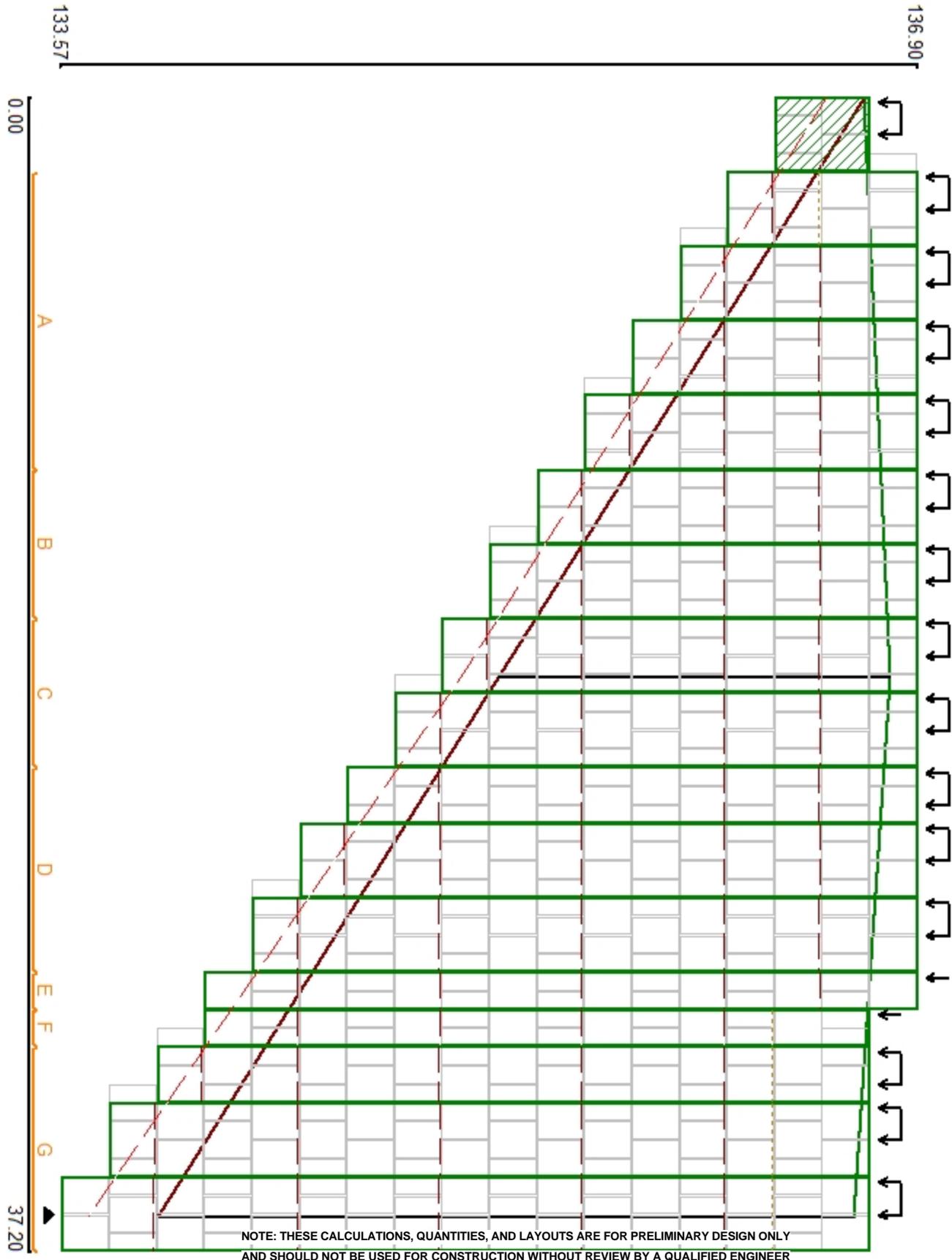
No.	Station [m]	Top [m]	Bottom [m]	Height [m]
1	0.00	136.70	136.70	0.00
2	18.68	136.80	135.28	1.52
3	36.09	136.66	133.95	2.71

Station Wall Length	36.09 m
Minimum Height	1.52 m
Maximum Height	2.71 m

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Section Geometry

Section Drawing



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Section Extents

Section	Top Elevation [m]	Base Elevation [m]	Left Side [m]	Right Side [m]	Bottom Grade Elevation [m]
1	136.72	136.35	0.00	2.40	136.52
2	136.90	136.16	2.40	4.80	136.33
3	136.90	135.98	4.80	7.20	136.15
4	136.90	135.79	7.20	9.60	135.97
5	136.90	135.60	9.60	12.00	135.79
6	136.90	135.42	12.00	14.40	135.60
7	136.90	135.24	14.40	16.80	135.42
8	136.90	135.05	16.80	19.20	135.24
9	136.90	134.87	19.20	21.60	135.05
10	136.90	134.68	21.60	23.40	134.92
11	136.90	134.50	23.40	25.80	134.73
12	136.90	134.31	25.80	28.20	134.55
13	136.90	134.13	28.20	29.40	134.46
14	136.72	134.13	29.40	30.60	134.37
15	136.72	133.94	30.60	32.40	134.23
16	136.72	133.76	32.40	34.80	134.05
17	136.72	133.57	34.80	37.20	133.95

Section Measurements

Section	Height [m]	Design Height [m]	Width [m]	Face Area [m ²]	Embedment [m]	Infill Volume [m ³]
1	0.37	0.37	2.40	0.9	0.17	0.0
2	0.74	0.74	2.40	1.8	0.17	1.3
3	0.93	0.93	2.40	2.2	0.18	1.7
4	1.11	1.11	2.40	2.7	0.18	2.1
5	1.30	1.30	2.40	3.1	0.18	2.6
6	1.48	1.48	2.40	3.6	0.18	4.0
7	1.66	1.66	2.40	4.0	0.18	4.6
8	1.85	1.85	2.40	4.4	0.19	6.4
9	2.03	2.03	2.40	4.9	0.19	7.1
10	2.22	2.22	1.80	4.0	0.24	6.9
11	2.41	2.41	2.40	5.8	0.24	9.9
12	2.59	2.59	2.40	6.2	0.24	10.7
13	2.78	2.78	1.20	3.3	0.33	6.6
14	2.59	2.59	1.20	3.1	0.24	5.7
15	2.78	2.78	1.80	5.0	0.29	10.6
16	2.96	2.96	2.40	7.1	0.29	15.0
17	3.15	3.15	2.40	7.5	0.38	15.8

Section Slopes

Section	Crest Slope [°]	Crest Offset [m]	Toe Slope [°]	Toe Offset [m]
1	0.00	0.00	0.00	0.00
2	0.00	0.00	0.00	0.00
3	0.00	0.00	0.00	0.00
4	0.00	0.00	0.00	0.00
5	0.00	0.00	0.00	0.00
6	0.00	0.00	0.00	0.00
7	0.00	0.00	0.00	0.00
8	0.00	0.00	0.00	0.00
9	0.00	0.00	0.00	0.00
10	0.00	0.00	0.00	0.00
11	0.00	0.00	0.00	0.00
12	0.00	0.00	0.00	0.00
13	0.00	0.00	0.00	0.00
14	0.00	0.00	0.00	0.00
15	0.00	0.00	0.00	0.00
16	0.00	0.00	0.00	0.00

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Section	Crest Slope [°]	Crest Offset [m]	Toe Slope [°]	Toe Offset [m]
17	0.00	0.00	0.00	0.00

Section Loads

Section	Live Load [kN/m ²]	Live Offset [m]	Dead Load [kN/m ²]	Dead Offset [m]
1	4.8	0.00	0.0	0.00
2	4.8	0.00	0.0	0.00
3	4.8	0.00	0.0	0.00
4	4.8	0.00	0.0	0.00
5	4.8	0.00	0.0	0.00
6	4.8	0.00	0.0	0.00
7	4.8	0.00	0.0	0.00
8	4.8	0.00	0.0	0.00
9	4.8	0.00	0.0	0.00
10	4.8	0.00	0.0	0.00
11	4.8	0.00	0.0	0.00
12	4.8	0.00	0.0	0.00
13	4.8	0.00	0.0	0.00
14	4.8	0.00	0.0	0.00
15	4.8	0.00	0.0	0.00
16	4.8	0.00	0.0	0.00
17	4.8	0.00	0.0	0.00

Reinforcement Details

Section	Course	Length [m]	Area [m ²]	Reinforcement
2	1	1.30	3.12	SGU60 - SGU60
3	3	1.30	3.12	SGU60 - SGU60
	1	1.30	3.12	SGU60 - SGU60
4	4	1.30	3.12	SGU60 - SGU60
	2	1.30	3.12	SGU60 - SGU60
5	5	1.30	3.12	SGU60 - SGU60
	3	1.30	3.12	SGU60 - SGU60
	1	1.30	3.12	SGU60 - SGU60
6	6	1.60	3.84	SGU60 - SGU60
	4	1.60	3.84	SGU60 - SGU60
	1	1.60	3.84	SGU60 - SGU60
7	7	1.60	3.84	SGU60 - SGU60
	5	1.60	3.84	SGU60 - SGU60
	2	1.60	3.84	SGU60 - SGU60
8	8	1.90	4.56	SGU60 - SGU60
	6	1.90	4.56	SGU60 - SGU60
	3	1.90	4.56	SGU60 - SGU60
	1	1.90	4.56	SGU60 - SGU60
9	9	1.90	4.56	SGU60 - SGU60
	7	1.90	4.56	SGU60 - SGU60
	4	1.90	4.56	SGU60 - SGU60
	1	1.90	4.56	SGU60 - SGU60
10	10	2.20	3.96	SGU60 - SGU60
	8	2.20	3.96	SGU60 - SGU60
	5	2.20	3.96	SGU60 - SGU60
	2	2.20	3.96	SGU60 - SGU60
11	11	2.20	5.28	SGU60 - SGU60
	9	2.20	5.28	SGU60 - SGU60
	6	2.20	5.28	SGU60 - SGU60
	3	2.20	5.28	SGU60 - SGU60
	1	2.20	5.28	SGU60 - SGU60
12	12	2.20	5.28	SGU60 - SGU60
	10	2.20	5.28	SGU60 - SGU60
	7	2.20	5.28	SGU60 - SGU60
	4	2.20	5.28	SGU60 - SGU60

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Section	Course	Length [m]	Area [m ²]	Reinforcement
13	1	2.20	5.28	SGU60 - SGU60
	13	2.50	3.00	SGU60 - SGU60
	11	2.50	3.00	SGU60 - SGU60
	8	2.50	3.00	SGU60 - SGU60
	5	2.50	3.00	SGU60 - SGU60
14	2	2.50	3.00	SGU60 - SGU60
	11	2.20	2.64	SGU60 - SGU60
	8	2.20	2.64	SGU60 - SGU60
	5	2.20	2.64	SGU60 - SGU60
	2	2.20	2.64	SGU60 - SGU60
15	12	2.50	4.50	SGU60 - SGU60
	9	2.50	4.50	SGU60 - SGU60
	6	2.50	4.50	SGU60 - SGU60
	3	2.50	4.50	SGU60 - SGU60
	1	2.50	4.50	SGU60 - SGU60
16	13	2.50	6.00	SGU60 - SGU60
	10	2.50	6.00	SGU60 - SGU60
	7	2.50	6.00	SGU60 - SGU60
	4	2.50	6.00	SGU60 - SGU60
	1	2.50	6.00	SGU60 - SGU60
17	14	2.50	6.00	SGU60 - SGU60
	11	2.50	6.00	SGU60 - SGU60
	8	2.50	6.00	SGU60 - SGU60
	5	2.50	6.00	SGU60 - SGU60
	2	2.50	6.00	SGU60 - SGU60

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Project: 202602029 - Mixed Use Residential Development

Site: 1493 Sixth Line, Oakville, ON

Date: 3/2/2026

Wall: Wall 4

Project Information

Client	February 2026		
Name	Mixed Use Residential Development	Number	202602029
Site	1493 Sixth Line, Oakville, ON	Designer	CYK
Revision	2	Created	3/2/2026
Standard	National Concrete Masonry Association 3rd Edition	Modified	3/2/2026
Seismic As	0.17	Default Deflection of 50.80 mm	
Comments			
Revision	Drawings provided on March 2, 2026		
Note			

Selected Facing Unit

Licenser/Product Line: Risi Stone Systems
Name: Sonoma Stone 375

NOTE: THESE CALCULATIONS, QUANTITIES, AND LAYOUTS ARE FOR PRELIMINARY DESIGN ONLY
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Project Summary

Quantities

Wall Length	34.20 m
Steps in Bottom of Wall	1
Total Wall Area	18.0 m ²
Exposed Area (includes cap)	12.1 m ²
Embedded Area	5.9 m ²
Corner Quantity	2
Tallest Panel Height	0.56 m
Base soil volume	5.3 m ³
Infill soil volume ‡	0.0 m ³
Gravity Face Drain	1.6 m ³

Reinforcement

SGU60 - SGU60	0.0 m ²
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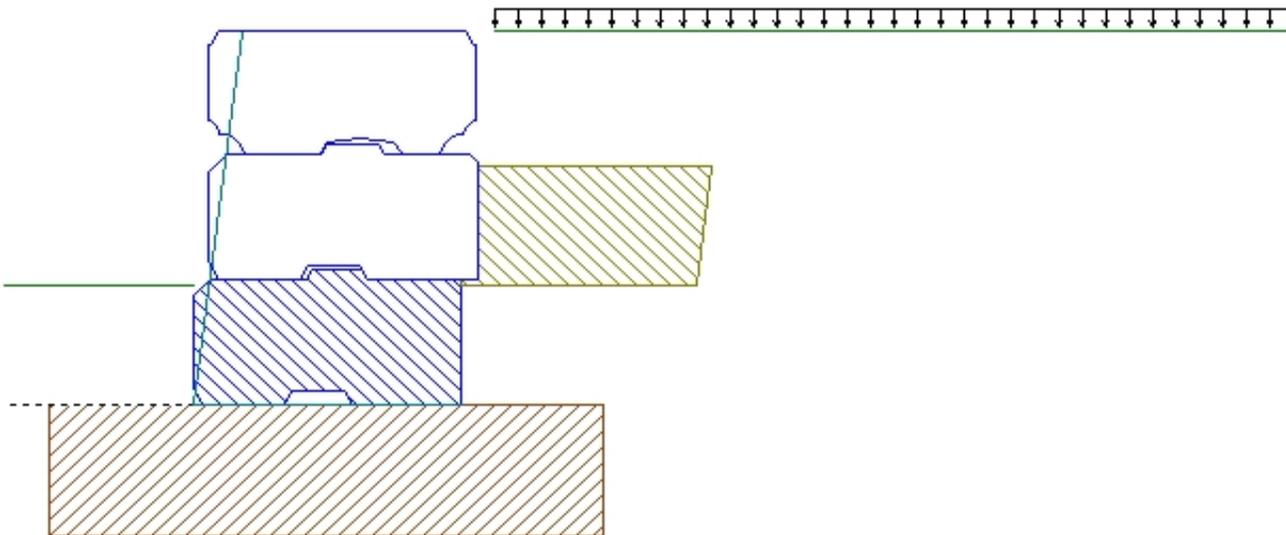
Note †: Total Facing Unit quantity is based on using full-sized units only on bottom course and an even mix of defined facing sizes, as identified elsewhere in this report, on remaining courses of each Section. The use of corners, tapered or cut units is not reflected in this quantity.

