



# 627 Lyons Lane, Oakville Transportation Impact Study (3<sup>rd</sup> Submission)

Paradigm Transportation Solutions Limited

May 2022



## Project Number

210060

## 627 Lyons Lane, Oakville

## Transportation Impact Study (3<sup>rd</sup> Submission)

May 2022

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# Executive Summary

## Content

Paradigm Transportation Solutions Limited (Paradigm) was retained to conduct this Transportation Impact Study (TIS) for a residential development located at 627 Lyons Lane in the Town of Oakville.

## Conclusions

This study evaluated the impacts of the construction of 281 apartment units in a 28-storey building on a parcel of land bounded by South Service Road and Lyons Lane. Vehicular access to the building's parking facilities and loading area is proposed via two accesses to the South Service Road located at the eastern limits of the property. The western driveway will be designated for pick-up and drop-off service for clarification. Overall, the proposed development will generate approximately 112 new vehicle trips during the weekday AM peak hour and 110 new vehicle trips during the weekday PM peak hour.

Detailed traffic analysis was conducted for each study area intersections under Base conditions, 2025, 2030 and 2035 Background and Total conditions.

The new traffic forecast to be added by full build-out of the development to the study area results in relatively small impacts at the various study intersections. The analysis has further determined that the proposed driveways to South Service Road and Lyons Lane will operate at LOS A during the weekday peak periods, and separate left-turn lanes are not warranted at either driveway connection under the 2035 Total conditions.

It is acknowledged that deficiencies currently exist at specific locations, primarily along the Trafalgar Road corridor within the study area. They can be expected to persist in the future with anticipated growth in traffic, independent of the development.

A variety of roadway improvements are planned within the study area to address the existing and long-term impacts of traffic growth, including the construction of new direct off-ramps for the QEW at Trafalgar Road, a revised local road network for Midtown Oakville, an extension of Cross Avenue and a variety of intersection improvements. It is understood that these improvements will provide some relief to operational issues. However, capacity constraints will persist for the overall transportation network.

Due to the high levels of congestion and the expected long-term transit network anticipated to be developed (BRT along Trafalgar Road), further remedial measures to improve intersection capacity are not likely to be implemented. Instead, future improvements to the transportation network are expected to focus on public transit infrastructure.



By focusing on shifting commuter travel to public transit, intersection operations could be expected to maintain the status quo (a capacity condition during peak hours) or possibly improve if fewer vehicles transverse the intersections during the peak hours of a typical weekday.

As requested by the Town, a sensitivity analysis has also been assessed, assuming Lyons Lane is closed off at Cross Avenue. If this occurs, and considering the MOEA roadway network is not established, the intersection of South Service Road at Argus Road would experience considerable delay and congestion, and mitigation measures would need to be implemented (i.e., the potential for restricted movements)

## Recommendations

- ▶ The site plan is updated to include short-term bicycle parking at the main entrances of the buildings in a visible well-lit area.
- ▶ Long-term bicycle parking in a safe and secure area (either a dedicated room or storage lockers) be included in the site plan.
- ▶ The on-site pedestrian sidewalks are well-lit and conform to the Town of Oakville's design standards and the Accessibility for Ontarians with Disabilities Act (AODA) design standards.
- ▶ Consider unbundling parking where parking spaces are provided as a separate cost to residents.
- ▶ Provide general education on all travel mode options (walking, cycling, and transit) that identify benefits and how residents can best utilize these modes. New residents should be provided with a welcome package that outlines proximity to transit, cycling facilities and the proximity to local activity centres
- ▶ If any, the site operator monitors the long-term desire lines created by the erosion caused by pedestrians crossing the site's landscaped areas. Should desire lines form, there may be an opportunity to adjust the site's landscaping to encourage the use of the designated on-site pedestrian sidewalks.
- ▶ The site operator monitors the on-site bicycle parking supply to ensure that the appropriate amount of bicycle parking is provided.



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# 1 Introduction

## 1.1 Overview

Paradigm Transportation Solutions Limited (Paradigm) was retained to conduct this Transportation Impact Study (TIS) for a residential development located at 627 Lyons Lane in the Town of Oakville, Ontario. **Figure 1.1** details the location of the subject development.

## 1.2 Previous Submission

Two previous submissions of the Transportation Impact Study for the subject lands were undertaken as described:

- ▶ 180307: 627 Lyons Lane, Oakville, Transportation Impact Study, August 2019.
- ▶ 210060: 627 Lyons Lane, Oakville, Transportation Impact Study (2<sup>nd</sup> Submission), August 2021.

In response to comments received from the Town and Region, Paradigm has prepared this third Submission to address comments received and minor changes to the concept plan. The comments and responses are provided in **Appendix A**.

## 1.3 Purpose and Scope

The purpose of the study is to assess current traffic and site conditions in the vicinity of the proposed development, forecast additional traffic that will be generated by the proposed development, analyze the impact that this traffic may have on the adjacent roadway network and provide the municipality and owner required to mitigate the identified impacts of the site generated traffic.

More specifically, the scope of this study is to:

- ▶ Forecast traffic from the proposed development;
- ▶ Assign the projected volumes to the surrounding road network based on the existing traffic patterns at the driveway connection;
- ▶ Assess total future traffic within the study area. The following horizons have been considered: Opening Year (2025), five years from Opening Year (2030), and ten years from Opening Year (2035); and
- ▶ Identify operational or safety concerns and any mitigation measures that may be required to improve operations; and
- ▶ Review and identify potential Transportation Demand Management (TDM) measures that can be implemented for the proposed development.



This report adheres to the terms of reference developed by Paradigm and agreed upon by the Region of Halton and the Ministry of Transportation, Ontario (MTO). It is noted that the Town of Oakville was included in terms of reference correspondence; however, the Town did not provide any feedback. The correspondence is provided in **Appendix B**. The study area, as agreed upon with the review agencies, includes the following intersections:

- ▶ Trafalgar Road and QEW Westbound Off-Ramp / North Service Road (Signalized);
- ▶ Trafalgar Road and QEW Eastbound Off-Ramp (Signalized);
- ▶ Trafalgar Road and Cross Avenue / South Service Road (Signalized);
- ▶ Trafalgar Road and Cornwall Road (Signalized);
- ▶ Cross Avenue and Argus Road / GO Station Driveway (Signalized);
- ▶ Cross Avenue and Lyons Lane / Commercial Driveway (Signalized);
- ▶ Cross Avenue and Lyons Lane (Unsignalized);
- ▶ Lyons Lane and South Service Road (Unsignalized);
- ▶ Trafalgar Road and Argus Road (Unsignalized); and
- ▶ Argus Road and South Service Road.





NTS

Image Source: OpenStreet Map



# Study Area & Development Location

627 Lyons Lane, Oakville  
210060

Figure 1.1

## 2 Existing Conditions

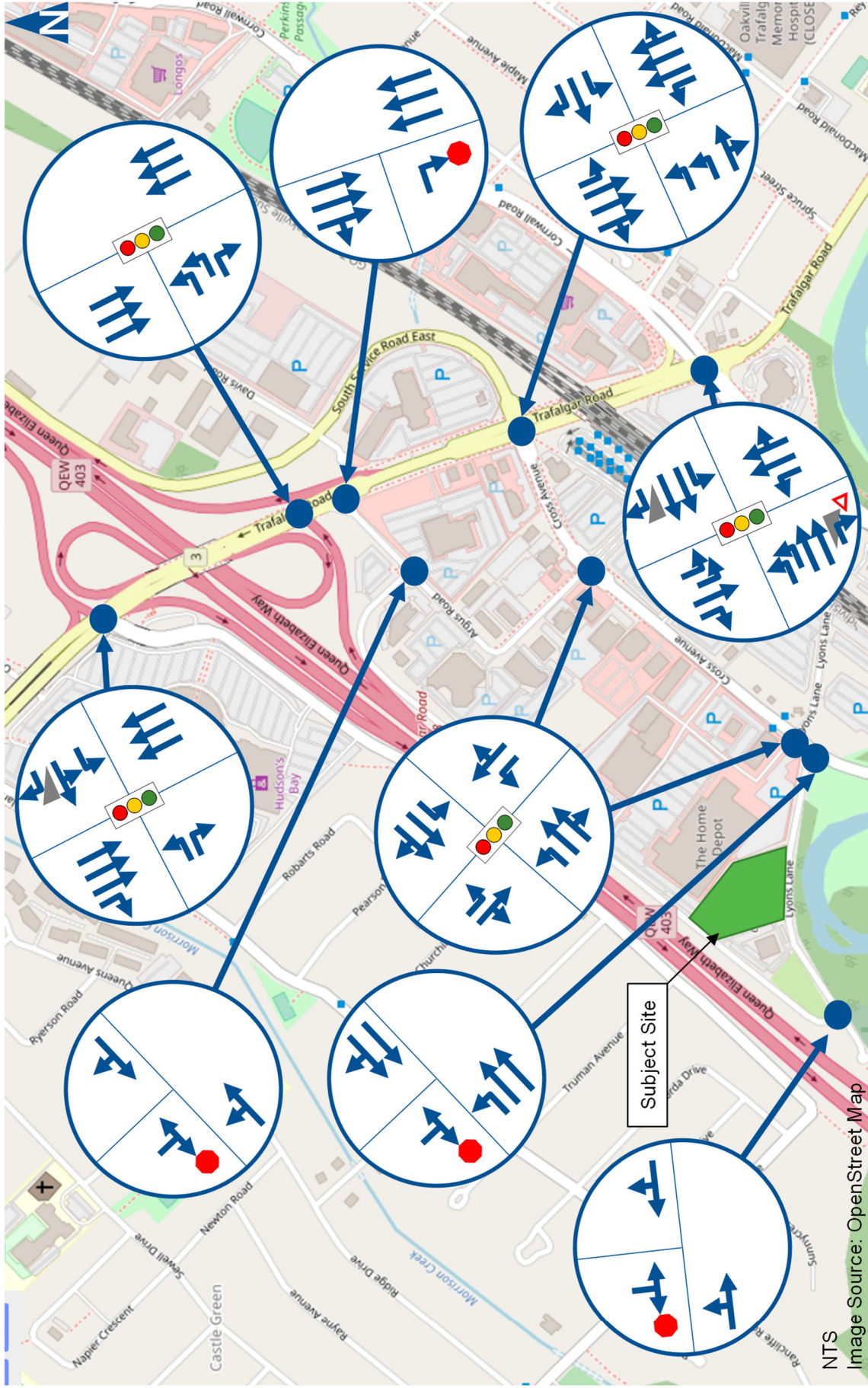
This section documents current traffic conditions, operational deficiencies, and constraints experienced by the public travelling at the intersections within the study area. The operational deficiencies and constraints identified at this stage will be fundamental to defining the required remedial measures.

### 2.1 Road Network

**Figure 2.1** displays the existing lane configurations and traffic control provisions at the study area intersections. The roadways that the subject development will directly impact include:

- ▶ **Trafalgar Road** (Halton Region Road 3) is an urban major arterial road from Cornwall Road northwards and an urban minor arterial road southward from Cornwall Road. The roadway consists of a six-lane cross-section from Cornwall Road northwards and tapers down to a two-lane cross-section southward. The posted speed limit along Trafalgar Road is 50 kilometres per hour. Pedestrian facilities are provided along both sides of the road in the study area.
- ▶ **Cross Avenue** is an urban minor arterial road from Trafalgar Road to Cornwall Road. It provides access to the Oakville GO Station and the Commercial businesses surrounding the station. East of Trafalgar Road, Cross Avenue continues as South Service Road, eastwards towards Royal Windsor Drive. The assumed speed limit of Cross Avenue is 50 kilometres per hour. Pedestrian facilities are provided along both sides of the road in the study area.
- ▶ **Lyons Lane** is a two-lane local road north of Cross Avenue. Lyons Lane terminates as a cul-de-sac west of its intersection with South Service Road. There is a sidewalk on the east side of Lyons Lane. The assumed speed limit of Lyons Lane is 50 kilometres per hour.
- ▶ **South Service Road East** is a two-lane local road that fronts the QEW and provides additional access to the existing built lands. From its west end of Lyons Lane, it runs east parallel to the QEW, turning south to intersect with Argus Road. The assumed speed limit of South Service Road is 50 kilometres per hour. There are no pedestrian facilities along South Service Road.
- ▶ **Argus Road** is a two-lane local road that connects Trafalgar Road to Cross Avenue. There is a sidewalk on the south and east side of the roadway. The assumed speed limit of Argus Road is 50 kilometres per hour.





NTS  
Image Source: OpenStreet Map



## Existing Lane Configuration & Traffic Control

627 Lyons Lane, Oakville  
210060

Figure 2.1

## 2.2 Existing Transit Service

### 2.2.1 Oakville Transit

Oakville Transit owns and operates the public transit system in Oakville. The subject site is located within the Midtown Oakville Urban Growth Area, one of the most transit-accessible locations within the Town. The subject site is located approximately 450 metres to the western end of the Oakville GO Station, which is currently serviced by 14 out of 21 Oakville Transit Routes. The majority of the transit routes which provide access to all of the Town of Oakville operate seven days a week from early morning to late evening, with headways generally between 10 and 30 minutes depending on the day of week and time of day.

**Figure 2.2** illustrates the existing Oakville Transit network

### 2.2.2 GO Inter-Regional Transit

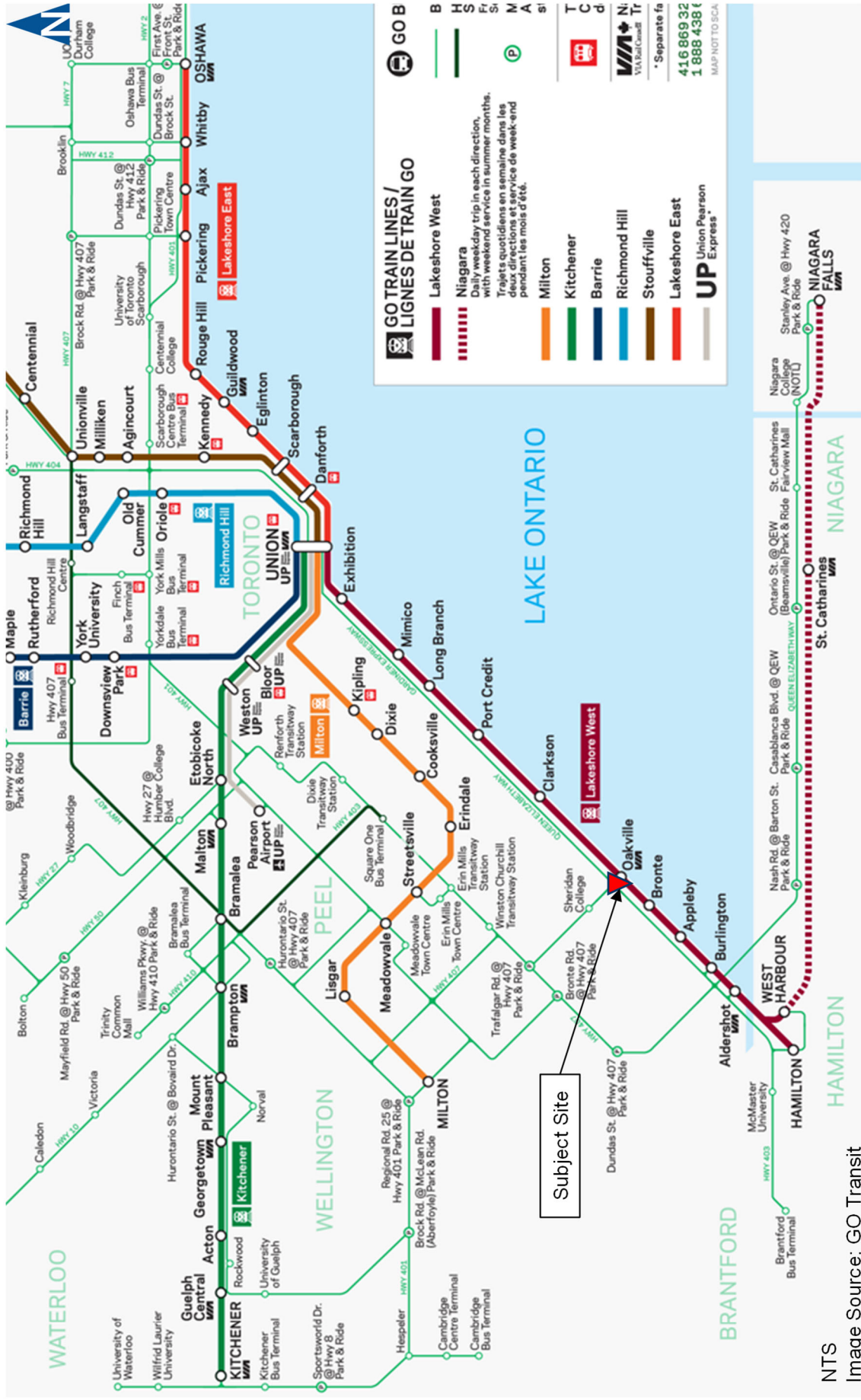
The proposed development is located approximately 450 metres to the western end of the Oakville GO Station. This station is located along the Lakeshore West Line, which currently operates a two-way all-day train service seven days a week and GO Bus connections to Hamilton and Sheridan College and York University via Highway 407.

**Figure 2.3** illustrates the GO Network's relation to the site.









# Existing GO Transit Network

Figure 2.3



## 2.3 Active Transportation

### 2.3.1 Pedestrians

The site is within walking distance of a number of retail opportunities providing for a range of destinations for prospective residents of the proposed development that can be readily accessed without using a vehicle.

Pedestrian sidewalks are provided on at least one side of streets through most study areas. Crosswalks, pedestrian pushbuttons, and indicators are provided for all approaches at the signalized intersections within the study area. A pedestrian connection under the QEW at the 16 Mile Creek connects both the north and south sides of the Highway.

The proximity of the site to such a range of amenities and destinations within walking distance reduces the need for residents to travel regularly using a car and own a vehicle.

### 2.3.2 Cycling

On-road cycling lanes are not currently provided on the streets in the study area. However, the Town of Oakville's Active Transportation Master Plan identifies that Trafalgar Road, Cross Avenue, and Lyons Lane are proposed to be reconstructed with on-street cycle lanes, as indicated in **Figure 2.4**.

In addition, as cyclists are permitted to ride on most roads except controlled-access highways, the lack of separate bicycle lanes on all other local and collector roadways will not prohibit this type of travel.

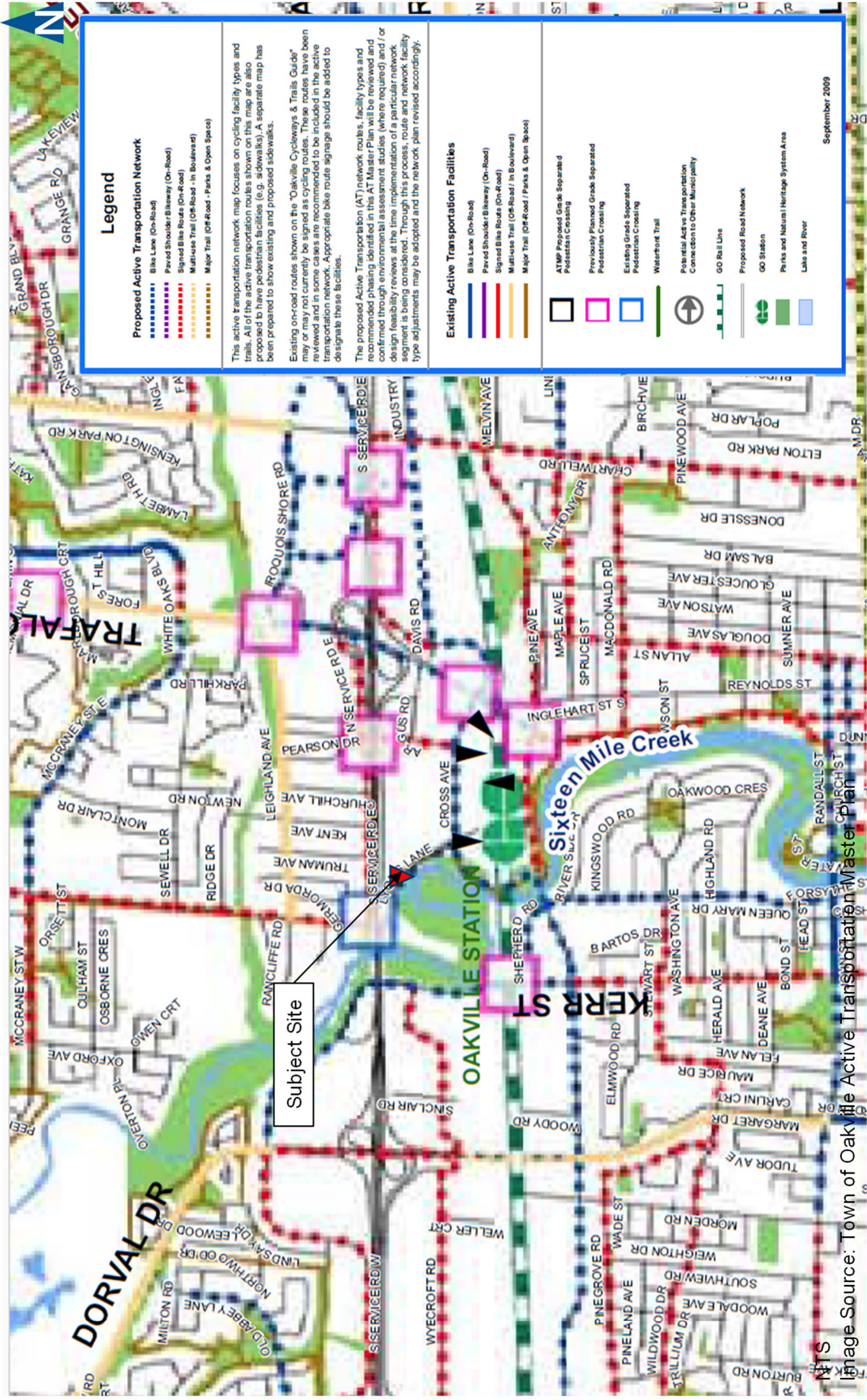
### 2.3.3 Travel Characteristics

A review of existing modes of transportation by area residents has been completed. Data on primary modes of transportation for all trips within Ward 3 was extracted from the 2016 Transportation Tomorrow Survey (TTS)<sup>1</sup>. The TTS data indicates that during the AM peak hour, the automobile accounts for 71 percent of trips made by residents, while transit accounts for 13 percent (one percent for local transit and 12 percent for GO Train). Walking and cycling (8 percent) and other modes (8 percent) account for the remaining percentages.

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<sup>1</sup> Transportation Tomorrow Survey 2016, Regional Municipality of Halton Summary by Ward, March 2018, Malatest





# Proposed Cycle & Trail Network

Figure 2.4



627 Lyons Lane, Oakville  
210060

## 2.4 Traffic Volumes

To assess intersection operation, turning movement counts are used to quantify the movement of vehicles through the area. Existing traffic data at an intersection or on a road section forms the foundation for analysis. The counts are usually taken during peak periods at an intersection to complete the level of service analysis. **Appendix C** contains the traffic data utilized in this report.

The MTO Traffic Impact Study Guidelines indicate the requirement for collecting new data for counts three years or older. Acknowledging this requirement and the current situation related to COVID-19, the ability to conduct up-to-date data collection is impacted.

Given that the GO Station essentially drives the traffic volumes along the study area roadways during weekday peak hours and since transit usage has been significantly hampered due to reduced demand, the ability to collect new traffic count data was not feasible as the volumes may not be representative of typical traffic volumes. However, the MTO recognizes the current situation and has supported the use of pre-COVID-19 TMCs, and, if deemed necessary, to apply a factor to the volumes.

As a result, historic TMC data has been used and adjusted using a growth rate of 2% per annum to provide reasonable traffic volumes for the baseline horizon (2022).

### 2.4.1 Traffic Data

Existing historical traffic counts were obtained from the Region of Halton, MTO, and count data Paradigm undertook as part of the 1<sup>st</sup> Submission. **Table 2.1** provides a summary of traffic count locations and sources.

**TABLE 2.1: TRAFFIC COUNT SUMMARY**

Intersection		Date
Trafalgar Road	QEW Westbound Off-Ramp/North Service Road	June 1, 2017
	QEW Eastbound Off-Ramp	June 1, 2017
	Argus Road	May 15, 2017
	Cross Avenue/South Service Road	June 1, 2017
	Cornwall Road	June 1, 2017
Cross Avenue	Argus Road/GO Station Driveway	January 10, 2019
	Lyons Lane	January 10, 2019
South Service Road	Argus Road	January 10, 2019
	Lyons Lane	January 10, 2019

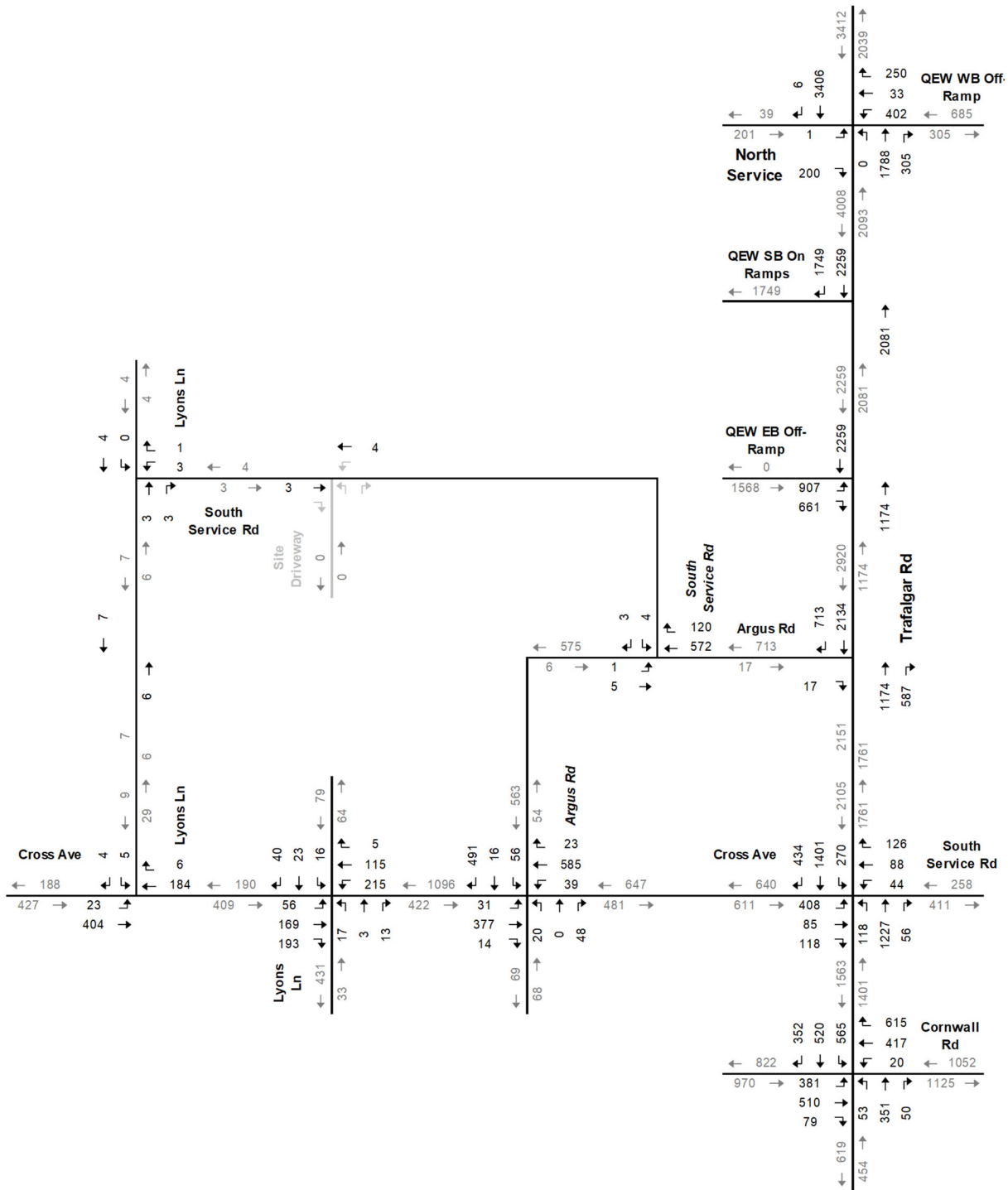


## 2.4.2 Volume Balancing

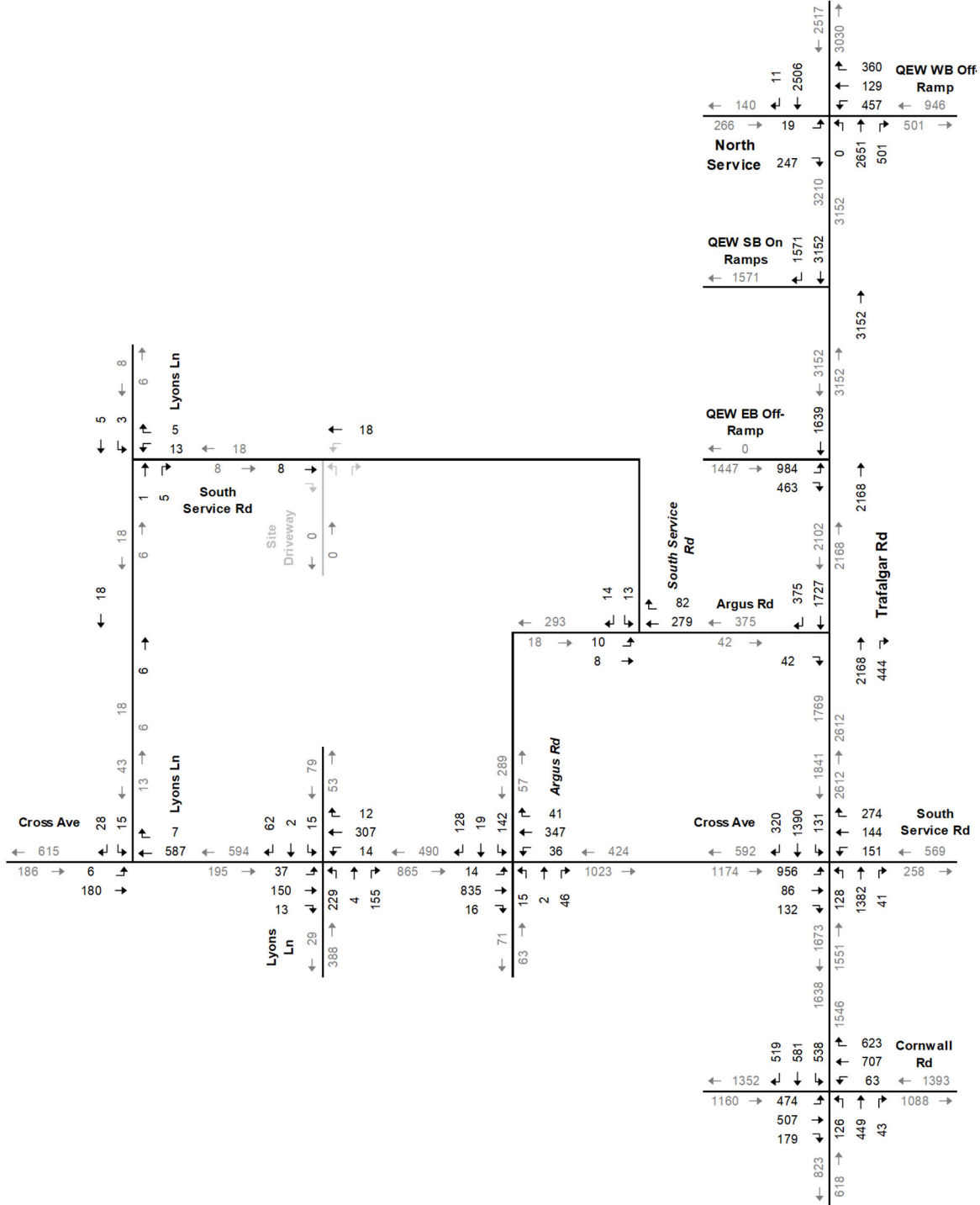
Volume balancing along Trafalgar Road has also been applied between the North Service Road and Argus Road to ensure that the corridor maintains proper upstream and downstream flow. No balancing occurred on Trafalgar Road, south of Argus Road and along Cross Avenue west of Argus Road to account for the GO Station and commercial driveways.

**Figures 2.5A** and **2.5B** illustrate the adjusted base year traffic volumes during the weekday AM and PM peak hours.





# Base Year Traffic Volumes AM Peak Hour



# Base Year Traffic Volumes PM Peak Hour

627 Lyons Lane, Oakville  
210060

Figure 2.5B



## 3 Development Concept

### 3.1 Development Description

The property at 627 Lyons Lane is located between Lyons Lane and South Service Road, east of the Lyons Lane and South Service Road intersection.

The development proposal for the site consists of a 28-storey building with 281 apartment units. Vehicular access to the building's parking facilities and loading area is proposed via two accesses to the South Service Road located at the eastern limits of the property. For clarification, the western driveway will be designated for pick-up and drop-off service and has not been assessed as part of the operational analysis. **Figure 3.1** illustrates the development site plan.