Note ‡: Reinforced fill values are calculated based on the average geogrid length in each Section. They do not account for anything beyond the reinforced zone (end of the geogrids). Actual infill values may be significantly higher.

Note : Drainage fill does not include the drainage stone within block. Core fill are calculated based on the percentage hollow core of the wall unit selected. If the percentage hollow core is not defined then the Core fill value within block will not be calculated.

Tallest Section

Section Height 0.56 m



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Project Design Inputs

Design Standard National Concrete Masonry Association 3rd Edition

Minimum Factors of Safety

Conventional

External		Value	Internal		Value	Facing	Value
FSsl	Base Sliding	1.50	FSsl	Internal Sliding	1.50		
FSbc	Bearing Capacity	2.00	FSsc	Shear Capacity	1.50		
FSot	Overturning	1.50					

MultiDepth

External		Value	Internal		Value	Facing	Value
FSsl	Base Sliding	1.50					
FSbc	Bearing Capacity	2.00					
FSsh	Interface Shear	1.50					
FSot	Overturning	1.50					

No Fines

External		Value	Internal		Value	Facing	Value
FSsl	Base Sliding	1.50					
FSbc	Bearing Capacity	2.00					
FSot	Overturning	1.50					

Reinforced

External		Value	Internal		Value	Facing		Value
FSsl	Base Sliding	1.50	FSsl	Internal Sliding	1.50	FScs	Connection Strength	1.50
FSbc	Bearing Capacity	2.00	FSpO	Pullout	1.50	FSsc	Facing Shear	1.50
FSct	Crest Toppling	1.50	FSto	Tensile Overstress	1.50			
FSot	Overturning	2.00						

Seismic

Conventional

External		Value	Internal		Value	Facing	Value
FSsl	Base Sliding	1.10	FSsl	Internal Sliding	1.10		
FSbc	Bearing Capacity	1.10	FSsc	Shear Capacity	1.10		
FSot	Overturning	1.10					

MultiDepth

External		Value	Internal		Value	Facing	Value
FSsl	Base Sliding	1.10					
FSbc	Bearing Capacity	1.10					
FSsh	Interface Shear	1.10					
FSot	Overturning	1.10					

No Fines

External		Value	Internal		Value	Facing	Value
FSsl	Base Sliding	1.10					
FSbc	Bearing Capacity	1.50					
FSot	Overturning	1.10					

Reinforced

External		Value	Internal		Value	Facing		Value
FSsl	Base Sliding	1.10	FSsl	Internal Sliding	1.10	FScs	Connection Strength	1.10
FSbc	Bearing Capacity	1.50	FSpO	Pullout	1.10	FSsc	Facing Shear	1.10
FSct	Crest Toppling	1.10	FSto	Tensile Overstress	1.10			
FSot	Overturning	1.50						

Design Factors

Term	Description	Minimum (as appl.)	Maximum (as appl.)
RC	Reinforced coverage ratio	1.00	0.00

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Selected Facing Unit

Licenser/Product Line:	Risi Stone Systems	
Name:	Sonoma Stone 375	
Facing Height	Hu	0.19 m
Facing Width	Lu	1.20 m
Facing Depth	Wu	0.38 m
Facing Weight	Xu	21.8 kN/m ³
Center of Gravity	Gu	0.19 m
Setback	u	0.02 m
Batter		7.12 °
Cap Height	Hcu	0.19 m
Initial Shear Capacity	au	64.00 kN/m
Apparent Shear Angle	u	46.00 °
Maximum Shear Capacity	Vu(max)	125.00 kN/m

Selected Reinforcement Types**Reinforcements**

SGU60 - SGU60	Supplier: Strata Systems, Inc., Product Line: Strata SGU SERIES, Fill Type: Sands						
Tult	60.00 kN/m	RFcr	1.47	RFd	1.10	LTDS	33.73 kN/m
RFid	1.10	Cds	0.80	Ci	0.80		

Connection/Shear Properties

cs1	7.30 kN/m	IP-1	26.90 kN/m	cs2	21.00 kN/m	IP-2	57.00 kN/m
cs max	24.80 kN/m	au	25.00 kN/m	u	48.00 kN/m	Vu(max)	88.00 kN/m

Selected Soil Types

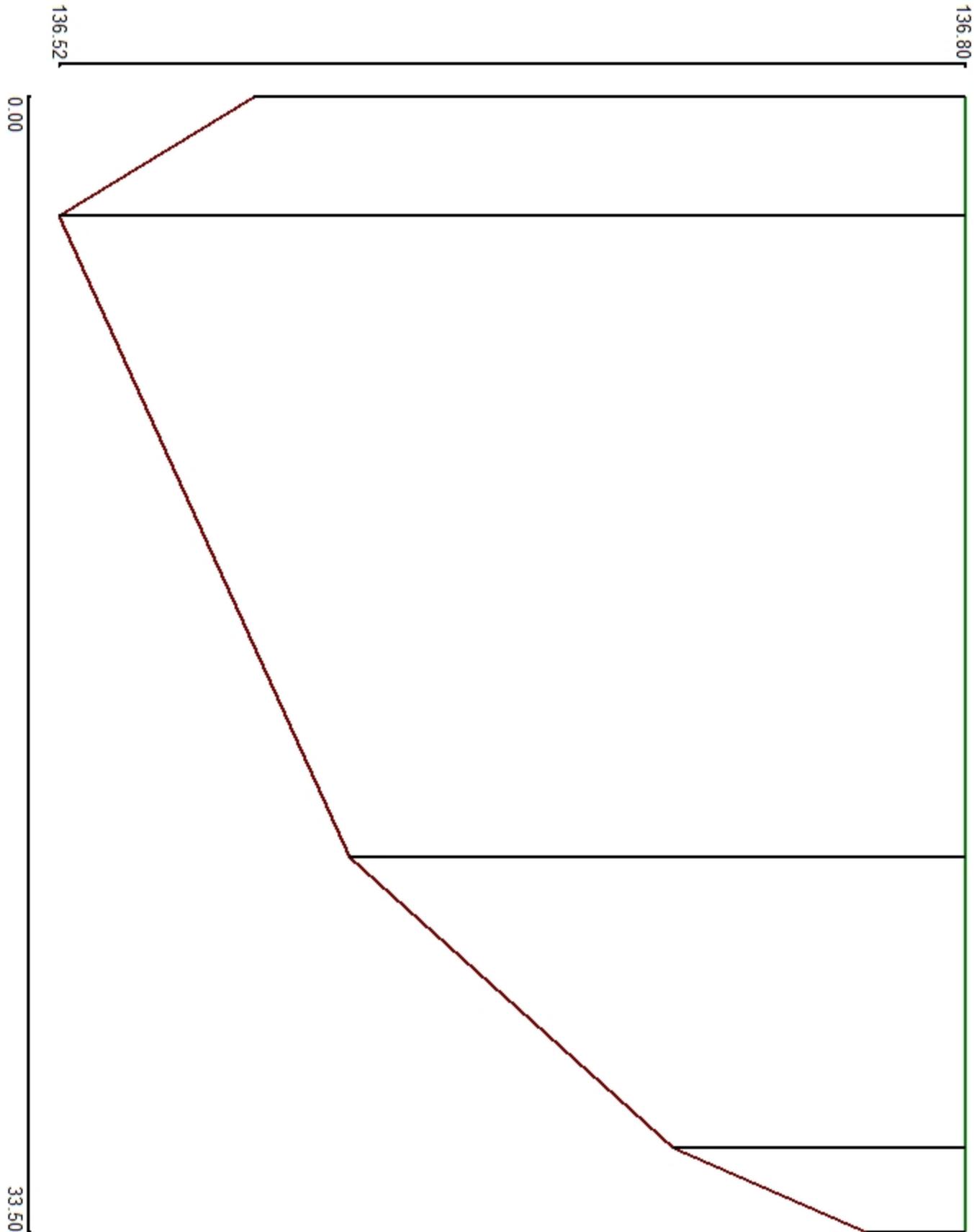
Soil Zone	Soil Type	In Situ		
		Friction Angle	Density [kN/m ³]	Cohesion Cf [kN/m ²]
Infill (i)	GW	33°	21.00	n/a
Retained (r)	CL	28°	19.95	n/a
Foundation (f)	CL	28°	19.95	0.00
Base (b)	GW	37°	22.00	n/a
Drainage (d)	GP	38°	19.64	n/a

Soil Glossary

CH:	Inorganic clays, high plasticity
CL:	Inorganic clays, low to medium plasticity, gravelly, sandy, silty, lean clays
GC:	Clayey gravels, poorly graded gravel-sand-clay mixtures
GM:	Silty gravels, poorly graded gravel-sand-silt mixtures
GP:	1/2"-3/4" clean crushed stone or crushed gravel
GW:	Well-graded gravels, gravel-sand. Little or no fines.
MH:	Inorganic clayey silts, elastic silts
ML:	Inorganic silts, very fine sands, silty or clayey, slight plasticity
SC:	Clayey sands, poorly graded sand-clay mixtures
SM:	Silty sands, poorly graded sand-silt mixtures
SP:	Poorly-graded sands, gravelly sands. Little or no fines.
SW:	Well-graded sands, gravelly sands. Little or no fines.

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Station Detail



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Note: Station Layout is the face view of the wall, looking at it from left to right

Station Layout

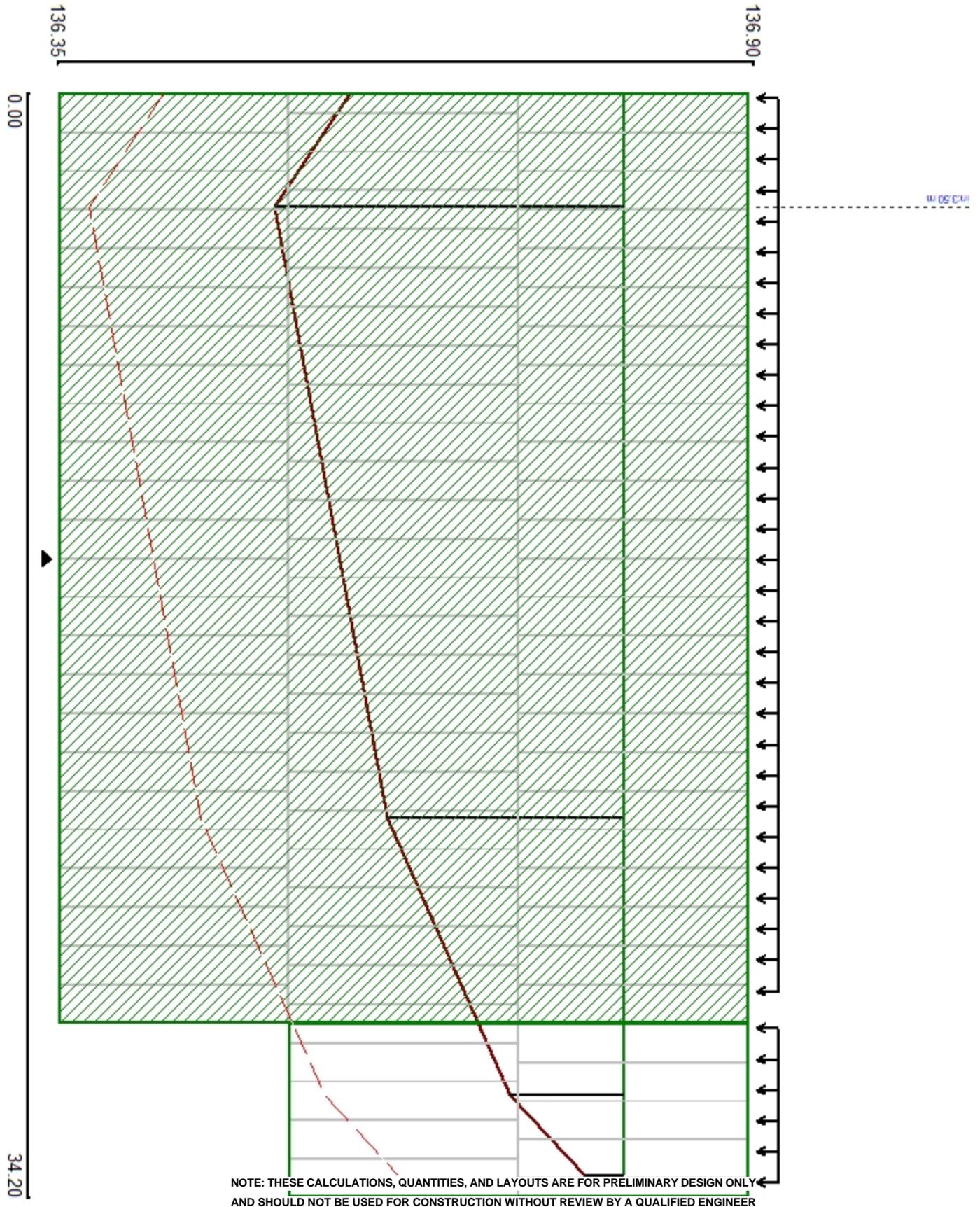
No.	Station [m]	Top [m]	Bottom [m]	Height [m]
1	0.00	136.80	136.58	0.22
2	3.50	136.80	136.52	0.28
3	22.45	136.80	136.61	0.19
4	31.00	136.80	136.71	0.09
5	33.50	136.80	136.77	0.03