#### 3.1.1 Pedestrian Routes

**Figure 3.2** illustrates the proposed pedestrian circulation plan related to the proposed development. Owing to its active transportation-oriented design, the proposed plan has a high degree of pedestrian connections providing three connections with the municipal sidewalks network along Lyons Lane, providing access to retail uses within the area and the Oakville GO Station.

#### 3.1.2 Swept Path Analysis

The truck turning assessment was completed using AutoTURN swept path analysis software to ensure adequate manoeuvrability through the site. The vehicle movements were examined using a CAD base file of the development plan dated 20 April 2022. The swept path analysis was conducted to investigate the on-site maneuverability of the larger design vehicles expected to utilize the site; Medium Single Unit (MSU) and Front-End Loader Garbage Truck (Custom). The vehicle dimensions for the garbage truck are based on specifications outlined in the Development Design Guidelines for Source Separation of Solid Waste<sup>17</sup>.




**Appendix D** provides the vehicle manoeuvring analysis and the profile and dimensions of the designed vehicles. The AutoTURN analyses indicate that the design vehicles have minor difficulty entering the loading bay.

Modifications to the loading bay are required to accommodate these vehicles entering the site. The AutoTURN swept path analysis indicates that large design vehicles will function adequately in terms of overall manoeuvring from a site circulation perspective.







	Municipal Sidewalk
	Proposed Walkway
	Pedestrian Connection



# Pedestrian Circulation Plan

627 Lyons Lane, Oakville  
210060

Figure 3.2

## 3.2 Development Trip Generation

The Town has requested that Multifamily Housing (Mid-Rise) Land Use (Land Use Code 221) from the Institute of Transportation Engineers (ITE) Trip Generation 10<sup>th</sup> Edition be used to estimate the weekday AM and PM peak hour traffic volumes that the development will generate.

As shown in **Table 3.1**, using the fitted curve equation, the estimated trip generation for the development indicates that 112 trips are forecasted to be generated during the AM peak hour and 110 trips during the PM peak hour.

**TABLE 3.1: TRIP GENERATION**

ITE Land Use	Units	AM Peak Hour			PM Peak Hour		
		In	Out	Total	In	Out	Total
221 - Multifamily Housing (Mid Rise)	281	26	86	112	67	43	110
<b>Trip Generation</b>		<b>26</b>	<b>86</b>	<b>112</b>	<b>67</b>	<b>43</b>	<b>110</b>

Trip reductions to account for transit usage (6.6%), active transportation modes (6.0%), and travel demand management (6.0%) have been applied to the base trip generation. The trip reductions were obtained from the Town of Oakville Transportation Master Plan<sup>2</sup>.

As shown in **Table 3.2**, the estimated trip generation after the reductions applied indicates 91 weekday AM peak hour and 89 weekday PM peak hour trips.

**TABLE 3.2: TRIP GENERATION WITH MODE SPLIT REDUCTION**

ITE Land Use	Units	AM Peak Hour			PM Peak Hour		
		In	Out	Total	In	Out	Total
221 - Multifamily Housing (Mid Rise)	281	26	86	112	67	43	110
<b>Trip Generation</b>		<b>26</b>	<b>86</b>	<b>112</b>	<b>67</b>	<b>43</b>	<b>110</b>
<i>Transit Mode Split</i>	6.6%	-2	-6	-7	-4	-3	-7
<i>Active Transportation Mode Split</i>	6.0%	-2	-5	-7	-4	-3	-7
<i>Travel Demand Management Split</i>	6.0%	-2	-5	-7	-4	-3	-7
<b>Total Net Trip Generation</b>		<b>20</b>	<b>70</b>	<b>91</b>	<b>55</b>	<b>34</b>	<b>89</b>

However, as per MTO comments received as part of the 1<sup>st</sup> Submission, MTO's opinion that this level of mode share is too optimistic. As a result, the analysis has not assumed any mode split for the proposed development.

<sup>2</sup> Town of Oakville Transportation Master Plan Review Final Draft January 2018, WSP + GLP



### 3.3 Development Trip Distribution and Assignment

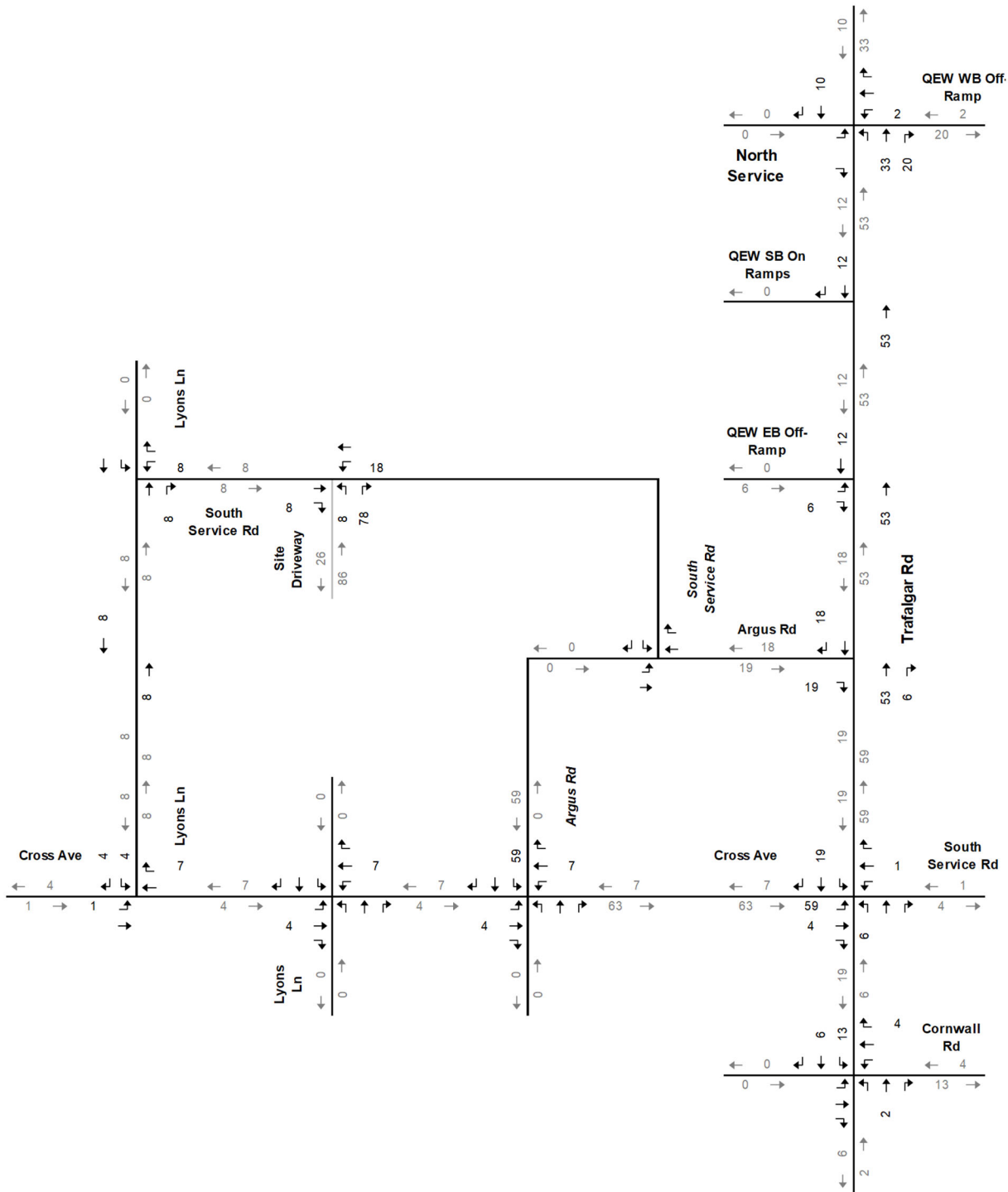
The site-generated trips were assigned to the road network based on the existing distribution of traffic at the study area intersections. The distribution is summarized in **Table 3.3**.

**Figure 3.2A** and **Figure 3.2B** display the resulting AM and PM peak hour site-generated trip assignments.

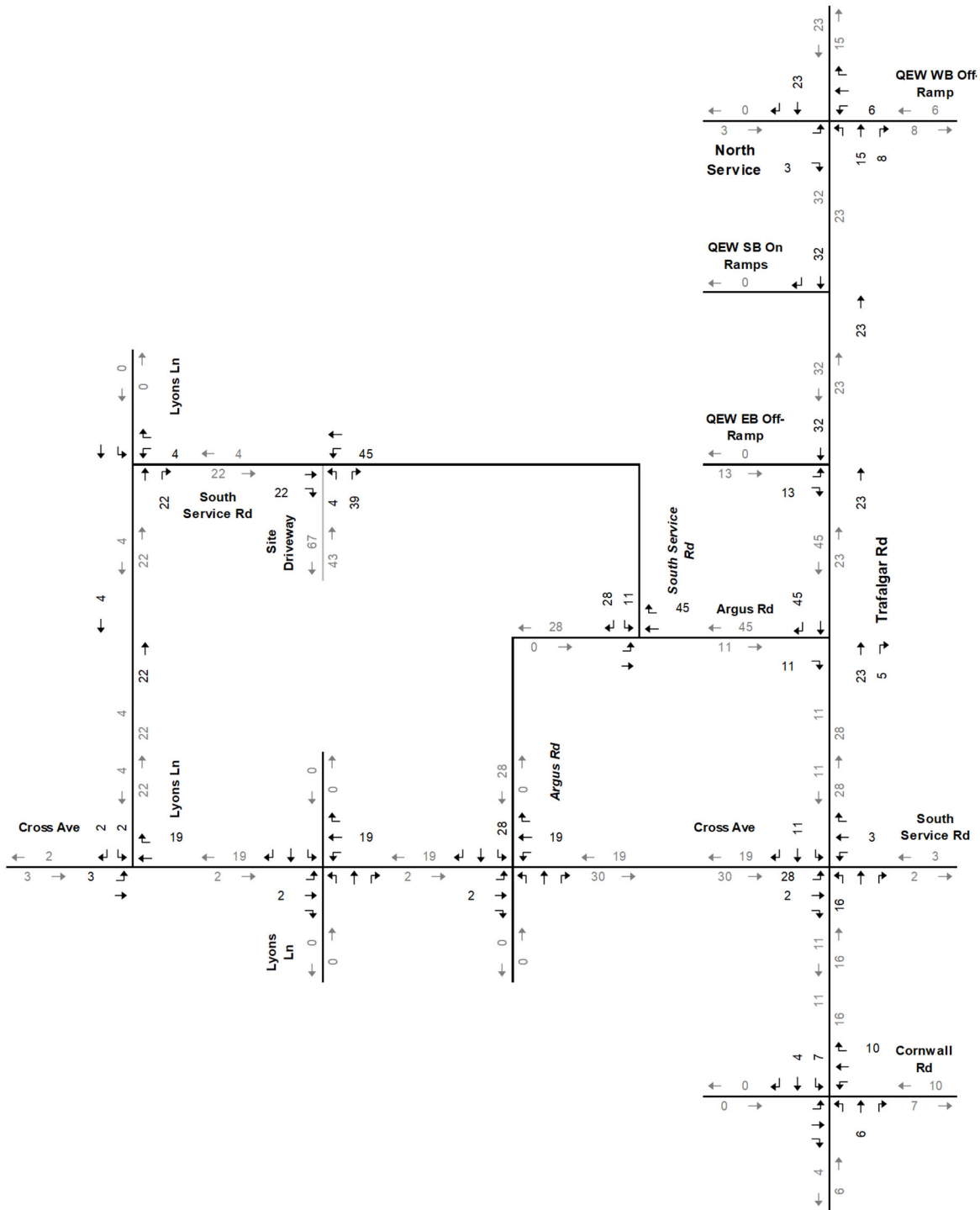
**TABLE 3.3: TRIP DISTRIBUTION**

Direction	Route	AM Peak Hour	PM Peak Hour
North	Trafalgar Road	39%	38%
South	Trafalgar Road	7%	9%
East	QEW	7%	9%
	South Service Road	5%	5%
	Cornwall Road	15%	15%
West	QEW	23%	19%
	Cross Ave	4%	5%
<b>Total</b>		<b>100%</b>	<b>100%</b>





# Site Generated Traffic Volumes AM Peak Hour



# Site Generated Traffic Volumes PM Peak Hour

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Figure 3.2B

## 4 Future Conditions

To remain consistent with MTO and Region traffic impact study guidelines, the horizon years of 2025, 2030 and 2035 have been utilized to analyze future traffic conditions.

### 4.1 Trafalgar Road EA

The Trafalgar Road (Regional Road 3)<sup>3</sup> Improvements Class Environmental Assessment Study from Cornwall Road to Highway 407 was completed in May 2015. It was recommended that Trafalgar Road be widened from four (4) to six (6) lanes and potentially converted the curb lanes to high occupancy vehicle (HOV) or bus rapid transit (BRT) lanes after completion of the road widening.

Trafalgar Road is currently a six-lane cross-section plus exclusive left-turn lanes in both directions within the study area. It is assumed that the Trafalgar corridor will operate as a six-lane corridor with HOV curb lanes for future horizons. The only modification to the road network for future analyses is removing the eastbound channelized right turn at Trafalgar Road and Cornwall Road to be consistent with the preferred design.

### 4.2 Midtown Oakville EA

The Town of Oakville completed a Class Environmental Assessment (EA) for Midtown Oakville (MOEA)<sup>4</sup> to guide the development of the transportation and municipal stormwater network needed to accommodate the planned growth in Midtown Oakville. The MOEA identified critical changes to the existing and planned road network that would be required to support intentional growth.

In addition, other master plans have been updated and technical studies completed, including the Halton Region Transportation Master Plan, the Town of Oakville Transportation Master Plan – Switching Gears, the Midtown Parking Strategy (2014), and Designing Midtown Oakville (2014). As a result, the Town has proposed an Official Plan Amendment (OPA) that would incorporate the results of these studies into the Official Plan and bring the policies and Schedules into alignment with the most current source documents.

The changes to the Midtown-related transportation network include modifications designed to the broader area network and changes to the local road network within Midtown Oakville. To accommodate traffic to and from Midtown Oakville and to provide an alternative to Trafalgar Road, several

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<sup>3</sup> Trafalgar Road Improvements Class Environmental Assessment Study From Cornwall Road to Highway 407, Town of Oakville, AECOM, April 2015.

<sup>4</sup> Midtown Oakville Transportation and Stormwater Municipal Class Environmental Assessment, Cole Engineering, June 2015.





improvements are provided, including direct off-ramps from eastbound QEW at Trafalgar Road and new ramps to/from the QEW at Royal Windsor Drive;

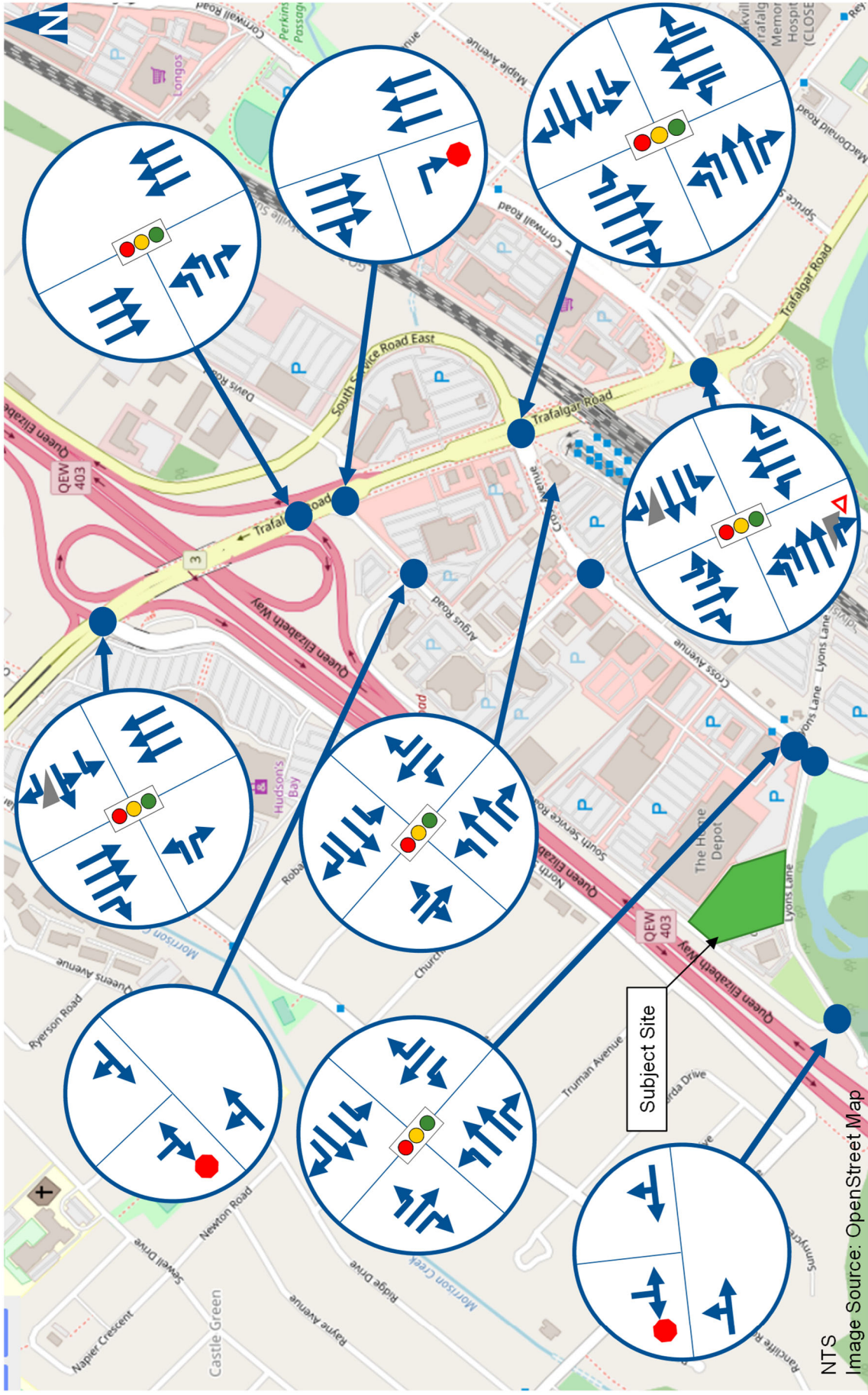
- ▶ A direct route from eastbound QEW to Midtown Oakville is provided via a new off-ramp that crosses under Trafalgar Road. This reduces the impacts of future traffic demand on the existing constrained intersections along Trafalgar Road at the off-ramp and Cross Avenue. The underpass of Trafalgar Road also provides the opportunity for improved active transportation connections into Midtown Oakville.
- ▶ A direct route from eastbound QEW to Midtown Oakville is provided via a new off-ramp to Cross Avenue at the Royal Windsor Drive interchange. A direct route from Midtown Oakville to eastbound QEW is provided via a new on-ramp at Royal Windsor Drive opposite Cross Avenue. A new westbound QEW off-ramp at Royal Windsor Drive will offer an alternative route to Midtown Oakville and surrounding areas.
- ▶ Cross Avenue is extended from Trafalgar Road to Royal Windsor Drive, connecting with the enhanced QEW interchange. Cross Avenue will provide accessible facilities for pedestrians and cyclists to travel safely, on-street parking where appropriate and four vehicular travel lanes.
- ▶ For access and circulation within Midtown Oakville, a revised local road network for Midtown Oakville is designed to support and align with the broader transportation network determined through the MOEA. As part of the revised road network, Lyons Lane at Cross Avenue is proposed to be realigned to form a four-way signalized intersection.

The proposed improvements are not expected to significantly impact the QEW mainline operations with the refinements to the design to accommodate weaving and merging better. The new ramps at Royal Windsor Drive and Trafalgar Road will accommodate the additional travel demand from Midtown Oakville's planned intensification, thereby relieving the existing Trafalgar Road interchange. **Figure 4.1A** illustrates the future lane configuration.

These improvements are assumed to be in place for the 2030 and 2035 horizons to assess the long-term impacts on the area. **Figures 4.2A and 4.2B** illustrate the proposed transportation network for Midtown Oakville.

**The MOEA recognized that the roadway improvements identified herein would relieve operational issues; however, capacity constraints will persist for the overall transportation network.**





NTS

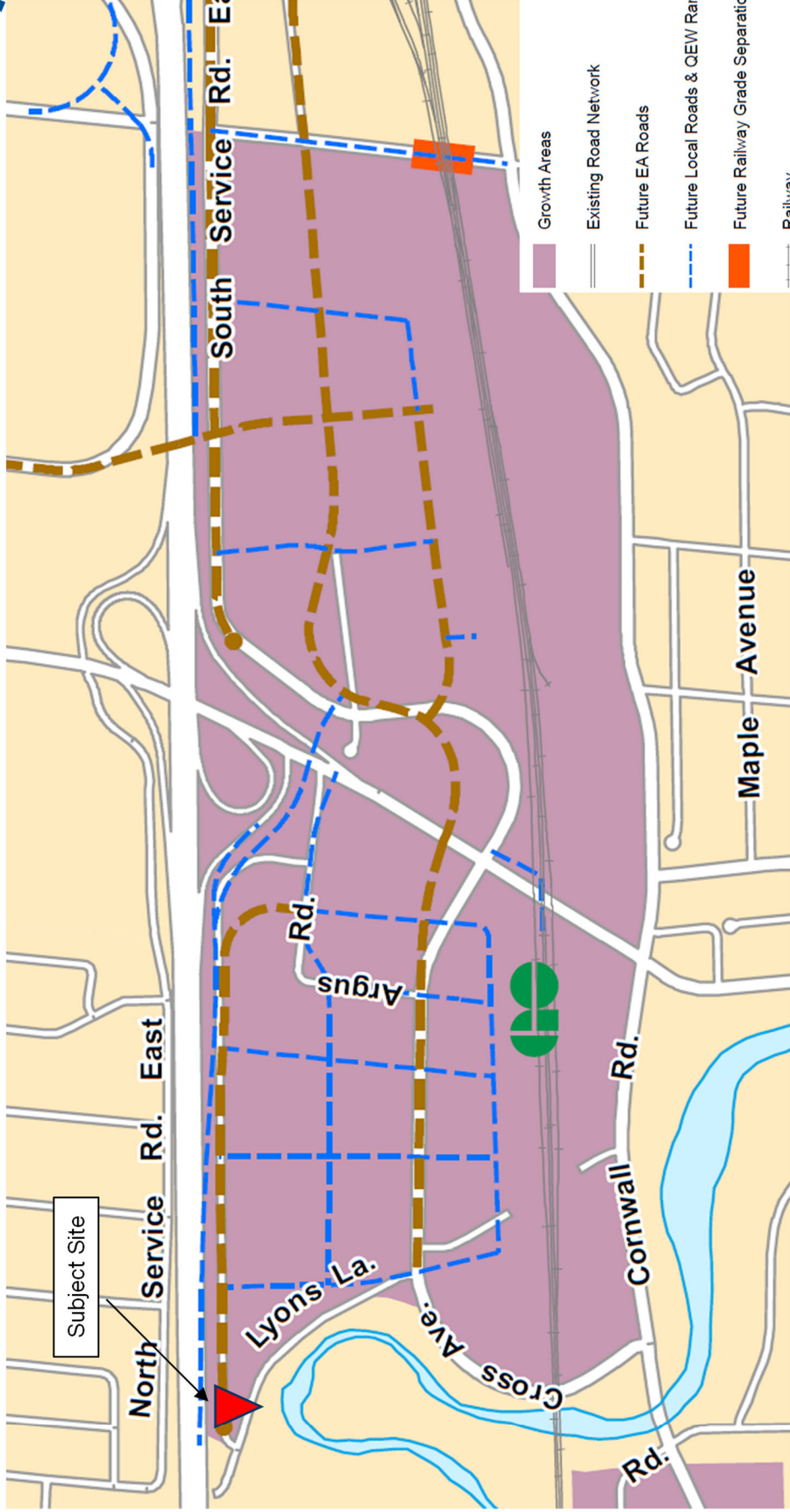
Image Source: OpenStreet Map



## Future Lane Configuration & Traffic Control

627 Lyons Lane, Oakville  
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Figure 4.1A



NTS  
Image Source: OPA14



# Midtown Oakville Overall Road Network

627 Lyons Lane, Oakville  
210060

Figure 4.1B



NTS  
Image Source: Midtown Oakville Class EA



# Midtown Oakville Broader Area Improvements

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Figure 4.1C

## 4.3 Future Forecasts

Traffic growth on area roadways is a function of the expected land development, economic activity, and changes in demographics. A frequently used procedure estimates an annual percentage increase and applies that increase to the study area traffic volumes. An alternative approach is to identify estimated traffic generated by specific planned significant developments that would be expected to affect the project study area roadways. For this assessment, both methods were utilized.

Based on discussions with Region of Halton staff, a general growth rate of 2.0% was applied to the area roadways to account for population and employment growth.

The MOEA<sup>5</sup> provides the best source for the overall development context and traffic growth expectations for the immediate study area. This report provides traffic forecast for a 17-year horizon with a development assumption of 5,960 residential units, 370,000 square feet of retail uses, 1.4 million square feet of office spaces with supporting institutional and recreational uses. In addition to the general traffic growth, it is assumed that a portion of Midtown Oakville's redevelopment will occur in the 2030 and 2035 horizons. This report considers 20% to 40% build-out for the 2030 and 2035 horizons.

Given that the MOEA most closely models the overall redevelopment plans, traffic projections contained within the MOEA were the primary source for developing background development projections. Adjustments were made to the traffic forecasts extracted from this study to account solely for site-specific growth. Specifically, as a growth rate of 2% per annum was most likely utilized in these forecasts, this growth has been removed.

**Appendix E** provides traffic projections related to site-specific growth.

### 4.3.1 Background projections

The weekday AM and PM peak hour background traffic volumes for 2025, 2030 and 2035 horizons are illustrated in **Figures 4.2 - 4.3**.

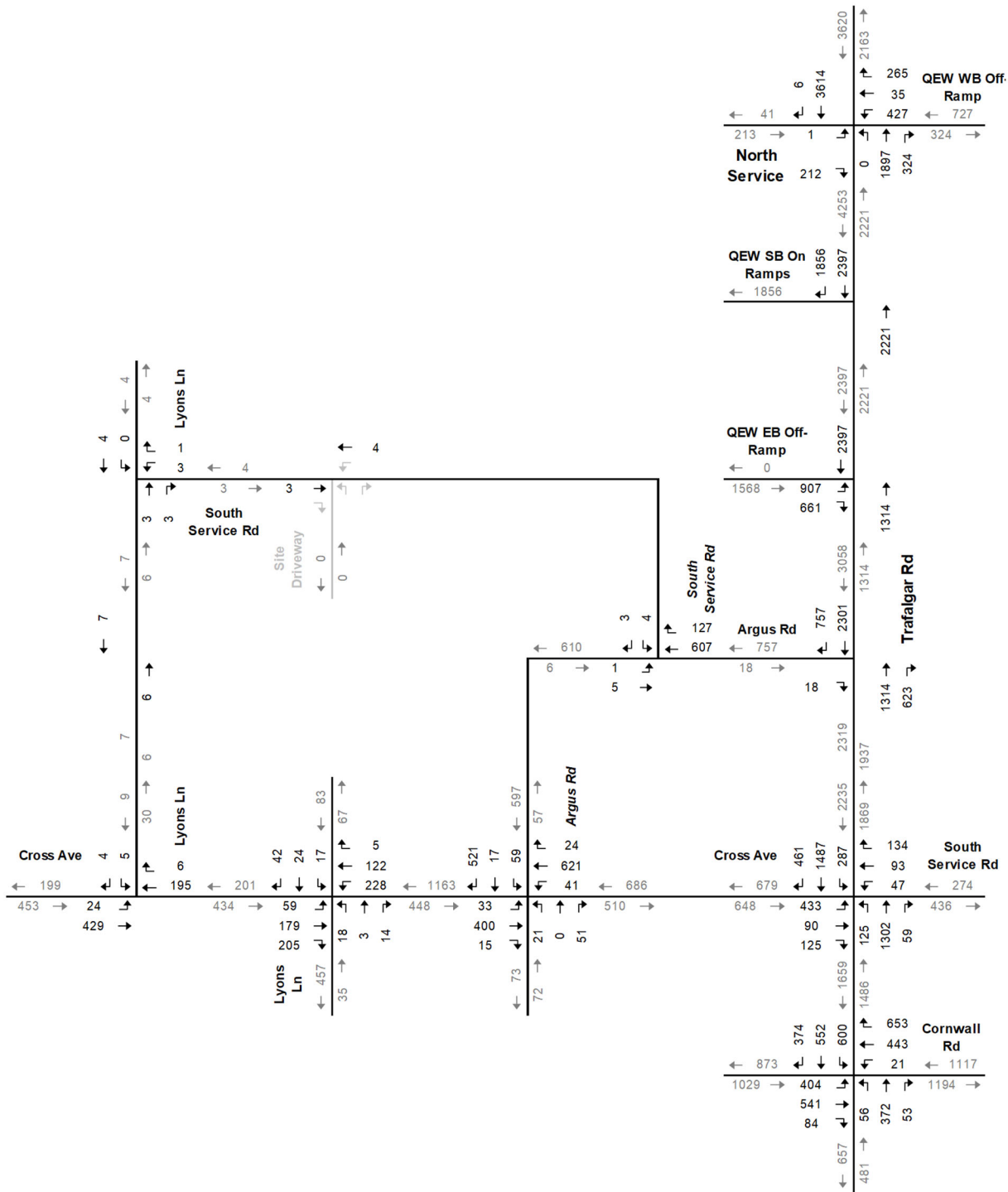
### 4.3.2 Total Projections

The projected site-generated traffic volumes were added to the Background projections to develop the Total traffic volumes. The weekday AM, and PM peak hours Total traffic volumes for 2025, 2030 and 2035 horizons are illustrated in **Figures 4.5 - 4.7**.

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<sup>5</sup> Midtown Oakville Transportation and Stormwater Municipal Class Environmental Assessment, Cole Engineering, June 2015.

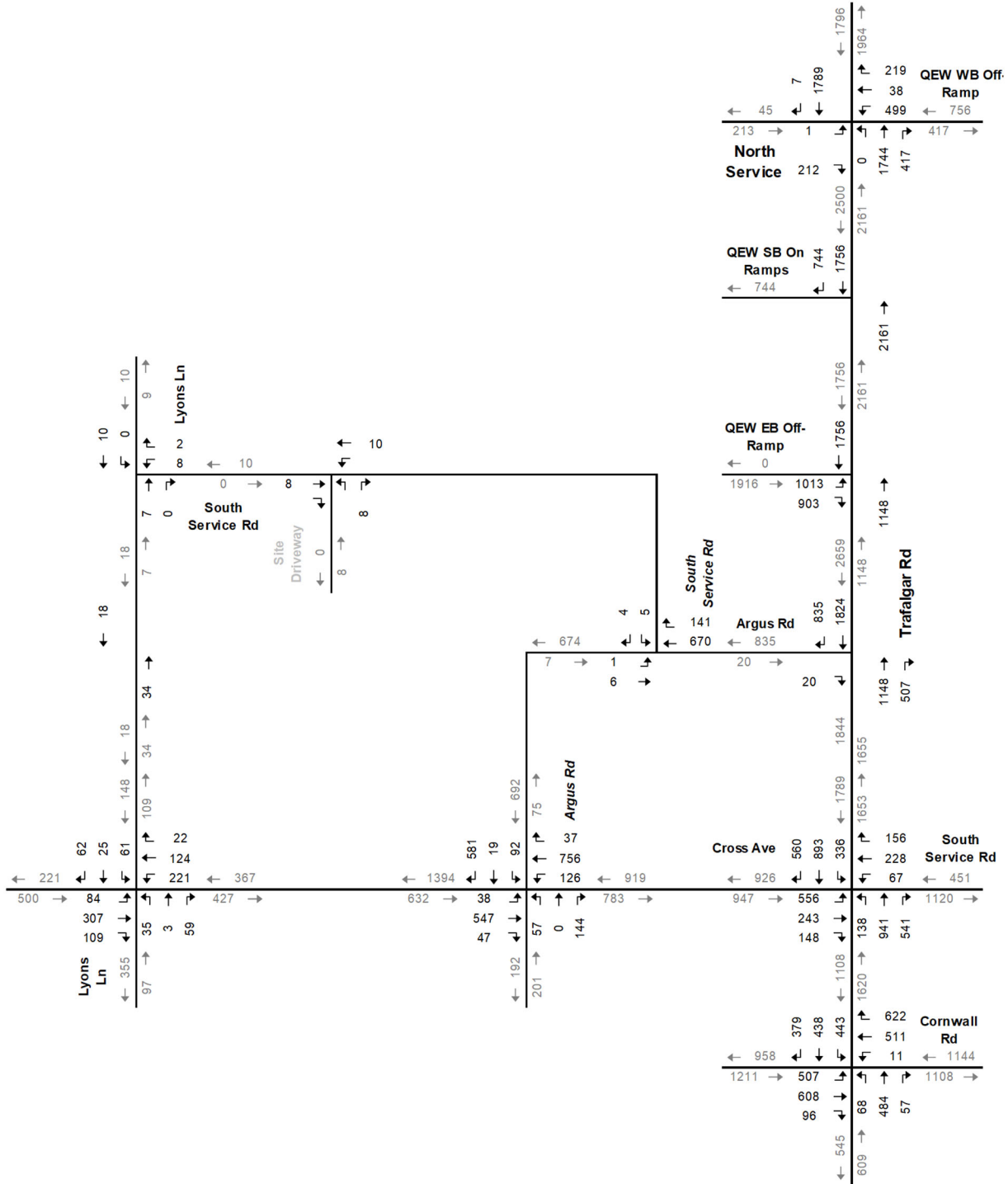




# 2025 Background Traffic Volumes AM Peak Hour

Figure 4.2A



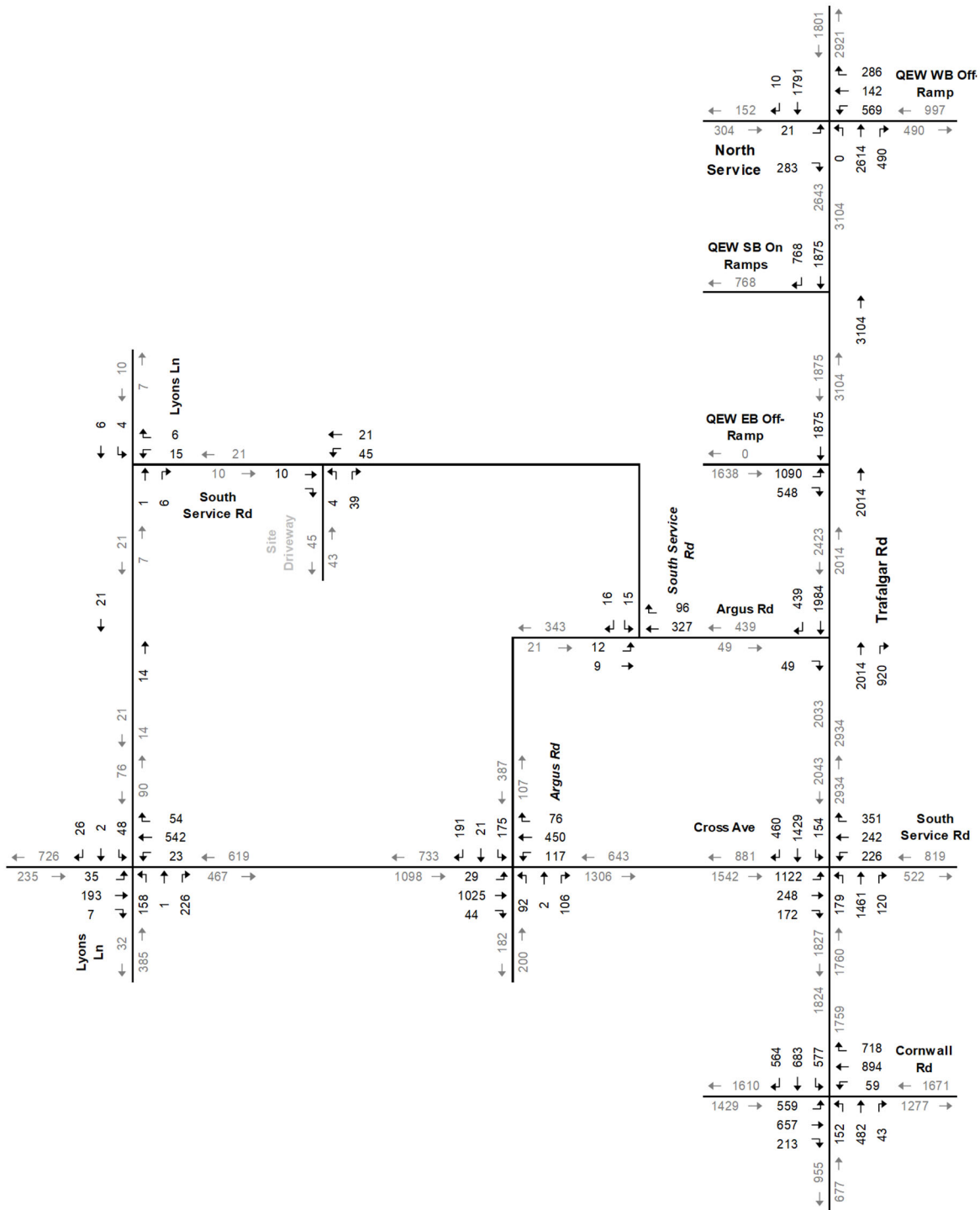


# 2030 Background Traffic Volumes AM Peak Hour

627 Lyons Lane, Oakville  
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Figure 4.3A



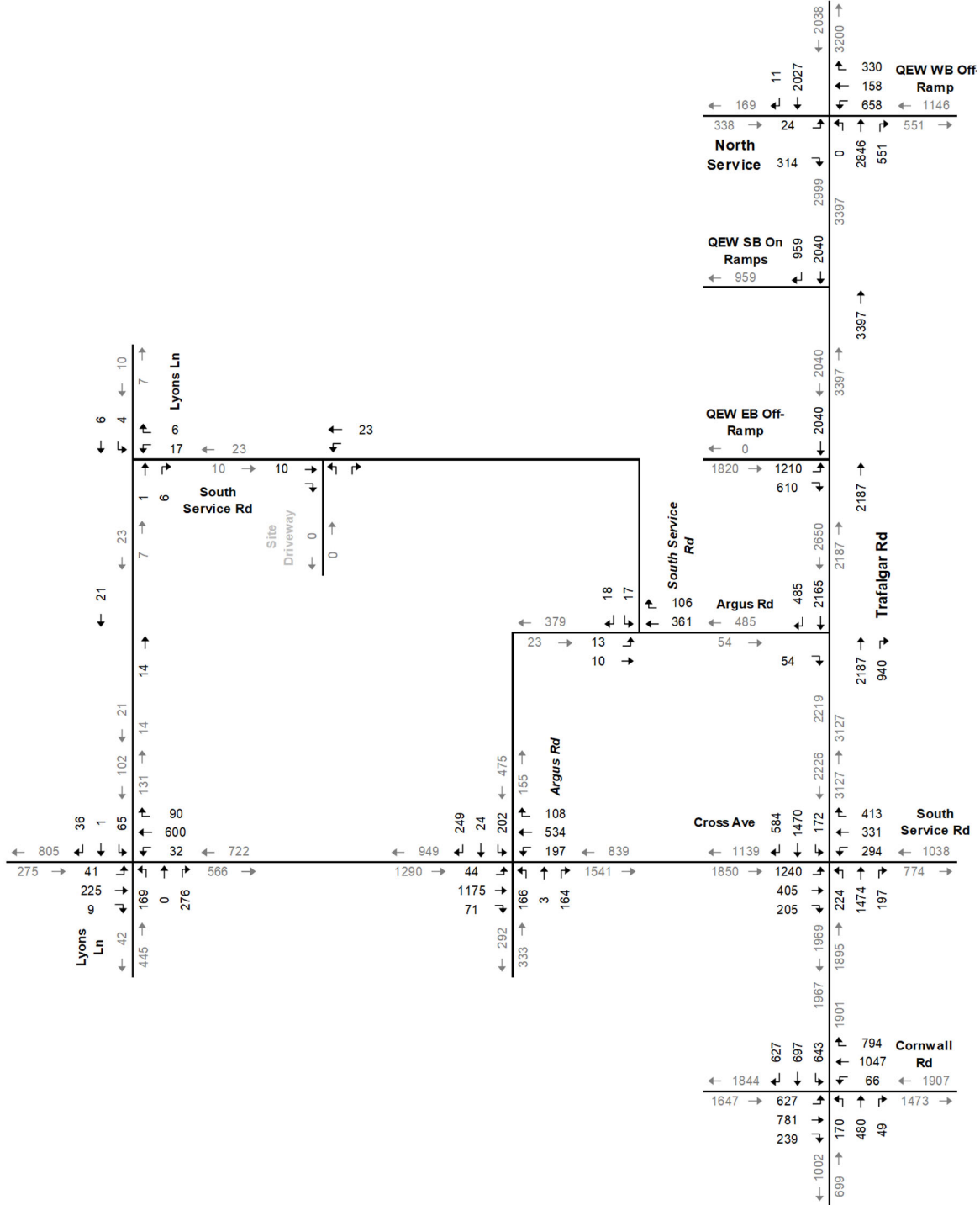


# 2030 Background Traffic Volumes PM Peak Hour

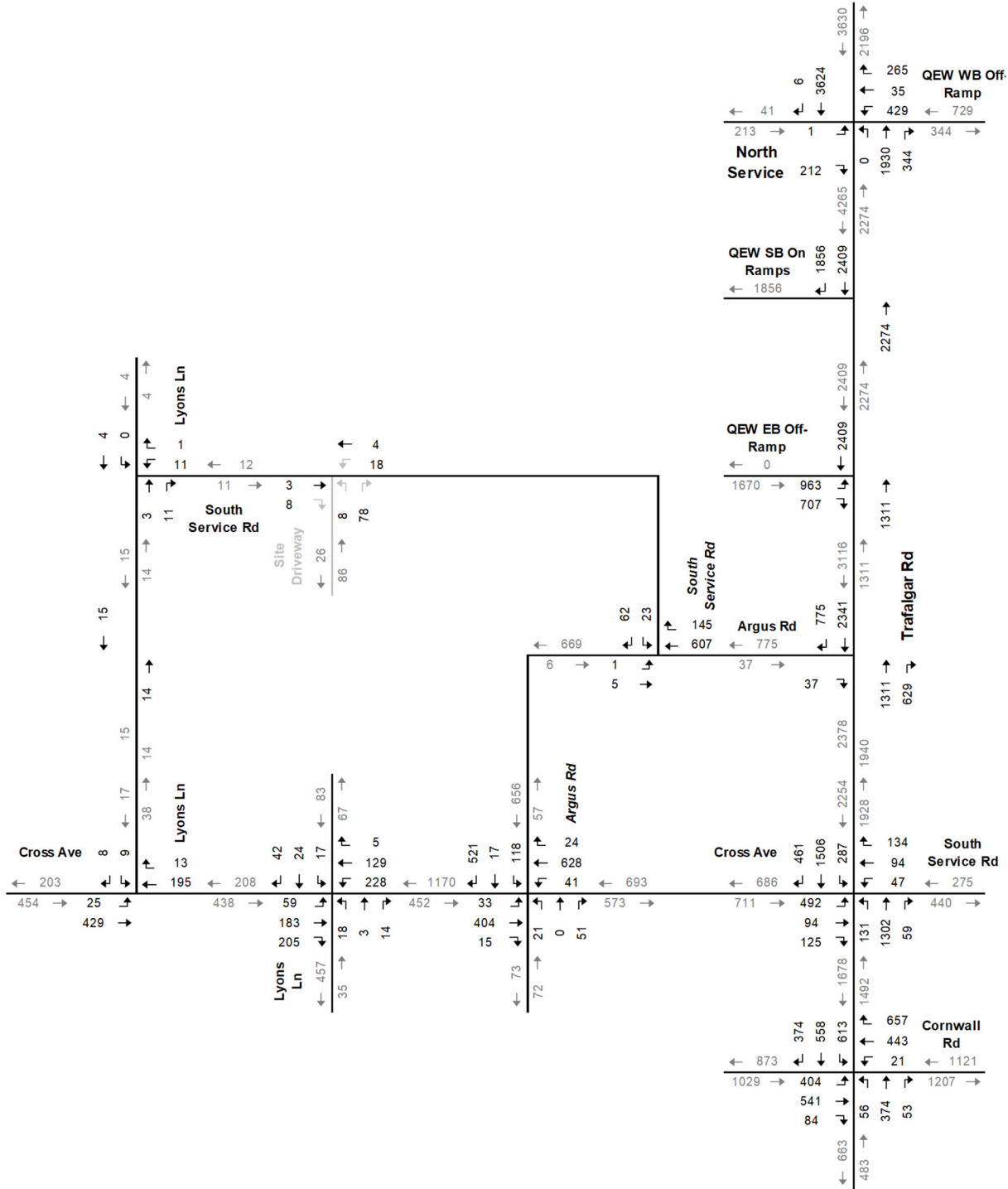
627 Lyons Lane, Oakville  
210060

Figure 4.3B





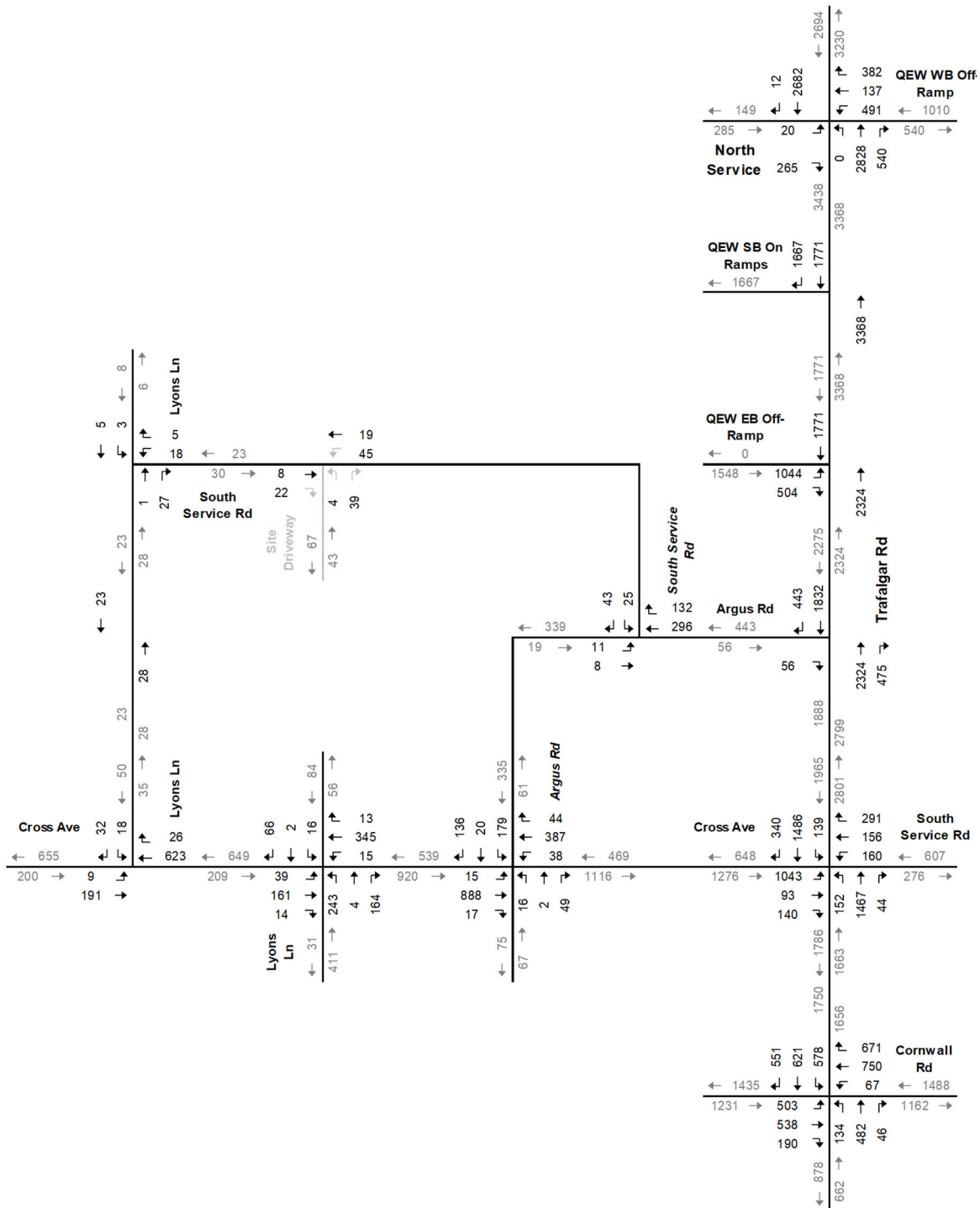
# 2035 Background Traffic Volumes PM Peak Hour



# 2025 Total Traffic Volumes AM Peak Hour

627 Lyons Lane, Oakville  
210060

Figure 4.5A

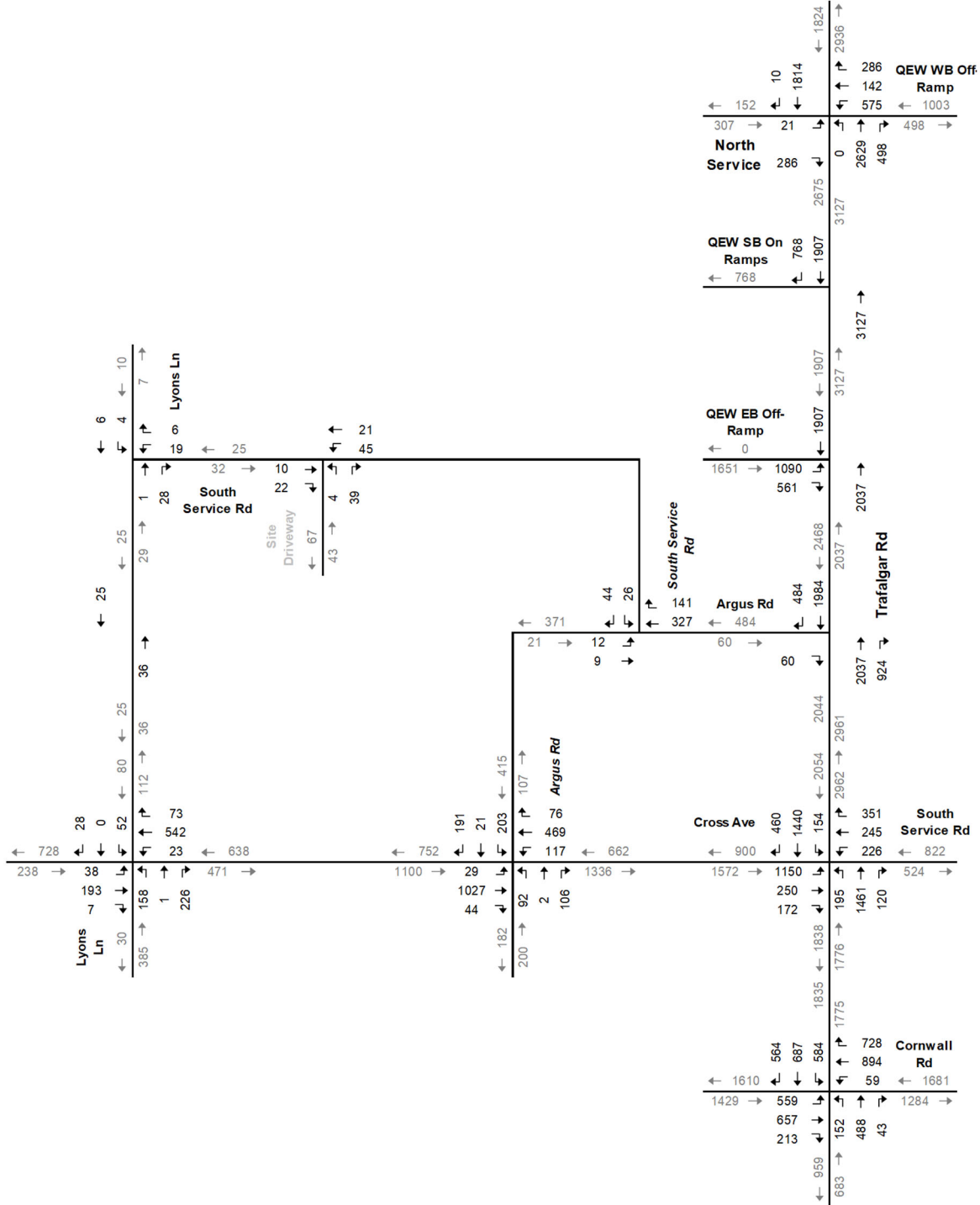


# 2025 Total Traffic Volumes PM Peak Hour

627 Lyons Lane, Oakville  
210060

Figure 4.5B

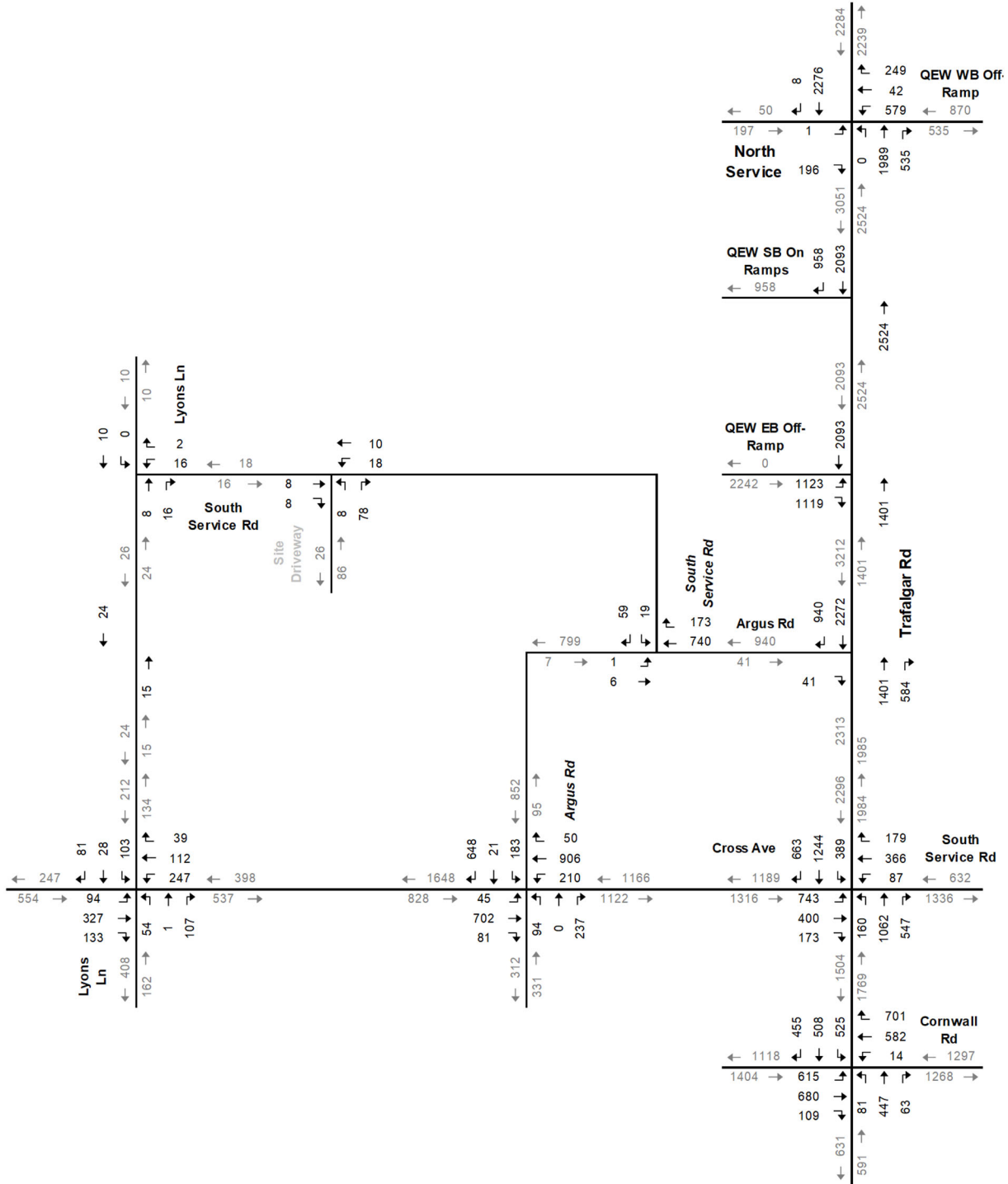




# 2030 Total Traffic Volumes PM Peak Hour

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Figure 4.6B

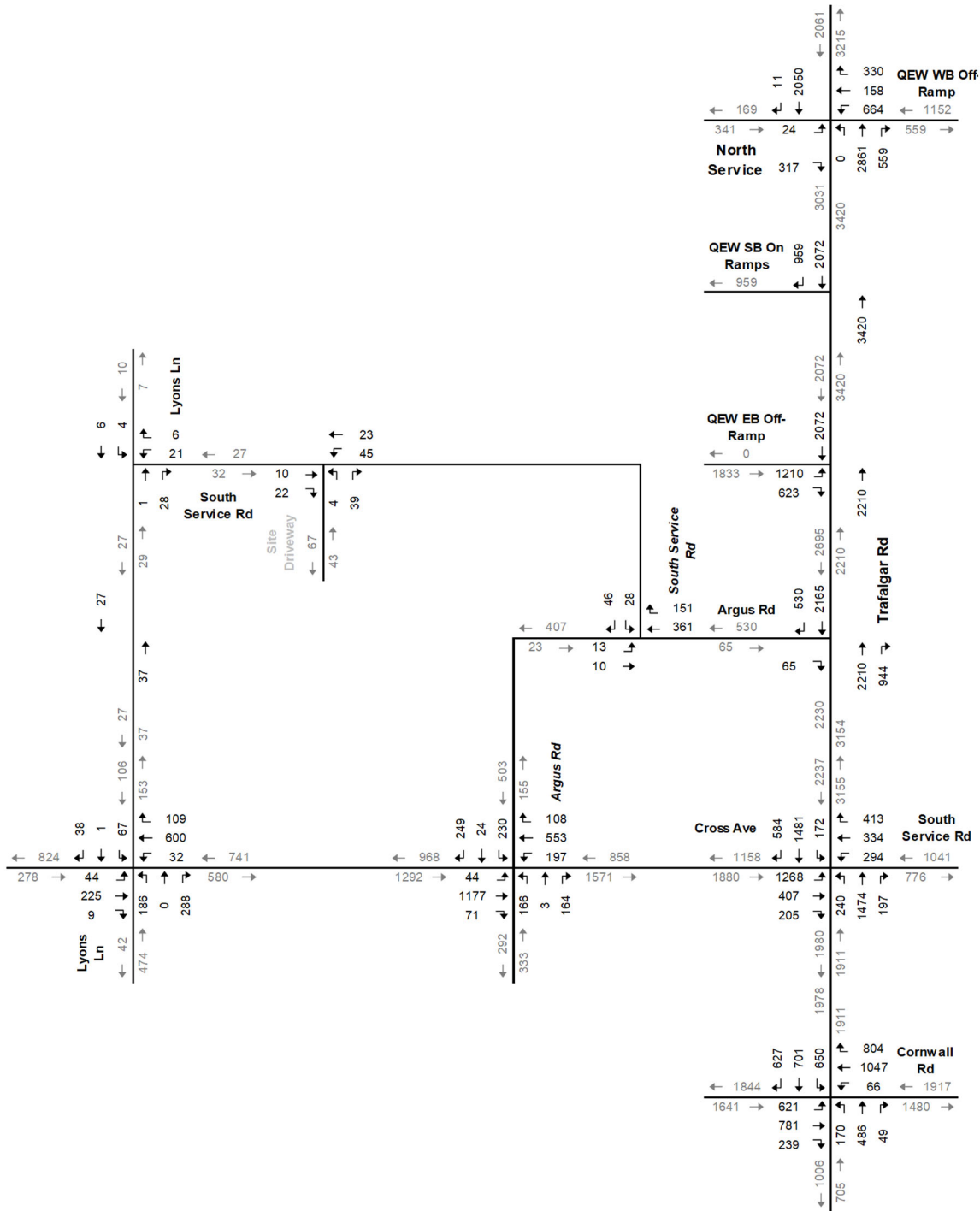


## 2035 Total Traffic Volumes AM Peak Hour

627 Lyons Lane, Oakville  
210060

Figure 4.7A





# 2035 Total Traffic Volumes PM Peak Hour

627 Lyons Lane, Oakville  
210060

Figure 4.7B

## 5 Operational Assessment

Measuring existing traffic volumes and projecting future traffic volumes quantifies traffic within the study area. To assess the quality of flow, roadway capacity analysis was conducted concerning Base conditions and projected future Background and Total conditions. The capacity analysis indicates how the roadway facilities serve the traffic demands. Calculated levels of service classify roadway operating conditions.

### 5.1 Level of Service Criteria

Level of service (LOS) is the term used to denote the different operating conditions that occur on a given roadway segment under various traffic volume loads. It is a qualitative measure that provides an index to the operational qualities of a roadway segment or an intersection with designations that range from A to F, with LOS A representing the best-operating conditions and LOS F representing the worst operating conditions.

For signalized intersections, the analysis considered the operation of each lane or lane group entering the intersection and the level of service for the overall conditions at the intersection.

For unsignalized intersections, the analysis assumes that traffic on the mainline is not affected by the traffic on the side streets. The level of service is only determined for left turns from the main road and all movements from the minor street.

The evaluation criteria used to analyze intersections are based on the 2000 Highway Capacity Manual (HCM)<sup>6</sup>.

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<sup>6</sup> Transportation Research Board, Highway Capacity Manual, Washing, D.C. 2003.



## 5.2 Intersection Capacity Analysis

Intersection capacity analyses were conducted at all intersections in the study area. Analyses were conducted for the Base Conditions and 2025, 2030 and 2035 Background and Total Conditions.

**Tables 5.1 and 5.2** summarize the capacity analyses for the study area intersections, respectively. The capacity analysis results are included in **Appendix F**. The following sub-sections outline the operations of the study area intersections.

### 5.2.1 Trafalgar Road at QEW Westbound Ramp

At the intersection of Trafalgar Road and QEW Westbound Ramp, several movements are operating at LOS E during the weekday peak hours.

Under the 2025 Background conditions, the southbound through a movement which presently operates at LOS E is projected to degrade to LOS F during the weekday AM and PM peak hours.

Under the 2030 Background conditions, the southbound through movement is projected to improve operation to LOS C. However, it will ultimately revert to LOS F under the 2035 Background conditions.

In terms of development traffic implications, similar levels of operation are expected under the Total conditions with only minor increases in delay resulting from site-generated traffic volumes.

### 5.2.2 Trafalgar Road at QEW Southbound On-Ramps

At the unsignalized intersection of Trafalgar Road and QEW Southbound On-Ramps, individual movements presently operate at LOS A during the weekday peak hours. Similar levels of operation are expected under future Background and Total traffic conditions with only a minor increase in delay resulting from site-generated traffic volumes.

### 5.2.3 Trafalgar Road at QEW Eastbound Ramp

At the intersection of Trafalgar Road at QEW Eastbound Ramp, the eastbound proper turn movement presently operates at LOS F during the weekday AM peak hour. The southbound through movement currently operate at LOS B during the weekday AM peak hour and is expected to degrade to LOS E under the 2025 Background conditions and ultimately to LOS F under the 2030 and 2035 Background conditions.

In terms of development traffic implications, similar levels of operation are expected under the Total conditions with only minor increases in delay resulting from site-generated traffic volumes.



#### **5.2.4 Trafalgar Road at Argus Road**

Individual movements at the unsignalized intersection of Trafalgar Road and Argus Road presently operate at LOS C or better during the weekday peak hours. Similar levels of operation are expected under future Background and Total traffic conditions with only a minor increase in delay resulting from site-generated traffic volumes.

#### **5.2.5 Trafalgar Road at Cross Avenue/South Service Road**

At the intersection of Trafalgar Road and Cross Avenue/South Service Road, several movements are operating at LOS E during the weekday AM peak hour and LOS E and F during the weekday PM peak hour.

Under 2025 Background traffic conditions, many movements are projected to continue to operate with high levels of delay during the weekday peak hours.

Under the 2030 and 2035 horizons, increased congestion and delay are projected for several movements degrading from LOS E and F during the weekday peak hours.

In terms of development traffic implications, similar levels of operation are expected under the Total conditions with only minor increases in delay resulting from site-generated traffic volumes.

#### **5.2.6 Trafalgar Road at Cornwall Road**

At the intersection of Trafalgar Road and Cornwall Road, several movements are operating at LOS E and F during the weekday peak hours.

Under 2025, 2030 and 2035 Background traffic conditions, many movements are projected to continue to operate with high levels of delay during the weekday peak hours.

In terms of development traffic implications, similar levels of operation are expected under the Total conditions with only minor increases in delay resulting from site-generated traffic volumes.

#### **5.2.7 Cross Avenue at Argus Road/GO Station Driveway**

Individual movements at the signalized intersection of Cross Avenue and Argus Road/GO Station Driveway presently operate at LOS D or better during the weekday peak hours, except the northbound left-turn movement that operates at LOS F during the weekday AM peak hour. Similar levels of operation are expected under the 2025 Background conditions.

Under 2030 Background conditions, the southbound through/right turn movement is projected to degrade to LOS F and the westbound left-turn movement to degrade to LOS F. With additional growth under the 2035 Background conditions; these movements are expected to operate with increased delay and congestion during the weekday peak hours.



In terms of development traffic implications, similar levels of operation are expected under the Total conditions with only minor increases in delay resulting from site-generated traffic volumes.

#### **5.2.8 Cross Avenue at Lyons Lane (Three Leg)**

Individual movements at the unsignalized intersection of Cross Avenue and Lyons Lane presently operate at LOS B or better during the weekday peak hours. Similar levels of operation are expected under future Background and Total traffic conditions with only a minor increase in delay resulting from site-generated traffic volumes.