Station Wall Length	33.50 m
Minimum Height	0.03 m
Maximum Height	0.28 m

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Section Geometry

Section Drawing



Markers

No.	Station	Code	Note
1	3.50	in	

Section Extents

Section	Top Elevation [m]	Base Elevation [m]	Left Side [m]	Right Side [m]	Bottom Grade Elevation [m]
1	136.90	136.35	0.00	28.80	136.52
2	136.90	136.53	28.80	34.20	136.68

Section Measurements

Section	Height [m]	Design Height [m]	Width [m]	Face Area [m ²]	Embedment [m]	Infill Volume [m ³]
1	0.56	0.56	28.80	16.0	0.18	0.0
2	0.37	0.37	5.40	2.0	0.15	0.0

Section Slopes

Section	Crest Slope [°]	Crest Offset [m]	Toe Slope [°]	Toe Offset [m]
1	0.00	0.00	0.00	0.00
2	0.00	0.00	0.00	0.00

Section Loads

Section	Live Load [kN/m ²]	Live Offset [m]	Dead Load [kN/m ²]	Dead Offset [m]
1	2.4	0.00	0.0	0.00
2	2.4	0.00	0.0	0.00

Reinforcement Details

Section	Course	Length [m]	Area [m ²]	Reinforcement
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RisiStone[®]
retaining wall systems

Construction Review & Inspection Guidelines

Inspection Checklist

(To be Used in Conjunction with Project Design, Specifications, and Sound Engineering Judgement)

Steps	Inspection Items	Remarks
Survey	<input type="checkbox"/> All stake locations and elevations in agreement with design.	<hr/> <hr/>
Excavation	<input type="checkbox"/> All utilities, structures, etc. are located prior to excavation and approval granted from governing bodies. <input type="checkbox"/> Excavation requirements are met or exceeded to allow for construction of wall, including required wall embedment and base depth. <input type="checkbox"/> The exposed retained and foundation soil conditions meet or exceed design requirements (internal friction angle, soil type, and unit weight). <input type="checkbox"/> All excavations conducted in accordance with regulatory requirements. In areas where safe excavations are not possible due to property line constraints/other structures, etc., temporary shoring may be required. <input type="checkbox"/> Presence of existing or proposed structures relative to the wall noted and designer is notified if these lie within impact zone of wall. <input type="checkbox"/> If water encountered, proper dewatering techniques used to ensure dry base construction.	<hr/> <hr/> <hr/> <hr/> <hr/> <hr/> <hr/>
Foundation Preparation	<input type="checkbox"/> The foundation soil (sub-grade) meets minimum allowable bearing capacity stated in the design. <input type="checkbox"/> Unsuitable soil removed and replaced under direction of Site Geotechnical Engineer. For geogrid reinforced structures, replacement of unsuitable material must include entire footprint of wall (facing AND geogrid reinforced zone). Replacement material must extend at 1H:1V from front and back of footprint to suitable founding depth. <input type="checkbox"/> Engineered fill material compacted to 95% SPD or as specified in the design.	<hr/> <hr/> <hr/> <hr/>
Base Preparation	<input type="checkbox"/> Base material is as specified in the design (well-graded angular gravel). <input type="checkbox"/> Compaction density not less than 98% SPD. <input type="checkbox"/> Base dimensions are as specified in the design. <input type="checkbox"/> The surface is level front to back and side to side. A 50mm (2in) unreinforced concrete leveling pad may be placed on top of the gravel base. <input type="checkbox"/> Base stepping as per design to ensure minimum required embedment is maintained at all times.	<hr/> <hr/> <hr/> <hr/> <hr/>

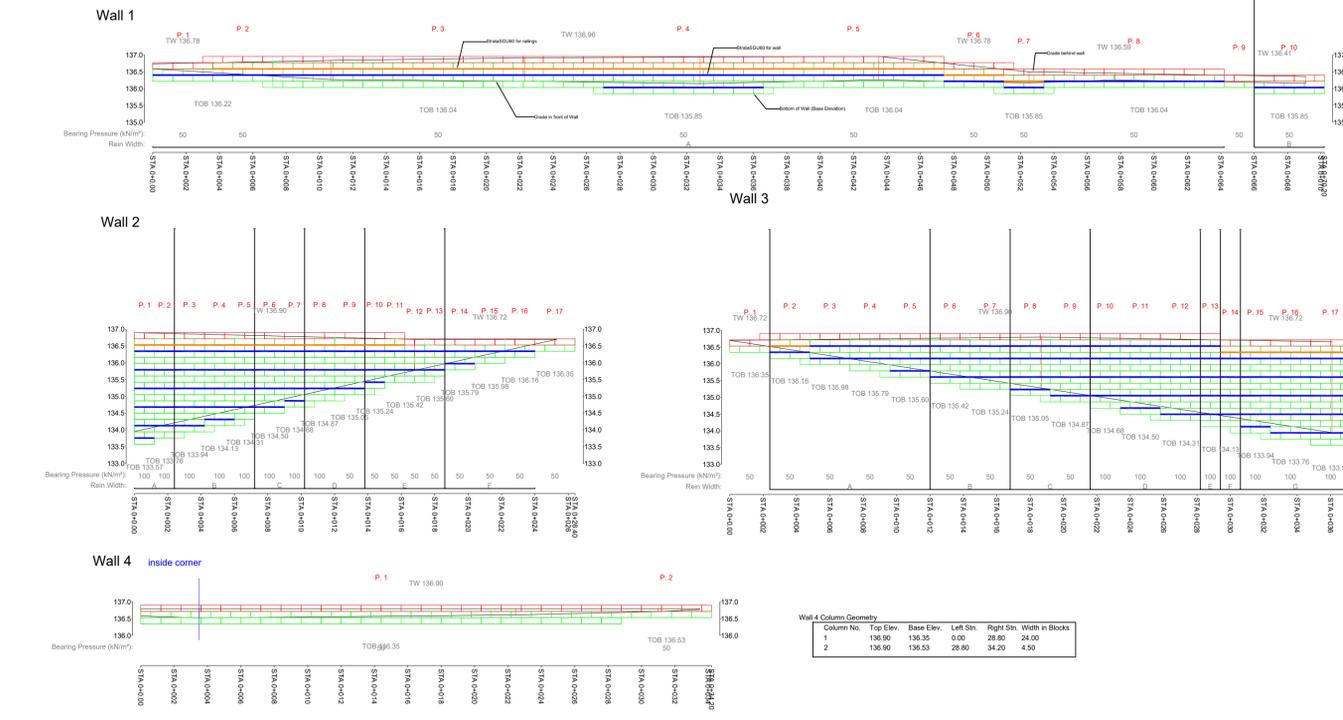


RisiStone[®]
retaining wall systems

Design Drawings

WALL FRONT ELEVATION VIEWS

Scale Vertical -1:100 Horizontal -1:200



PANEL GEOMETRY & GEOGRID GROUPS

Wall 1 Column Geometry

Column No.	Top Elev.	Base Elev.	Left Stn.	Right Stn.	Width in Blocks
1	136.78	136.22	3.60	7.20	3.00
2	136.96	136.22	3.60	7.20	3.00
3	136.96	136.04	7.20	27.00	16.80
4	136.96	135.85	27.00	36.00	8.00
5	136.96	136.04	36.00	47.40	9.00
6	136.78	136.04	47.40	51.00	3.00
7	136.59	135.85	51.00	53.40	2.00
8	136.59	136.04	53.40	64.20	9.00
9	136.41	136.04	64.20	66.00	1.50
10	136.41	135.85	66.00	70.20	3.50

Wall 1 Geogrids

Group	Layer	Length (m)	Panels	Wall Span (m)
A	All	1.20	1-8	0.90-64.20
B	All	1.20	10-10	66.00-70.20

Wall 2 Column Geometry

Column No.	Top Elev.	Base Elev.	Left Stn.	Right Stn.	Width in Blocks
1	136.90	133.57	0.00	1.20	1.00
2	136.90	133.76	1.20	2.40	1.00
3	136.90	133.94	2.40	4.20	1.50
4	136.90	134.13	4.20	6.00	1.50
5	136.90	134.31	6.00	7.20	1.00
6	136.90	134.50	7.20	9.00	1.50
7	136.90	134.68	9.00	10.20	1.00
8	136.90	134.87	10.20	12.00	1.50
9	136.90	135.05	12.00	13.80	1.50
10	136.90	135.24	13.80	15.00	1.00
11	136.90	135.42	15.00	16.20	1.00
12	136.72	135.42	16.20	17.40	1.00
13	136.72	135.60	17.40	18.60	1.00
14	136.72	135.78	18.60	20.40	1.50
15	136.72	135.96	20.40	22.20	1.50
16	136.72	136.16	22.20	24.00	1.50
17	136.72	136.35	24.00	26.40	2.00

Wall 2 Geogrids

Group	Layer	Length (m)	Panels	Wall Span (m)
A	All	2.70	1-2	0.00-2.40
B	All	2.40	3-5	2.40-7.20
C	All	2.10	6-7	7.20-10.20
D	All	1.80	8-9	10.20-13.80
E	All	1.50	10-13	13.80-18.60
F	All	1.20	14-16	18.60-24.00

Wall 3 Column Geometry

Column No.	Top Elev.	Base Elev.	Left Stn.	Right Stn.	Width in Blocks
1	136.72	136.35	0.00	2.40	2.00
2	136.90	136.16	2.40	4.80	2.00
3	136.90	135.98	4.80	7.20	2.00
4	136.90	135.79	7.20	9.60	2.00
5	136.90	135.60	9.60	12.00	2.00
6	136.90	135.42	12.00	14.40	2.00
7	136.90	135.24	14.40	16.80	2.00
8	136.90	135.05	16.80	19.20	2.00
9	136.90	134.87	19.20	21.60	2.00
10	136.90	134.68	21.60	24.00	1.50
11	136.90	134.50	23.40	25.80	2.00
12	136.90	134.31	25.80	28.20	2.00
13	136.90	134.13	28.20	29.40	1.00
14	136.72	134.13	29.40	30.60	1.00
15	136.72	133.94	30.60	32.40	1.50
16	136.72	133.76	32.40	34.80	2.00
17	136.72	133.57	34.80	37.20	2.00

Wall 3 Geogrids

Group	Layer	Length (m)	Panels	Wall Span (m)
A	All	1.30	2-5	2.40-12.00
B	All	1.60	6-7	12.00-16.80
C	All	1.90	8-9	16.80-21.60
D	All	2.20	10-12	21.60-28.20
E	All	2.50	13-13	28.20-29.40
F	All	2.20	14-14	29.40-30.60
G	All	2.50	15-17	30.60-37.20

Wall 4 Column Geometry

Column No.	Top Elev.	Base Elev.	Left Stn.	Right Stn.	Width in Blocks
1	136.90	136.35	0.00	28.80	24.00
2	136.90	136.53	28.80	34.00	4.50

Wall 4 Geogrids

Group	Layer	Length (m)	Panels	Wall Span (m)
A	All	1.20	1-2	0.00-2.40
B	All	1.20	10-10	66.00-70.20

TABLE 1.0 - SOIL ZONES & ASSUMED PROPERTIES

MUST BE VERIFIED BY GENERAL REVIEW ENGINEER - REFER TO SECTION 3.03 OF SPECIFICATION

Soil Region	Drainage Fill (as req. - ref to section)			
	Reinforced Fill	Retained Fill/Soil**	Foundation Soil**	Base
Description (by USCS)	GW Well graded gravel Max. 5% fines	ML-CL Lean Silty Clay Low Plasticity	ML-CL Lean Silty Clay Low Plasticity	GW Well graded gravel Max. 5% fines
Effective Internal Friction Angle (Deg.)	33°	28°	28°	37°
Compaction Requirement (Eng. Fills Only)	95% SPD (+/- 2% opt. moist.)	95% SPD (+/- 2% opt. moist.)	98% SPD (+/- 2% opt. moist.)	98% SPD (+/- 2% opt. moist.)
Moist Unit Weight (kN/cu.m)	21	20	20	18
Effective Cohesion (kPa)	NA	NA	NA	NA
Soil Notes	Max. 150-200mm Compaction Lifts	Max. 150-200mm Compaction Lifts	See Section for req. ALLOW Bearing Capacities	Max. 150-200mm Compaction Lifts
Geotextile at Interface	Interface: Reinforced/Retained Geotextile Not Req. if gradation listed below are met. Otherwise: TBD	NA	NA	NA
Assumed Gradation for Filtration Req.†	D(15) < 0.3mm D(50) < 1.18mm MAX 5% FINES	D(85) > 0.075mm D(15) < 0.002mm D(50) < 0.05mm	NA	NA

* If the above gradation requirements are not met, an alternative filter fabric will be required. Contact RSS to discuss alternatives.

† Engineered Fill is defined as Clean earth fill placed and compacted in maximum lift thicknesses of 150mm to at least 98 percent Standard Proctor Density for Foundation Soils and 95 percent Standard Proctor Density for Retained Soils, under the full-time inspection and testing of a geotechnical engineering firm who provides written confirmation and certification of the completed Engineered Fill.