#### **5.2.9 Cross Avenue at Lyons Lane (Four Leg)**

Individual movements at the signalized intersection of Cross Avenue and Lyons Lane presently operate at LOS D or better during the weekday peak hours. Similar levels of operation are expected under future Background and Total traffic conditions with only a minor increase in delay resulting from site-generated traffic volumes.

#### **5.2.10 South Service Road at Lyons Lane**

Individual movements at the signalized intersection of Cross Avenue and Lyons Lane presently operate at LOS A during the weekday peak hours. Similar levels of operation are expected under future Background and Total traffic conditions with only a minor increase in delay resulting from site-generated traffic volumes.

#### **5.2.11 Argus Road at South Service Road East**

Individual movements at the unsignalized intersection of Argus Road and South Service Road East presently operate at LOS C or better during the weekday peak hours. Similar levels of operation are expected under 2025, 2030 and 2035 Background conditions.

Under the Total conditions (2025, 2030 and 2035), the increased delay is projected to the southbound approach, which will operate at LOS F. This is likely a result of high east-west traffic along Argus Road, leaving few gaps for southbound stop-controlled movements.

#### **5.2.12 Development Driveway**

Under the future traffic conditions with full-build out of the development, the new driveway connection is expected to operate at LOS A during the weekday peak hours under the Total conditions.



**TABLE 5.1A: AM PEAK HOUR OPERATIONS - 2021-2035**

Analysis Period	Intersection	Horizon	Control Type	MOE	Direction / Movement / Approach																Overall
					Eastbound				Westbound				Northbound				Southbound				
					Left	Through	Right	Approach	Left	Through	Right	Approach	Left	Through	Right	Approach	Left	Through	Right	Approach	
AM Peak Hour	Trafalgar Road & QEW Westbound Off-Ramp / North Service Road	Base Year	TCS	LOS Delay V/C Q	E 68 0.04 2	D 46 0.49 64	D 46	E 57 0.69 92	E 58 0.71 93	D 47 0.30 37	D 54	B 11 0.57 126	A 10 0.25 16	E 59 1.07 511	A 7 0.00 0	F 58 0	D 42				
		2025 Background	TCS	LOS Delay V/C Q	E 68 0.04 2	D 45 0.50 67	D 45	E 57 0.71 97	E 58 0.72 97	D 47 0.35 43	D 54	A 6 0.61 106	A 6 0.27 25	F 91.7 1.15 564	A 7 0.00 0	F 58.2	E 58.2				
		2025 Total	TCS	LOS Delay V/C Q	E 68 0.04 2	D 45 0.50 67	D 45	E 57 0.71 97	E 58 0.72 97	D 47 0.35 43	D 54	A 6 0.62 105	A 6 0.29 26	F 94 1.15 567	A 7 0.00 0	F 59	E 59				
		2030 Background	TCS	LOS Delay V/C Q	E 68 0.04 2	C 27 0.25 39	C 27	C 27 0.25 173	D 42 0.84 22	C 29 0.15 13	D 42	B 13 0.74 155	A 13 0.41 38	C 28 0.74 217	B 28 0.01 0	C 23	C 23				
		2030 Total	TCS	LOS Delay V/C Q	E 68 0.04 2	C 27 0.25 39	C 27	D 49 0.84 174	C 28 0.15 22	C 28 0.09 13	D 42	B 14 0.75 170	A 8 0.44 39	C 28 0.75 219	B 28 0.01 0	C 23	C 23				
		2035 Background	TCS	LOS Delay V/C Q	E 68 0.04 2	B 19 0.24 47	B 19	C 34 0.76 236	C 21 0.16 29	C 20 0.11 15	C 30	D 51 1.02 255	C 46 0.60 62	F 119 1.16 326	C 24 0.01 0	E 71	E 71				
		2035 Total	TCS	LOS Delay V/C Q	E 68 0.04 2	B 19 0.24 47	B 19	C 34 0.76 237	C 21 0.16 29	C 20 0.11 15	C 30	E 60 1.05 263	C 53 0.63 67	F 124 1.17 329	C 24 0.01 0	E 76	E 76				
AM Peak Hour	Trafalgar Road & QEW Southbound On-Ramps	Base Year	FF	LOS Delay V/C Q							A 0 0.44 0	A 0 1.12 0	A 0 0 0	A 0 1.19 0	A 0						
		2025 Background	FF	LOS Delay V/C Q								A 0 0.47 0	A 0 1.19 0	A 0 0 0	A 0 1.19 0	A 0					
		2025 Total	FF	LOS Delay V/C Q								A 0 0.48 0	A 0 1.19 0	A 0 0 0	A 0 1.19 0	A 0					
		2030 Background	FF	LOS Delay V/C Q								A 0 0.46 0	A 0 0.48 0	A 0 0.48 0	A 0 0.48 0	A 0					
		2030 Total	FF	LOS Delay V/C Q								A 0 0.47 0	A 0 0.48 0	A 0 0.48 0	A 0 0.48 0	A 0					
		2035 Background	FF	LOS Delay V/C Q								A 0 0.53 0	A 0 0.61 0	A 0 0.61 0	A 0 0.61 0	A 0					
		2035 Total	FF	LOS Delay V/C Q								A 0 0.54 0	A 0 0.61 0	A 0 0.61 0	A 0 0.61 0	A 0					
AM Peak Hour	Trafalgar Road & QEW Eastbound Off-Ramp	Base Year	TCS	LOS Delay V/C Q	D 40 0.76 155	F 120 1.13 311	E 74				D 42 0.89 275	D 42 0.46 51	B 15 1.04 196	B 15 0.00 0	D 46						
		2025 Background	TCS	LOS Delay V/C Q	C 33 0.70 150	F 95 1.08 327	E 59				D 50 0.55 168	D 50 1.04 196	D 52.5 1.20 265	D 53	D 53						
		2025 Total	TCS	LOS Delay V/C Q	C 33 0.70 150	F 98 1.08 331	E 59				D 46 0.58 172	D 46 1.05 197	E 55 1.20 265	E 55	D 54						
		2030 Background	TCS	LOS Delay V/C Q	D 45 0.85 180	F 175 1.60 493	F 175				C 33 0.45 151	C 33 0.67 69	B 16 0.67 69	B 16 0.00 0	F 84						
		2030 Total	TCS	LOS Delay V/C Q	D 45 0.85 180	F 178 1.61 496	F 178				D 35 0.48 158	D 35 0.67 70	B 16 0.67 70	B 16 0.00 0	F 85						
		2035 Background	TCS	LOS Delay V/C Q	C 21 0.64 143	F 106 1.31 544	F 106				D 52 0.79 180	D 52 1.20 265	F 136.8 1.20 265	F 136.8	F 104.5						
		2035 Total	TCS	LOS Delay V/C Q	C 21 0.63 140	F 104 1.33 544	F 104				D 51 0.84 183	D 51 1.23 268	F 152 1.23 268	F 152	F 108						

MOE - Measure of Effectiveness  
 LOS - Level of Service  
 Delay - Average Delay per Vehicle in Seconds  
 Q - 95th Percentile Queue Length (m)  
 TCS - Traffic Control Signal  
 TWSC - Two-Way Stop Control  
 < - Shared Left-Turn  
 > - Shared Right-Turn



**TABLE 5.1B: AM PEAK HOUR OPERATIONS - 2021-2035 (CONT.)**

Analysis Period	Intersection	Horizon	Control Type	MOE	Direction / Movement / Approach																			
					Eastbound				Westbound				Northbound				Southbound				Overall			
					Left	Through	Right	Approach	Left	Through	Right	Approach	Left	Through	Right	Approach	Left	Through	Right	Approach				
AM Peak Hour	Trafalgar Road & Argus Road	Base Year	TWSC	LOS Delay V/C Q		C 16								A 0				A 0						
		2025 Background	TWSC	LOS Delay V/C Q		B 0									A 0				A 0					
		2025 Total	TWSC	LOS Delay V/C Q		B 12									A 0				A 0					
		2030 Background	TWSC	LOS Delay V/C Q		A 10									A 0				A 0					
		2030 Total	TWSC	LOS Delay V/C Q		A 10									A 0				A 0					
		2035 Background	TWSC	LOS Delay V/C Q		B 11									A 0				A 0					
		2035 Total	TWSC	LOS Delay V/C Q		B 11									A 0				A 0					
AM Peak Hour	Trafalgar Road & Cross Avenue / South Service Road	Base Year	TCS	LOS Delay V/C Q	E 54	D 47	D 47	D 47	D 54	D 45	E 58	E 55	D 54	D 37	D 41	D 41	D 40	D 52	C 34	C 34	D 36	D 41		
		2025 Background	TCS	LOS Delay V/C Q	E 63	D 51	D 51	D 51	E 63	D 45	E 58	E 55	D 54	D 36	D 42	D 42	D 41	D 41	D 54	D 20	D 20	C 24	D 37	
		2025 Total	TCS	LOS Delay V/C Q	E 77	D 51	D 51	D 51	E 77	D 45	E 59	E 59	D 54	D 38	D 42	D 42	D 42	D 42	D 54	D 20	D 20	C 25	D 40	
		2030 Background	TCS	LOS Delay V/C Q	E 55	D 47	D 44	D 44	E 55	D 59	E 36	C 32	D 50	D 32	D 43	D 40	D 41	D 41	D 41	D 41	D 21	D 12	C 22	D 38
		2030 Total	TCS	LOS Delay V/C Q	E 54	D 45	D 42	D 42	E 54	D 53	E 37	D 37	D 51	D 34	D 45	D 43	D 43	D 43	D 44	D 23	D 15	C 24	D 39	
		2035 Background	TCS	LOS Delay V/C Q	E 57	D 44	D 39	D 39	E 57	D 54	E 33	C 33	E 55	D 38	D 53	D 60	D 54	D 54	D 50	D 24	D 12	C 25	D 43	
		2035 Total	TCS	LOS Delay V/C Q	E 63	D 45	D 41	D 41	E 63	D 52	E 34	C 34	E 56	D 40	D 54	D 56	D 53	D 53	D 51	D 25	D 13	C 26	D 45	
AM Peak Hour	Trafalgar Road & Cornwall Road	Base Year	TCS	LOS Delay V/C Q	F 54	C 35	C 30	C 30	D 54	E 68	D 46	E 71	E 61	E 79	D 41	D 41	D 46	F 82	C 52	D 56	E 56	E 56		
		2025 Background	TCS	LOS Delay V/C Q	F 52	C 29	C 25	C 25	D 52	E 69	D 40	F 83	E 66	F 94	D 48	D 48	D 53	F 100	D 39	D 48	E 65	E 61		
		2025 Total	TCS	LOS Delay V/C Q	F 52	C 29	C 25	C 25	D 52	E 69	D 40	F 85	E 67	F 94	D 48	D 48	D 53	F 105	D 40	D 48	E 68	E 62		
		2030 Background	TCS	LOS Delay V/C Q	F 51	C 23	C 23	C 23	D 51	E 70	D 36	F 80	E 60	F 89	D 53	D 45	D 56	F 112	D 32	D 11	E 54	E 55		
		2030 Total	TCS	LOS Delay V/C Q	F 51	C 23	C 23	C 23	D 51	E 70	D 36	F 82	E 61	F 89	D 53	D 45	D 56	F 122	D 31	D 11	E 57	E 57		
		2035 Background	TCS	LOS Delay V/C Q	F 74	C 24	C 24	C 24	E 74	E 71	D 38	F 146	F 97	F 116	D 50	D 44	D 59	F 186	D 37	D 28	F 86	F 82		
		2035 Total	TCS	LOS Delay V/C Q	F 74	C 24	C 24	C 24	E 74	E 71	D 38	F 149	F 98	F 116	D 50	D 44	D 59	F 197	D 39	D 29	F 92	F 84		

MOE - Measure of Effectiveness  
 LOS - Level of Service  
 Delay - Average Delay per Vehicle in Seconds  
 Q - 95th Percentile Queue Length (m)  
 TCS - Traffic Control Signal  
 TWSC - Two-Way Stop Control  
 < - Shared Left-Turn  
 > - Shared Right-Turn



**TABLE 5.1C: AM PEAK HOUR OPERATIONS – 2021-2035 (CONT.)**

Analysis Period	Intersection	Horizon	Control Type	MOE	Direction / Movement / Approach																Overall		
					Eastbound				Westbound				Northbound				Southbound						
					Left	Through	Right	Approach	Left	Through	Right	Approach	Left	Through	Right	Approach	Left	Through	Right	Approach			
AM Peak Hour	Cross Avenue & Argus Road / GO Station Driveway	Base Year	TCS	LOS Delay	B 18	B 18	B 18	B 18	A 10	A 10	A 10	A 10	F 38	B 38	B 38	B 38	B 38	B 38	B 38	B 38	B 38	B 19	
		V/C	0.27	0.42	0.42	0.22	0.41	0.41	0.22	0.41	0.41	0.79	0.10	0.10	0.10	0.10	0.10	0.10	0.10	0.10	0.10	0.10	0.10
		Q	8	35	35	7	38	38	7	38	38	10	0	0	0	0	0	0	0	0	0	0	0
		2025 Background	TCS	LOS Delay	C 20	C 20	C 20	C 20	B 12	B 12	B 12	B 12	F 47	B 47	B 47	B 47	B 47	B 47	B 47	B 47	B 47	B 47	C 23
		V/C	0.30	0.46	0.46	0.25	0.45	0.45	0.25	0.45	0.45	0.87	0.10	0.10	0.10	0.10	0.10	0.10	0.10	0.10	0.10	0.10	0.10
		Q	9	40	40	8	45	45	8	45	45	12	0	0	0	0	0	0	0	0	0	0	0
		2025 Total	TCS	LOS Delay	C 20	C 20	C 20	C 20	B 12	B 12	B 12	B 12	F 47	B 47	B 47	B 47	B 47	B 47	B 47	B 47	B 47	B 47	C 23
V/C	0.31	0.47	0.47	0.25	0.46	0.46	0.25	0.46	0.46	0.87	0.10	0.10	0.10	0.10	0.10	0.10	0.10	0.10	0.10	0.10	0.10		
Q	9	40	40	8	45	45	8	45	45	12	0	0	0	0	0	0	0	0	0	0	0		
2030 Background	TCS	LOS Delay	C 22	C 22	C 22	C 22	E 23	E 23	E 23	F 56	A 56	B 56	E 56	C 56	F 56	F 56	F 56	F 56	F 56	F 56	F 56	E 71	
V/C	0.38	0.56	0.10	0.96	0.54	0.03	0.96	0.54	0.03	1.06	0.00	0.28	1.06	0.00	0.28	0.28	0.28	0.28	0.28	0.28	0.28		
Q	10	55	0	41	61	0	41	61	0	17	0	0	17	0	0	28	111	111	111	111	111		
2030 Total	TCS	LOS Delay	C 22	C 22	C 22	C 22	E 23	E 23	E 23	F 56	A 56	B 56	E 56	C 56	F 56	F 56	F 56	F 56	F 56	F 56	F 56	E 70	
V/C	0.39	0.56	0.10	0.96	0.55	0.03	0.96	0.55	0.03	1.06	0.00	0.28	1.06	0.00	0.28	0.28	0.53	1.39	1.39	1.39	1.39		
Q	10	55	0	41	62	0	41	62	0	18	0	0	18	0	0	44	111	111	111	111	111		
2035 Background	TCS	LOS Delay	C 23	C 23	C 23	C 23	F 89	F 89	F 89	F 175	A 175	D 175	F 175	C 175	F 175	F 175	F 175	F 175	F 175	F 175	F 175	F 139	
V/C	0.52	0.65	0.18	1.18	0.61	0.05	1.18	0.61	0.05	1.82	0.00	0.79	1.82	0.00	0.79	0.46	1.76	1.76	1.76	1.76	1.76		
Q	13	73	2	106	76	2	106	76	2	39	0	36	39	0	36	37	152	152	152	152	152		
2035 Total	TCS	LOS Delay	C 23	C 23	C 23	C 23	F 90	F 90	F 90	F 178	A 178	D 178	F 178	C 178	F 178	F 178	F 178	F 178	F 178	F 178	F 178	F 138	
V/C	0.53	0.65	0.18	1.82	0.62	0.05	1.82	0.62	0.05	1.84	0.00	0.79	1.84	0.00	0.79	0.68	1.76	1.76	1.76	1.76	1.76		
Q	13	74	2	106	77	2	106	77	2	39	0	36	39	0	36	55	153	153	153	153	153		
AM Peak Hour	Cross Avenue & Lyons Lane / Commercial Driveway	Base Year	TCS	LOS Delay	A 9	A 9	A 9	A 9	A 5	A 5	A 5	A 5	C 24	C 24	C 24	C 24	C 24	C 24	C 24	C 24	C 24	A 9	
		V/C	0.13	0.24	0.24	0.56	0.08	0.08	0.56	0.08	0.08	0.10	0.03	0.03	0.10	0.03	0.03	0.10	0.03	0.03	0.10	0.03	
		Q	10	12	12	15	4	4	15	4	4	7	5	5	7	5	5	6	14	14	14	14	
		2025 Background	TCS	LOS Delay	A 10	A 10	A 10	A 10	A 5	A 5	A 5	A 5	C 25	C 25	C 25	C 25	C 25	C 25	C 25	C 25	C 25	C 25	A 10
		V/C	0.14	0.25	0.25	0.59	0.08	0.08	0.59	0.08	0.08	0.12	0.03	0.03	0.12	0.03	0.03	0.16	0.14	0.14	0.14	0.14	
		Q	12	15	15	16	5	5	16	5	5	8	5	5	8	5	5	7	15	15	15	15	
		2025 Total	TCS	LOS Delay	A 10	A 10	A 10	A 10	A 5	A 5	A 5	A 5	C 25	C 25	C 25	C 25	C 25	C 25	C 25	C 25	C 25	C 25	A 10
V/C	0.14	0.26	0.26	0.59	0.09	0.09	0.59	0.09	0.09	0.12	0.03	0.03	0.12	0.03	0.03	0.14	0.14	0.14	0.14	0.14			
Q	12	15	15	16	5	5	16	5	5	8	5	5	8	5	5	7	15	15	15	15			
2030 Background	TCS	LOS Delay	C 24	C 24	C 24	C 24	B 12	B 12	B 12	B 14	B 14	B 14	B 14	B 14	B 14	B 14	B 14	B 14	B 14	B 14	B 18		
V/C	0.43	0.67	0.08	0.59	0.10	0.03	0.59	0.10	0.03	0.08	0.00	0.04	0.08	0.00	0.04	0.20	0.04	0.11	0.11	0.11			
Q	21	30	11	31	9	1	31	9	1	11	2	7	11	2	7	15	9	0	0	0			
2030 Total	TCS	LOS Delay	C 24	C 24	C 24	C 24	B 11	B 11	B 11	B 13	B 13	B 13	B 13	B 13	B 13	B 13	B 13	B 13	B 13	B 13	B 18		
V/C	0.43	0.67	0.08	0.56	0.10	0.04	0.56	0.10	0.04	0.08	0.00	0.05	0.08	0.00	0.05	0.21	0.04	0.11	0.11	0.11			
Q	21	30	11	29	9	1	29	9	1	11	1	7	11	1	7	16	8	0	0	0			
2035 Background	TCS	LOS Delay	C 25	C 25	C 25	C 25	B 13	B 13	B 13	B 15	B 15	B 15	B 15	B 15	B 15	B 15	B 15	B 15	B 15	B 15	C 20		
V/C	0.45	0.69	0.10	0.65	0.09	0.05	0.65	0.09	0.05	0.13	0.00	0.08	0.13	0.00	0.08	0.33	0.05	0.16	0.16	0.16			
Q	25	34	13	36	8	1	36	8	1	17	1	11	17	1	11	25	10	0	0	0			
2035 Total	TCS	LOS Delay	C 25	C 25	C 25	C 25	B 13	B 13	B 13	B 15	B 15	B 15	B 15	B 15	B 15	B 15	B 15	B 15	B 15	B 15	C 20		
V/C	0.45	0.69	0.10	0.65	0.09	0.06	0.65	0.09	0.06	0.13	0.00	0.08	0.13	0.00	0.08	0.34	0.05	0.14	0.14	0.14			
Q	25	34	13	36	8	1	36	8	1	17	0	0	17	0	0	26	10	0	0	0			
AM Peak Hour	Cross Avenue & Lyons Lane	Base Year	TCS	LOS Delay	A 0	A 0	A 0	A 0	A 0	A 0	A 0	A 0				B 12	B 12	B 12	B 12	B 12	B 12	A 1	
		V/C	0.02	0.20		0.08	0.08		0.08	0.08							0.03	0.03	0.03	0.03	0.03	0.03	
		Q	1	0		0	0		0	0							1	1	1	1	1	1	
2025 Background	TCS	LOS Delay	A 0	A 0	A 0	A 0	A 0	A 0	A 0	A 0	A 0				B 12	B 12	B 12	B 12	B 12	B 12	A 1		
V/C	0.03	0.22		0.09	0.05		0.09	0.05								0.03	0.03	0.03	0.03	0.03	0.03		
Q	0	0		0	0		0	0								1	1	1	1	1	1		
2025 Total	TCS	LOS Delay	A 0	A 0	A 0	A 0	A 0	A 0	A 0	A 0	A 0				B 12	B 12	B 12	B 12	B 12	B 12	A 1		
V/C	0.03	0.22		0.09	0.06		0.09	0.06								0.06	0.06	0.06	0.06	0.06	0.06		
Q	1	0		0	0		0	0								2	2	2	2	2	2		

MOE - Measure of Effectiveness  
 LOS - Level of Service  
 Delay - Average Delay per Vehicle in Seconds  
 Q - 95th Percentile Queue Length (m)  
 TCS - Traffic Control Signal  
 TWSC - Two-Way Stop Control  
 < - Shared Left-Turn  
 > - Shared Right-Turn





**TABLE 5.1D: AM PEAK HOUR OPERATIONS – 2021-2035 (CONT.)**

Analysis Period	Intersection	Horizon	Control Type	MOE	Direction / Movement / Approach																Overall		
					Eastbound				Westbound				Northbound				Southbound						
					Left	Through	Right	Approach	Left	Through	Right	Approach	Left	Through	Right	Approach	Left	Through	Right	Approach			
AM Peak Hour	Lyons Lane & South Service Road	Base Year	TWSC	LOS Delay V/C Q	A 0 0.00 0	A 0 0.00 0	A 0 0.00 0	A 0 0.00 0	A 0 0.00 0	A 0 0.00 0	A 0 0.00 0	A 0 0.00 0	A 0 0.00 0	A 0 0.00 0	A 0 0.00 0	A 0 0.00 0	A 0 0.00 0	A 9 0.01 0	A 9 0.01 0	A 9 0.01 0	A 9 0.01 0	A 9 0.01 0	
		2025 Background	TWSC	LOS Delay V/C Q	A 0 0.00 0	A 0 0.00 0	A 0 0.00 0	A 0 0.00 0	A 0 0.00 0	A 0 0.00 0	A 0 0.00 0	A 0 0.00 0	A 0 0.00 0	A 0 0.00 0	A 0 0.00 0	A 0 0.00 0	A 0 0.00 0	A 0 0.00 0	A 9 0.01 0	A 9 0.01 0	A 9 0.01 0	A 9 0.01 0	A 9 0.01 0
		2025 Total	TWSC	LOS Delay V/C Q	A 0 0.00 0	A 0 0.00 0	A 0 0.00 0	A 0 0.00 0	A 0 0.00 0	A 0 0.00 0	A 0 0.00 0	A 0 0.00 0	A 0 0.00 0	A 0 0.00 0	A 0 0.00 0	A 0 0.00 0	A 0 0.00 0	A 0 0.00 0	A 9 0.04 1	A 9 0.04 1	A 9 0.04 1	A 9 0.04 1	A 9 0.04 1
		2030 Background	TWSC	LOS Delay V/C Q	A 0 0.00 0	A 0 0.00 0	A 0 0.00 0	A 0 0.00 0	A 0 0.00 0	A 0 0.00 0	A 0 0.00 0	A 0 0.00 0	A 0 0.00 0	A 0 0.00 0	A 0 0.00 0	A 0 0.00 0	A 0 0.00 0	A 0 0.00 0	A 9 0.03 1	A 9 0.03 1	A 9 0.03 1	A 9 0.03 1	A 9 0.03 1
		2030 Total	TWSC	LOS Delay V/C Q	A 0 0.00 0	A 0 0.00 0	A 0 0.00 0	A 0 0.00 0	A 0 0.00 0	A 0 0.00 0	A 0 0.00 0	A 0 0.00 0	A 0 0.00 0	A 0 0.00 0	A 0 0.00 0	A 0 0.00 0	A 0 0.00 0	A 0 0.00 0	A 9 0.06 1	A 9 0.06 1	A 9 0.06 1	A 9 0.06 1	A 9 0.06 1
		2035 Background	TWSC	LOS Delay V/C Q	A 0 0.00 0	A 0 0.00 0	A 0 0.00 0	A 0 0.00 0	A 0 0.00 0	A 0 0.00 0	A 0 0.00 0	A 0 0.00 0	A 0 0.00 0	A 0 0.00 0	A 0 0.00 0	A 0 0.00 0	A 0 0.00 0	A 0 0.00 0	A 9 0.03 1	A 9 0.03 1	A 9 0.03 1	A 9 0.03 1	A 9 0.03 1
		2035 Total	TWSC	LOS Delay V/C Q	A 0 0.00 0	A 0 0.00 0	A 0 0.00 0	A 0 0.00 0	A 0 0.00 0	A 0 0.00 0	A 0 0.00 0	A 0 0.00 0	A 0 0.00 0	A 0 0.00 0	A 0 0.00 0	A 0 0.00 0	A 0 0.00 0	A 0 0.00 0	A 9 0.06 1	A 9 0.06 1	A 9 0.06 1	A 9 0.06 1	A 9 0.06 1
AM Peak Hour	Argus Road & South Service Road	Base Year	TWSC	LOS Delay V/C Q	A 3 0.01 0	A 3 0.01 0	A 3 0.01 0	A 3 0.01 0	A 3 0.01 0	A 3 0.01 0	A 3 0.01 0	A 3 0.01 0	A 3 0.01 0	A 3 0.01 0	A 3 0.01 0	A 3 0.01 0	A 3 0.01 0	A 15 0.07 2	A 15 0.07 2	A 15 0.07 2	A 15 0.07 2	A 15 0.07 2	
		2025 Background	TWSC	LOS Delay V/C Q	A 3 0.01 0	A 3 0.01 0	A 3 0.01 0	A 3 0.01 0	A 3 0.01 0	A 3 0.01 0	A 3 0.01 0	A 3 0.01 0	A 3 0.01 0	A 3 0.01 0	A 3 0.01 0	A 3 0.01 0	A 3 0.01 0	A 3 0.01 0	A 16 0.08 2	A 16 0.08 2	A 16 0.08 2	A 16 0.08 2	A 16 0.08 2
		2025 Total	TWSC	LOS Delay V/C Q	A 3 0.01 0	A 3 0.01 0	A 3 0.01 0	A 3 0.01 0	A 3 0.01 0	A 3 0.01 0	A 3 0.01 0	A 3 0.01 0	A 3 0.01 0	A 3 0.01 0	A 3 0.01 0	A 3 0.01 0	A 3 0.01 0	A 3 0.01 0	A 63 0.92 77	A 63 0.92 77	A 63 0.92 77	A 63 0.92 77	A 63 0.92 77
		2030 Background	TWSC	LOS Delay V/C Q	A 3 0.01 0	A 3 0.01 0	A 3 0.01 0	A 3 0.01 0	A 3 0.01 0	A 3 0.01 0	A 3 0.01 0	A 3 0.01 0	A 3 0.01 0	A 3 0.01 0	A 3 0.01 0	A 3 0.01 0	A 3 0.01 0	A 3 0.01 0	A 17 0.10 3	A 17 0.10 3	A 17 0.10 3	A 17 0.10 3	A 17 0.10 3
		2030 Total	TWSC	LOS Delay V/C Q	A 3 0.01 0	A 3 0.01 0	A 3 0.01 0	A 3 0.01 0	A 3 0.01 0	A 3 0.01 0	A 3 0.01 0	A 3 0.01 0	A 3 0.01 0	A 3 0.01 0	A 3 0.01 0	A 3 0.01 0	A 3 0.01 0	A 3 0.01 0	A 72 0.94 77	A 72 0.94 77	A 72 0.94 77	A 72 0.94 77	A 72 0.94 77
		2035 Background	TWSC	LOS Delay V/C Q	A 3 0.01 0	A 3 0.01 0	A 3 0.01 0	A 3 0.01 0	A 3 0.01 0	A 3 0.01 0	A 3 0.01 0	A 3 0.01 0	A 3 0.01 0	A 3 0.01 0	A 3 0.01 0	A 3 0.01 0	A 3 0.01 0	A 3 0.01 0	A 19 0.13 3	A 19 0.13 3	A 19 0.13 3	A 19 0.13 3	A 19 0.13 3
		2035 Total	TWSC	LOS Delay V/C Q	A 3 0.01 0	A 3 0.01 0	A 3 0.01 0	A 3 0.01 0	A 3 0.01 0	A 3 0.01 0	A 3 0.01 0	A 3 0.01 0	A 3 0.01 0	A 3 0.01 0	A 3 0.01 0	A 3 0.01 0	A 3 0.01 0	A 3 0.01 0	A 109 1.06 96	A 109 1.06 96	A 109 1.06 96	A 109 1.06 96	A 109 1.06 96
AM Peak Hour	South Service Road & Site Driveway	2025 Total	TWSC	LOS Delay V/C Q	A 0 0.01 0	A 0 0.01 0	A 0 0.01 0	A 0 0.01 0	A 0 0.01 0	A 0 0.01 0	A 0 0.01 0	A 0 0.01 0	A 0 0.01 0	A 0 0.01 0	A 0 0.01 0	A 0 0.01 0	A 0 0.01 0	A 9 0.09 2	A 9 0.09 2	A 9 0.09 2	A 9 0.09 2	A 9 0.09 2	
		2030 Total	TWSC	LOS Delay V/C Q	A 0 0.01 0	A 0 0.01 0	A 0 0.01 0	A 0 0.01 0	A 0 0.01 0	A 0 0.01 0	A 0 0.01 0	A 0 0.01 0	A 0 0.01 0	A 0 0.01 0	A 0 0.01 0	A 0 0.01 0	A 0 0.01 0	A 0 0.01 0	A 9 0.09 2	A 9 0.09 2	A 9 0.09 2	A 9 0.09 2	A 9 0.09 2
		2035 Total	TWSC	LOS Delay V/C Q	A 0 0.01 0	A 0 0.01 0	A 0 0.01 0	A 0 0.01 0	A 0 0.01 0	A 0 0.01 0	A 0 0.01 0	A 0 0.01 0	A 0 0.01 0	A 0 0.01 0	A 0 0.01 0	A 0 0.01 0	A 0 0.01 0	A 0 0.01 0	A 9 0.09 2	A 9 0.09 2	A 9 0.09 2	A 9 0.09 2	A 9 0.09 2

MOE - Measure of Effectiveness  
 LOS - Level of Service  
 Delay - Average Delay per Vehicle in Seconds

Q - 95th Percentile Queue Length (m)  
 TCS - Traffic Control Signal  
 TWSC - Two-Way Stop Control  
 < - Shared Left-Turn  
 > - Shared Right-Turn