TABLE 2.0 - DESIGN INFORMATION

Retaining Wall System	Max. Slope Above Wall	Max. Surcharge Above Wall (kPa)	Batter of Wall (Degrees)	Maximum Height (mm)
Sonomastone Manufactured by Unilock Inc.	horizontal	12	7.12	3330

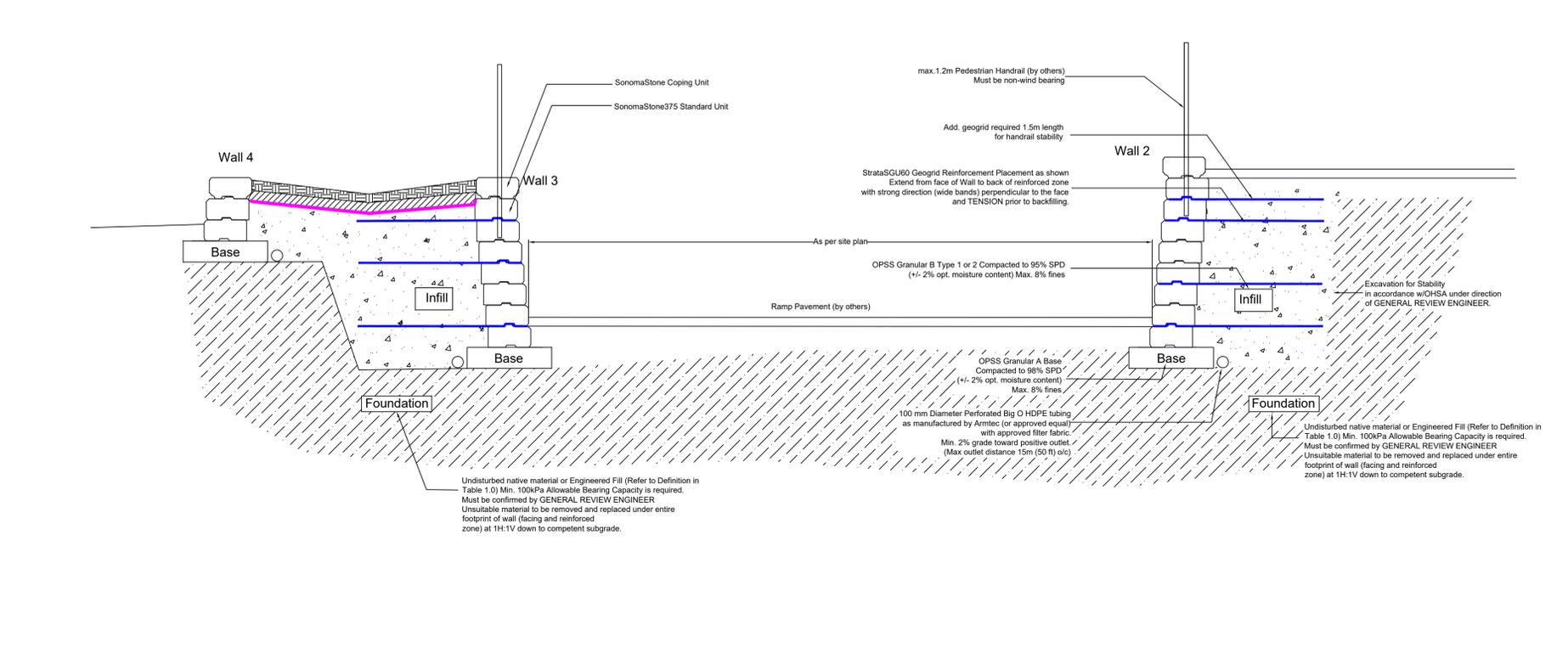
Geogrid Type	Min. Geogrid LTDS (kN/m)	Max. Slope Below Wall	Depth of Embedment (mm)	Compacted Base Dimensions (min height x mm width)
SGU 60 by Stratagysystems	31	None	As shown	200 x 750

GENERAL NOTES

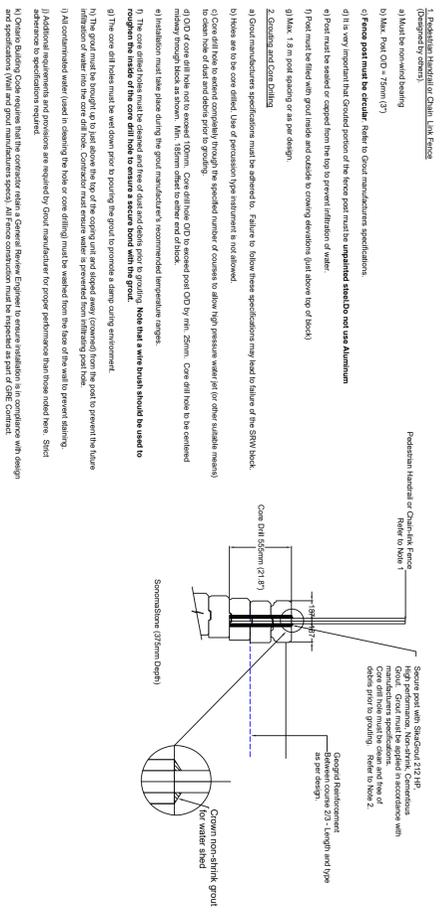
- THE INFORMATION PROVIDED ON THIS SHEET MUST BE USED IN CONJUNCTION WITH THE ATTACHED SPECIFICATIONS.
- THIS DESIGN IS BASED ON INFORMATION PROVIDED IN DRAWING NO. C01 BY ALPINE & MARTIN CONSULTANTS LTD ON MARCH 2, 2026. THESE WALL DESIGN DRAWINGS ARE NOT INTENDED TO BE "STAND ALONE" DRAWINGS. THE WALL CONTRACTOR AND GENERAL CONTRACTOR ARE REQUIRED TO HAVE A COMPLETE UNDERSTANDING OF ANY AND ALL OTHER STRUCTURES THAT MAY INTERACT WITH THIS SEGMENTAL RETAINING WALL. THE WALL CONTRACTOR AND GENERAL CONTRACTOR MUST REFER TO A FULL SET OF CIVIL, STRUCTURAL AND ARCHITECTURAL DRAWINGS (AS APPLICABLE) FOR THE PROJECT TO ENSURE SUCCESSFUL CONSTRUCTION AND PERFORMANCE OF THE WALL SYSTEM. THIS WALL DESIGN DRAWING SHOULD NOT BE REFERRED TO FOR MANHOLE LOCATIONS, ELEVATIONS, OR ANY OTHER CIVIL OR SITE INFRASTRUCTURE INFORMATION BECAUSE DATA MAY HAVE BEEN SELECTIVELY REMOVED FROM THIS DRAWING FOR CLARITY OF WALL ILLUSTRATION.
- DESIGN ASSUMPTIONS:
A) THE SRW DESIGN ASSUMES THE FOLLOWING:
A) THE FOUNDATION SOILS WILL PRODUCE ACCEPTABLE TOTAL AND DIFFERENTIAL SETTLEMENT GIVEN THE APPLIED LOAD OF THE SRW (MAX. 25 mm TOTAL OR DIFFERENTIAL SETTLEMENT AS VERIFIED BY GRE).
B) THE MAXIMUM GROUNDWATER ELEVATION IS BELOW THE BASE OF THE SRW.
C) THERE WILL BE NO HYDROSTATIC PRESSURE WITHIN OR BEHIND THE SRW.
D) THE SURROUNDING STRUCTURES WILL NOT EXERT ANY ADDITIONAL LOADING ON THE SRW (I.E. AN ADJACENT STRUCTURAL FOUNDATION IS AT OR BELOW PROPOSED LEVELING BASE OR OUTSIDE OF A CIRCUMFERENTIAL ZONE OF INFLUENCE AS DETERMINED BY THE GENERAL REVIEW ENGINEER).
E) THERE ARE NO STRUCTURES (UTILITIES SUCH AS GAS/WATER MAINS, SRW SEWERS, ELECTRICAL/COMMUNICATIONS CABLES, ETC.) TO BE PLACED WITHIN OR BELOW THE REINFORCED FILL DURING OR AFTER CONSTRUCTION.
- AT THIS STAGE IN THE DESIGN, RISI STONE SYSTEMS HAS NOT RECEIVED SITE SPECIFIC GEOTECHNICAL INFORMATION / GEOTECHNICAL REPORT. FOR DESIGN PURPOSES, WE HAVE ASSUMED A SET OF GEOTECHNICAL PARAMETERS. UPON EXCAVATION OR FURTHER EXPLORATION IN THE WALL LOCATION(S), THESE DESIGN PARAMETERS MUST BE VERIFIED AND ACCEPTABLE BY THE GENERAL REVIEW ENGINEER (REFER TO NOTE 6) OR REVISED PARAMETERS MUST BE PROVIDED FOR A REDESIGN. BOTH THE CONTRACTOR AND THE GENERAL CONTRACTANT MUST BE ADVISED THAT THE DESIGN MAY HAVE TO BE ALTERED BASED ON ACTUAL CONDITIONS FOUND ON SITE. ALTERATION OF THE DESIGN MAY RESULT IN ADDITIONAL CONSTRUCTION COSTS AND PROJECT DELAYS. IT IS RECOMMENDED THAT CONTRACTOR AND GENERAL CONTRACTOR BE ADVISED THAT THE WALL CONSTRUCTION FOR DEALING WITH THE DISCOVERY OF UNFAVORABLE SOIL CONDITIONS.
- THIS DESIGN MUST BE CHECKED WITH THE FINAL GRADING PLAN TO VERIFY ACCURACY. IT IS THE CONTRACTOR'S RESPONSIBILITY TO ENSURE THE WALL LAYOUT(S) PROVIDED MATCH THE FINAL SITE GRADING. CONTRACTOR MUST VERIFY ALL DIMENSIONS AND ELEVATIONS PRIOR TO BEGINNING CONSTRUCTION. RISI STONE SYSTEMS MAKES EVERY EFFORT TO ENSURE ACCURACY OF THE DESIGN. HOWEVER, AS INFORMATION PROVIDED MAY HAVE BEEN UNKNOWNLY OUT OF DATE, UNCLEAR IN AREAS, OR INCOMPLETE, IT IS ULTIMATELY THE CONTRACTOR'S RESPONSIBILITY TO VERIFY THE DIMENSIONS AND ELEVATIONS (QUANTITIES) OF THE WALL(S) WITH THE MOST RECENT GRADING PLAN AND ACTUAL SITE CONDITIONS.
- THE ONTARIO BUILDING CODE REQUIRES THAT THE CONSTRUCTION OF EVERY BUILDING DESIGNED BY AN ARCHITECT AND/OR PROFESSIONAL ENGINEER IS TO BE REVIEWED FOR GENERAL CONFORMITY TO THE APPROVED DESIGN BY PROFESSIONALS (RETAINING WALLS FALL UNDER THE CATEGORY OF DESIGNATED STRUCTURES AND THEREFORE INCLUDED UNDER THE OBC). RISI STONE SYSTEMS AND/OR THEIR LICENSEE DOES NOT PROVIDE THIS SERVICE. THE CONTRACTOR MUST ENSURE THAT A THIRD PARTY ENGINEER HAS BEEN RETAINED TO PROVIDE GENERAL REVIEW OF THE WALL CONSTRUCTION IN ACCORDANCE WITH PART 3 EXECUTION SUB SECTION 3.03 OF RISI STONE SYSTEMS STANDARD SPECIFICATIONS.
- THIS DESIGN HAS BEEN CONDUCTED IN ACCORDANCE WITH ASSHTO AT THE REQUEST. AS THE ONTARIO BUILDING CODE OR NATIONAL BUILDING CODE DO NOT PROVIDE EXPLICIT METHODOLOGIES TO ADDRESS THE DESIGN OF SEGMENTAL RETAINING WALLS, COMPLIANCE OF THIS DESIGN WITH THE OBC AND NBC IS BY WAY OF THE OPEL RECOMMENDATIONS. SEISMIC ANALYSIS HAS BEEN CONDUCTED AND ASSUMES A PGA OF 0.17 (OBC - SITE CLASS C). SITE CLASS MUST BE VERIFIED BY GENERAL REVIEW ENGINEER UPON INSPECTION OF SUBGRADE (AS DETAILED ON SECTION). ANALYSIS OF OVERALL GLOBAL AND/OR COMPOUND STABILITY HAS NOT BEEN CONDUCTED. IT IS REQUIRED THAT THE PROJECT GEOTECHNICAL ENGINEER BE RETAINED BY THE OWNER TO ASSESS THE NEED FOR A GLOBAL STABILITY ANALYSIS AND PROVIDE THIS, IF NECESSARY. RISI STONE SYSTEMS CAN WORK WITH THE GEOTECHNICAL ENGINEER TO PROVIDE DETAILS OF THE WALL DESIGN TO BE INCORPORATED INTO THE GLOBAL STABILITY ANALYSIS.
- THE LOCATION OF EXISTING OR PROPOSED UTILITIES MUST BE VERIFIED PRIOR TO CONSTRUCTION. GENERALLY IT IS RECOMMENDED THAT UTILITIES BE OFFSET FROM THE WALL TO A) PREVENT ADDITIONAL LOADING ON THE CONDUIT (I.E. A 1H:1V LINE OF INFLUENCE FROM THE BASE OF THE WALL SHOULD BE ASSUMED) UNLESS ACCOUNTED FOR IN DESIGN OF THE UTILITY B) TO ENSURE FUTURE ACCESS TO THE UTILITY WITHOUT UNDERMINING THE WALL. THE ENGINEERED FILL ABOVE THESE UTILITIES MUST BE COMPACTED TO 98% SPD. THE CIVIL ENGINEER MUST REVIEW THE DESIGN TO VERIFY THE ABOVE (REFER TO NOTE 9 AND SPECIFICATION FOR FURTHER DETAILS).
- THE RETAINING WALL DRAWINGS AND SPECIFICATIONS MUST BE REVIEWED BY THE CIVIL ENGINEER, LANDSCAPE ARCHITECT/ARCHITECT, AND GENERAL REVIEW ENGINEER PRIOR TO THE GENERAL REVIEW ENGINEER AUTHORIZING THE DRAWINGS TO BE USED FOR CONSTRUCTION IN ACCORDANCE WITH SECTION 3.02, SEGMENTAL RETAINING WALL DESIGN REVIEW, OF THE SPECIFICATIONS.

SECTION A - TYPICAL SECTION

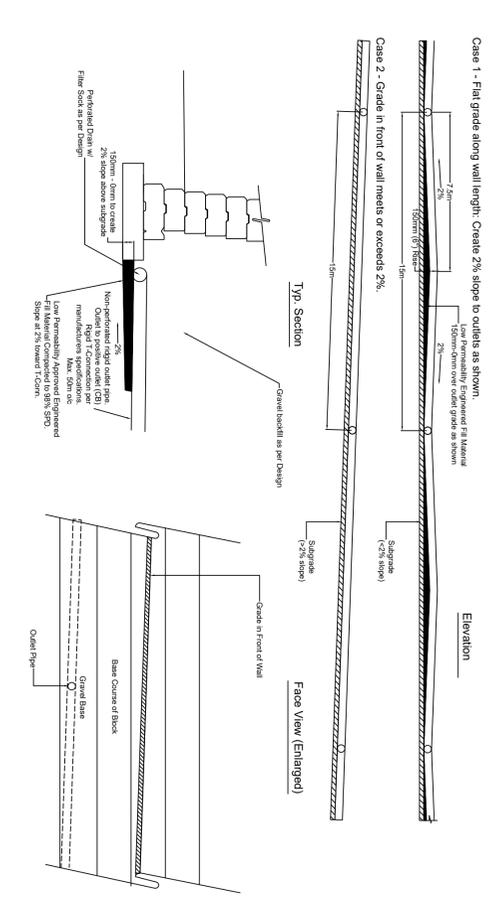
Scale NTS



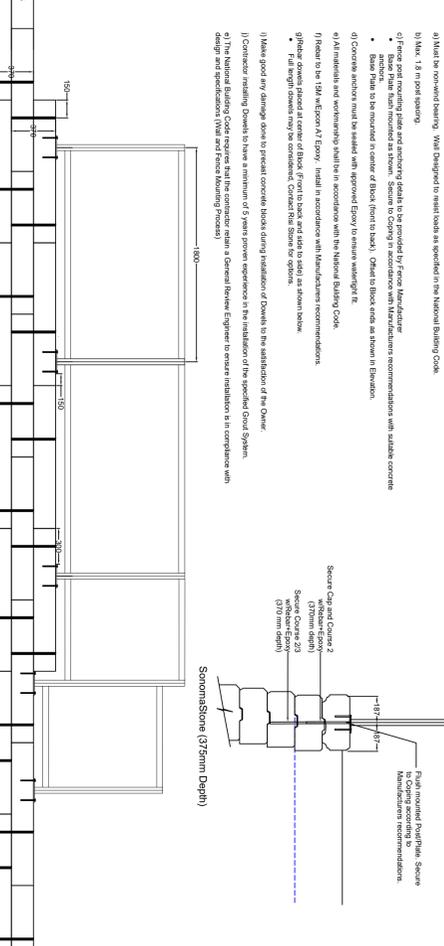
Detail - Pedestrian Handrail/Chain Link Fence Circular Steel Post Core Drill Installation



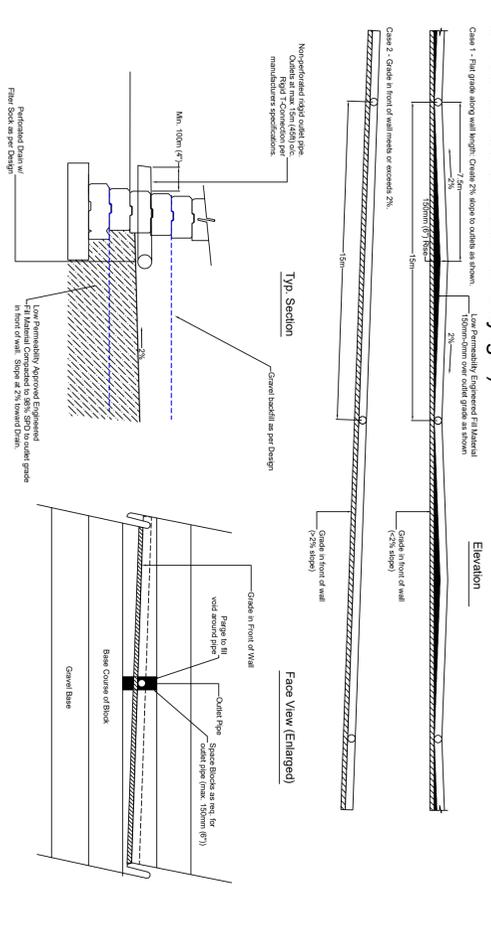
b) Drain Outlet at /near base of the wall (as shown) or just above the Base of Wall Outlet from Behind (To Nearest Catch Basin)



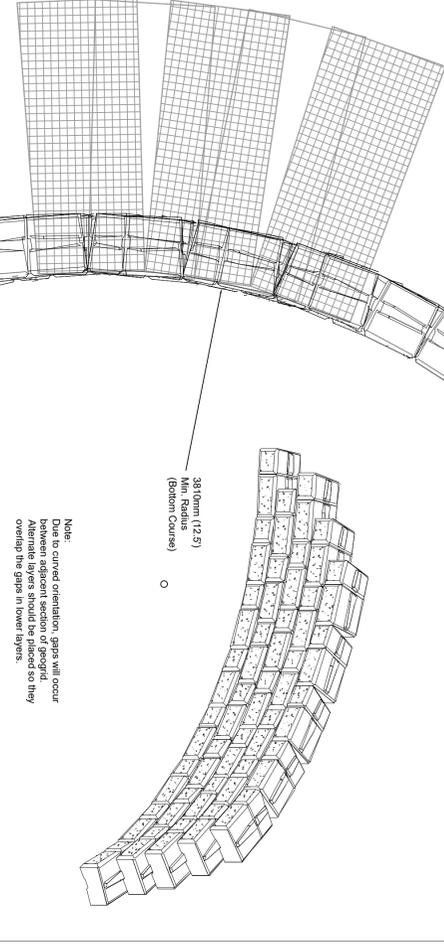
Detail - Pedestrian Handrail/Chain Link Fence Surface Mounted



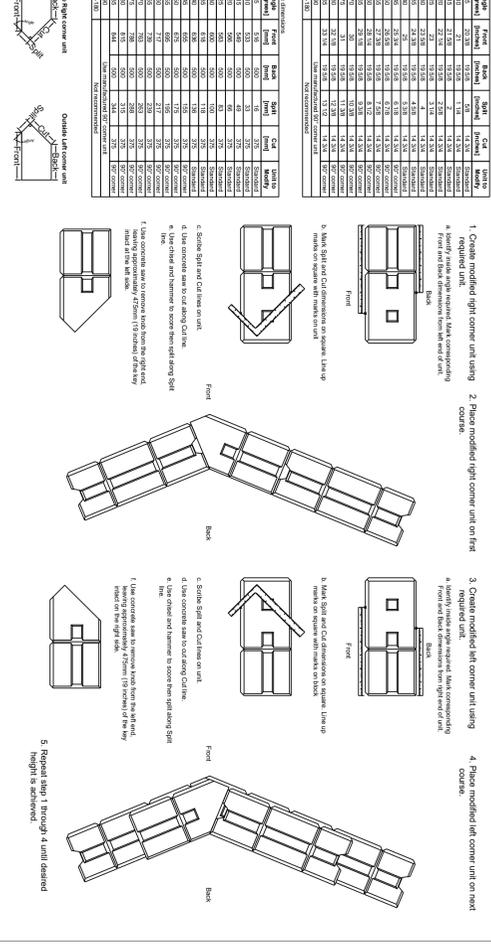
Detail - Either Drainage Outlet Option a) Drain Outlet at /near or just above the Base of Wall (as shown) Outlet Thru Block Face



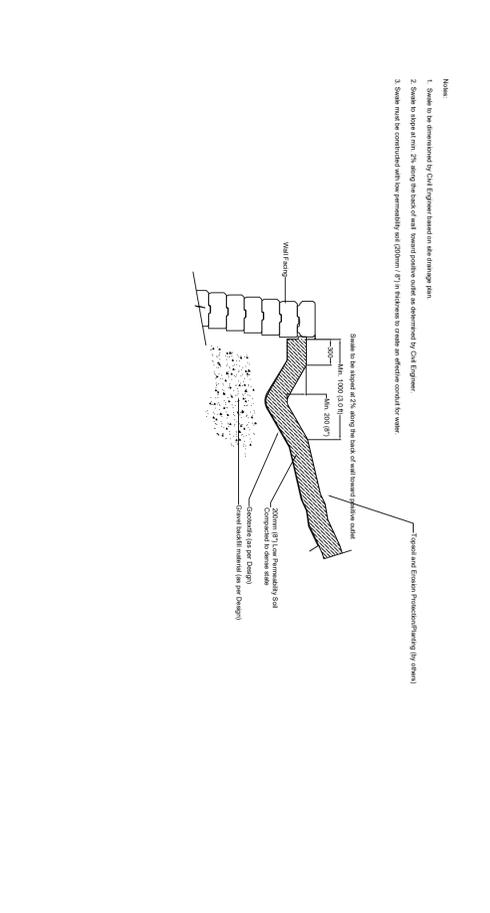
Detail - Concave Curve



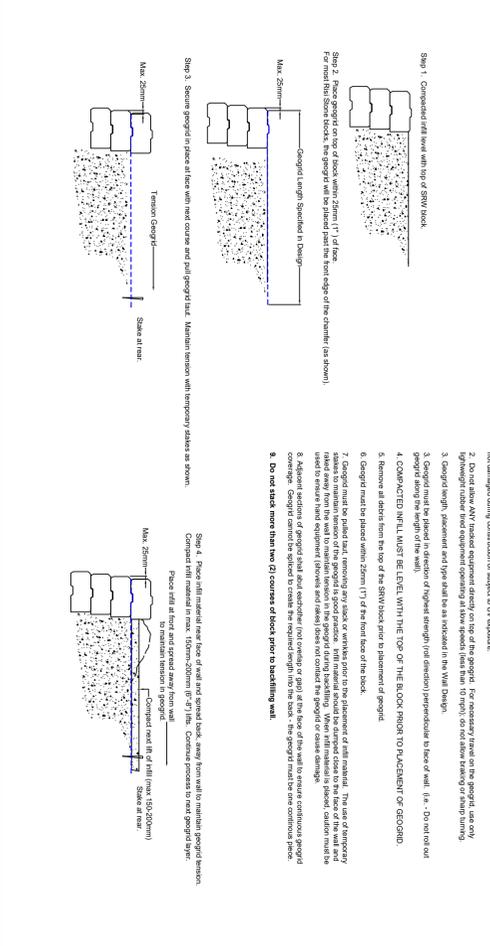
Detail - Outside Corner Construction



Detail - Swale Detail (Typical - Dim. to be provided by Civil Engineer)



Detail - Geogrid Installation



SonomasStone Wall System Information

Sonomas Stone @ System Units	Face Width	Block Width	Height	Depth	Weight
Standard Unit	47.25"	47.25"	7.25"	14.175"	395 lbs
Tapered Unit	1200 mm	1200 mm	185 mm	375 mm	175 kg
750 Unit	600 mm	542 mm	185 mm	375 mm	82 kg
Left Corner Unit	38.5"	38.5"	7.25"	14.175"	309 lbs
Right Corner Unit	38.5"	38.5"	7.25"	14.175"	309 lbs
Coping Unit	47.25"	47.25"	7.25"	14.175"	375 lbs
Tapered Coping Unit	23.25"	21.5"	7.25"	14.175"	180 lbs

* Indicates dimensions of alternate units available in some locations.

PROFESSIONAL ENGINEER
Y. K. K. CHEN
1006/189
1006/226
REGISTERED PROFESSIONAL ENGINEER
PROF. OF CIVIL ENGINEERING

PROJECT
Risi Project No. 202602029
Mixed Use Residential Development
1493 Sixth Line, Oakville, ON

SonomasStone®
Geogrid Reinforced Segmental Retaining Wall

Drawn By: CNK
Checked By: CNK
Date: March 2026
Drawing No.: 202602029V2

REVISIONS

No.	Date	By	Revisions

WALL LOCATION PLAN n.t.s.

SHEET
RW-2

DRAWING NO.
2 of 2

RISI PROJECT NO.
202602029

DETAIL SHEET

10 - 480 Harry Walker Parkway S
Newmarket, Ontario, Canada L3Y 0B3
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RisiStone®
retaining wall systems



RisiStone[®]
retaining wall systems

Specifications

PART 1 | **GENERAL**

Last Modified | January 2017

1.01 Description

- A. The work covered by this section includes the furnishing of all labour, materials, equipment, and incidentals for the Design, inspection, and construction of a modular concrete Segmental Retaining Wall ("SRW") including drainage system and geosynthetic reinforcement as shown in the Construction Documents and as described by this Specification. The work included in this section consists of, but is not limited, to the following:
- 1) Design of an SRW system.
 - 2) Review of the site conditions with respect to suitability of the SRW Design.
 - 3) Inspection of all construction operations and materials related to the SRW.
 - 4) Excavation and foundation soil preparation.
 - 5) Furnishing and placement of the Leveling Base.
 - 6) Furnishing and placement of the Drainage system.
 - 7) Furnishing and placement of Geotextile Filter (if applicable).
 - 8) Furnishing and placement of SRW units.
 - 9) Furnishing and placement of Geosynthetic Reinforcement.
 - 10) Furnishing, placement, and compaction of Reinforced, Drainage, and Retained Fills.
 - 11) Furnishing of final grading.