**TABLE 5.2A: PM PEAK HOUR OPERATIONS - 2021-2035**

Analysis Period	Intersection	Horizon	Control Type	MOE	Direction / Movement / Approach																
					Eastbound				Westbound				Northbound				Southbound				Overall
					Left	Through	Right	Approach	Left	Through	Right	Approach	Left	Through	Right	Approach	Left	Through	Right	Approach	
PM Peak Hour	Trafalgar Road & QEW Westbound Off-Ramp / North Service Road	Base Year	TCS	LOS Delay V/C Q	E 67 0.28 15	D 39 0.48 73	D 41	E 57 0.19 118	E 57 0.19 121	A 0 0.25 0	D 35	C 24 0.91 362	A 1 0.35 0	C 20	D 53 1.03 380	A 10 0.01 0	D 53	D 35			
		2025 Background	TCS	LOS Delay V/C Q	E 67 0.29 16	D 39 0.50 78	D 41	E 56 0.77 126	E 56 0.77 130	A 0 0.27 0	D 35	C 27 0.98 399	A 0 0.37 0	C 23	F 85 1.12 415	B 11 0.01 0	F 85	D 48			
		2025 Total	TCS	LOS Delay V/C Q	E 67 0.29 16	D 38 0.50 80	D 41	E 56 0.77 127	E 56 0.77 131	A 0 0.27 0	D 35	C 29 0.99 402	A 0 0.38 0	C 24	F 90 1.13 421	B 11 0.01 0	F 90	D 51			
		2030 Background	TCS	LOS Delay V/C Q	E 73 0.45 17	D 41 0.56 97	D 44	E 66 0.87 167	E 65 0.86 169	D 49 0.64 99	E 61	B 16 0.89 212	A 0 0.44 34	B 15	C 20 0.74 166	A 0 0.01 0	C 20	C 25			
		2030 Total	TCS	LOS Delay V/C Q	E 73 0.45 17	D 41 0.57 98	D 43	E 66 0.87 171	E 65 0.86 170	D 49 0.64 99	E 61	B 16 0.90 218	A 10 0.45 34	B 15	C 20 0.75 170	A 0 0.01 0	C 20	C 25			
		2035 Background	TCS	LOS Delay V/C Q	E 75 0.51 18	D 40 0.59 112	D 42	E 70 0.91 212	E 72 0.92 219	D 50 0.71 130	E 65	C 29 1.01 358	A 10 0.52 25	C 26	C 27 0.86 203	B 11 0.01 0	C 27	C 34			
		2035 Total	TCS	LOS Delay V/C Q	E 75 0.51 18	D 40 0.60 114	D 43	E 72 0.92 215	E 73 0.93 221	D 50 0.71 130	E 66	C 32 1.01 362	B 11 0.52 25	C 29	C 28 0.87 207	B 11 0.01 0	C 28	D 35			
	Trafalgar Road & QEW Southbound On-Ramps	Base Year	TCS	LOS Delay V/C Q								A 0 0.35 0	A 0	A 0 0.67 0	A 1.00 0	A 0	A 0	A 0			
		2025 Background	TCS	LOS Delay V/C Q								A 0 0.71 0	A 0	A 0 0.37 0	A 1.07 0	A 0	A 0	A 0			
		2025 Total	TCS	LOS Delay V/C Q								A 0 0.72 0	A 0	A 0 0.38 0	A 1.07 0	A 0	A 0	A 0			
		2030 Background	TCS	LOS Delay V/C Q								A 0 0.66 0	A 0	A 0 0.40 0	A 0.49 0	A 0	A 0	A 0			
		2030 Total	TCS	LOS Delay V/C Q								A 0 0.67 0	A 0	A 0 0.41 0	A 0.49 0	A 0	A 0	A 0			
		2035 Background	TCS	LOS Delay V/C Q								A 0 0.72 0	A 0	A 0 0.43 0	A 0.61 0	A 0	A 0	A 0			
		2035 Total	TCS	LOS Delay V/C Q								A 0 0.69 0	A 0	A 0 0.44 0	A 0.61 0	A 0	A 0	A 0			
	Trafalgar Road & QEW Eastbound Off-Ramp	Base Year	TCS	LOS Delay V/C Q	D 47 0.85 173	E 57 0.88 193	D 50					C 32 0.79 218	C 32	B 17 0.60 95	B 17	C 32					
		2025 Background	TCS	LOS Delay V/C Q	D 50 0.89 188	E 62 0.92 215	D 54					D 43 0.85 278	D 43	B 16 0.64 83	B 16	D 37					
		2025 Total	TCS	LOS Delay V/C Q	D 44 0.84 179	E 56 0.90 212	D 48					D 46 0.89 287	D 46	B 19 0.68 93	B 18.5	D 38					
		2030 Background	TCS	LOS Delay V/C Q	D 38 0.79 173	E 50 0.88 215	D 42					D 42 0.84 243	D 42	C 23 0.78 172	C 23	D 36					
2030 Total		TCS	LOS Delay V/C Q	D 37 0.78 173	E 51 0.90 229	D 42					D 45 0.95 243	D 45	C 24 0.80 179	C 24	D 37						
2035 Background		TCS	LOS Delay V/C Q	D 40 0.85 205	E 62 0.96 267	D 47					D 46 0.93 246	D 46	C 28 0.87 216	C 28	D 41						
2035 Total		TCS	LOS Delay V/C Q	D 38 0.84 203	E 62 0.96 273	D 46					D 47 0.95 247	D 47	C 31 0.89 228	C 31	D 41						

MOE - Measure of Effectiveness  
 LOS - Level of Service  
 Delay - Average Delay per Vehicle in Seconds

Q - 95th Percentile Queue Length (m)  
 TCS - Traffic Control Signal  
 TWSC - Two-Way Stop Control

< - Shared Left-Turn  
 > - Shared Right-Turn





**TABLE 5.2C: PM PEAK HOUR OPERATIONS – 2021-2035 (CONT.)**

Analysis Period	Intersection	Horizon	Control Type	MOE	Direction / Movement / Approach																		
					Eastbound				Westbound				Northbound				Southbound				Overall		
					Left	Through	Right	Approach	Left	Through	Right	Approach	Left	Through	Right	Approach	Left	Through	Right	Approach			
PM Peak Hour	Cross Avenue & Argus Road / GO Station Driveway	Base Year	TCS	LOS Delay V/C Q	B 11 0.05 4	B 17 0.69 80	B 17 0.69 80	B 17 0.69 80	B 17	A 9 0.25 6	A 6 0.20 20	A 6 0.20 20	A 6	C 23 0.19 7	C 23 0.14 0	C 23 0.14 0	C 23	C 29 0.58 44	C 23 0.21 12	C 23 0.21 12	C 26	B 16	
		2025 Background	TCS	LOS Delay V/C Q	B 0.06 5	B 18 0.73 89	B 18 0.73 89	B 18 0.73 89	B 18	A 10 0.28 7	A 6 0.21 22	A 6 0.21 22	A 6	C 24 0.21 7	C 23 0.15 0	C 23 0.15 0	C 23	C 30 0.62 46	C 23 0.22 12	C 23 0.22 12	C 27	B 17	
		2025 Total	TCS	LOS Delay V/C Q	B 0.06 5	B 19 0.74 90	B 19 0.74 90	B 19 0.74 90	B 19	A 11 0.29 7	A 6 0.22 23	A 6 0.22 23	A 7	C 24 0.20 7	C 23 0.15 0	C 23 0.15 0	C 23	C 34 0.69 54	C 23 0.21 12	C 23 0.21 12	C 28	B 18	
		2030 Background	TCS	LOS Delay V/C Q	B 0.14 10	D 43 0.94 145	B 19 0.08	D 41	D 41	F 89 0.94 57	A 10 0.25 35	A 10 0.07	C 25	F 93 0.96 36	C 26 0.03	C 26 0.23	E 56	E 56	D 51 0.77 65	C 34 0.29 14	C 34 0.29 14	D 41	D 39
		2030 Total	TCS	LOS Delay V/C Q	C 0.14 10	D 19 0.96 145	D 19 0.08	D 44	D 44	F 92 0.95 57	B 12 0.27 37	A 10 0.07	C 25	F 83 0.92 35	C 26 0.03	C 26 0.23	D 51	D 51	E 61 0.85 84	C 34 0.28 14	C 34 0.28 14	D 46	D 41
		2035 Background	TCS	LOS Delay V/C Q	B 0.21 12	E 58 1.03 163	B 16 0.13	D 54	D 54	F 497 1.98 98	A 9 0.31 38	A 9 0.10	F 123	F 632 2.28 85	B 35 0.04	D 35 0.68	F 321	F 321	D 43 0.78 68	C 30 0.47 23	C 30 0.47 23	D 35	F 105
		2035 Total	TCS	LOS Delay V/C Q	B 0.22 12	E 16 1.05 164	B 16 0.13	E 59	E 59	F 514 2.02 98	B 11 0.32 39	A 9 0.10	F 125	F 586 2.17 85	B 34 0.04	C 34 0.67	F 298	F 298	D 52 0.85 82	C 29 0.48 26	C 29 0.48 26	D 39	F 105
PM Peak Hour	Cross Avenue & Lyons Lane / Commercial Driveway	Base Year	TCS	LOS Delay V/C Q	B 15 0.17 10	B 15 0.15 16	B 15 0.15 16	B 15	A 9 0.05 3	A 9 0.19 22	A 9 0.19 22	A 9	D 41 0.83 61	C 33 0.16 0	C 33 0.16 0	C 33	D 20 0.07 6	C 20 0.08 1	C 20 0.08 1	C 20	C 21		
		2025 Background	TCS	LOS Delay V/C Q	B 0.18 10	B 16 0.16 17	B 16 0.16 17	B 16	A 9 0.05 4	A 9 0.21 24	A 9 0.21 24	A 9	D 44 0.86 66	C 35 0.17 0	C 35 0.17 0	D 35	D 35	C 20 0.08 6	C 20 0.09 0	C 20 0.09 0	C 20	C 22	
		2025 Total	TCS	LOS Delay V/C Q	B 0.19 10	B 16 0.16 17	B 16 0.16 17	B 16	A 9 0.05 4	A 9 0.22 25	A 9 0.22 25	A 9	D 44 0.86 66	C 35 0.17 0	C 35 0.17 0	D 35	D 35	C 20 0.08 6	C 20 0.09 0	C 20 0.09 0	C 20	C 22	
		2030 Background	TCS	LOS Delay V/C Q	C 0.61 9	B 18 0.25 18	B 16 0.01	C 20	C 20	B 17 0.08 7	C 22 0.71 49	B 17 0.07	C 22	A 8 0.24 24	A 7 0.00	A 7 0.17	A 7	A 7	A 7 0.15 7	A 7 0.00	A 7 0.05	A 7	B 16
		2030 Total	TCS	LOS Delay V/C Q	C 0.64 10	B 17 0.24 18	B 16 0.01	C 21	C 21	B 16 0.08 7	C 22 0.70 48	B 16 0.09	C 21	A 8 0.24 24	A 7 0.00	A 7 0.17	A 7	A 7	A 7 0.16 7	A 7 0.00	A 7 0.03	A 7	B 16
		2035 Background	TCS	LOS Delay V/C Q	D 0.71 11	B 16 0.26 19	B 15 0.01	C 22	C 22	B 16 0.11 9	C 20 0.71 52	B 16 0.11	C 20	A 9 0.27 28	A 8 0.00	A 8 0.21	A 8	A 8	A 8 0.21 10	A 8 0.00	A 8 0.06	A 8	B 16
		2035 Total	TCS	LOS Delay V/C Q	D 0.74 11	B 16 0.26 19	B 15 0.01	C 23	C 23	B 16 0.11 9	C 20 0.70 52	B 16 0.13	C 20	A 9 0.30 33	A 8 0.00	A 8 0.22	A 9	A 9	A 9 0.22 11	A 9 0.00	A 9 0.07	A 8	B 16
PM Peak Hour	Cross Avenue & Lyons Lane	Base Year	TCS	LOS Delay V/C Q	A 0.01 0	A 0 0.06 0	A 0 0.06 0	A 1	A 0 0.27 0	A 0 0.14 0	A 0 0.14 0	A 0						B 14 0.20 6	B 14 0.20 6	B 14 0.20 6	A 2		
		2025 Background	TCS	LOS Delay V/C Q	A 0.01 0	A 0 0.06 0	A 0 0.06 0	A 1	A 0 0.29 0	A 0 0.15 0	A 0 0.15 0	A 0						B 14 0.22 0	B 14 0.22 0	B 14 0.22 0	A 1.5		
		2025 Total	TCS	LOS Delay V/C Q	A 0.02 0	A 0 0.06 0	A 0 0.06 0	A 1	A 0 0.29 0	A 0 0.17 0	A 0 0.17 0	A 0						B 15 0.25 0	B 15 0.25 0	B 15 0.25 0	A 2		

MOE - Measure of Effectiveness  
 LOS - Level of Service  
 Delay - Average Delay per Vehicle in Seconds  
 Q - 95th Percentile Queue Length (m)  
 TCS - Traffic Control Signal  
 TWSC - Two-Way Stop Control  
 < - Shared Left-Turn  
 > - Shared Right-Turn





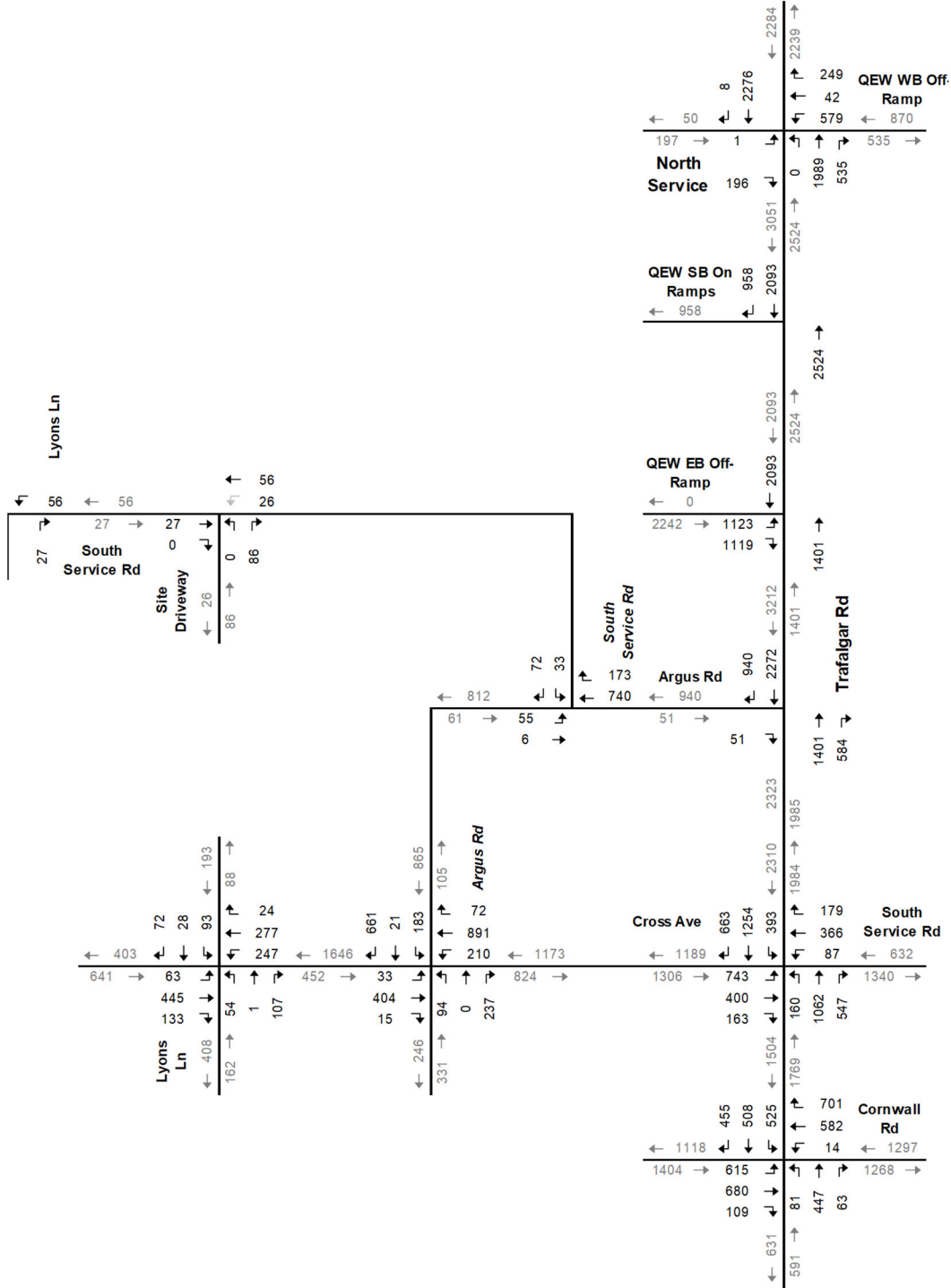
## 5.2 Lyons Lane Closure

North of Cross Avenue, Lyons Lane currently exhibits minimal traffic during the weekday peak hours, with approximately 40 two-way trips during the AM peak hour and 55 two-way trips during the PM peak hour. If Lyons Lane at Cross Avenue was closed before constructing the new MOEA road network, these existing trips would be reassigned to the intersection of South Service Road and Argus Road.

A sensitivity analysis has been completed that assessed the operations of the study area intersections with the closure of Lyons Lane, north of Cross Avenue, under the 2035 horizon. **Figure 5.1** illustrates the reassigned traffic volumes.

**Table 5.3** summarizes the sensitivity analysis results, while **Appendix G** contains the detailed Synchro reports. Overall, the study area intersections are expected to operate with similar operations without the closure of Lyons Lane. An exception is the intersection of South Service Road and Argus Road, which is projected to operate with considerable delay. If Lyons Lane is closed to vehicular traffic without establishing the MOEA roadway network, South Service Road at Argus Road would likely require mitigation measures (i.e. potential restricted movements).





# 2035 Total Traffic Volumes AM Peak Hour (Lyons Lane Closed)





**TABLE 5.3A: 2035 AM PEAK HOUR OPERATIONS (LYONS LANE CLOSED)**

Analysis Period	Intersection	Horizon	Control Type	MOE	Direction / Movement / Approach																Overall	
					Eastbound				Westbound				Northbound				Southbound					
					Left	Through	Right	Approach	Left	Through	Right	Approach	Left	Through	Right	Approach	Left	Through	Right	Approach		
AM Peak Hour	Trafalgar Road & QEW Westbound Off-Ramp / North Service Road	2035 Total	TCS	LOS Delay V/C Q	E 68 0.04 2	D 18 0.24 47	B 19		C 34 0.76 237	C 21 0.16 29	C 20 0.11 15	C 30		E 60 1.05 263	C 29 0.63 67	D 53		F 125 1.17 329	C 24 0.01 0	F 124	E 76	
	Trafalgar Road & QEW Southbound On-Ramps	2035 Total	FF	LOS Delay V/C Q										A 0 0.54 0		A 0		A 0 0.45 0	A 0 0.61 0	A 0		
	Trafalgar Road & QEW Eastbound Off-Ramp	2035 Total	TCS	LOS Delay V/C Q	C 21 0.63 140	F 187 1.33 544	F 104							D 51 0.84 183		D 51		F 152 1.23 268		F 152	F 108	
	Trafalgar Road & Argus Road	2035 Total	TWSC	LOS Delay V/C Q		B 11 0.08 2	B 11							A 0 0.30 0	A 0 0.37 0	A 0		A 0 0.58 0	A 0 0.89 0	A 0		
	Trafalgar Road & Cross Avenue / South Service Road	2035 Total	TCS	LOS Delay V/C Q	E 77 0.98 165	D 45 0.50 82	D 41 0.13 22	E 63	D 52 0.19 20	E 67 0.81 80	C 34 0.29 45	E 56		D 40 0.84 38	D 54 0.87 99	E 56 0.92 78	D 53	D 51 0.98 60	C 25 0.69 80	B 13 0.70 13	C 26	D 45
	Trafalgar Road & Cornwall Road	2035 Total	TCS	LOS Delay V/C Q	F 139 1.14 156	C 24 0.51 116	C 24 0.51 116	E 74	E 71 0.33 12	D 38 0.54 102	F 149 1.19 289	F 98		F 116 0.87 60	D 50 0.58 83	D 44 0.04 0	E 59	F 197 1.19 145	D 39 0.92 205	C 29 0.55 44	F 92	F 84
	Cross Avenue & Argus Road / GO Station Driveway	2035 Total	TCS	LOS Delay V/C Q	C 23 0.41 9	C 20 0.41 40	B 17 0.03 0	C 21	F 191 1.31 93	B 16 0.64 76	B 11 0.07 5	D 48		F 390 1.74 39	A 0 0.00 0	C 20 0.46 4	F 147	C 29 0.64 55	F 356 1.71 156	F 356 1.71 156	F 280	F 127
	Cross Avenue & Lyons Lane / Commercial Driveway	2035 Total	TCS	LOS Delay V/C Q	C 22 0.32 19	C 31 0.82 50	C 20 0.10 13	C 29	C 20 0.71 48	A 10 0.21 19	A 9 0.03 1	B 14		B 18 0.14 18	B 17 0.00 1	B 18 0.08 12	B 18	C 21 0.33 25	B 17 0.05 10	B 18 0.12 0	B 19	C 25
	Argus Road & South Service Road	2035 Total	TWSC	LOS Delay V/C Q	D 26 0.58 28	D 26 0.58 28		D 26		A 0 0.65 0	A 0 0.65 0	A 0						F Err 3.10	F Err 3.10	F Err		
	South Service Road & Site Driveway	2035 Total	TWSC	LOS Delay V/C Q	A 0 0.02 0	A 0 0.02 0		A 0	A 2 0.02 0	A 2 0.02 0	A 2			A 9 0.09 2	A 9 0.09 2	A 9						

MOE - Measure of Effectiveness

LOS - Level of Service

Delay - Average Delay per Vehicle in Seconds

Q - 95th Percentile Queue Length (m)

TCS - Traffic Control Signal

TWSC - Two-Way Stop Control

< - Shared Left-Turn

> - Shared Right-Turn



**TABLE 5.3B: 2035 PM PEAK HOUR OPERATIONS (LYONS LANE CLOSED)**

Analysis Period	Intersection	Horizon	Control Type	MOE	Direction / Movement / Approach																Overall
					Eastbound				Westbound				Northbound				Southbound				
					Left	Through	Right	Approach	Left	Through	Right	Approach	Left	Through	Right	Approach	Left	Through	Right	Approach	
PM Peak Hour	Trafalgar Road & QEW Westbound Off-Ramp / North Service Road	2035 Total	TCS	LOS Delay V/C Q	E 75 0.51 18	D 40 0.60 114	D 43	E 72 0.92 215	E 73 0.93 221	D 50 0.71 130	E 66	C 32 1.01 362	B 11 0.52 25	C 29	C 28 0.87 207	B 11 0.01 0	C 28	D 35			
	Trafalgar Road & QEW Southbound On-Ramps	2035 Total	TCS	LOS Delay V/C Q								A 0 0.69 0	A 0 0 0	A 0	A 0 0.44 0	A 0 0.61 0	A 0	A 0			
	Trafalgar Road & QEW Eastbound Off-Ramp	2035 Total	TCS	LOS Delay V/C Q	D 38 0.84 203	E 62 0.96 273	D 46					D 47 0.95 247	A 47 0.95 247	D 47	C 31 0.89 228	C 31	D 41				
	Trafalgar Road & Argus Road	2035 Total	TWSC	LOS Delay V/C Q		B 12 0.15 4	B 12					A 0 0.47 0	A 0 0.60 0	A 0	A 0 0.55 0	A 0 0.62 0	A 0				
	Trafalgar Road & Cross Avenue / South Service Road	2035 Total	TCS	LOS Delay V/C Q	F 126 1.16 288	C 32 0.36 65	C 32 0.20 27	F 96	E 66 0.74 63	E 62 0.72 72	F 128 1.08 163	F 89	F 143 1.19 66	E 62 1.01 123	E 58 0.22 15	E 72	E 64 0.93 46	E 76 1.07 223	D 38 0.92 153	E 65	F 79
	Trafalgar Road & Cornwall Road	2035 Total	TCS	LOS Delay V/C Q	F 182 1.24 165	D 36 0.76 172	D 36 0.76 172	F 91	D 36 0.46 20	F 86 1.04 236	F 253 1.43 343	F 154	F 181 1.20 97	D 48 0.57 90	D 39 0.03 0	E 79	F 198 1.26 136	F 109 1.10 235	F 86 0.91 124	F 131	F 122
	Cross Avenue & Argus Road / GO Station Driveway	2035 Total	TCS	LOS Delay V/C Q	B 18 0.28 15	E 58 1.03 159	B 16 0.13 1	D 53	F 514 2.02 98	B 11 0.31 37	A 10 0.14 7	F 121	F 766 2.58 81	B 19 0.04 1	C 34 0.67 24	F 385	D 52 0.85 82	C 32 0.60 34	C 32 0.60 34	D 40	F 112
	Cross Avenue & Lyons Lane / Commercial Driveway	2035 Total	TCS	LOS Delay V/C Q	C 22 0.53 8	B 17 0.30 20	B 16 0.01 2	B 18	B 16 0.12 9	C 21 0.68 44	B 16 0.10 2	C 20	A 8 0.28 27	A 0 0.00 0	A 7 0.22 0	A 9	A 7 0.22 6	A 7 0.14 1	A 6 0.00 1	A 7	B 15
	Argus Road & South Service Road	2035 Total	TWSC	LOS Delay V/C Q	A 9 0.02 5	A 9 0.37 5	A 9	A 9	A 0 0.37 0	A 0 0.37 0	A 0 0 0	A 0				C 22 0.49 21	C 22 0.49 21	C 22			
	South Service Road & Site Driveway	2035 Total	TWSC	LOS Delay V/C Q	A 0 0.04 0	A 0 0.04 0	A 0	A 4	A 4 0.05 1	A 4 0.05 1	A 4	A 4	A 9 0.05 1	A 9 0.05 1	A 9						

MOE - Measure of Effectiveness  
 LOS - Level of Service  
 Delay - Average Delay per Vehicle in Seconds

Q - 95th Percentile Queue Length (m)  
 TCS - Traffic Control Signal  
 TWSC - Two-Way Stop Control  
 < - Shared Left-Turn  
 > - Shared Right-Turn



## 6 Mitigation

As summarized in the analysis tables in the previous chapter, several study area locations either currently experience or are projected to experience operational deficiencies independent of the development. **The analysis also identified that the development would have minimal impacts on traffic conditions in the study area.**

### 6.1 Midtown Oakville Improvements

Mitigation measures at several study area intersections have been identified through the Midtown Oakville Environmental Assessment. The critical transportation network improvements include the following:

- ▶ The extension of Cross Avenue eastward to Royal Windsor Drive and the QEW. This extension will likely divert eastbound left-turns at Cross Avenue and Trafalgar Road to the QEW and Royal Windsor Drive interchange. In addition to the extension, Cross Avenue will be widened from Trafalgar Road to Lyons Lane.
- ▶ A new north-south crossing of the QEW east of Trafalgar Road. This north-south road will connect the Cross Avenue extension with Iroquois Shore Road and Trafalgar Road at McCraney Street. This new street will have pedestrian and cycling infrastructure, dedicated transit lanes, and general-purpose lanes. The purpose of the new roadway is to divert north/south traffic on Trafalgar Road;
- ▶ Realignment of the Trafalgar Road and QEW Eastbound Off-Ramp Terminal. A new eastbound QEW direct off-ramp will be constructed under Trafalgar Road and connect with the Cross Avenue extension. This new off-ramp will divert eastbound right-turns that will generally turn onto Trafalgar Road. The realignment of South Service Road and Argus Road to the west of its current location will be needed to accommodate this new off-ramp;
- ▶ Two new active transportation crossings of the QEW are west and east of Trafalgar Road. The west crossing will connect Oakville Place to the realigned Argus Road. The east crossing will be separate and east of Trafalgar Road; and
- ▶ Improvements to the QEW and Royal Windsor Drive interchange will include a new westbound off-ramp, a new eastbound on-ramp, a new eastbound direct off-ramp to Cross Avenue extension, and the widening and extension of Royal Windsor Drive to Iroquois Shore Road at Eighth Line. The improvements to this interchange will likely divert traffic to/from the Trafalgar Road interchanges.



Because the GO Oakville Station is located centrally within the study area, the Trafalgar Road corridor experiences many trips during the weekday peak hours. In most circumstances, widening existing roads or building new ones will infringe on private property, impact mature trees and green space, or compromise the current public realm (e.g. sidewalks, boulevards).

The Midtown Oakville and Trafalgar Road EAs recognize this and identify roadway improvements, which will relieve operational issues; however, capacity constraints will persist for the overall transportation network. Instead, the strategy is to further focus on providing a sustainable transportation strategy to move more people per kilometre by walking, cycling and transit or in combination with high occupancy vehicles.

## 6.2 Transportation Network Overview

The vehicle traffic analyses of intersection performance conducted overall conditions are expected to experience congestion during the weekday peak hours. This area surrounds one of the busiest GO Transit stations within the province; the area is expected to experience vehicle capacity constraints two to four hours a day on a typical weekday. The other 20 hours of the weekday, weekends and holidays would be expected to exhibit better vehicle traffic conditions.

Widening any arterial roads to accommodate vehicular traffic volumes is not recommended in the study area. Any potential road widening would accommodate dedicated bus lanes to improve transit capacity and efficiency. This would be counter-intuitive to the vision of a people-centric, pedestrian-friendly environment that expects people to use more sustainable modes to travel.

Conditions for pedestrians, cyclists and transit users would be expected to be significantly improved from existing conditions in the study area. The smaller block size and wide sidewalks on both sides of any new roads will aid in pedestrian connectivity through pedestrian pathways. Wider sidewalks and multi-use pathways are anticipated in areas of the highest pedestrian demand.

Cycling infrastructure presently is limited in the study area. The cycling network is expected to expand through redevelopment, and gaps in the network are expected to be filled.

Increased frequency of GO trains and buses and Oakville Transit buses are expected to make transit more convenient. Transit priority measures are recommended to be explored further by the Town to help make transit schedules more reliable and make transit a more attractive option.

There is forecast to be more vehicle traffic congestion, longer delays and long queues at full build-out of the study area. At the same time, with the people-centric approach to Midtown Oakville, there are expected to be more amenities and better connectivity for pedestrians and cyclists and more



frequent, reliable, and convenient service for transit passengers. The finer grain road network is proposed to provide enhanced multi-modal connectivity and access.

## 6.3 Right-In/Right-Out Conditions

### 6.3.1 Argus Road and South Service Road

Future southbound left-turn movements from South Service Road East are forecast to operate at LOS F due to high east-west volumes along Argus Road.

For this study, a significant portion of site traffic from South Service Road East was assumed to utilize Argus Road to access Cross Avenue as a "worst-case scenario." However, Midtown Oakville plans to introduce a revised local road network to alleviate congestion at this intersection by providing access to Cross Avenue via alternative north-south local roads.

To avoid situations where southbound drivers are experiencing significant delays, the southbound Leg of the South Service Road may be limited to right-in/right-out movements. A 2035 sensitivity test has been analyzed with restricted movements. Results of the Sensitivity test can be found in **Appendix H**.

## 6.4 Left Turn Lane

The need for auxiliary turn lanes was reviewed at the development's new driveway connection. A design speed of 10 kilometres per hour over the posted and assumed speed limit had been utilized. The warrants for left-turn lanes follow the Ministry of Transportation's (MTO) Geometric Design Standards<sup>7</sup> requirements. **Table 6.1** summarizes the results of the left-turn lane warrant analyses.

**TABLE 6.1: LEFT TURN LANE WARRANT**

South Service Road at Site Driveway		
Approach Direction Design Speed	Westbound 60 km/h	
Horizon	2034 Total	
Peak Hour	AM	PM
Advancing Volume	25	66
Opposing Volumes	15	32
Left Turning Traffic	15	44
% of Left Turning Traffic	60.0%	66.7%
Figure Used*	9A-9 (40%)	9A-9 (40%)
<b>Warranted</b>	<b>No</b>	<b>No</b>
Storage Length Required	N/A	N/A

Based on MTO Design Supplement for TAC Geometric Design Guide for Canadian Road - June 2017

<sup>7</sup> MTO Design Supplement- TAC Geometric Design Guide for Canadian Road, June 2017



## 7 Travel Demand Management

The Town of Oakville is actively engaging the development community to integrate Travel Demand Management (TDM) in all current and future development applications. A TDM Study aims to outline a transparent process for selecting and implementing TDM measures.

### 7.1 Area Opportunities

#### 7.1.1 Pedestrians

This area in the Town of Oakville is primarily commercial and within proximity of the Oakville GO Station; therefore, pedestrian facilities are limited to the sidewalk network along the east side of Lyons Lane. Directly to the west of the subject property, a pedestrian path leads under the Queen Elizabeth Way/Highway 403 to enable pedestrians to safely cross the Highway to the residential area to the north. There are no pedestrian facilities along South Service Road.

The site is within walking distance of numerous and significant retail and transportation opportunities providing a range of destinations for prospective residents of the proposed building that can be readily accessed without using a car.

Pedestrian sidewalks are provided on a least one side of streets throughout most of the study area. Crosswalks, pedestrian pushbuttons, and indicators are provided for all approaches at the signalized intersections within the study area. These facilities will further help foster and promote walking trips to/from the development.

Furthermore, MOEA identified additional links for pedestrians and cyclists are recommended by two grade-separated, active transportation crossings of the QEW. The crossing proposed on the west side of Trafalgar Road is located approximately 600 metres from the proposed development. This crossing will encourage residents of the development and general area to explore active transportation mode choices and improve access to transit.

#### 7.1.2 Cycling

On-road cycling lanes are not currently provided on the streets in the study area. However, as cyclists are permitted to ride on most roads except controlled-access highways, the lack of a separate bicycle lane will not prohibit this type of travel, particularly for this development.

Reviewing the Town of Oakville's Active Transportation Master Plan (ATPM) document for cycling network plans and existing facilities within the proposed development indicates that there are currently no cycling facilities present. However, within the ATMP, on-street bike lanes are proposed along Lyons Lane and are proposed to be completed in the Long-Term Phase (11-20+ years), and Cross Avenue to be completed in 2020.



### **7.1.3 Oakville Transit**

Oakville Transit owns and operates the public transit system in Oakville. The subject site is located within the Midtown Oakville Urban Growth Area, one of the most transit-accessible locations within the Town. The subject site is located approximately 450 metres to the western end of the Oakville GO Station, which is currently serviced by 14 out of 21 Oakville Transit Routes. The majority of the transit routes operate seven days a week from early morning to late evening and provide access to all of the Town of Oakville.

The Trafalgar Road Class EA recommended that the curb lane of Trafalgar Road be converted to a high occupancy vehicle (HOV) or bus rapid transit (BRT) lane. With the presence of a BRT line along Trafalgar Road, ridership in the service area typically increases by as much as 50%. This future transit line provides a further incentive for residents of the development to choose transit as a primary commuting option.

### **7.1.4 GO Inter-Regional Transit**

The proposed development is located approximately 450 metres to the western end of the Oakville GO Station. This station is located along the Lakeshore West Line, which currently operates a two-way all-day train service seven days a week plus GO Bus connections to Hamilton, Sheridan College and York University via Highway 407.

### **7.1.5 Ride-Hailing & Car Services**

Ride-hailing car services in the Town of Oakville include Oakville Taxi, Oakville United Taxi, A1 Oakville Taxi, Halton Taxi Svc, Blue Line Taxi, Oakville Para-transit and Uber/Lyft. These services offer on-demand private car services.



## 7.2 Site Specific Measures

A Transportation Demand Management Plan (TDMP) is a strategy using policies, programs, services, and products that result in more efficient use of transportation resources. A TDMP is designed to assist people and organizations in getting where they are going by being environmentally, economically, and socially responsible for their transportation actions.

This plan intends to support employee and patron's travel patterns to the subject site using sustainable transportation options.

### 7.2.1 Walking

The proposed site plan does not show sidewalks connections from the building to the municipal streets. Well-lit sidewalks and walkways between the building should be included to/from roads, parking areas and main entrances. Weather protection by main entrances and on adjacent sidewalks should also be considered.

It is recommended that the on-site pedestrian sidewalks be well-lit and conform to the Town of Oakville's design standards and the Accessibility for Ontarians with Disabilities Act (AODA) design standards.

### 7.2.2 Cycling

The site plan provides for a total of 221 long-term and 74 short-term bicycle parking spaces. The facilities are located on the first two-level of the underground parking garage and the grade floor.

### 7.2.3 Transit

No transit-related infrastructure is proposed as part of this development application. The existing transit stops located south of the subject site on Cross Avenue are expected to continue to service the area. The addition of the Oakville GO station nearby helps reduce the vehicle demand from the subject site.

### 7.2.4 Unbundled Parking

Rather than automatically including a certain amount of parking with building space, users can pay parking costs directly by "unbundling," which means that parking is rented or sold separately. This is more equitable and efficient since occupants are not forced to pay for parking they do not need and allows consumers to adjust their parking supply to reflect their needs. The applicant will charge for parking at a separate cost to occupants. If it is found that not all car parking spaces are utilized, then the vacant spaces can be repurposed (i.e. converted to bicycle storage).





### **7.2.5 Travel Planning/Education/Promotion**

Increasing awareness of sustainable transportation opportunities for site residents should be considered. Residents should be provided with a welcome package that outlines the available transit routes and active transportation options, such as the availability of bicycle parking and proximity to the existing bicycle network.

## **7.3 Site Specific Monitoring**

### **7.3.1 Walking Infrastructure Monitoring**

It is recommended that the site operator monitor the long-term desire lines, if any, created by the erosion caused by pedestrians crossing the site's landscaped areas. Should desire lines form, there may be an opportunity to adjust the site's landscaping to encourage the use of the designated on-site pedestrian sidewalks.

### **7.3.2 Cycling Infrastructure Monitoring**

It is recommended that the site operator monitor the on-site bicycle parking spaces to ensure demand matches supply. An indicator to suggest that the site's bicycle parking demand exceeds supply is observing bicycles locked to the street furniture on-site or immediately adjacent to the subject site.

Should the site's bicycle parking demand regularly exceed the supply, consideration should be given to expanding the amount of on-site bicycle parking provided.



## 7.4 TDM Conclusions

The site plan does not indicate TDM measures such as short-term or long-term bicycle parking or sidewalk connections. Measures that are currently not included on the site plan that is recommended to help further promote and encourage TDM include:

- ▶ The on-site pedestrian sidewalks are well-lit and conform to the Town of Oakville's and AODA design standards.
- ▶ Consider unbundling parking where parking spaces are provided as a separate cost to residents.
- ▶ Provide general education on all travel mode options (walking, cycling, and transit) that identify benefits and how residents can best utilize these modes. New residents should be provided with a welcome package that outlines proximity to transit, cycling facilities and the proximity to local activity centres
- ▶ The site operator monitors the long-term desired lines and adjusts the site's landscaping to encourage the use of the designated on-site pedestrian sidewalks.
- ▶ The site operator monitors the on-site bicycle parking supply to ensure that the appropriate amount of bicycle parking is provided.



## 7 Conclusions and Recommendations

### 7.1 Conclusions

This study evaluated the impacts of the construction of 281 apartment units in a 28-storey building on a parcel of land bounded by South Service Road and Lyons Lane. Vehicular access to the building's parking facilities and loading area is proposed via two accesses to the South Service Road located at the eastern limits of the property. The western driveway will be designated for pick-up and drop-off service for clarification. Overall, the proposed development will generate approximately 112 new vehicle trips during the weekday AM peak hour and 110 new vehicle trips during the weekday PM peak hour.

Detailed traffic analysis was conducted for each study area intersections under Base conditions, 2025, 2030 and 2035 Background and Total conditions.

The new traffic forecast to be added by full build-out of the development to the study area results in relatively small impacts at the various study intersections. The analysis has further determined that the proposed driveways to South Service Road and Lyons Lane will operate at LOS A during the weekday peak periods, and separate left-turn lanes are not warranted at either driveway connection under the 2035 Total conditions.

It is acknowledged that deficiencies currently exist at specific locations, primarily along the Trafalgar Road corridor within the study area. They can be expected to persist in the future with anticipated growth in traffic, independent of the development.

A variety of roadway improvements are planned within the study area to address the existing and long-term impacts of traffic growth, including the construction of new direct off-ramps for the QEW at Trafalgar Road, a revised local road network for Midtown Oakville, an extension of Cross Avenue and a variety of intersection improvements. It is understood that these improvements will provide some relief to operational issues. However, capacity constraints will persist for the overall transportation network.

Due to the high levels of congestion and the expected long-term transit network anticipated to be developed (BRT along Trafalgar Road), further remedial measures to improve intersection capacity are not likely to be implemented. Instead, future improvements to the transportation network are expected to focus on public transit infrastructure.

By focusing on shifting commuter travel to public transit, intersection operations could be expected to maintain the status quo (a capacity condition during peak hours) or possibly improve if fewer vehicles transverse the intersections during the peak hours of a typical weekday.



As requested by the Town, a sensitivity analysis has also been assessed, assuming Lyons Lane is closed off at Cross Avenue. If this occurs, and considering the MOEA roadway network is not established, the intersection of South Service Road at Argus Road would experience considerable delay and congestion, and mitigation measures would need to be implemented (i.e., the potential for restricted movements)

## 7.2 Recommendations

Based on the findings of this study, it is recommended that the development be allowed to proceed with the following TDM measures:

- ▶ The site plan is updated to include short-term bicycle parking at the main entrances of the buildings in a visible well-lit area.
- ▶ Long-term bicycle parking in a safe and secure area (either a dedicated room or storage lockers) be included in the site plan.
- ▶ The on-site pedestrian sidewalks are well-lit and conform to the Town of Oakville's design standards and the Accessibility for Ontarians with Disabilities Act (AODA) design standards.
- ▶ Consider unbundling parking where parking spaces are provided as a separate cost to residents.
- ▶ Provide general education on all travel mode options (walking, cycling, and transit) that identify benefits and how residents can best utilize these modes. New residents should be provided with a welcome package that outlines proximity to transit, cycling facilities and the proximity to local activity centres
- ▶ If any, the site operator monitors the long-term desire lines created by the erosion caused by pedestrians crossing the site's landscaped areas. Should desire lines form, there may be an opportunity to adjust the site's landscaping to encourage the use of the designated on-site pedestrian sidewalks.
- ▶ The site operator monitors the on-site bicycle parking supply to ensure that the appropriate amount of bicycle parking is provided.



# Appendix A

## First Submission Comments



27 August 2021  
Project: 190665

Town of Oakville  
Tricia Collingwood  
1225 Trafalgar Rd,  
Oakville, ON L6H 0H3

**RE: 627 LYONS LANE – TOWN OF OAKVILLE TRANSPORTATION PLANNING  
COMMENTS**

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In August 2019, Paradigm Transportation Solutions Limited (Paradigm) prepared a Transportation Impact Study (TIS)<sup>1</sup> for the proposed development located at 627 Lyons Lane in the Town of Oakville. Town of Oakville Transportation Planning staff provided comments on the TIS in a letter dated 16 March 2020.

This letter provides additional information and responds to the Transportation Planning comments. For ease of review, each comment is copied below followed by our response/clarification to the comment.

**Town Comment #1**

Please provide a pedestrian circulation plan to show the interim connections to the existing roadways, and ultimate connections with the future roadways outlined in the Midtown Oakville EA. [Circ. 1]

This has been provided in the updated TIS.

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<sup>1</sup> 180307: 627 Lyons Lane, Oakville, Transportation Impact Study, Paradigm, August 2019

### **Town Comment #2**

The Metrolinx Midtown Oakville Mobility Hub Study proposed the relocation of the Oakville GO station and bus terminal from the existing location to the east side of Trafalgar Road. The future GO station will be slightly further east from the site than it is today. [Circ. 1].

Acknowledged.

### **Town Comment #3**

The nearest existing bus stops to the site are located on Cross Avenue at Lyons Lane. The site driveway to South Service Road does not provide a convenient walking option to Cross Avenue to access the bus stops and the GO station. Please consider a pedestrian only site access from Lyons Lane and provide a pedestrian circulation plan to show the ultimate connections with the future roadways outlined in the Midtown Oakville plan. [Circ. 1]

This has been considered in the updated TIS.

### **Town Comment #4**

Oakville Transit provides door-to-door paratransit service called care-A-van for persons with disabilities. Service is provided by low-floor, fully accessible 26ft buses supplemented in partnership with local taxi providers. Drivers will leave the vehicle and escort the customer to the first accessible public entrance. Please identify paratransit pick up/drop off location on the site plan in the Site Plan process. [Circ. 1]

Acknowledged.

### **Town Comment #5**

The proposed development is envisioned with one full movement access on South Service Road. What is the plan for a secondary access upon full build-out? Most of the study area intersections are operating at or below capacity under the existing conditions. Until such time, the Midtown Oakville Infrastructure improvement study is finalized, of which the timing of implementation is unknown, what is the interim plan to safely accommodate the additional (proposed site-generated) traffic on the network? TIS needs to evaluate the minimum required/necessary improvements accordingly for vehicular, transit, pedestrian and bicycle traffic in, from, and within the site. [Circ. 1]

To our understanding a single connection is only contemplated. Appropriate sidewalks, crossing and bicycle storage facilities will be provided to accommodate active transportation within the site.



### **Town Comment #6**

1. Town of Oakville Transportation Master Plan projects anticipated to be constructed by 2031 (by the town or by others), and the new proposed project is anticipated completed in 2022. From the TIS report LOS, and queue at QEW off-ramp intersections are not acceptable at the time when new proposed building is completed. **Please provide a recommendation that improve operational conditions to an acceptable level at the intersections from 2022 to before the Master Plan implemented.**

It is understood that even with improvements as identified in the Trafalgar Road EA and Midtown Oakville EA, capacity constraints will persist for the overall transportation network. Future improvements to the transportation network are expected to focus on public transit infrastructure rather than typical geometric improvements to improve operational conditions. The implementation of interim improvements would be considered temporary and will involve considerable throw away costs. It is also acknowledged that increased delay is projected at the QEW off-ramps without build-out of the proposed development.

### **Town Comment #7**

Meanwhile, the consultant is required to explain in more details the questions listed below:

- Trip Generation with Mode Split - Section 3.2

At reports Page 17, "Trip reductions to account to transit usage (6.6%), active transportation modes (6.0%), and travel demand management (6.0%) have been applied to the base trip generation. The trip reductions were obtained from the Town of Oakville Transportation Master Plan".

- The percentage is a target, and by year 2031, the new proposed development will be completed by year 2022, the report applied mode split from 2022. Suggesting the mode split should apply after the Mater Plan projects completed, and please provide detail schedule of the projects including design and construction stage etc...

The TIS included a review of three horizons (2022, 2027 and 2032) representing near term, short term and long-term implications. The use of the modal split as provided provides for a most likely case planning scenario for the long-term implications. It is noted that use of this modal split for the near term and short term provides for a reasonable best-case scenario, however removal of this modal split assumption would not impact, alter and result in modifications to the recommendations given the modal split accounts for a reduction in 20-25 two-way vehicle trips during the weekday peak hours.





### Town Comment #8

In general, trips at QEW have longer travel distance than local road such as Town of Oakville, and origin/destination is not within Oakville only, wider such as GTHA. The mode split should be different at different class of roadway. For example, what type of 6% active transportation mode will be on QEW ramps?

Due to the proximity of the proposed site to a major transit station (Oakville GO Station), we would expect that active transportation mode split to be high for this development.

### Town Comment #9

- Trip Assignment - Table 3.3.
  - New trip assigned at QEW East (Toronto) is 7% - 9%, and West (Hamilton) is 23% - 19%, during AM and PM peak hours, based on existing distribution of traffic. Please provide more detail information of the assumption.

The trip distribution was developed based the existing link volumes for the study area roadways. See below the weekday AM and PM peak hour link volumes.

Weekday AM Peak Hour				Weekday AM Peak Hour			
North	Trafalgar Road	4937	37%	North	Trafalgar Road	5024	35%
South	Trafalgar Road	972	7%	South	Trafalgar Road	1305	9%
East	QEW	896	7%	East	QEW	1311	9%
	South Service Road	607	5%		South Service Road	749	5%
	Cornwall Road	1972	15%		Cornwall Road	2246	15%
West	QEW	3005	23%	West	QEW	2733	19%
	South Service Road	580	4%		South Service Road	755	5%
	Cornwall Road	217	2%		Cornwall Road	368	3%
		<b>13186</b>	<b>100%</b>			<b>14491</b>	<b>100%</b>



### **Town Comment #10**

- Future forecast - Section 4

Appendix C provides traffic projections related to site specific growth in 2027 and 2032, reference from MOEA report 20% and 40% built-out assumption. Figure C.1 - C.4, (Figure C.2 same as Report Figure 4.3B). please clarify more in detail below points.

- Traffic volume on Northbound of Trafalgar Rd has significantly reduced, especially QEW off-ramps turn to Northbound Trafalgar Rd. E.g. in 2022 is 346, and 2032 is 286, 10 years' after the volume has a negative growth.

Volumes were developed based on the projections as contained in the MOEA. The MOEA outlines a number of movements are expected to see a decrease due to travel pattern changes. The following calculations were completed to derive the 20% and 40% build-out assumption:

MOEA Volumes utilized.

2031 Preferred Network Volumes were subtracted from 2014 Existing Volumes to provide for a general basis of growth expected and travel pattern changes to the study area roadways. A growth rate of 2% per annum between year 2014 and 2031 was factored out of the volumes to remove general growth assumptions.

For the 20% build out scenario, 20% of the MOEA projections as derived in Step 1 were utilized. For the 40% build out scenario, 40% of the MOEA projections as derived were utilized.

Adjustments and further refinements were completed to maintain balancing along the study area corridors

### **Town Comment #11**

- Traffic pattern or trip origin/destination at QEW off-ramps are different with local road of Town of Oakville. E.g. the trips at QEW off-ramps generated is not by Town of Oakville only, by GTHA.

Acknowledged. The trip distribution was based on a wider area and included additional roadways in the development of the trip distribution.



### **Town Comment #12**

- According to MOEA, major growth area is located south of QEW, but at North of QEW of Oakville will still has a minor growth, so the traffic from/to northbound of Trafalgar Rd should not decrease significantly.

We disagree based on the projections outlined in the MOEA. 2014 existing two-way link volumes along Trafalgar Road north of the North Service Road/QEW WB Ramp are between 4,000 and 5,100 vph. The 2031 projected volumes as contained in the MOEA results in between 2,900 and 3,300 vph.

### **Town Comment #13**

- Population, total households, total vehicles, and average vehicle ownership are increasing, but how traffic volume is decreasing.

Given the needs for the Midtown Oakville area, potential infrastructure improvements range from revised road alignments to new road, transit, and active transportation connections, including additional crossings of the QEW.

The MOEA identified that to accommodate traffic to and from Midtown Oakville and to provide an alternate to Trafalgar Road, several improvements are contemplated within the preferred concept. Through these improvements, traffic isn't so much decreasing but will be re-routed through new connections away from the heavily used Trafalgar Road corridor. The net result is a reduction in volumes along Trafalgar Road. The volumes projected in the MOEA identified this reduction in consultation with the MTO.



## Town Comment #14

2. Turning Movement Counts, at the report had compared two years' TMCs of 2015 and 2017, and from the comparison concluded that 2017 traffic volumes had decreased from 2015 at intersection of QEW ramps at Trafalgar Rd. The major volume decreased legs were off-ramps of QEW. But there have discrepancies with MTO database. For example, when comparing TMC data of 2017 (page 95-97) QEW EB off-ramp intersection, MTO data is showing the volume was not decreased. Please see below table for the detail and recommended conduct new TMCs, or using 2015 TMCs data applying annual growth rate.

AM Peak							
MTO database				2017 TMC			Difference
Day	Ending	Total		Left	Right	Total	
6/1/2017	8:00	449	1254				331
6/1/2017	8:15	418	1472				
6/1/2017	8:30	471	1666				
6/1/2017	8:45	414	1752	822	599	1421	
Noon Peak							
MTO database				2017 TMC			Difference
Day	Ending	Total		Left	Right	Total	
6/1/2017	11:45	306	1350				253
6/1/2017	12:00	375	1381				
6/1/2017	12:15	317	1350				
6/1/2017	12:30	364	1362	673	436	1109	
PM peak							
MTO database				2017 TMC			Difference
Day	Ending	Total		Left	Right	Total	
6/1/2017	17:15	378	1415				298
6/1/2017	17:30	437	1476				
6/1/2017	17:45	402	1536				
6/1/2017	18:00	391	1608	891	419	1310	

The other factor is population, total households, total vehicles, and average vehicle ownership of Town of Oakville is increasing over the years, as well as GTA, so the traffic volume should have increasing trend.

As stated in the comment above, the ramp terminals experienced a decrease, thus a factor applied to these movements is not warranted. In terms of through volumes along Trafalgar Road, the upstream and downstream volumes were balanced along the Trafalgar Road corridor to compensate for daily volume fluctuations along the study area roadways.



### **Town Comment #15**

- 3 Synchro reports and analysis, please update below points according to TIS guideline.
  - For ramps, a v/c ratio for terminal ramp approaches with a value greater than 0.75 would be deemed critical and should be evaluated for possible operational improvements.

As stated in the report, the MOEA identified that even with the proposed improvements implemented, capacity constraints is still projected to occur.

### **Town Comment #16**

- 95<sup>th</sup>ile Queue length at intersections, should not exceeding existing available storage length. The queue length should determine by Sim Traffic, and simulation time of seeding is 15min, recording time is 60 min.

A SimTraffic analysis can be undertaken, however as stated in the MOEA, with the proposed improvements implemented, capacity constraints are still projected to occur.

### **Town Comment #17**

- Synchro simulation should ne matching with existing roadway conditions such as lane configuration. For example, QEW WB off-ramp SBT, number of lanes of approaching and receiving is balanced, missing on-ramp leg.

Will be reviewed.

### **Town Comment #18**

- Default value of lost time adjustment is zero, if adjust the value please provide more detail information such as filed study.

We have reviewed the MTO TIS Guidelines and have not come across any reference to lost time. If available please provide. As a result, we have defaulted to the Region of Halton that stipulates four seconds of lost time be included in the Synchro calculations.



If you have any questions or would like to discuss our responses in more detail, please feel free to contact us.

Yours very truly,

**PARADIGM TRANSPORTATION SOLUTIONS LIMITED**



**Adam J. Makarewicz**  
Dipl.T., C.E.T. MITE  
Senior Project Manager



**Stew Elkins**  
B.E.S., MITE  
Vice President



04 May 2022  
Project: 190665

Town of Oakville  
Tricia Collingwood  
1225 Trafalgar Rd,  
Oakville, ON L6H 0H3

**RE: 627 LYONS LANE – TOWN OF OAKVILLE TRANSPORTATION PLANNING  
COMMENTS (2<sup>ND</sup> SUBMISSION)**

---

In August 2021, Paradigm Transportation Solutions Limited (Paradigm) prepared a Transportation Impact Study (TIS)<sup>1</sup> for the proposed development located at 627 Lyons Lane in the Town of Oakville. The Review Agencies provided comments on the TIS in a letter dated 22 February 2022.

This letter provides additional information and responds to the Transportation Planning comments. For ease of review, each comment is copied below followed by our response/clarification to the comment.

**Town Comment #1a**

- a. Confirm that the site plan is the current site plan and that there is no proposed building structure in the 50m buffer zone.

Updated site plan reflects the 50m buffer in the updated TIS.

**Town Comment #2a**

- a. Confirm if floor plan 3 & 4 is the same as floor plan 2, as Floor plan 3&4 was not provided.

Addressed by others.

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<sup>1</sup> 210060: 627 Lyons Lane, Oakville, Transportation Impact Study, Paradigm, August 2021

**Town Comment #3a**

a. The traffic site plan illustrate three (3) pedestrian connectivity to Lyons Lane, please identify on the current site plan.

Addressed by others.

**Town Comment #3b**

The Land Use Code for the development should be LU 221 rather than LU 222 as LU 221 provided a small sample for the rates provided.

LUC 221 has been used in the updated TIS.

**Town Comment #3c**

Provide Lane configuration figures to represent the difference in the existing lanes and the proposed future lanes.

Addressed in updated TIS.

**Town Comment #3d**

Hwy 403 does not intersect Trafalgar Road, please update for future report.

Acknowledged

**Town Comment #3e**

The assessment for the intersection of North Service Road and Trafalgar Road is separate from Trafalgar Road and QEW WB on ramp/Off ramp.

The QEW WB Ramp terminal are located opposite the North Service Road and has been assessed as such. Traffic data provided by the Region at Trafalgar Road and North Service Road includes the QEW WB ramp volumes as the east leg of the intersection.





**Town Comment #3f**

The assessment for the intersection of North Service Road and Trafalgar Road is separate from Trafalgar Road and QEW WB on ramp/Off ramp.

Trafalgar Road and QEW EB Off Ramp has been reassessed solely with off-ramp volumes rather than including the on-ramp volumes. Rather, a new intersection has been included along Trafalgar Road that includes the QEW On-Ramp for Southbound traffic.

**Town Comment #3g**

Please provide a sensitivity analysis summarizing the impacts of closing Lyons Lane to future vehicular traffic to the surrounding intersections within the study area.

Addressed in updated TIS.

**Town Comment #3h**

Please ensure that there are proper identification on any site plan being use for turning movement plan. There are no dimensions or road name to identify the site.

Addressed in updated TIS.

**Town Comment #3i**

Provide the turning movement plans with visual of the whole site not in pieces.

Addressed in updated TIS.



## Region Comment Transportation Impact Study (Part 1)

A Transportation Impact Study was completed by Paradigm dated August 2021. **A study resubmission will be required** to address the issue outlined below.

The study states in section 2.4.2:

*“The traffic data collected for this project was obtained in 2017 and 2019. To quantify a reasonable factor that should be applied, historic 2015 traffic data at the Trafalgar Road and QEW Ramp terminals were also reviewed. Based on the growth comparison outlined herein, an average decrease of 3.5% has occurred between 2017 and 2019. As a result, no factors to the 2017 or 2019 turning movement counts were applied.”*

It should be noted, in a 2017 memo by Town staff on another area development file, it was stated:

*“Please note the former hospital site was closed on December 13, 2015 and the bridge work along Trafalgar Road has yet to be completed. MTO is completing the bridge work in phases and although was expected to be completed in December 2016, will now be completed by the summer of 2017. Currently all six (6) lanes of traffic are open, however, please confirm with MTO that the lanes will remain open at the time of any traffic counts.”*

Therefore, the decrease in traffic counts when comparing 2015 to 2017 was, in all likelihood, due to the MTO bridge works during the 2016-2017 time period. Therefore, the 3-4% decrease is explainable and, not applying growth rates to the 2017 and 2019 count data is unacceptable. **Town of Oakville comments from the previous submission also did not support a decrease in traffic volumes to be used in the study (MTO did not support this decrease as well).** The following strategy is recommended for a more accurate assessment:

- The Study terms of reference comments provided by Halton Region stated:

*“Halton’s transportation model predicts growth rates averaging two percent per annum, to the year 2031, on various sections of Trafalgar Road and the crossings streets.”*

Addressed in updated TIS. Historical counts have been factored with the use of 2% per annum growth rate.



## Region Comment Transportation Impact Study (Part 2)

The Ministry of Transportation (MTO) had previously reviewed the study and required several changes, and a study resubmission was necessary. While MTO will review and confirm their previous comments, it does not look like MTO concerns have been addressed. Some of the previous comments unresolved with this study submission

include:

- MTO data is showing the volume was not decreased. The other factor is population, total households, total vehicles, and average vehicle ownership of Town of Oakville is increasing over the years, as well as GTA, so the traffic volume should have increasing trend.
- For ramps, a v/c ratio for terminal ramp approaches with a value greater than 0.75 would be deemed critical and should be evaluated for possible operational improvements.
- Synchro simulation should be matching with existing roadway conditions such as lane configuration. For example, QEW WB off-ramp SBT, number of lanes of approaching and receiving is balanced, missing on-ramp leg.

To our understanding, the MTO had no further comments regarding the 2<sup>nd</sup> Submission. Regardless, these comments have been further addressed as part of the updated TIS:

- Historical counts have been factored with the use of 2% per annum growth rate
- As stated in the report, the MOEA identified that even with the proposed improvements implemented, capacity constraints over 0.75 will occur.
- Synchro lane Configuration has been updated to reflect existing conditions.



If you have any questions or would like to discuss our responses in more detail, please feel free to contact us.

Yours very truly,

**PARADIGM TRANSPORTATION SOLUTIONS LIMITED**



**Adam J. Makarewicz**  
Dipl.T., C.E.T. MITE  
Senior Project Manager



**Stew Elkins**  
B.E.S., MITE  
Vice President



# Appendix B

## Terms of Reference



**From:** [Gariscsak, Anne](#)  
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**Subject:** FW: 627 Lyons Lane, Town of Oakville -TIS Scope of Work- Halton Region Comments  
**Date:** December 11, 2018 9:01:43 AM  
**Attachments:** [image001.png](#)  
**Importance:** High

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Andrew,

Halton Region staff have reviewed the subject TIS scope of work and offer the following comments.

Data Requests:

-Data requests for Halton Region information (traffic counts/signal timing) should be completed via the data request process as noted on the Halton Region website <http://www.halton.ca/cms/One.aspx?portalId=8310&pageId=12457> or from the Town of Oakville. **If traffic data is older than 2 years, then updated turning movement counts are required.**

-Town of Oakville will provide area development details, as well as details related to planned town roadway improvements.

Transportation Impact Study Requirements (TIS):

-The following intersections have been proposed for the Transportation Impact Study area, and are acceptable:

- South Service Road at Lyons Lane;
- South Service Road at Argus Road;
- Cross Avenue at Lyons Lane;
- Cross Avenue at Argus Road / Oakville GO Station East Entrance;
- Trafalgar Road at Cross Avenue;
- Highway 403 at Eastbound Off-Ramp; and
- Highway 403 at Westbound Off-Ramp.

-The following intersections must also be included in the Transportation Impact Study area:

- Trafalgar Road and Cornwall Road
- Trafalgar Road and Argus Road

Analysis Assumptions:

-Horizon years: The proposed horizon years analysis is acceptable.

The TIS report shall include:

- Site Plan and Map,
- Size & Number of Development Phases,
- Existing Conditions (Study Area Intersections, Road Network, Pedestrian Routes, Cycling Routes, Transit Services),
- Existing Traffic Conditions (Site Operating Characteristics, Data Collection/Traffic Counts, Analysis Periods (5 years Ahead),
- Future Background Conditions (Horizon Years, Horizon Year Volumes)
- Background Traffic Demand (with TMC's < 2 years old),
- Background Traffic Demand Forecast (with acceptable growth rates)
- Site Generated Traffic (Transit Modal Split, Trip Generation/Distribution/Assignment)
- Future Total Traffic Demand,
- Capacity Analysis (by Intersection, with LOS, Avg. Delay, V/C ratios),
- Traffic Impacts (Tables – Total Traffic with/without Mitigation)
- Access Considerations – Existing, Proposed, Geometrics (turn lanes, sight lines),
- Recommendations,
- TDM recommendations (Transit, Pedestrian & Cycling Facilities Analysis)
- Conclusions

#### Background Traffic:

-Halton's transportation model predicts growth rates averaging two percent per annum, to the year 2031, on various sections of Trafalgar Road and the crossings streets.

-Growth rates for Town roads must be verified with the Town of Oakville.

-Proposed Transit mode splits, as well as TDM & Active Transportation assumptions must be provided to Halton Region & the Town of Oakville for review/approval, due to the proximity of the proposed site to a major transit station (Oakville GO Station).

#### Site Access:

-Study must be in accordance to Halton Region's Transportation Impact Guidelines

<http://www.halton.ca/cms/One.aspx?portalId=8310&pageId=12612>

-Identify required/recommended road improvements either as a result of the development impacts, or general non-development improvements.

Should you have any detailed technical questions with respect to Halton Region's comments, please contact Matt Krusto (cc'd) directly.

Thank you,  
Anne Garicsak.

**Anne Gariscsak, MCIP RPP**

**Planner**

Planning Services

Legislative & Planning Services

**Halton Region**

905-825-6000, ext. 7109 | 1-866-442-5866



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**From:** Krusto, Matt

**Sent:** Thursday, December 06, 2018 1:36 PM

**To:** Gariscsak, Anne

**Cc:** Natywary, Laurielle

**Subject:** FW: (180307) 627 Lyons Lane, Town of Oakville -TIS Scope of Work

Anne – please circulate through the one window for review.

Thanks!

Matt

**Matt Krusto**

**Transportation Planning Coordinator**

Infrastructure Planning & Policy

Public Works

**Halton Region**

905-825-6000, ext. 7225 | 1-866-442-5866



---

**From:** Andrew Evans [<mailto:aevans@ptsl.com>]



**Sent:** Thursday, December 06, 2018 1:12 PM

**To:** Syed Rizvi;; Jakaitis, Alicia; [Asia.Polus@ontario.ca](mailto:Asia.Polus@ontario.ca)

**Subject:** (180307) 627 Lyons Lane, Town of Oakville -TIS Scope of Work

Greetings,

Paradigm Transportation Solutions Limited have been retained to undertake a Transportation Impact Study (TIS) for a proposed residential development located at 627 Lyons Lane in the Town of Oakville. The developer is envisioning 220 residential units with vehicle access provided by a westerly driveway to Lyons Lane and a driveway to South Service Road.

As the development is in the permit control area of the Ministry of Transportation (MTO), the TIS will be completed following the MTO Traffic Impact Study Guidelines.

The study area will comprise of the following intersections:

- South Service Road at Lyons Lane;
- South Service Road at Argus Road;
- Cross Avenue at Lyons Lane;
- Cross Avenue at Argus Road;
- Trafalgar Road at Cross Avenue;
- Highway 403 at Eastbound Off-Ramp; and
- Highway 403 at Westbound Off-Ramp.

Traffic forecasts and analysis for the weekday AM and PM peak hours will be completed for three (3) horizon years:

- Opening Date;
- Five (5) years after Opening; and
- Ten (10) years after Opening.

Future Background Traffic will be estimated by applying a growth rate to existing traffic volumes and adding anticipated trips from nearby approved and in-stream developments.

- Growth rates and background developments will need to be confirmed by Town of Oakville.

Development generated trips will be forecast based on rates developed from the Institute of Transportation Engineers (ITE) Trip Generation Manual (10th Edition). These trips will be assigned to the study area intersections based on existing traffic patterns and/or Transportation Tomorrow Survey (TTS) data.

We will analyze the operation of the study area intersections for Existing, Future Background and Future Total traffic conditions for each horizon year and analysis period using Synchro software. Volume to capacity (v/c) ratios, Level of Service (LOS) and queuing will be assessed. We will identify any operational deficiencies as well as the net impact of the proposed development on the study area road network. The need for improvements (e.g. auxiliary turn lanes) and/or other mitigating measures (e.g. traffic control device modifications) to address deficiencies will be

determined. We will assess whether these measures are required due to non-site traffic (i.e. Existing or Future Background) or the increase in volumes resulting from the proposed development (i.e. Future Total).

If you have any questions or comments, please feel free to contact me.

Thank you and regards.