1.02 Related Work

- A. Section 31 10 00 - Site Preparation
- B. Section 31 20 00 - Earth Moving

1.03 Reference Standards (Refer to most recent versions)

- A. Segmental Retaining Wall Design
- 1) Design Manual for Segmental Retaining Walls, National Concrete Masonry Association, Third Edition which will be referred to as the "NCMA Design Manual"
- B. Segmental Retaining Wall Units
- 1) ASTM C140, "Standard Test Methods for Sampling and Testing Concrete Masonry Units and Related Units"
 - 2) ASTM C1262, "Standard Test Method for Evaluating the Freeze-Thaw Durability of Manufactured Concrete Masonry Units and Related Concrete Units"
 - 3) ASTM C1372, "Standard Specification for Dry-Cast Segmental Retaining Wall Units"
 - 4) ASTM D6638, "Test Method for Determining Connection Strength Between Geosynthetic Reinforcement and Segmental Concrete Units (Modular Concrete Blocks)"

- 5) ASTM D6916, "Standard Test Method for Determining the Shear Strength Between Segmental Concrete Units (Modular Concrete Blocks)"
- C. Geotextile Filter
- 1) ASTM D4491, "Standard Test Methods for Water Permeability of Geotextiles by Permittivity"
 - 2) ASTM D4751, "Standard Test Method for Determining Apparent Opening Size of a Geotextile"
 - 3) ASTM D5261, "Standard Test Method for Measuring Mass per Unit Area of Geotextiles"
- D. Geosynthetic Reinforcement
- 1) ASTM D4595, "Standard Test Method for Tensile Properties of Geotextiles by the Wide-Width Strip Method"
 - 2) ASTM D5262, "Standard Test Method for Evaluating the Unconfined Tension Creep Rupture Behavior of Geosynthetics"
 - 3) ASTM D5321, "Standard Test Method for Determining the Coefficient of Soil and Geosynthetic or Geosynthetic and Geosynthetic Friction by Direct Shear Method"
 - 4) ASTM D5818, "Standard Practice for Exposure and Retrieval of Samples to Evaluate Installation Damage of Geosynthetics"
 - 5) ASTM D6637, "Standard Test Method for Determining Tensile Properties of Geogrids by the Single or Multi-Rib Tensile Method"
 - 6) ASTM D6706, "Standard Test Method for Measuring Geosynthetic Pullout Resistance in Soil"
 - 7) ASTM D6992 – Standard Test Method for Accelerated Tensile Creep and Creep-Rupture of Geosynthetic Materials Based on Time-Temperature Superposition Using Stepped Isothermal Method.
- E. Soils
- 1) ASTM D422, "Standard Test Method for Particle-Size Analysis of Soils"
 - 2) ASTM D698, "Standard Test Methods for Laboratory Compaction Characteristics of Soil Using Standard Effort (12,400 ft-lbf/ft³ (600 kN-m/m³))"
 - 3) ASTM D1556, "Standard Test Method for Density and Unit Weight of Soil in Place by the Sand-Cone Method"
 - 4) ASTM D1557, "Standard Test Method for Laboratory Compaction Characteristics of Soil Using Modified Effort (56,000 ft-lbf/ft³ (2,700 kN-m/m³))"
 - 5) ASTM D2487 "Standard Practice for Classification of Soils for Engineering Purposes (Unified Soil Classification System)"
 - 6) ASTM D6938, "Standard Test Method for In-Place Density and Water Content of Soil and Soil-Aggregate by Nuclear Methods"
 - 7) ASTM D4318, "Standard Test Methods for Liquid Limit, Plastic Limit, and Plasticity Index of Soils"
 - 8) ASTM D6919, "Standard Test Methods for Particle-Size Distribution (gradation) of Soils Using Sieve Analysis"
 - 9) ASTM G51, "Standard Test Method for Measuring pH of Soil for Use in Corrosion Testing"

- F. Drainage Pipe
 - 1) ASTM F758, "Standard Specification for Smooth-Wall Poly(Vinyl Chloride) (PVC) Plastic Underdrain Systems for Highway, Airport, and Similar Drainage"
 - 2) ASTM F405, "Standard Specification for Corrugated Polyethylene (PE) Pipe and Fittings"
- G. Where specifications and reference documents conflict, the Owner or Owner's Representative shall make the final determination of applicable document.

1.04 Delivery, Material Handling, and Storage

- A. The Installer shall check all materials delivered to the site to ensure that the materials specified in the Construction Documents have been received and are in good condition.
- B. The Installer shall store and handle all materials in accordance with manufacturer's recommendations and in a manner to prevent deterioration or damage due to moisture, temperature changes, contaminants, handling, or other causes.

1.05 Roles and Responsibilities

Although other parties may have responsibilities related to the Retaining Wall, the following Four (4) main entities have direct responsibilities for the Design, Review and Construction of the Segmental Retaining Wall. This outline of roles and responsibilities is based on Section 3 and Section 12 of the NCMA Design Manual for Segmental Retaining Walls, 3rd Edition.

- A. The term **Installer** shall refer to the individual or Firm that will construct the SRW. The Installer must have the necessary experience and understanding of SRWs for the project and have successfully completed projects of similar scope and size.
- B. The **Site Geotechnical Engineer** is the individual Professional Geotechnical Engineer or Geotechnical Engineering Firm that has been retained to provide all Geotechnical verifications for the Wall, including verifying Site Soils and Groundwater conditions, Materials testing, and Global Stability. Refer to Section 3.02 and 3.03.
- C. The term **General Review Engineer** refers to the individual Professional Engineer or Professional Engineering Firm that has been retained to provide "General Review" of the Wall construction to ensure that the Wall is constructed in general conformance with the Design and Specifications. The General Review Engineer and the Site Geotechnical Engineer can be, and are often the same Party. Refer to Section 3.02 and 3.03.
- D. The term **Wall Designer** refers to the individual Professional Engineer or Professional Engineering Firm that is experienced in the design of SRWs and is responsible for generating a sealed SRW Design based on information that is provided to the Designer, created in accordance with Section 3.01. The Designer may retain the services of other professionals to augment their own capabilities, skills, and knowledge. The Wall Designer and General Review Engineer (GRE) are not required to be the same individual or firm. Any issues in the field, such as differences between assumed Design conditions and actual field conditions, will be brought to the attention of the Wall Designer by the GRE.

1.06 Submittals - per Contract Documents

1.07 Measurement for Payment - per Contract Documents

1.08 Approved Segmental Retaining Wall System

The Segmental Retaining Wall (SRW) System shall be the Risi Stone SRW System noted in the attached Design.

PART 2 | MATERIALS

2.01 Definitions

- A. Segmental Retaining Wall ("SRW") is the entire retaining wall structure(s) including: SRW Units, Coping, Drainage Pipe, Geotextile Filter, Geosynthetic Reinforcement and Drainage, Reinforced, Retained, and Base Fills. A Segmental Retaining wall structure can be classified as follows:
 - 1) Conventional SRW - SRW Units stacked on a Leveling Base with a Drainage system behind.
 - 2) Multi-Depth SRW - SRW Units of different depths with larger units at the bottom, and smaller units at the top, stacked on a Leveling Base with a Drainage system behind.
 - 3) Reinforced SRW - SRW Units stacked on a Leveling Base with a Drainage system, Reinforced Fill including Geosynthetic Reinforcement located behind.
 - 4) Crib SRW - SRW Units stacked parallel and perpendicular to the SRW direction forming bin like structures, built on a Leveling Base with a Drainage system behind.
- B. Segmental Retaining Wall Units are modular, solid, dry-cast concrete blocks, designed specifically for the task of earth retention, that form the external facia of an SRW system.
- C. Coping Units are the last course of concrete units used to finish the top of the SRW. Coping Units are also referred to as cap units.
- D. Leveling Base is the compacted granular soil, or if specified in the Construction Documents, an unreinforced concrete footing, placed beneath the first course of SRW units.
- E. Drainage Fill is a free draining aggregate with high permeability placed directly behind the modular concrete units. This will include a Drainage Pipe and may be separated from other Fill with a suitable Geotextile Filter.
- F. Reinforced Fill is placed directly behind the Drainage Fill, placed in layers and compacted, that will include horizontal layers of Geosynthetic Reinforcement. If the Reinforced Fill is considered to be a "draining material", the Drainage Fill may not be required.
- G. Retained Fill is the soil placed between the Reinforced Fill and the Retained Soil in Reinforced SRWs or between the Drainage Fill and Retained Soil in Conventional SRWs.
- H. Retained Soil in cut situations is the undisturbed native soil embankment. In soil fill situations this will be the compacted engineered site fill.
- I. Foundation Soil is the undisturbed native soil or engineered fill beneath the SRW structure.
- J. Drainage Pipe is a perforated pipe used to carry water, collected from within the SRW, to outlets, to prevent pore water pressures from building up within the SRW and specifically behind the SRW Units.
- K. Geotextile Filter is a permeable planar polymer structure that will allow the passage of water from one soil medium to another while preventing the migration of fine particles that might clog the downstream fill. Selection of a Geotextile Filter is based on the characteristics of the different soils used in and surrounding the SRW.
- L. Geosynthetic Reinforcement is an open planar polymer structure having tensile strength and durability properties that are suitable for soil reinforcement applications. Geogrid is a commonly used type of Geosynthetic Reinforcement.

- M. All values stated in metric units shall be considered as accurate. Values in parenthesis stated in imperial units are the nominal equivalents.

2.02 Material Requirements

- A. All approved products will be identified in the Construction Documents. No substitutions will be allowed unless approved in writing by the Designer.
- B. The Risi Stone SRW units will be specified in the Construction Documents which shall include the manufacturer's name, product name, dimensions, colour, and finish. Additionally the SRW units must:
- 1) Meet the minimum standard as defined by ASTM C1372 for:
 - a) Strength
 - b) Absorption
 - c) Freeze - Thaw durability
 - d) Permissible variation in dimensions
 - e) Finish and Appearance
 - 2) Meet the physical properties listed below as tested using ASTM C140:
 - a) Dimensional tolerance shall be +/- 3 mm (1/8 in.) for height, width, and length.
 - b) The minimum 28-day compressive strength of 35 MPa (5000 psi).
 - c) The maximum moisture absorption shall be 1.0 kN/cubic m (6.5 lbs/cubic ft).
 - 3) Use an integral shear key connection that shall be offset to create, as specified in the Construction Documents, either:
 - a) A minimum batter as stated in the Construction Documents, or
 - b) A near vertical alignment. Special construction procedures are required for vertical SRWs. See Section 3.04.D.
 - 4) If required, summary test data shall be provided with the SRW Design and shall include:
 - a) SRW Unit shear strength as per ASTM D6916
 - b) SRW Unit - Geosynthetic Reinforcement connection strength as per ASTM D6638
- C. Reinforced Fill
- 1) If the SRW Units by themselves provide sufficient stability, the Designer may choose to omit the Reinforced Fill
 - 2) The Reinforced Fill shall be specified in the Construction Documents as "select imported fill"
 - a) Unified Soil Classification System designation as per ASTM D2487
 - b) % passing #200 sieve
 - c) Effective friction angle (direct shear or triaxial test)
 - d) Minimum compacted density
 - 3) Additional information may be required which could include:

- a) Soil gradation curve (ASTM D422)
 - b) Liquid limit, plastic limit, and plasticity index (ASTM D4318)
 - c) Soil pH (ASTM G51)
 - d) Permeability coefficient "Q"
- D. Leveling Base
- 1) The leveling base material shall be non-frost susceptible, well-graded, compacted angular gravel-sand mixture (GW as per ASTM D2487).
 - 2) Additional information may be required which could include:
 - a) Effective friction angle (direct shear or triaxial)
 - b) Soil gradation curve (ASTM D422)
 - c) Soil pH (ASTM G51)
 - d) Permeability coefficient "Q"
 - e) Potential for consolidation
 - 3) Alternately, the Construction Documents may specify the leveling base shall be an unreinforced concrete footing with specified dimensions.
- E. Drainage Fill
- 1) If the Reinforced Fill has adequate drainage characteristics, the Designer may choose to omit the Drainage Fill.
 - 2) The Drainage Fill shall be a free-draining angular, gravel material of uniform particle size smaller than 25 mm (1 inch) and greater than 6mm (1/4 inch). If shown in the Construction Documents, the Drainage Fill shall be separated from the Reinforced Fill or Retained Fill by a specified Geotextile Filter.
 - 3) Additional information may be required which could include:
 - a) Effective friction angle (direct shear or triaxial)
 - b) Soil gradation curve (ASTM D422)
 - c) Soil pH (ASTM G51)
 - d) Permeability coefficient "Q"
 - e) Potential for consolidation
- F. Drainage Pipe
- 1) The Drainage Pipe shall be specified in the Construction Documents and shall either be a perforated corrugated polyethylene or perforated PVC pipe, with a minimum diameter of 100 mm (4 inches), protected by a Geotextile Filter to prevent the migration of soil particles into the Drainage Pipe.
- G. Geotextile Filter
- 1) If the gradation of adjacent soils permits, the Geotextile Filter may not be required per the Design.

- 2) If required, summary test data shall be provided with the SRW Design and shall include:
 - a) Apparent opening size "AOS" (ASTM D4751)
 - b) Unit weight (ASTM D5261)
 - c) Coefficient of permeability (ASTM D4491)
- H. Geosynthetic Reinforcement
- 1) If the SRW Units by themselves provide sufficient stability, the Designer may choose to omit the Geosynthetic Reinforcement.
 - 2) The Geosynthetic Reinforcement shall be specified in the Construction Documents and shall include the manufacturer's name, product name, and Long Term Design Strength ("LTDS") as calculated according to section 3.01.A.5.
 - 3) If required, summary test data shall be provided with the SRW Design and shall include:
 - a) Tensile strength (ASTM D6637)
 - b) Creep potential reduction factor (ASTM D5262)
 - c) Installation damage reduction factor
 - d) Durability reduction factor (chemical and biological)
 - e) Soil pullout resistance (ASTM D6706)
 - f) Connection strength (ASTM D6638)
 - g) Coefficient of interaction "Ci"
 - h) Coefficient of interaction "Cds"
 - i) Concrete Adhesive
- 1) If the Coping Unit by itself provides sufficient stability, the Designer may choose to omit the Coping Adhesive.
 - 2) The adhesive is used to permanently secure the coping unit to the top course of the SRW. The adhesive must provide sufficient strength and remain flexible for the expected life of the SRW.