**Andrew Evans, M.Sc.**

*Transportation Planner*



**Paradigm Transportation Solutions Limited**

5A-150 Pinebush Road Cambridge ON N1R 8J8

p: 905.381.2229 x **305 <<< New Extension Number**

m: 519.497.3239

e: [aevans@ptsl.com](mailto:aevans@ptsl.com)

w: [www.ptsl.com](http://www.ptsl.com)

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**From:** [Polus, Asia \(MTO\)](#)  
**To:** [Andrew Evans](#)  
**Cc:** [Scholz, Kevin \(MTO\)](#); [Lawrence, Morgan \(MTO\)](#); [Syed Rizvi](#); [alicia.jakaitis@halton.ca](mailto:alicia.jakaitis@halton.ca); [Adam Makarewicz](#)  
**Subject:** RE: (180307) 627 Lyons Lane, Town of Oakville -TIS Scope of Work  
**Date:** December 21, 2018 10:14:47 AM  
**Attachments:** [image001.png](#)

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Hi Andrew,

Further to your request, please note that after additional review of your previously provided Scope of work the ministry's Traffic Office offers the following comments:

- The proponent can remove our previous requirement to analyst the QEW at Dorval/Kerr ramp terminals, however all other requirements are still applicable.

I trust that the above is clear, however if you have any questions please feel free to ask.

Best Regards

**W. Asia Polus**

*Corridor Management Planner*

Ministry of Transportation  
Central Region, Highway Corridor Management Section  
159 Sir William Hearst Ave. 7th Floor  
Toronto, ON M3M 0B7  
Tel. 416 - 235-3991  
Fax 416 - 235-4267

---

**From:** Polus, Asia (MTO)  
**Sent:** December-18-18 12:46 PM  
**To:** 'Andrew Evans' <aevans@ptsl.com>  
**Cc:** Scholz, Kevin (MTO) <Kevin.Scholz@ontario.ca>; Lawrence, Morgan (MTO) <Morgan.Lawrence@ontario.ca>; Syed Rizvi: <syed.rizvi@oakville.ca>; alicia.jakaitis@halton.ca; Adam Makarewicz <amakarewicz@ptsl.com>  
**Subject:** RE: (180307) 627 Lyons Lane, Town of Oakville -TIS Scope of Work

Hi Andrew,

Your request was sent to the ministry's traffic Office for review and I will contact you as soon as any comments are available.

Regards

**W. Asia Polus**

*Corridor Management Planner*

Ministry of Transportation  
Central Region, Highway Corridor Management Section

159 Sir William Hearst Ave. 7th Floor  
Toronto, ON M3M 0B7  
Tel. 416 - 235-3991  
Fax 416 - 235-4267

---

**From:** Andrew Evans [<mailto:aevans@ptsl.com>]  
**Sent:** December-18-18 10:49 AM  
**To:** Polus, Asia (MTO) <[Asia.Polus@ontario.ca](mailto:Asia.Polus@ontario.ca)>  
**Cc:** Scholz, Kevin (MTO) <[Kevin.Scholz@ontario.ca](mailto:Kevin.Scholz@ontario.ca)>; Lawrence, Morgan (MTO) <[Morgan.Lawrence@ontario.ca](mailto:Morgan.Lawrence@ontario.ca)>; Syed Rizvi: <[syed.rizvi@oakville.ca](mailto:syed.rizvi@oakville.ca)>; [alicia.jakaitis@halton.ca](mailto:alicia.jakaitis@halton.ca);  
Adam Makarewicz <[amakarewicz@ptsl.com](mailto:amakarewicz@ptsl.com)>  
**Subject:** RE: (180307) 627 Lyons Lane, Town of Oakville -TIS Scope of Work

Hi Asia,

Thank you for responding to our scope. The need to include the QEW/Dorval and QEW/Kerr ramp terminals does not seem to be warranted based on the expected distribution.

The proposed development is for 220 residential units which based on the Institute Transportation of Engineers Trip Generation Manual (10<sup>th</sup> Edition) will generate approximately 74 trips during the weekday AM peak hour and 83 trips during the weekday PM peak hour (not including any reductions for transit and active transportation modes).

Referring to the Midtown Oakville Class EA (2014) , this urban growth centre includes over 100 hectares of developable land and includes the subject development. The study area for this class EA did not include the QEW/Dorval and QEW/Kerr ramp terminal and the residential distribution as outlined in the Class EA does not project any trips to utilize these ramp terminals.

Trips generated by this development will utilize the Trafalgar Road ramp terminals (which are included in the study) rather than the Dorval/Kerr ramp terminals due to travel time and distances between the site and ramp terminals. Any trips that would likely use the Dorval/Kerr ramp terminals would be less than 1% of the hourly volumes at those intersections and not likely to significantly impact their operations.

Based on the above, these intersections will not experience a significant increase in traffic related to the proposed development. We respectfully request you review the following and reconsider the need for an expanded scope.

Thank you and regards.

**Andrew Evans, M.Sc.**  
*Transportation Planner*



## Paradigm Transportation Solutions Limited

p: 905.381.2229 x 305 <<< New Extension Number  
m: 519.497.3239

---

**From:** Polus, Asia (MTO) <[Asia.Polus@ontario.ca](mailto:Asia.Polus@ontario.ca)>

**Sent:** December 13, 2018 2:37 PM

**To:** Andrew Evans <[aevans@ptsl.com](mailto:aevans@ptsl.com)>; Syed Rizvi: <[syed.rizvi@oakville.ca](mailto:syed.rizvi@oakville.ca)>; [alicia.jakaitis@halton.ca](mailto:alicia.jakaitis@halton.ca)

**Cc:** Scholz, Kevin (MTO) <[Kevin.Scholz@ontario.ca](mailto:Kevin.Scholz@ontario.ca)>; Lawrence, Morgan (MTO) <[Morgan.Lawrence@ontario.ca](mailto:Morgan.Lawrence@ontario.ca)>

**Subject:** RE: (180307) 627 Lyons Lane, Town of Oakville -TIS Scope of Work

Hi Andrew,

Thank you for your e-mail, please note that your request was reviewed by the ministry's Traffic Office and the following requirements were provided:

- The Study must include the both ramp terminals at QEW/Dorval and QEW/Trafalgar intersections as well as the QEW WB ramp terminal at Kerr St.

Please note that any submission regarding this development proposal has to be sent through the Town of Oakville with the attention to Kevin Scholz, included in this e-mail, he is a new Corridor Management Officer for this area and he can be reached at 416-235-5383.

If you have any questions or require some clarification please do not hesitate to contact us.

Best Regards

**W. Asia Polus**

*Corridor Management Planner*

Ministry of Transportation  
Central Region, Highway Corridor Management Section  
159 Sir William Hearst Ave. 7th Floor  
Toronto, ON M3M 0B7  
Tel. 416 - 235-3991  
Fax 416 - 235-4267

---

**From:** Andrew Evans [<mailto:aevans@ptsl.com>]

**Sent:** December-06-18 1:12 PM

**To:** Syed Rizvi: <[syed.rizvi@oakville.ca](mailto:syed.rizvi@oakville.ca)>; [alicia.jakaitis@halton.ca](mailto:alicia.jakaitis@halton.ca); Polus, Asia (MTO) <[Asia.Polus@ontario.ca](mailto:Asia.Polus@ontario.ca)>

**Subject:** (180307) 627 Lyons Lane, Town of Oakville -TIS Scope of Work

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The study area will comprise of the following intersections:

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- South Service Road at Argus Road;
- Cross Avenue at Lyons Lane;
- Cross Avenue at Argus Road;
- Trafalgar Road at Cross Avenue;
- Highway 403 at Eastbound Off-Ramp; and
- Highway 403 at Westbound Off-Ramp.

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If you have any questions or comments, please feel free to contact me.

Thank you and regards.

**Andrew Evans, M.Sc.**

*Transportation Planner*



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# Appendix C

## Turning Movement Counts





# Trafalgar Rd @ Cornwall Rd

## Morning Peak Diagram

### Specified Period

**From:** 7:00:00

**To:** 9:00:00

### One Hour Peak

**From:** 8:00:00

**To:** 9:00:00

**Municipality:** Halton Region  
**Site #:** 0000003011  
**Intersection:** Trafalgar Rd & Cornwall Rd  
**TFR File #:** 4  
**Count date:** 1-Jun-2017

**Weather conditions:**  
Clear/Dry  
**Person(s) who counted:**  
Cam

**\*\* Signalized Intersection \*\***

**Major Road:** Trafalgar Rd runs N/S

North Leg Total: 2557  
 North Entering: 1337  
 North Peds: 25  
 Peds Cross:  $\times$

Heavys	9	6	19	34
Trucks	10	6	4	20
Cars	300	471	512	1283
<b>Totals</b>	<b>319</b>	<b>483</b>	<b>535</b>	



Heavys	14
Trucks	9
Cars	1197
<b>Totals</b>	<b>1220</b>

East Leg Total: 1995  
 East Entering: 953  
 East Peds: 18  
 Peds Cross:  $\times$

Heavys	Trucks	Cars	Totals
19	13	713	745

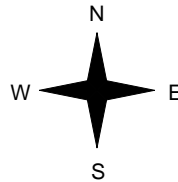


Trafalgar Rd

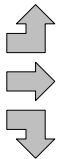
Cars	Trucks	Heavys	Totals
543	5	9	557
366	3	9	378
17	1	0	18
<b>926</b>	<b>9</b>	<b>18</b>	



Cornwall Rd



Heavys	Trucks	Cars	Totals
3	3	339	345
12	7	443	462
1	0	71	72
<b>16</b>	<b>10</b>	<b>853</b>	



Cornwall Rd



Cars	Trucks	Heavys	Totals
1000	11	31	1042

Peds Cross:  $\times$   
 West Peds: 9  
 West Entering: 879  
 West Leg Total: 1624

Cars	559	Cars	47	315	45	407
Trucks	7	Trucks	0	1	0	1
Heavys	7	Heavys	1	2	0	3
<b>Totals</b>	<b>573</b>	<b>Totals</b>	<b>48</b>	<b>318</b>	<b>45</b>	



Peds Cross:  $\times$   
 South Peds: 7  
 South Entering: 411  
 South Leg Total: 984

## Comments

# Trafalgar Rd @ Cornwall Rd

## Mid-day Peak Diagram

### Specified Period

**From:** 11:00:00  
**To:** 14:00:00

### One Hour Peak

**From:** 12:00:00  
**To:** 13:00:00

**Municipality:** Halton Region  
**Site #:** 0000003011  
**Intersection:** Trafalgar Rd & Cornwall Rd  
**TFR File #:** 4  
**Count date:** 1-Jun-2017

**Weather conditions:**  
Clear/Dry  
**Person(s) who counted:**  
Cam

**\*\* Signalized Intersection \*\***

**Major Road:** Trafalgar Rd runs N/S

North Leg Total: 2692  
North Entering: 1309  
North Peds: 24  
Peds Cross:  $\times$

Heavys	8	1	12	21
Trucks	10	3	6	19
Cars	186	516	567	1269
Totals	204	520	585	



Heavys	22
Trucks	23
Cars	1338
Totals	1383

East Leg Total: 2106  
East Entering: 1076  
East Peds: 13  
Peds Cross:  $\times$

Heavys	Trucks	Cars	Totals
21	17	576	614

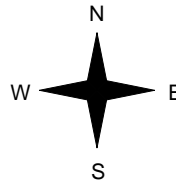


Trafalgar Rd

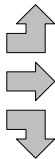
Cars	Trucks	Heavys	Totals
626	13	13	652
359	7	13	379
44	1	0	45
1029	21	26	



Cornwall Rd



Heavys	Trucks	Cars	Totals
5	6	324	335
6	5	368	379
0	2	37	39
11	13	729	



Cornwall Rd



Peds Cross:  $\times$   
West Peds: 6  
West Entering: 753  
West Leg Total: 1367

Cars	597	Cars	31	388	66	485
Trucks	6	Trucks	0	4	0	4
Heavys	1	Heavys	0	4	0	4
Totals	604	Totals	31	396	66	



Trafalgar Rd

Peds Cross:  $\times$   
South Peds: 8  
South Entering: 493  
South Leg Total: 1097

## Comments

# Trafalgar Rd @ Cornwall Rd

## Afternoon Peak Diagram

### Specified Period

**From:** 15:00:00

**To:** 18:00:00

### One Hour Peak

**From:** 17:00:00

**To:** 18:00:00

**Municipality:** Halton Region  
**Site #:** 0000003011  
**Intersection:** Trafalgar Rd & Cornwall Rd  
**TFR File #:** 4  
**Count date:** 1-Jun-2017

**Weather conditions:**  
Clear/Dry  
**Person(s) who counted:**  
Cam

**\*\* Signalized Intersection \*\***

**Major Road:** Trafalgar Rd runs N/S

North Leg Total: 2883  
 North Entering: 1483  
 North Peds: 21  
 Peds Cross:  $\times$

Heavys	6	0	11	17
Trucks	2	2	1	5
Cars	462	524	475	1461
<b>Totals</b>	<b>470</b>	<b>526</b>	<b>487</b>	



Heavys	8
Trucks	7
Cars	1385
<b>Totals</b>	<b>1400</b>

East Leg Total: 2246  
 East Entering: 1261  
 East Peds: 10  
 Peds Cross:  $\times$

Heavys	Trucks	Cars	Totals
11	5	1208	1224

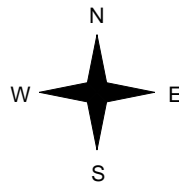


Trafalgar Rd

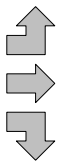
Cars	Trucks	Heavys	Totals
556	3	5	564
634	1	5	640
57	0	0	57
<b>1247</b>	<b>4</b>	<b>10</b>	



Cornwall Rd



Heavys	Trucks	Cars	Totals
3	2	424	429
6	6	447	459
1	0	161	162
<b>10</b>	<b>8</b>	<b>1032</b>	



Cornwall Rd



Peds Cross:  $\times$   
 West Peds: 17  
 West Entering: 1050  
 West Leg Total: 2274

Cars	742	Cars	112	405	39	556
Trucks	2	Trucks	2	2	0	4
Heavys	1	Heavys	0	0	0	0
<b>Totals</b>	<b>745</b>	<b>Totals</b>	<b>114</b>	<b>407</b>	<b>39</b>	



Trafalgar Rd

Peds Cross:  $\times$   
 South Peds: 14  
 South Entering: 560  
 South Leg Total: 1305

## Comments

# Trafalgar Rd @ Cornwall Rd

## Total Count Diagram

**Municipality:** Halton Region  
**Site #:** 0000003011  
**Intersection:** Trafalgar Rd & Cornwall Rd  
**TFR File #:** 4  
**Count date:** 1-Jun-2017

**Weather conditions:**  
 Clear/Dry  
**Person(s) who counted:**  
 Cam

**\*\* Signalized Intersection \*\***

**Major Road:** Trafalgar Rd runs N/S

North Leg Total: 20400  
 North Entering: 10198  
 North Peds: 147  
 Peds Cross:  $\times$

Heavys	58	29	112	199
Trucks	60	29	40	129
Cars	2277	3661	3932	9870
<b>Totals</b>	<b>2395</b>	<b>3719</b>	<b>4084</b>	



Heavys	146
Trucks	111
Cars	9945
<b>Totals</b>	<b>10202</b>

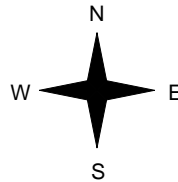
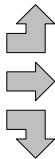
East Leg Total: 15937  
 East Entering: 8450  
 East Peds: 79  
 Peds Cross:  $\times$

Heavys	151	Trucks	98	Cars	6219	Totals	6468
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Cornwall Rd

Heavys	40	Trucks	36	Cars	2602	Totals	2678
	68		35		2939		3042
	5		9		490		504
<b>Totals</b>	<b>113</b>	<b>80</b>	<b>6031</b>				



Trafalgar Rd

Cars	4396	Trucks	51	Heavys	87	Totals	4534
	3495		36		87		3618
	293		4		1		298
<b>Totals</b>	<b>8184</b>	<b>91</b>	<b>175</b>				

Cornwall Rd



Cars	7227	Trucks	79	Heavys	181	Totals	7487
------	------	--------	----	--------	-----	--------	------

Peds Cross:  $\times$   
 West Peds: 79  
 West Entering: 6224  
 West Leg Total: 12692

Cars	4444
Trucks	42
Heavys	35
<b>Totals</b>	<b>4521</b>



Cars	447	2947	356	3750
Trucks	2	24	4	30
Heavys	6	19	1	26
<b>Totals</b>	<b>455</b>	<b>2990</b>	<b>361</b>	

Peds Cross:  $\times$   
 South Peds: 68  
 South Entering: 3806  
 South Leg Total: 8327

### Comments

# Trafalgar Rd @ South Service Rd

## Morning Peak Diagram

### Specified Period

**From:** 7:00:00

**To:** 9:00:00

### One Hour Peak

**From:** 8:00:00

**To:** 9:00:00

**Municipality:** Halton Region  
**Site #:** 0000003012  
**Intersection:** Trafalgar Rd & South Service Rd  
**TFR File #:** 5  
**Count date:** 1-Jun-2017

**Weather conditions:**  
Clear/Dry  
**Person(s) who counted:**  
Cam

**\*\* Signalized Intersection \*\***

**Major Road:** Trafalgar Rd runs N/S

North Leg Total: 3502  
 North Entering: 1907  
 North Peds: 1  
 Peds Cross:  $\bowtie$

Heavys	14	16	3	33
Trucks	2	21	5	28
Cars	377	1232	237	1846
<b>Totals</b>	<b>393</b>	<b>1269</b>	<b>245</b>	



Heavys	54
Trucks	15
Cars	1526
<b>Totals</b>	<b>1595</b>

East Leg Total: 607  
 East Entering: 234  
 East Peds: 52  
 Peds Cross:  $\bowtie$

Heavys	Trucks	Cars	Totals
28	5	547	580

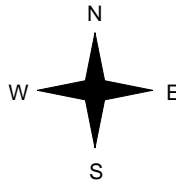


Trafalgar Rd

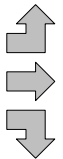
Cars	Trucks	Heavys	Totals
109	1	4	114
74	1	5	80
39	1	0	40
<b>222</b>	<b>3</b>	<b>9</b>	



Cross Ave



Heavys	Trucks	Cars	Totals
27	6	337	370
5	0	72	77
9	4	94	107
<b>41</b>	<b>10</b>	<b>503</b>	



South Service Rd



Cars	Trucks	Heavys	Totals
360	5	8	373

Peds Cross:  $\bowtie$   
 West Peds: 10  
 West Entering: 554  
 West Leg Total: 1134

Cars	1365	Cars	96	1080	51	1227
Trucks	26	Trucks	2	8	0	10
Heavys	25	Heavys	9	23	0	32
<b>Totals</b>	<b>1416</b>	<b>Totals</b>	<b>107</b>	<b>1111</b>	<b>51</b>	



Peds Cross:  $\bowtie$   
 South Peds: 4  
 South Entering: 1269  
 South Leg Total: 2685

## Comments

# Trafalgar Rd @ South Service Rd

## Mid-day Peak Diagram

### Specified Period

**From:** 11:00:00

**To:** 14:00:00

### One Hour Peak

**From:** 12:00:00

**To:** 13:00:00

**Municipality:** Halton Region  
**Site #:** 0000003012  
**Intersection:** Trafalgar Rd & South Service Rd  
**TFR File #:** 5  
**Count date:** 1-Jun-2017

**Weather conditions:**  
Clear/Dry  
**Person(s) who counted:**  
Cam

**\*\* Signalized Intersection \*\***

**Major Road:** Trafalgar Rd runs N/S

North Leg Total: 3320  
 North Entering: 1509  
 North Peds: 0  
 Peds Cross:  $\times$

Heavys	9	12	4	25
Trucks	6	21	2	29
Cars	232	1101	122	1455
<b>Totals</b>	<b>247</b>	<b>1134</b>	<b>128</b>	



Heavys	42
Trucks	25
Cars	1744
<b>Totals</b>	<b>1811</b>

East Leg Total: 594  
 East Entering: 336  
 East Peds: 8  
 Peds Cross:  $\times$

Heavys	Trucks	Cars	Totals
17	8	450	475



Trafalgar Rd

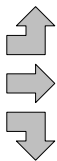
Cars	Trucks	Heavys	Totals
151	3	6	160
89	0	2	91
80	2	3	85
<b>320</b>	<b>5</b>	<b>11</b>	



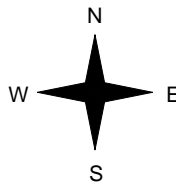
South Service Rd



Heavys	Trucks	Cars	Totals
21	2	407	430
2	2	70	74
6	0	88	94
<b>29</b>	<b>4</b>	<b>565</b>	



Cross Ave



Trafalgar Rd



Cars	Trucks	Heavys	Totals
246	6	6	258

Peds Cross:  $\times$   
 West Peds: 20  
 West Entering: 598  
 West Leg Total: 1073

Cars	1269
Trucks	23
Heavys	21
<b>Totals</b>	<b>1313</b>



Cars	129	1186	54	1369
Trucks	2	20	2	24
Heavys	6	15	0	21
<b>Totals</b>	<b>137</b>	<b>1221</b>	<b>56</b>	

Peds Cross:  $\times$   
 South Peds: 28  
 South Entering: 1414  
 South Leg Total: 2727

## Comments

# Trafalgar Rd @ South Service Rd

## Afternoon Peak Diagram

### Specified Period

**From:** 15:00:00

**To:** 18:00:00

### One Hour Peak

**From:** 17:00:00

**To:** 18:00:00

**Municipality:** Halton Region  
**Site #:** 0000003012  
**Intersection:** Trafalgar Rd & South Service Rd  
**TFR File #:** 5  
**Count date:** 1-Jun-2017

**Weather conditions:**  
Clear/Dry  
**Person(s) who counted:**  
Cam

**\*\* Signalized Intersection \*\***

**Major Road:** Trafalgar Rd runs N/S

North Leg Total: 4034  
 North Entering: 1668  
 North Peds: 0  
 Peds Cross:  $\times$

Heavys	9	5	0	14
Trucks	0	4	0	4
Cars	281	1250	119	1650
<b>Totals</b>	<b>290</b>	<b>1259</b>	<b>119</b>	



Heavys	34
Trucks	9
Cars	2323
<b>Totals</b>	<b>2366</b>

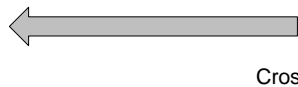
East Leg Total: 749  
 East Entering: 515  
 East Peds: 70  
 Peds Cross:  $\times$

Heavys	Trucks	Cars	Totals
20	0	516	536

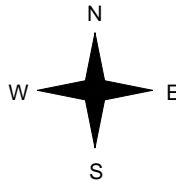


Trafalgar Rd

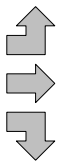
Cars	Trucks	Heavys	Totals
246	1	1	248
125	0	5	130
134	1	2	137
<b>505</b>	<b>2</b>	<b>8</b>	



Cross Ave



Heavys	Trucks	Cars	Totals
28	1	837	866
6	0	72	78
11	0	109	120
<b>45</b>	<b>1</b>	<b>1018</b>	



South Service Rd



Cars	Trucks	Heavys	Totals
228	0	6	234



Trafalgar Rd

Peds Cross:  $\times$   
 West Peds: 18  
 West Entering: 1064  
 West Leg Total: 1600

Cars	1493
Trucks	5
Heavys	18
<b>Totals</b>	<b>1516</b>



Cars	110	1240	37	1387
Trucks	0	7	0	7
Heavys	6	5	0	11
<b>Totals</b>	<b>116</b>	<b>1252</b>	<b>37</b>	

Peds Cross:  $\times$   
 South Peds: 15  
 South Entering: 1405  
 South Leg Total: 2921

## Comments

# Trafalgar Rd @ South Service Rd

## Total Count Diagram

**Municipality:** Halton Region  
**Site #:** 0000003012  
**Intersection:** Trafalgar Rd & South Service Rd  
**TFR File #:** 5  
**Count date:** 1-Jun-2017

**Weather conditions:**  
 Clear/Dry  
**Person(s) who counted:**  
 Cam

**\*\* Signalized Intersection \*\***

**Major Road:** Trafalgar Rd runs N/S

North Leg Total: 27051  
 North Entering: 12565  
 North Peds: 1  
 Peds Cross:  $\times$

Heavys	93	116	17	226
Trucks	14	124	15	153
Cars	2294	8870	1022	12186
<b>Totals</b>	<b>2401</b>	<b>9110</b>	<b>1054</b>	



Heavys	349
Trucks	148
Cars	13989
<b>Totals</b>	<b>14486</b>

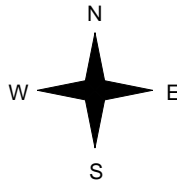
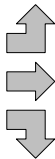
East Leg Total: 4526  
 East Entering: 2674  
 East Peds: 256  
 Peds Cross:  $\times$

Heavys	Trucks	Cars	Totals
167	27	3820	4014



Cross Ave

Heavys	Trucks	Cars	Totals
197	24	3662	3883
21	8	474	503
59	7	728	794
<b>277</b>	<b>39</b>	<b>4864</b>	



Trafalgar Rd

Cars	Trucks	Heavys	Totals
1272	20	34	1326
736	3	18	757
569	8	14	591
<b>2577</b>	<b>31</b>	<b>66</b>	



South Service Rd



Cars	Trucks	Heavys	Totals
1776	32	44	1852

Peds Cross:  $\times$   
 West Peds: 102  
 West Entering: 5180  
 West Leg Total: 9194

Cars	10167
Trucks	139
Heavys	189
<b>Totals</b>	<b>10495</b>



Cars	790	9055	280	10125
Trucks	10	104	9	123
Heavys	56	118	6	180
<b>Totals</b>	<b>856</b>	<b>9277</b>	<b>295</b>	

Peds Cross:  $\times$   
 South Peds: 83  
 South Entering: 10428  
 South Leg Total: 20923

### Comments



# Trafalgar Rd @ Argus Rd

## Morning Peak Diagram

### Specified Period

**From:** 7:00:00

**To:** 9:00:00

### One Hour Peak

**From:** 7:45:00

**To:** 8:45:00

**Municipality:** Halton Region  
**Site #:** 0000003013  
**Intersection:** Trafalgar Rd & Argus Rd  
**TFR File #:** 2  
**Count date:** 15-May-2017

**Weather conditions:**  
Sunny/Dry  
**Person(s) who counted:**  
Bronek  
Radek

**\*\* Non-Signalized Intersection \*\***

**Major Road:** Trafalgar Rd runs N/S

North Leg Total: 3496  
 North Entering: 2579  
 North Peds: 0  
 Peds Cross:  $\nabla$

Heavys	10	39	49
Trucks	3	46	49
Cars	633	1848	2481
<b>Totals</b>	<b>646</b>	<b>1933</b>	



Heavys	42
Trucks	26
Cars	849
<b>Totals</b>	<b>917</b>

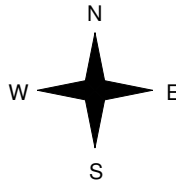
Heavys	Trucks	Cars	Totals
10	3	633	646



Trafalgar Rd



Argus Rd



Heavys	Trucks	Cars	Totals
0	0	0	0
0	1	14	15
0	1	14	



Trafalgar Rd



Peds Cross:  $\nabla$   
 West Peds: 11  
 West Entering: 15  
 West Leg Total: 661

Cars	1862
Trucks	47
Heavys	39
<b>Totals</b>	<b>1948</b>



Cars	0	849	849
Trucks	0	26	26
Heavys	0	42	42
<b>Totals</b>	<b>0</b>	<b>917</b>	

Peds Cross:  $\nabla$   
 South Peds: 0  
 South Entering: 917  
 South Leg Total: 2865

## Comments

# Trafalgar Rd @ Argus Rd

## Mid-day Peak Diagram

### Specified Period

**From:** 11:00:00

**To:** 14:00:00

### One Hour Peak

**From:** 12:15:00

**To:** 13:15:00

**Municipality:** Halton Region  
**Site #:** 0000003013  
**Intersection:** Trafalgar Rd & Argus Rd  
**TFR File #:** 2  
**Count date:** 15-May-2017

**Weather conditions:**  
Sunny/Dry  
**Person(s) who counted:**  
Bronek  
Radek

**\*\* Non-Signalized Intersection \*\***

**Major Road:** Trafalgar Rd runs N/S

North Leg Total: 2892  
 North Entering: 1647  
 North Peds: 0  
 Peds Cross:  $\nabla$

Heavys	11	30	41
Trucks	5	32	37
Cars	279	1290	1569
<b>Totals</b>	<b>295</b>	<b>1352</b>	



Heavys	29
Trucks	22
Cars	1194
<b>Totals</b>	<b>1245</b>

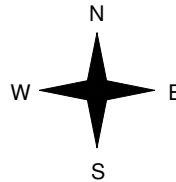
Heavys	Trucks	Cars	Totals
11	5	279	295



Trafalgar Rd



Argus Rd



Heavys	Trucks	Cars	Totals
0	0	0	0
1	2	35	38
1	2	35	



Trafalgar Rd



Peds Cross:  $\nabla$   
 West Peds: 9  
 West Entering: 38  
 West Leg Total: 333

Cars	1325
Trucks	34
Heavys	31
<b>Totals</b>	<b>1390</b>



Cars	0	1194	1194
Trucks	0	22	22
Heavys	0	29	29
<b>Totals</b>	<b>0</b>	<b>1245</b>	

Peds Cross:  $\nabla$   
 South Peds: 0  
 South Entering: 1245  
 South Leg Total: 2635

## Comments

# Trafalgar Rd @ Argus Rd

## Afternoon Peak Diagram

### Specified Period

**From:** 15:00:00

**To:** 18:00:00

### One Hour Peak

**From:** 17:00:00

**To:** 18:00:00

**Municipality:** Halton Region  
**Site #:** 0000003013  
**Intersection:** Trafalgar Rd & Argus Rd  
**TFR File #:** 2  
**Count date:** 15-May-2017

**Weather conditions:**  
Sunny/Dry  
**Person(s) who counted:**  
Bronek  
Radek

**\*\* Non-Signalized Intersection \*\***

**Major Road:** Trafalgar Rd runs N/S

North Leg Total: 3579  
 North Entering: 1674  
 North Peds: 0  
 Peds Cross:  $\nabla$

Heavys	9	21	30
Trucks	1	8	9
Cars	330	1305	1635
<b>Totals</b>	<b>340</b>	<b>1334</b>	



Heavys	29
Trucks	8
Cars	1868
<b>Totals</b>	<b>1905</b>

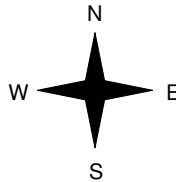
Heavys	Trucks	Cars	Totals
9	1	330	340



Trafalgar Rd



Argus Rd



Heavys	Trucks	Cars	Totals
0	0	0	0
2	1	35	38
2	1	35	



Trafalgar Rd



Peds Cross:  $\nabla$   
 West Peds: 24  
 West Entering: 38  
 West Leg Total: 378

Cars	1340
Trucks	9
Heavys	23
<b>Totals</b>	<b>1372</b>



Cars	0	1868	1868
Trucks	0	8	8
Heavys	0	29	29
<b>Totals</b>	<b>0</b>	<b>1905</b>	

Peds Cross:  $\nabla$   
 South Peds: 0  
 South Entering: 1905  
 South Leg Total: 3277

## Comments

# Trafalgar Rd @ Argus Rd

## Total Count Diagram

**Municipality:** Halton Region  
**Site #:** 0000003013  
**Intersection:** Trafalgar Rd & Argus Rd  
**TFR File #:** 2  
**Count date:** 15-May-2017

**Weather conditions:**  
 Sunny/Dry  
**Person(s) who counted:**  
 Bronek  
 Radek

**\*\* Non-Signalized Intersection \*\***

**Major Road:** Trafalgar Rd runs N/S

North Leg Total: 24276  
 North Entering: 14428  
 North Peds: 0  
 Peds Cross:  $\nabla$

Heavys	85	240	325
Trucks	21	245	266
Cars	3022	10815	13837
<b>Totals</b>	<b>3128</b>	<b>11300</b>	



Heavys	267
Trucks	169
Cars	9412
<b>Totals</b>	<b>9848</b>

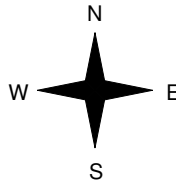
Heavys	Trucks	Cars	Totals
85	21	3022	3128



Trafalgar Rd



Argus Rd



Heavys	Trucks	Cars	Totals
0	0	0	0
6	8	215	229
6	8	215	



Trafalgar Rd



Peds Cross:  $\nabla$   
 West Peds: 99  
 West Entering: 229  
 West Leg Total: 3357

Cars	11030
Trucks	253
Heavys	246
<b>Totals</b>	<b>11529</b>



Cars	0	9412	9412
Trucks	0	169	169
Heavys	0	267	267
<b>Totals</b>	<b>0</b>	<b>9848</b>	

Peds Cross:  $\nabla$   
 South Peds: 0  
 South Entering: 9848  
 South Leg Total: 21377

### Comments

# Trafalgar Rd @ QEW EB Off Ramp

## Morning Peak Diagram

### Specified Period

**From:** 7:00:00

**To:** 9:00:00

### One Hour Peak

**From:** 7:45:00

**To:** 8:45:00

**Municipality:** Halton Region  
**Site #:** 0000003014  
**Intersection:** Trafalgar Rd & QEW EB Off Ramp  
**TFR File #:** 6  
**Count date:** 1-Jun-2017