PART 3 | EXECUTION

3.01 Segmental Retaining Wall Design

- A. Design Standard
- 1) The Designer is responsible for providing an SRW Design based on the proposed site development documents. The design life of the structure shall be 75 years unless otherwise specified in the Construction Documents.
 - 2) The Designer shall create the SRW Design in accordance with recommendations of the NCMA Design Manual for Segmental Retaining Walls, Third Edition, for Internal, External, and Internal Compound Stability under Static and Seismic conditions.
 - 3) If required, an alternate design method may be used and must be identified in the SRW Design. The alternate design method must be comprehensive and adequately evaluate all possible modes of failure.
 - 4) The Wall Designer is not responsible for analyzing the global stability of the SRW structure for circular slip failure planes that are completely external to the SRW structure. The Global Stability analysis is to be conducted by the Geotechnical Engineer (SGE) in accordance with NCMA guidelines. Refer to Section 1.05.
- B. Design Assumptions – Refer to Notes on Design Drawing
- C. Design Parameters
- 1) Site Parameters
 - a) The length, height, and overall elevations of the SRW Design must be derived from the provided site grading plan, elevation details, cross-section details, and station information.
 - b) Surcharges, anticipated usage and slopes above, as well as slopes below, all sections of the SRW must be indicated on the site grading plan.
 - c) The minimum SRW embedment shall be the greater of:
 - i. The height of an SRW unit, or
 - ii. The minimum embedment required based on the slope below the SRW.

Slope Below SRW	Minimum Embedment
No Slope	H/10
3 : 1 (18.4 deg)	H/10
2 : 1 (26.5 deg)	H/7
 - iii. The Site Geotechnical Engineer may determine it is necessary to increase embedment due to erosion potential or global stability requirements.
- 2) Site Soil Parameters
 - a) All site soil parameters used in the design shall be stated in the SRW Design. This should include soil classification (ASTM D2487), effective friction angle, compacted density, and cohesion.

- b) Site-specific soil parameters obtained from site geotechnical investigations shall be used in the design calculations. If a site geotechnical investigation is not available or does not provide specific parameters for the SRW, assumed soil parameters may be used and the SRW Design shall state the assumed values and that assumed soil parameters have been used.
 - c) If select on-site soils are to be used as SRW fill materials, additional testing of the re-compacted soil will be required for the design calculations. Soil parameters for the select on-site fill shall be used in the design calculations. If fill parameters are not available, assumed fill parameters may be used and the Design Drawings shall state the assumed values and that assumed fill parameters have been used.
- 3) Product Design Parameters
- a) All relevant Product Design Parameters for materials incorporated in the SRW shall be obtained from the supplier or manufacturer and used in the design calculations. All values used shall be obtained from testing conducted in accordance with the Reference Standards identified in Section 1.03. If product test results are not available, assumed parameters may be used and the Design Drawings shall state the assumed values and that assumed product design parameters have been used.

3.02 Segmental Retaining Wall Design Review

This section states the minimum review process that is required prior to construction of an SRW. Other parties such as municipalities, architects, developers, owners, and other designers should review the SRW Design prior to acceptance to ensure specific requirements of each party are met.

- A. Review of Design by the GRE (General Review Engineer). The General Review Engineer is not responsible for the Wall Design. The role of the GRE is to ensure that the Design produced by the Wall Designer is followed in the field. As such, the GRE must review and understand the Design. Refer to 1.05.C and 3.03.
- B. Review of the Design by the SGE (Site Geotechnical Engineer). The SGE must review the Design to verify that the Site Soil and Groundwater Conditions assumed in the Design are correct for the Site, or provide new values/conditions to the Wall Designer. The SGE must also review the Design to determine if a Global Stability analysis is required based on soil conditions, Wall geometry and slopes, groundwater, etc.
- C. Review of the Design by the Civil Engineer. The Project Civil Engineer must be provided with a copy of the SRW Design so they may review it for general compatibility with the site.
 - 1) Review should include, but is not limited to, the following specific elements:
 - a) All surface drainage must direct water away from the SRW including slopes and paved surfaces.
 - b) The SRW drainage system delivers outflow to approved locations.
 - c) All site services must be located outside of SRW construction area unless otherwise noted in Design.
 - d) The SRW structure or excavation limits must not cross over property boundaries unless approved prior to construction.
 - e) All structures located near the SRW must be shown in the Construction Documents.

- f) Anticipated use above wall during and after construction must be as shown in the Construction Documents.
 - 2) The Project Civil Engineer must contact the Designer to address any outstanding issues, questions, or concerns regarding the SRW Design and resolve these issues prior to the General Review Engineer authorizing the SRW Design to be used as Construction Documents.
- D. Review of the Design by the Landscape Architect. If applicable, the Project Landscape Architect must be provided with a copy of the SRW Design so they may review it for general compatibility with the site.
- 1) The review should include, but is not limited to, the following specific elements:
 - a) Ensure plant and tree species to be placed above the SRW are suited to the environment created by the SRW.
 - b) Limit irrigation near SRW structure.
 - c) Grading above and below the SRW structure.
 - d) It may be necessary to incorporate a root barrier (as required by others) to prevent the migration of tree roots into the drainage layer.
 - e) Larger plants and trees must be kept outside of the Reinforced Fill to ensure
 - i. The Geosynthetic Reinforcement is not damaged by excavation for the root ball
 - ii. The SRW is not subjected to any additional load from plants or trees.
 - 2) The Project Landscape Architect must contact the Designer to address any outstanding issues, questions, or concerns regarding the SRW Design and resolve these issues prior to the General Review Engineer issuing Construction Documents or authorising the SRW Design to be used as Construction Documents.

3.03 Inspection

Wall Construction must be regularly inspected as follows.

- A. Geotechnical Inspection. This is to be performed by a Geotechnical Engineer (SGE) retained by either the Installer or Owner (depending on the requirements of the Contract Documents). The Geotechnical Inspection includes, but may not be limited to, the following:
- 1) Verifying assumed Design soil parameters and groundwater conditions are acceptable for the Site, or provide the Wall Designer with alternate values/conditions.
 - 2) Verifying subgrade Bearing Capacity meets or exceeds values required by the Design, or provide recommendations to the Installer to achieve the required values (i.e. removal and replacement of subgrade materials, foundation improvement, etc).
 - 3) Determining the need for Global Stability Analysis, and supplying this analysis if necessary per the NCMA guidelines (Section 12).
 - 4) Providing Construction inspection and testing of on-site and fill soils (i.e. compaction testing).
 - 5) Ensuring groundwater conditions and/or other water sources have been identified and compared with the assumptions made in the design. Additional water sources noted on site such as seepage from the cut embankment must be identified and the Designer notified if these are not noted in the Construction Documents.

- B. General Review of Construction. The General Review Engineer is retained by the Installer or Owner (depending on the requirements of the Contract Documents) to provide the following services. (Note that the General Review Engineer may be the same individual as the Site Geotechnical Engineer. This is often the most efficient method of ensuring proper Inspection).
- 1) Inform the Designer in writing that they will be acting as the General Review Engineer for the project prior to construction.
 - 2) The GRE is to ensure that the Site Geotechnical Engineer (SGE) has verified the Geotechnical conditions as noted above.
 - 3) The GRE is to ensure that the SGE has determined if Global Stability analysis is required and conducted if need be.
 - 4) Testing and acceptance of all materials used to construct the SRW.
 - 5) Inspection of the methods used to construct the SRW.
 - 6) Determine if the wall is constructed in general conformance with the Construction Documents.
 - 7) The General Review Engineer must contact the Designer to address any outstanding issues, questions, or concerns regarding the SRW Design and resolve these issues prior to issuing Construction Documents or authorize the SRW Design to be used as Construction Documents. During construction, the GRE should notify the Designer of any discrepancies between the Design and actual Site Conditions.
 - 8) Ensure the SRW and associated excavation remains outside of the loading influence of other adjacent structures, unless they have been specifically accounted for in the SRW Design and shown in the Construction Documents and ensure stability of excavations and conformance with applicable regulations.
 - 9) Ensure that surface water runoff and/or other sources of water are being controlled during construction and directed away from the SRW to a functioning drain.
- C. The Owner may engage a testing and inspection agency for their own quality assurance, but this does not replace the Site Geotechnical Engineer and General Review Engineer's inspection function described in Section 1.05 and Section 3.03.
- D. Installer's Quality Assurance Program
- 1) The Installer is responsible to ensure the SRW is constructed in accordance with the Construction Documents. The Installer must be qualified in the construction of SRWs, knowledgeable of acceptable methods of construction, and have thoroughly reviewed and understood the Construction Documents.
 - 2) It is recommended that the Installer shall keep a construction journal to document the construction of the SRW as part of a thorough quality control program. The General Review Engineer shall be provided with copies of the construction journal throughout the construction process.
 - 3) The Installer's field construction supervisor shall have demonstrated experience and be qualified to direct all work related to the SRW construction.
 - 4) The Installer must notify the General Review Engineer of critical stages in the construction of the SRW in order that they may be present to observe and inspect the work. The General Review Engineer must be notified reasonably well in advance of the scheduled date(s) for construction.

E. Construction Tolerances

- 1) Installation of SRW facia shall be within all the following acceptable tolerances:

Vertical Control	+/- 1.25 inches over a 10 ft distance
Horizontal Control	Straight lines: +/- 1.25 inches over a 10 ft distance
Rotation of the SRW face	Maximum 2.0 degrees from established SRW plan batter or +/- 10.0% from total established horizontal setback
Bulging	+/- 1.25 inch over a 10 ft distance

3.04 Construction

A. Site Preparation

- 1) Comply with all current Federal, Provincial/State, and local regulations for execution of the work, including local building codes and excavation regulations. Provide excavation support as required to maintain stability of the area during excavation and SRW construction and to protect existing structures, utilities, landscape features, property, or improvements.
- 2) Prior to grading or excavation of the site, confirm the location of the SRW and all underground features, including utility locations within the area of construction. Ensure surrounding structures are protected from effects of SRW excavation.
- 3) Coordinate installation of underground utilities with SRW installation.
- 4) Control surface water drainage and prevent inundation of the SRW construction area during the construction process.
- 5) The Foundation Soil shall be excavated or filled as required to the grades and dimensions shown in the Construction Documents.
- 6) The Foundation Soil shall be proof rolled and examined by the General Review Engineer to ensure that it meets the minimum strength requirements specified in the Construction Documents. If unacceptable Foundation Soil is encountered, the General Review Engineer should contact the Designer to discuss options and determine the most appropriate course of action.
- 7) In cut situations, the native soil shall be excavated to the lines and grades shown in the Construction Documents and removed from the site or stockpiled for reuse as Reinforced or Retained Fill as identified in the Construction Documents. Care should be taken not to contaminate or overly saturate the stockpiled fill material.

B. Installing Drainage System

- 1) If specified in the Construction Documents, the approved Geotextile Filter shall be set against the back of the first SRW Unit, over the prepared foundation soil extending towards the back of the excavation, up the excavation face and eventually over the top of the Drainage Fill to the back of the SRW Units near the top of the wall or as shown in the Construction Documents. Geotextile overlaps shall be a minimum of 300 mm (1 ft.) and shall be shingled down the face of the excavation in order to prevent the migration of particles from one fill type to another.

- 2) The Drainage Pipe shall be placed as shown in the Construction Documents, in accordance with the overall drainage plan for the site. The main collection drain pipe shall be a minimum of 100mm (4 inches) in diameter. The pipe shall be laid to ensure gravity flow of water from the Reinforced Fill. Connect drainage collection pipe at a storm sewer catch basin or daylight along slope at an elevation lower than lowest point of pipe within Reinforced Fill mass, every 15 m (50 feet) maximum.
 - 3) If other sources of water are discovered during excavation or anticipated, other drainage measures/systems such as chimney or blanket drains may be required. The General Review Engineer should contact the Designer to discuss options and determine the most appropriate course of action.
- C. Leveling Base or Spread Footing Placement
- 1) The Leveling Base shall be the specified material placed in the location to the dimensions shown in the Construction Documents.
- D. Installation of Segmental Retaining Wall Units
- 1) The bottom row of SRW Units shall be placed on the Leveling Base as shown in the Construction Documents. The units shall be placed in the middle of the Leveling Base. Care shall be taken to ensure that the SRW Units are aligned properly, leveled from side to side and front to back, and are in complete contact with the Leveling Base.
 - 2) The SRW Units above the bottom course shall be placed to interconnect the shear key and then pushed forward, creating the specified batter of the SRW face.
 - 3) The SRW Units shall be swept clean before placing additional courses to ensure that no dirt, concrete, or other foreign materials become lodged between successive lifts of the SRW Units.
 - 4) Successive courses shall be placed to create a running bond pattern with the edge of all units being approximately aligned with the middle of the unit in the course below it. Cut SRW Units may need to be placed to ensure the vertical line between adjacent SRW Units remains within the middle third of the SRW Unit below.
 - 5) A maximum of three courses of SRW units can be placed above the level of the Reinforced Fill at any time.
 - 6) The Installer shall check the level of SRW Units with each lift to ensure that no gaps are formed between successive lifts that may affect the performance of the SRW.
 - 7) Care shall be taken to ensure that the SRW Units and Geosynthetic Reinforcement, where applicable, are not damaged during handling and placement.
 - 8) No heavy equipment, for compaction, fill placement or other, shall be allowed within 1 metre (3 ft.) of the back of the SRW Units.
- E. Drainage Fill
- 1) Drainage Fill may not be required as indicated in the Construction Documents.
 - 2) The Drainage Fill will be placed behind the SRW Units with a minimum width of 300 mm (1 ft.) and separated from other soils using the specified Geotextile Filter.
 - 3) Drainage Fill shall be placed behind the SRW facing in maximum lifts of 150 mm (6 inches) and compacted to a minimum density of 95% Standard Proctor.

F. Reinforced Fill

- 1) Reinforced Fill may not be required as indicated in the Construction Documents.
- 2) Reinforced Fill shall be placed behind the SRW Units or Drainage Fill with a maximum lift thickness of 150 mm (6 inches) and compacted to a minimum density of 95% Standard Proctor Maximum Dry Density (ASTM D698) at a moisture content from 2% below to 2% above optimum.
- 3) The Reinforced Fill shall be placed and compacted level with the top of the SRW Units at the specified Geosynthetic Reinforcement elevations to ensure no voids exist under the Geosynthetic Reinforcement as it extends out over the Reinforced Fill.
- 4) Care shall be taken to ensure that the Geosynthetic Reinforcement lays flat and taut during placement of the Reinforced Fill. This is best achieved by placing the Reinforced Fill on top of the Geosynthetic Reinforcement near the SRW facia and spreading toward the back of the Reinforced Fill.
- 5) At the end of each day's operation, slope the last lift of Reinforced Fill away from the SRW facing to rapidly direct runoff away from the SRW facia. Do not allow surface runoff from adjacent areas to enter the SRW construction area.

G. Geosynthetic Reinforcement

- 1) Geosynthetic Reinforcement may not be required as indicated in the Construction Documents.
- 2) Verify type and primary strength direction of the Geosynthetic Reinforcement.
- 3) Cut Geosynthetic Reinforcement in sheets to the length shown in the Construction Documents.
- 4) Geosynthetic Reinforcement sheets shall be placed horizontally with the primary strength direction perpendicular to the SRW face, at the elevations shown in the Construction Documents. The sheets are to be placed adjacent to one another, without overlapping and without gaps between them.
- 5) Sweep the top of the SRW Units to ensure the SRW Units are clean and free of debris.
- 6) The Geosynthetic Reinforcement shall be placed over the compacted Reinforced Fill and the SRW Units with the outside edge extending over the shear key of the SRW Unit to within 25 mm (1 in.) of the front facing unit.
- 7) The next course of SRW Units shall be carefully placed on top of the lower course to ensure that no pieces of concrete are chipped off and become lodged between courses and the Geosynthetic Reinforcement is in complete contact with the top and bottom surfaces of the successive SRW courses.
- 8) With the Geosynthetic Reinforcement secured in place, the Geosynthetic Reinforcement shall be pulled taut away from the back the SRW Units during placement of Reinforced Fill. Alternatively, suitable anchoring pins or staples can be used to ensure that there are no wrinkles or slackness prior to placement of the Reinforced Fill. The Geosynthetic Reinforcement shall lay flat when pulled back perpendicular to the back of the SRW facia.
- 9) No construction equipment shall be allowed to operate directly on top of the Geosynthetic Reinforcement until a minimum thickness of 150 mm (6 inches) of fill has been placed. Equipment may drive on Reinforced Fill at slow speeds and should exercise care not to stop

suddenly or make sharp turns. No heavy equipment shall be allowed within 1 metre (3 ft.) of the back of the SRW Units.

H. Retained Fill

- 1) Retained Fill may not be required as indicated in the Construction Documents.
- 2) Retained Fill shall be placed and compacted behind the Reinforced Fill or Drainage Fill in Conventional SRW applications, in maximum lift thickness of 150 mm (6 inches).

I. Continuing Wall Construction

- 1) Repeat section 3.04.D through to 3.04.H until the grades indicated in the Construction Documents are achieved.

J. Secure Coping

- 1) The Coping Adhesive may not be required as indicated in the Construction Documents.
- 2) If coping adhesive is required by Design, coping units shall be secured to the top of the SRW with two 10 mm (3/8 inch) beads of Concrete Adhesive positioned 50mm (2 inches) in front and behind the tongue of the last course of SRW units.

K. Finishing SRW

- 1) Finish grading above the SRW to direct surface runoff water away from the SRW. A swale system must be used above the SRW if the grade slopes toward the back of the wall. Construct the swale with the materials and to the dimensions specified in the Construction Documents. Final grading must be established immediately to ensure the Reinforced Fill is protected from water infiltration.
- 2) Upon completion of the SRW, additional structures (fences, handrails, vehicular guardrails, buildings, pools/ponds, etc.) or changes to grading/loading (increased height, slopes, parking areas, changes in proximity to water flow, etc.), other than those shown in the Construction Documents, cannot be installed/implemented without the review and consent of the General Review Engineer who will typically have to consult the Designer.
- 3) If the Installer is not responsible for the final landscaping and grading above or around the SRW, the Installer must ensure the firm who is responsible for the final landscaping and grading understands the SRW's limitations with respect to allowable depth of topsoil, excavation behind the SRW for planting, offset for heavy equipment and allowable surcharge. This also extends to firms who will be responsible for installations like handrails, fences, and signs that will apply additional loads to the SRW and will impact the SRW's performance.

Retaining Wall Budget & Design



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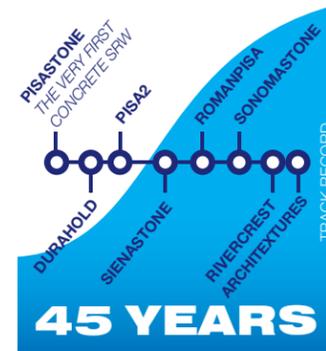


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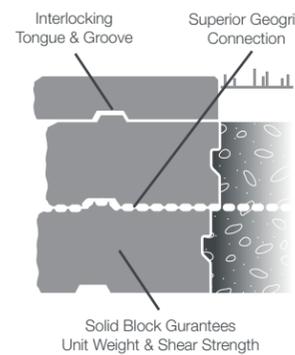
Comparing the Installed Cost

The "Installed" Cost of a Retaining Wall will vary based on a number of factors including the Wall Height, Application (heavy loading, water & steep slopes), Site Access, Aesthetics, Etc. Usually, the higher the wall and more critical the application, the greater the Final Installed Cost. The Installed Cost should include the Block, as well as Infill/Drainage Materials, Base, Wall Excavation (within footprint), Drainage pipe and labour/machine time. It is always important to compare product design and quality, as not all products are correct for every application.

Light duty walls, like Rivercrest, Pisa2 and RomanPisa are great for complex layouts (tight radii, intricate geometries) and applications where site access may be limited for heavy equipment (residential). Heavy duty walls, such as SienaStone, SonomaStone, and DuraHold are ideal for more critical applications, commercial use, and large scale installations where machine placing can save time and labour costs. For additional detailed information about selecting the right product for your project, please contact your Unilock Sales Representative, or Risi Stone Engineer.



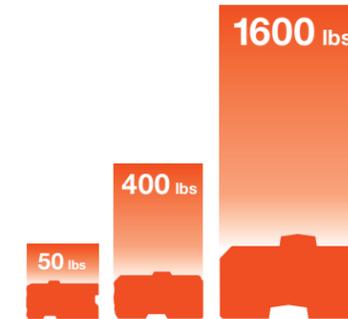
We have the longest, most proven track record in the SRW industry. The very first Concrete SRW System was invented by Angelo Risi in 1974.



Our simple solid block design ensures guaranteed performance. Block weight, shear strength, wall alignment & geogrid connection strength are all integrated right into the block.



Our Engineers have over 75 years of combined SRW design experience. We strive to provide you with precise, efficient information, advice & accurate Engineer Sealed designs.



We offer a complete range of product sizes, from hand-placed, to massive machine placed blocks. All of our products are purposely dimensioned for maximum versatility in any application.



Our SienaStone system has been evaluated & MTO approved against the most stringent design, manufacturing & quality checks.

Budgeting & Selecting the Right Product

The chart below provides comprehensive details about products, wall height restrictions and optimal application.

	Rivercrest® Natural	Pisa2® Classic	PisaSmooth™ Modern	SienaSmooth Modern Colossal	SienaStone® Colossal	SonomaStone® Colossal	DuraHold® Colossal
Block Dimensions	Hand-Placed 31 x 5.7 x 24 cm 7 Kg / 15 Lbs Vertical	Hand-Placed 20 x 15 x 30 cm 21 Kg / 46 Lbs Vertical / Battered	Hand-Placed 55 x 8.5 x 27 cm 24 Kg / 54 Lbs Near / Vertical	Machine-Placed 120 x 18 x 37.5 cm 170 Kg / 375 Lbs Near / Vertical	Machine-Placed 120 x 18.5 x 50 cm 256 Kg / 570 Lbs Vertical / Battered	Machine-Placed 120 x 18.5 x 37.5 cm 175 Kg / 385 Lbs Vertical / Battered	Machine-Placed 183 x 30.5 x 61 cm 732 Kg / 1610 Lbs Vertical / Battered
Wall Height	< 0.6m	< 1.8m	< 1m	< 1.2m	< 1.5m	< 1.5-3m	< 1.8m
Wall Configuration	Gravity	Reinforced	Gravity	Gravity	Gravity	Gravity & Multi-Depth	Gravity
Application	Complex Geometry, Residential	Complex Geometry, Commercial, Residential	Complex Geometry, Residential	Modern Linear Geometry, Residential	Commercial, Machine Placed, Space Restrictions Property Lines	Higher Wall, Space Restrictions Property Lines	Critical Application, Commercial, High Wall, Machine Placed
Style & Appearance	Natural	Classic	Modern	Modern	Colossal	Colossal	Colossal

Retaining Wall Design Process



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Comprehensive Designs & Solid Support

At Risi Stone Systems, we know Wall Design and we know what a comprehensive Design involves. While many competitors offer some form of "typical" or preliminary design, we are able to offer complete Designs that are specific to your Project and Region. Our Engineers use Vespa.RS, a cutting edge SRW design software that can layout and analyze your wall design ensuring it's fully compliant with NCMA or AASHTO Design Methodologies.

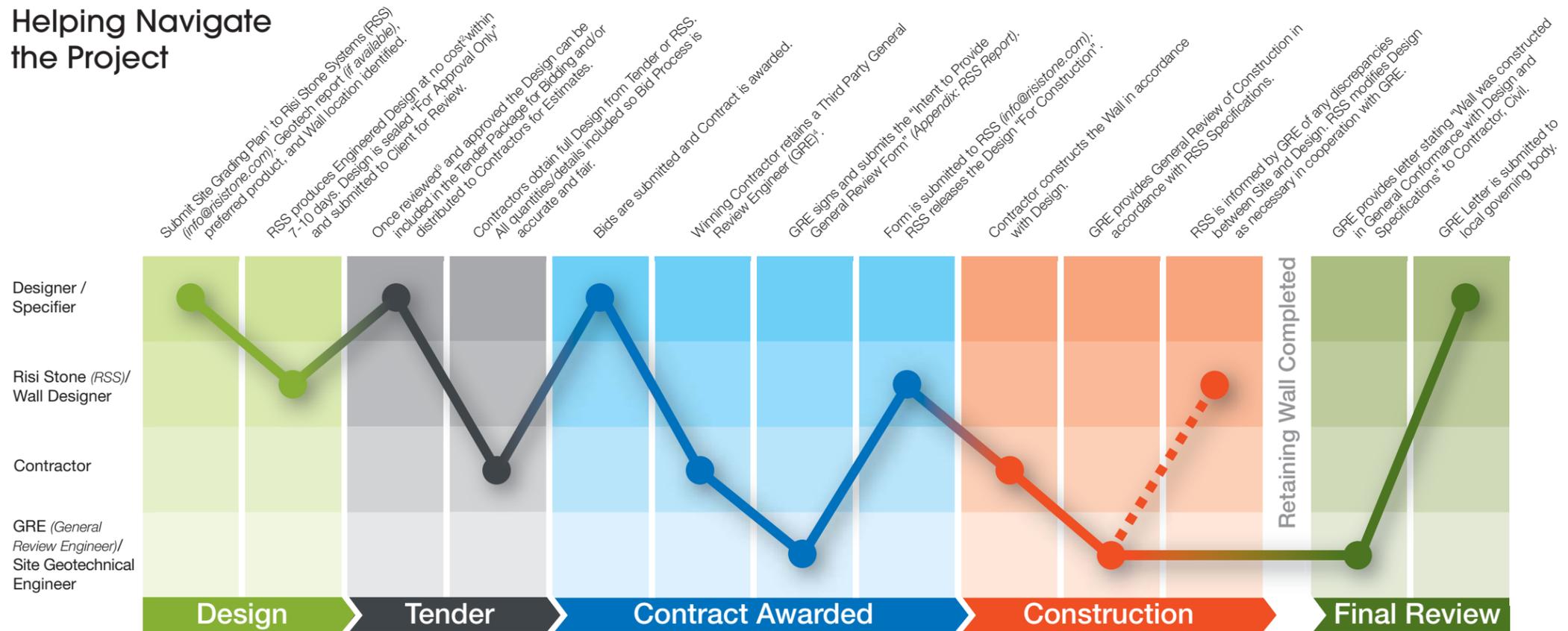
Total Cost vs Block Cost

On any Wall Project, the actual cost of the block is approximately 25%-30% of the Total cost of the Installed Wall, therefore, minor differences in "block" price rarely have a significant impact on the Total Installed Wall Cost.

Block Quality

Unilock blocks are the highest quality in the industry ensuring long term performance. With a minimum 5000 psi compressive strength and maximum 5% water absorption, our Systems are manufactured under the strictest quality control for proven long term durability and performance. Many other systems on the market are 2500-3000 psi compressive strength, are hollow, and require more labour time to place and level the units (core filling, shimming due to dimensional problems, addition of connectors). In the end, the "cheaper" alternative ends up costing about the same, but more of the project dollars are directed into the labour costs to install the block, not the quality of the block itself. And remember, it is the Block that will be there for the next 75+ years. The laborer leaves the site after the job is done.

Helping Navigate the Project



Appearance



¹ If you are requiring a Fully Engineered Wall Design, please provide us with what you consider to be a "Final" grading plan, so that we do not incur numerous and costly revisions as you continually revise the Plan. We appreciate it!
² A nominal fee is charged to the Contractor who is awarded the Project. For very complex Designs, with multiple revisions, a subsidized Design Fee may be negotiated between Risi Stone and the Client for Wall Design Services.
³ Wall Design is not reviewed with respect to Structural Stability, Compliance with the Building Code, etc. Review is only to ensure the Design, as shown, fits within the constraints (space, property lines) of your Project and meets the Grading Plan requirements. RSS is the Structural Designer of the Wall and take full responsibility for the Wall Design.
⁴ Ideally, the GRE is the same individual or firm providing Geotechnical Inspection on the Site. The GRE is not responsible to review the Design for Structural adequacy; they are only to ensure that the Wall is being constructed in General Conformance with the Design.