**Weather conditions:**  
Clear/Dry  
**Person(s) who counted:**  
Cam

**\*\* Signalized Intersection \*\***

**Major Road:** Trafalgar Rd runs N/S

North Leg Total: 3931

North Entering: 2046

North Peds: 0

Peds Cross:  $\nabla$

Heavys	0	63	63
Trucks	0	17	17
Cars	0	1966	1966
Totals	0	2046	



Heavys	68
Trucks	21
Cars	1796
Totals	1885

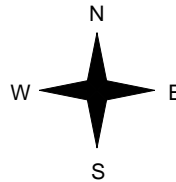
Heavys	Trucks	Cars	Totals
0	0	0	0



Trafalgar Rd



QEW EB Off Ramp



Heavys	Trucks	Cars	Totals
12	11	799	822
6	2	591	599
18	13	1390	



Trafalgar Rd



Peds Cross:  $\nabla$   
 West Peds: 6  
 West Entering: 1421  
 West Leg Total: 1421

Cars	2557
Trucks	19
Heavys	69
Totals	2645



Cars	0	997	997
Trucks	0	10	10
Heavys	0	56	56
Totals	0	1063	

Peds Cross:  $\nabla$   
 South Peds: 0  
 South Entering: 1063  
 South Leg Total: 3708

## Comments

# Trafalgar Rd @ QEW EB Off Ramp

## Mid-day Peak Diagram

### Specified Period

**From:** 11:00:00

**To:** 14:00:00

### One Hour Peak

**From:** 11:30:00

**To:** 12:30:00

**Municipality:** Halton Region  
**Site #:** 0000003014  
**Intersection:** Trafalgar Rd & QEW EB Off Ramp  
**TFR File #:** 6  
**Count date:** 1-Jun-2017

**Weather conditions:**  
Clear/Dry  
**Person(s) who counted:**  
Cam

**\*\* Signalized Intersection \*\***

**Major Road:** Trafalgar Rd runs N/S

North Leg Total: 3410  
 North Entering: 1392  
 North Peds: 0  
 Peds Cross:  $\nabla$

Heavys	0	32	32
Trucks	0	13	13
Cars	0	1347	1347
Totals	0	1392	



Heavys	56
Trucks	25
Cars	1937
Totals	2018

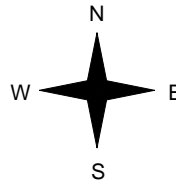
Heavys	Trucks	Cars	Totals
0	0	0	0



Trafalgar Rd



QEW EB Off Ramp



Heavys	Trucks	Cars	Totals
21	8	644	673
6	9	421	436
27	17	1065	



Trafalgar Rd



Peds Cross:  $\nabla$   
 West Peds: 11  
 West Entering: 1109  
 West Leg Total: 1109

Cars	1768
Trucks	22
Heavys	38
Totals	1828



Cars	0	1293	1293
Trucks	0	17	17
Heavys	0	35	35
Totals	0	1345	

Peds Cross:  $\nabla$   
 South Peds: 2  
 South Entering: 1345  
 South Leg Total: 3173

## Comments

# Trafalgar Rd @ QEW EB Off Ramp

## Afternoon Peak Diagram

### Specified Period

**From:** 15:00:00

**To:** 18:00:00

### One Hour Peak

**From:** 17:00:00

**To:** 18:00:00

**Municipality:** Halton Region  
**Site #:** 0000003014  
**Intersection:** Trafalgar Rd & QEW EB Off Ramp  
**TFR File #:** 6  
**Count date:** 1-Jun-2017

**Weather conditions:**  
Clear/Dry  
**Person(s) who counted:**  
Cam

**\*\* Signalized Intersection \*\***

**Major Road:** Trafalgar Rd runs N/S

North Leg Total: 4360  
 North Entering: 1485  
 North Peds: 0  
 Peds Cross:  $\nabla$

Heavys	0	26	26
Trucks	0	5	5
Cars	0	1454	1454
Totals	0	1485	



Heavys	34
Trucks	6
Cars	2835
Totals	2875

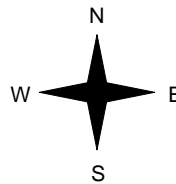
Heavys	Trucks	Cars	Totals
0	0	0	0



Trafalgar Rd



QEW EB Off Ramp



Heavys	Trucks	Cars	Totals
1	2	888	891
5	0	414	419
6	2	1302	



Trafalgar Rd



Peds Cross:  $\nabla$   
 West Peds: 17  
 West Entering: 1310  
 West Leg Total: 1310

Cars	1868
Trucks	5
Heavys	31
Totals	1904



Cars	0	1947	1947
Trucks	0	4	4
Heavys	0	33	33
Totals	0	1984	

Peds Cross:  $\nabla$   
 South Peds: 2  
 South Entering: 1984  
 South Leg Total: 3888

## Comments

# Trafalgar Rd @ QEW EB Off Ramp

## Total Count Diagram

**Municipality:** Halton Region  
**Site #:** 0000003014  
**Intersection:** Trafalgar Rd & QEW EB Off Ramp  
**TFR File #:** 6  
**Count date:** 1-Jun-2017

**Weather conditions:**  
 Clear/Dry  
**Person(s) who counted:**  
 Cam

**\*\* Signalized Intersection \*\***

**Major Road:** Trafalgar Rd runs N/S

North Leg Total: 28637  
 North Entering: 12310  
 North Peds: 0  
 Peds Cross:  $\nabla$

Heavys	0	296
Trucks	0	118
Cars	0	11896
Totals	0	12310

296  
118  
11896



Heavys	411
Trucks	143
Cars	15773
Totals	16327

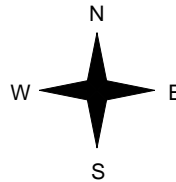
Heavys	Trucks	Cars	Totals
0	0	0	0



Trafalgar Rd



QEW EB Off Ramp



Heavys	Trucks	Cars	Totals
114	48	5398	5560
59	36	3438	3533
173	84	8836	



Trafalgar Rd



Peds Cross:  $\nabla$   
 West Peds: 87  
 West Entering: 9093  
 West Leg Total: 9093

Cars	15334
Trucks	154
Heavys	355
Totals	15843



Cars	0	10375	10375
Trucks	0	95	95
Heavys	0	297	297
Totals	0	10767	

Peds Cross:  $\nabla$   
 South Peds: 7  
 South Entering: 10767  
 South Leg Total: 26610

### Comments



# Trafalgar Rd @ North Service Rd E

## Morning Peak Diagram

### Specified Period

**From:** 7:00:00

**To:** 9:00:00

### One Hour Peak

**From:** 7:45:00

**To:** 8:45:00

**Municipality:** Halton Region  
**Site #:** 0000003015  
**Intersection:** Trafalgar Rd & North Service Rd E  
**TFR File #:** 7  
**Count date:** 1-Jun-2017

**Weather conditions:**  
Clear/Dry  
**Person(s) who counted:**  
Cam

**\*\* Signalized Intersection \*\***

**Major Road:** Trafalgar Rd runs N/S

North Leg Total: 4937  
 North Entering: 3090  
 North Peds: 0  
 Peds Cross:  $\times$

Heavys	0	77	0	77
Trucks	0	18	0	18
Cars	5	2990	0	2995
Totals	5	3085	0	



Heavys	59
Trucks	30
Cars	1758
Totals	1847

East Leg Total: 896  
 East Entering: 620  
 East Peds: 5  
 Peds Cross:  $\times$

Heavys	Trucks	Cars	Totals
3	4	28	35

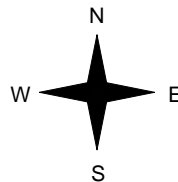


Trafalgar Rd

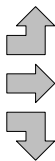
Cars	Trucks	Heavys	Totals
213	7	6	226
23	4	3	30
344	12	8	364
580	23	17	



North Service Rd E



Heavys	Trucks	Cars	Totals
0	0	1	1
0	0	0	0
3	2	176	181
3	2	177	



QEW WB On/Off Ramp



Cars	Trucks	Heavys	Totals
261	3	12	276

Trafalgar Rd



Peds Cross:  $\times$   
 West Peds: 8  
 West Entering: 182  
 West Leg Total: 217

Cars	3510
Trucks	32
Heavys	88
Totals	3630



Cars	0	1544	261	1805
Trucks	0	23	3	26
Heavys	0	53	12	65
Totals	0	1620	276	

Peds Cross:  $\times$   
 South Peds: 0  
 South Entering: 1896  
 South Leg Total: 5526

## Comments

# Trafalgar Rd @ North Service Rd E

## Mid-day Peak Diagram

### Specified Period

**From:** 11:00:00

**To:** 14:00:00

### One Hour Peak

**From:** 12:00:00

**To:** 13:00:00

**Municipality:** Halton Region  
**Site #:** 0000003015  
**Intersection:** Trafalgar Rd & North Service Rd E  
**TFR File #:** 7  
**Count date:** 1-Jun-2017

**Weather conditions:**  
 Clear/Dry  
**Person(s) who counted:**  
 Cam

**\*\* Signalized Intersection \*\***

**Major Road:** Trafalgar Rd runs N/S

North Leg Total: 3903  
 North Entering: 1954  
 North Peds: 3  
 Peds Cross:  $\bowtie$

Heavys	0	62	0	62
Trucks	0	27	0	27
Cars	10	1855	0	1865
<b>Totals</b>	<b>10</b>	<b>1944</b>	<b>0</b>	



Heavys	64
Trucks	63
Cars	1822
<b>Totals</b>	<b>1949</b>

East Leg Total: 1092  
 East Entering: 742  
 East Peds: 9  
 Peds Cross:  $\bowtie$

Heavys	0
Trucks	2
Cars	81
<b>Totals</b>	<b>83</b>

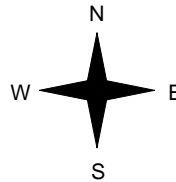


Trafalgar Rd

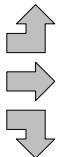
Cars	230	Trucks	16	Heavys	18	<b>Totals</b>	<b>264</b>
	71		2		0		73
	387		13		5		405
<b>Totals</b>	<b>688</b>		<b>31</b>		<b>23</b>		



North Service Rd E



Heavys	1	Trucks	1	Cars	24	<b>Totals</b>	<b>26</b>
	0		0		0		0
	4		3		214		221
<b>Totals</b>	<b>5</b>		<b>4</b>		<b>238</b>		



QEW WB On/Off Ramp



Cars	331	Trucks	11	Heavys	8	<b>Totals</b>	<b>350</b>
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Trafalgar Rd



Peds Cross:  $\bowtie$   
 West Peds: 15  
 West Entering: 247  
 West Leg Total: 330

Cars	2456
Trucks	43
Heavys	71
<b>Totals</b>	<b>2570</b>



Cars	0	1568	331	1899
Trucks	0	46	11	57
Heavys	0	45	8	53
<b>Totals</b>	<b>0</b>	<b>1659</b>	<b>350</b>	

Peds Cross:  $\bowtie$   
 South Peds: 3  
 South Entering: 2009  
 South Leg Total: 4579

## Comments

# Trafalgar Rd @ North Service Rd E

## Afternoon Peak Diagram

### Specified Period

**From:** 15:00:00  
**To:** 18:00:00

### One Hour Peak

**From:** 17:00:00  
**To:** 18:00:00

**Municipality:** Halton Region  
**Site #:** 0000003015  
**Intersection:** Trafalgar Rd & North Service Rd E  
**TFR File #:** 7  
**Count date:** 1-Jun-2017

**Weather conditions:**  
Clear/Dry  
**Person(s) who counted:**  
Cam

**\*\* Signalized Intersection \*\***

**Major Road:** Trafalgar Rd runs N/S

North Leg Total: 5024  
North Entering: 2280  
North Peds: 2  
Peds Cross:  $\times$

Heavys	0	37	0	37
Trucks	0	13	0	13
Cars	10	2220	0	2230
Totals	10	2270	0	



Heavys	35
Trucks	10
Cars	2699
Totals	2744

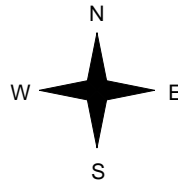
East Leg Total: 1173  
East Entering: 857  
East Peds: 14  
Peds Cross:  $\times$

Heavys	0	Trucks	1	Cars	126	Totals	127
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North Service Rd E

Heavys	0	Trucks	0	Cars	17	Totals	17
	0		0		0		0
	1		0		223		224
	1		0		240		



Trafalgar Rd



Cars	321	Trucks	2	Heavys	3	Totals	326
	116		1		0		117
	407		2		5		414
	844		5		8		

QEW WB On/Off Ramp



Cars	313	Trucks	1	Heavys	2	Totals	316
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Peds Cross:  $\times$   
West Peds: 14  
West Entering: 241  
West Leg Total: 368

Cars	2850
Trucks	15
Heavys	43
Totals	2908



Cars	0	2361	313	2674
Trucks	0	8	1	9
Heavys	0	32	2	34
Totals	0	2401	316	

Peds Cross:  $\times$   
South Peds: 0  
South Entering: 2717  
South Leg Total: 5625

## Comments

# Trafalgar Rd @ North Service Rd E

## Total Count Diagram

**Municipality:** Halton Region  
**Site #:** 0000003015  
**Intersection:** Trafalgar Rd & North Service Rd E  
**TFR File #:** 7  
**Count date:** 1-Jun-2017

**Weather conditions:**  
 Clear/Dry  
**Person(s) who counted:**  
 Cam

**\*\* Signalized Intersection \*\***

**Major Road:** Trafalgar Rd runs N/S

North Leg Total: 33764  
 North Entering: 17883  
 North Peds: 9  
 Peds Cross:  $\times$

Heavys	1	476	0	477
Trucks	0	166	0	166
Cars	63	17177	0	17240
<b>Totals</b>	<b>64</b>	<b>17819</b>	<b>0</b>	



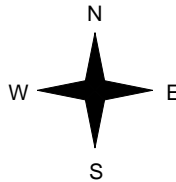
Heavys	458
Trucks	232
Cars	15191
<b>Totals</b>	<b>15881</b>

East Leg Total: 8515  
 East Entering: 5956  
 East Peds: 67  
 Peds Cross:  $\times$

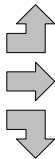
Heavys	Trucks	Cars	Totals
9	11	557	577



North Service Rd E



Heavys	Trucks	Cars	Totals
1	2	129	132
0	0	0	0
20	22	1547	1589
<b>21</b>	<b>24</b>	<b>1676</b>	



Trafalgar Rd

Cars	Trucks	Heavys	Totals
2172	61	101	2334
494	11	8	513
3005	62	42	3109
<b>5671</b>	<b>134</b>	<b>151</b>	

QEW WB On/Off Ramp



Cars	Trucks	Heavys	Totals
2456	46	57	2559

Peds Cross:  $\times$   
 West Peds: 82  
 West Entering: 1721  
 West Leg Total: 2298

Cars	21729	Cars	0	12890	2456	15346
Trucks	250	Trucks	0	169	46	215
Heavys	538	Heavys	0	356	57	413
<b>Totals</b>	<b>22517</b>	<b>Totals</b>	<b>0</b>	<b>13415</b>	<b>2559</b>	



Peds Cross:  $\times$   
 South Peds: 4  
 South Entering: 15974  
 South Leg Total: 38491

### Comments



Paradigm Transportation Solutions Limited  
22 King Street South, Suite 300

Waterloo, Ontario, Canada N2J 1N8  
519-896-3163 cbowness@ptsl.com

Count Name: Argus Road & South Service Road  
Site Code:  
Start Date: 01/10/2019  
Page No: 1

### Turning Movement Data

Start Time	Argus Road Eastbound						Argus Road Westbound						Northbound Approach Northbound						South Service Road Southbound						Int. Total
	Left	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	Right	U-Turn	Peds	App. Total	
7:00 AM	0	1	0	0	1	1	1	102	17	0	4	120	1	0	0	0	2	1	0	0	0	1	0	122	
7:15 AM	0	0	0	0	0	0	0	128	17	0	2	145	0	0	0	0	1	0	0	0	0	0	0	145	
7:30 AM	1	1	1	0	0	3	0	127	25	0	1	152	0	0	0	0	1	0	1	0	0	0	1	156	
7:45 AM	0	0	0	0	0	0	2	157	28	0	2	187	0	1	0	0	2	1	0	0	1	0	1	189	
Hourly Total	1	2	1	0	1	4	3	514	87	0	9	604	1	1	0	0	6	2	1	0	1	0	2	612	
8:00 AM	0	1	1	0	0	2	1	130	21	0	1	152	0	0	0	0	2	0	2	2	2	1	0	7	161
8:15 AM	0	3	0	0	1	3	1	125	39	0	1	165	1	0	0	0	1	1	1	0	0	0	0	1	170
8:30 AM	2	2	0	0	0	4	0	60	27	0	0	87	1	1	0	0	1	2	0	1	2	0	0	3	96
8:45 AM	2	2	0	0	0	4	0	85	30	0	1	115	0	1	0	0	0	1	1	2	0	0	0	3	123
Hourly Total	4	8	1	0	1	13	2	400	117	0	3	519	2	2	0	0	4	4	4	5	4	1	0	14	550
*** BREAK ***	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
11:00 AM	2	5	1	1	0	9	0	30	18	0	0	48	1	3	0	0	1	4	4	3	3	1	0	11	72
11:15 AM	2	1	0	0	0	3	1	33	19	0	0	53	0	2	1	0	0	3	2	2	8	0	0	12	71
11:30 AM	4	4	0	0	0	8	1	32	25	0	3	58	0	1	0	0	2	1	2	1	3	0	0	6	73
11:45 AM	1	0	0	0	0	1	0	38	20	0	1	58	1	4	0	0	0	5	2	4	2	0	0	8	72
Hourly Total	9	10	1	1	0	21	2	133	82	0	4	217	2	10	1	0	3	13	10	10	16	1	0	37	288
12:00 PM	1	4	1	0	0	6	1	22	24	1	1	48	1	3	0	0	0	4	3	3	5	0	0	11	69
12:15 PM	0	0	0	0	0	0	1	32	15	0	1	48	0	1	0	0	0	1	2	3	3	0	0	8	57
12:30 PM	0	2	0	1	0	3	2	30	20	0	2	52	0	0	1	0	0	1	2	0	5	0	0	7	63
12:45 PM	1	6	1	0	0	8	0	37	20	0	0	57	0	1	0	0	0	1	1	0	3	0	0	4	70
Hourly Total	2	12	2	1	0	17	4	121	79	1	4	205	1	5	1	0	0	7	8	6	16	0	0	30	259
1:00 PM	1	2	0	0	0	3	0	32	24	0	2	56	0	0	0	0	3	0	0	0	5	0	0	5	64
1:15 PM	1	3	1	0	0	5	0	44	10	0	0	54	0	1	0	0	0	1	3	0	4	0	0	7	67
1:30 PM	1	2	0	1	0	4	0	30	27	0	4	57	1	0	0	0	5	1	2	1	2	0	0	5	67
1:45 PM	1	3	1	0	0	5	0	38	17	0	2	55	0	2	0	0	0	2	1	2	2	0	0	5	67
Hourly Total	4	10	2	1	0	17	0	144	78	0	8	222	1	3	0	0	8	4	6	3	13	0	0	22	265
*** BREAK ***	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
3:00 PM	5	3	0	0	1	8	0	36	16	0	4	52	1	0	0	0	5	1	3	2	5	0	0	10	71
3:15 PM	1	2	0	0	0	3	0	31	18	0	0	49	0	1	0	0	2	1	1	1	1	0	0	3	56
3:30 PM	1	7	1	0	0	9	0	31	6	0	1	37	0	2	1	0	3	3	4	2	4	0	0	10	59
3:45 PM	1	5	0	0	0	6	1	36	12	0	1	49	0	1	0	0	1	1	2	1	5	1	0	9	65
Hourly Total	8	17	1	0	1	26	1	134	52	0	6	187	1	4	1	0	11	6	10	6	15	1	0	32	251
4:00 PM	5	1	0	1	0	7	0	44	12	0	3	56	1	3	0	0	1	4	4	3	2	0	0	9	76
4:15 PM	0	3	0	0	0	3	0	46	19	0	1	65	1	0	0	0	2	1	3	0	2	0	0	5	74
4:30 PM	3	4	0	0	0	7	0	37	9	0	1	46	0	0	0	0	1	0	1	0	4	0	0	5	58
4:45 PM	1	0	0	0	0	1	0	51	12	0	0	63	1	0	0	0	1	1	4	2	5	0	0	11	76
Hourly Total	9	8	0	1	0	18	0	178	52	0	5	230	3	3	0	0	5	6	12	5	13	0	0	30	284

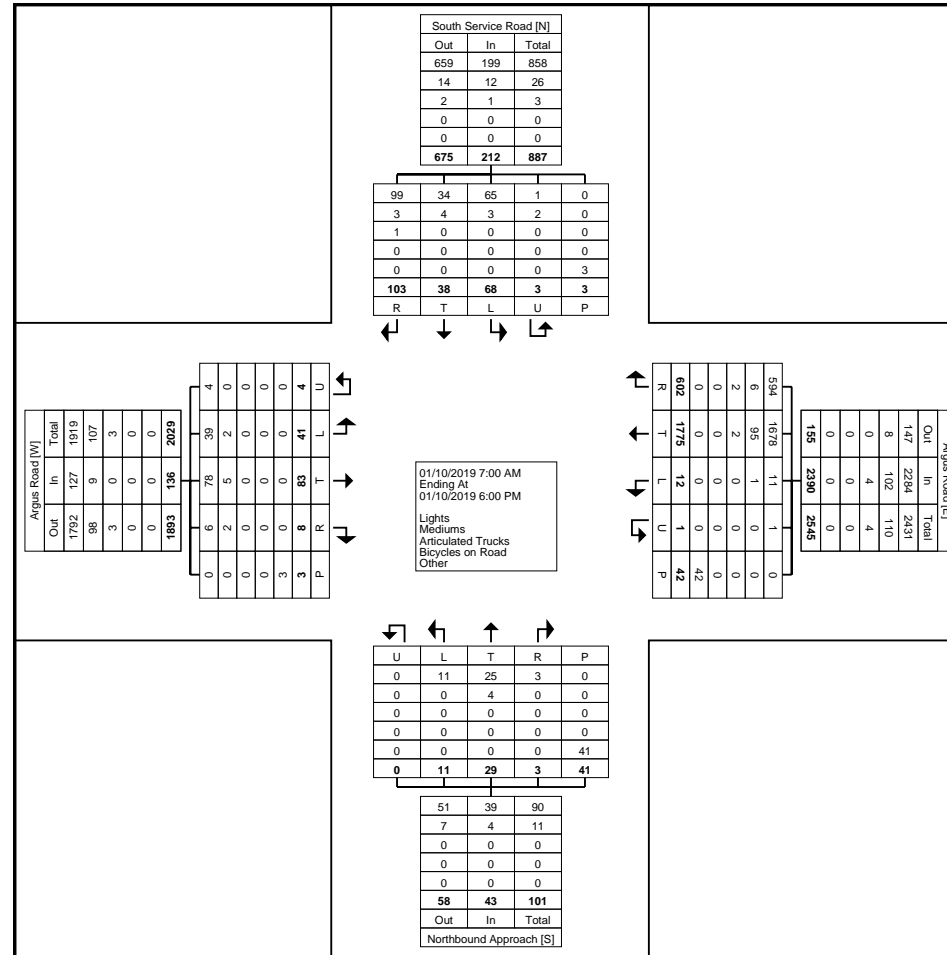
5:00 PM	2	5	0	0	0	7	0	27	16	0	0	43	0	1	0	0	0	1	10	1	13	0	1	24	75
5:15 PM	1	3	0	0	0	4	0	40	10	0	1	50	0	0	0	0	1	0	1	1	5	0	0	7	61
5:30 PM	0	4	0	0	0	4	0	36	15	0	1	51	0	0	0	0	2	0	1	1	2	0	0	4	59
5:45 PM	1	4	0	0	0	5	0	48	14	0	1	62	0	0	0	0	1	0	5	0	5	0	0	10	77
Hourly Total	4	16	0	0	0	20	0	151	55	0	3	206	0	1	0	0	4	1	17	3	25	0	1	45	272
Grand Total	41	83	8	4	3	136	12	1775	602	1	42	2390	11	29	3	0	41	43	68	38	103	3	3	212	2781
Approach %	30.1	61.0	5.9	2.9	-	-	0.5	74.3	25.2	0.0	-	-	25.6	67.4	7.0	0.0	-	-	32.1	17.9	48.6	1.4	-	-	-
Total %	1.5	3.0	0.3	0.1	-	4.9	0.4	63.8	21.6	0.0	-	85.9	0.4	1.0	0.1	0.0	-	1.5	2.4	1.4	3.7	0.1	-	7.6	-
Lights	39	78	6	4	-	127	11	1678	594	1	-	2284	11	25	3	0	-	39	65	34	99	1	-	199	2649
% Lights	95.1	94.0	75.0	100.0	-	93.4	91.7	94.5	98.7	100.0	-	95.6	100.0	86.2	100.0	-	-	90.7	95.6	89.5	96.1	33.3	-	93.9	95.3
Mediums	2	5	2	0	-	9	1	95	6	0	-	102	0	4	0	0	-	4	3	4	3	2	-	12	127
% Mediums	4.9	6.0	25.0	0.0	-	6.6	8.3	5.4	1.0	0.0	-	4.3	0.0	13.8	0.0	-	-	9.3	4.4	10.5	2.9	66.7	-	5.7	4.6
Articulated Trucks	0	0	0	0	-	0	0	2	2	0	-	4	0	0	0	0	-	0	0	0	1	0	-	1	5
% Articulated Trucks	0.0	0.0	0.0	0.0	-	0.0	0.0	0.1	0.3	0.0	-	0.2	0.0	0.0	0.0	-	-	0.0	0.0	0.0	1.0	0.0	-	0.5	0.2
Bicycles on Road	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0
% Bicycles on Road	0.0	0.0	0.0	0.0	-	0.0	0.0	0.0	0.0	0.0	-	0.0	0.0	0.0	0.0	-	-	0.0	0.0	0.0	0.0	0.0	-	0.0	0.0
Bicycles on Crosswalk	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	0	-	-
% Bicycles on Crosswalk	-	-	-	-	0.0	-	-	-	-	-	0.0	-	-	-	-	-	0.0	-	-	-	-	-	0.0	-	-
Pedestrians	-	-	-	-	3	-	-	-	-	-	42	-	-	-	-	-	41	-	-	-	-	-	3	-	-
% Pedestrians	-	-	-	-	100.0	-	-	-	-	-	100.0	-	-	-	-	-	100.0	-	-	-	-	-	100.0	-	-



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Count Name: Argus Road & South Service Road  
Site Code:  
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Turning Movement Data Plot



Paradigm Transportation Solutions Limited  
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Count Name: Argus Road & South Service Road  
Site Code:  
Start Date: 01/10/2019  
Page No: 4

### Turning Movement Peak Hour Data (7:30 AM)

Start Time	Argus Road Eastbound						Argus Road Westbound						Northbound Approach Northbound						South Service Road Southbound						Int. Total
	Left	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	Right	U-Turn	Peds	App. Total	
7:30 AM	1	1	1	0	0	3	0	127	25	0	1	152	0	0	0	0	1	0	1	0	0	0	0	1	156
7:45 AM	0	0	0	0	0	0	2	157	28	0	2	187	0	1	0	0	2	1	0	0	1	0	1	1	189
8:00 AM	0	1	1	0	0	2	1	130	21	0	1	152	0	0	0	0	2	0	2	2	2	1	0	7	161
8:15 AM	0	3	0	0	1	3	1	125	39	0	1	165	1	0	0	0	1	1	1	0	0	0	0	1	170
Total	1	5	2	0	1	8	4	539	113	0	5	656	1	1	0	0	6	2	4	2	3	1	1	10	676
Approach %	12.5	62.5	25.0	0.0	-	-	0.6	82.2	17.2	0.0	-	-	50.0	50.0	0.0	0.0	-	-	40.0	20.0	30.0	10.0	-	-	-
Total %	0.1	0.7	0.3	0.0	-	1.2	0.6	79.7	16.7	0.0	-	97.0	0.1	0.1	0.0	0.0	-	0.3	0.6	0.3	0.4	0.1	-	1.5	-
PHF	0.250	0.417	0.500	0.000	-	0.667	0.500	0.858	0.724	0.000	-	0.877	0.250	0.250	0.000	0.000	-	0.500	0.500	0.250	0.375	0.250	-	0.357	0.894
Lights	0	5	2	0	-	7	4	524	113	0	-	641	1	1	0	0	-	2	4	2	3	1	-	10	660
% Lights	0.0	100.0	100.0	-	-	87.5	100.0	97.2	100.0	-	-	97.7	100.0	100.0	-	-	-	100.0	100.0	100.0	100.0	100.0	-	100.0	97.6
Mediums	1	0	0	0	-	1	0	15	0	0	-	15	0	0	0	0	-	0	0	0	0	0	-	0	16
% Mediums	100.0	0.0	0.0	-	-	12.5	0.0	2.8	0.0	-	-	2.3	0.0	0.0	-	-	-	0.0	0.0	0.0	0.0	0.0	-	0.0	2.4
Articulated Trucks	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0
% Articulated Trucks	0.0	0.0	0.0	-	-	0.0	0.0	0.0	0.0	-	-	0.0	0.0	0.0	-	-	-	0.0	0.0	0.0	0.0	0.0	-	0.0	0.0
Bicycles on Road	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0
% Bicycles on Road	0.0	0.0	0.0	-	-	0.0	0.0	0.0	0.0	-	-	0.0	0.0	0.0	-	-	-	0.0	0.0	0.0	0.0	0.0	-	0.0	0.0
Bicycles on Crosswalk	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	0	-	-
% Bicycles on Crosswalk	-	-	-	-	0.0	-	-	-	-	-	0.0	-	-	-	-	-	0.0	-	-	-	-	-	0.0	-	-
Pedestrians	-	-	-	-	1	-	-	-	-	-	5	-	-	-	-	-	6	-	-	-	-	-	1	-	-
% Pedestrians	-	-	-	-	100.0	-	-	-	-	-	100.0	-	-	-	-	-	100.0	-	-	-	-	-	100.0	-	-

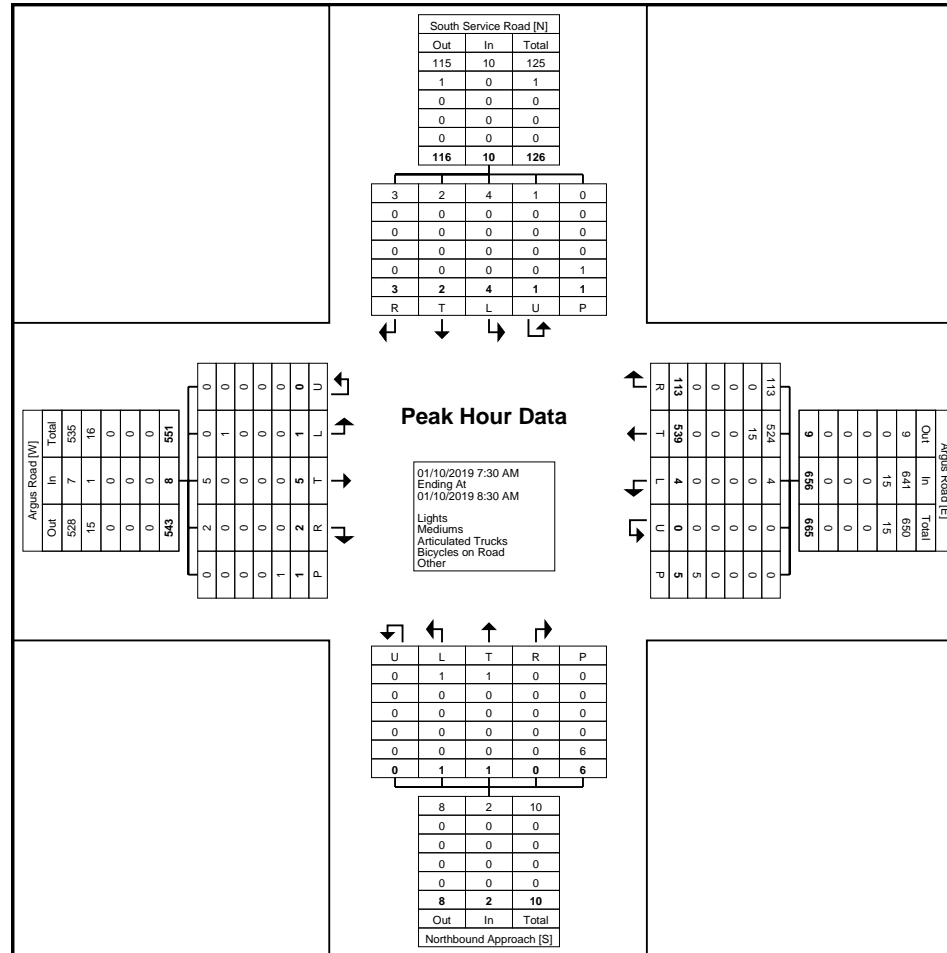




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Count Name: Argus Road & South Service Road  
Site Code:  
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Turning Movement Peak Hour Data Plot (7:30 AM)



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Count Name: Argus Road & South Service Road  
Site Code:  
Start Date: 01/10/2019  
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### Turning Movement Peak Hour Data (11:00 AM)

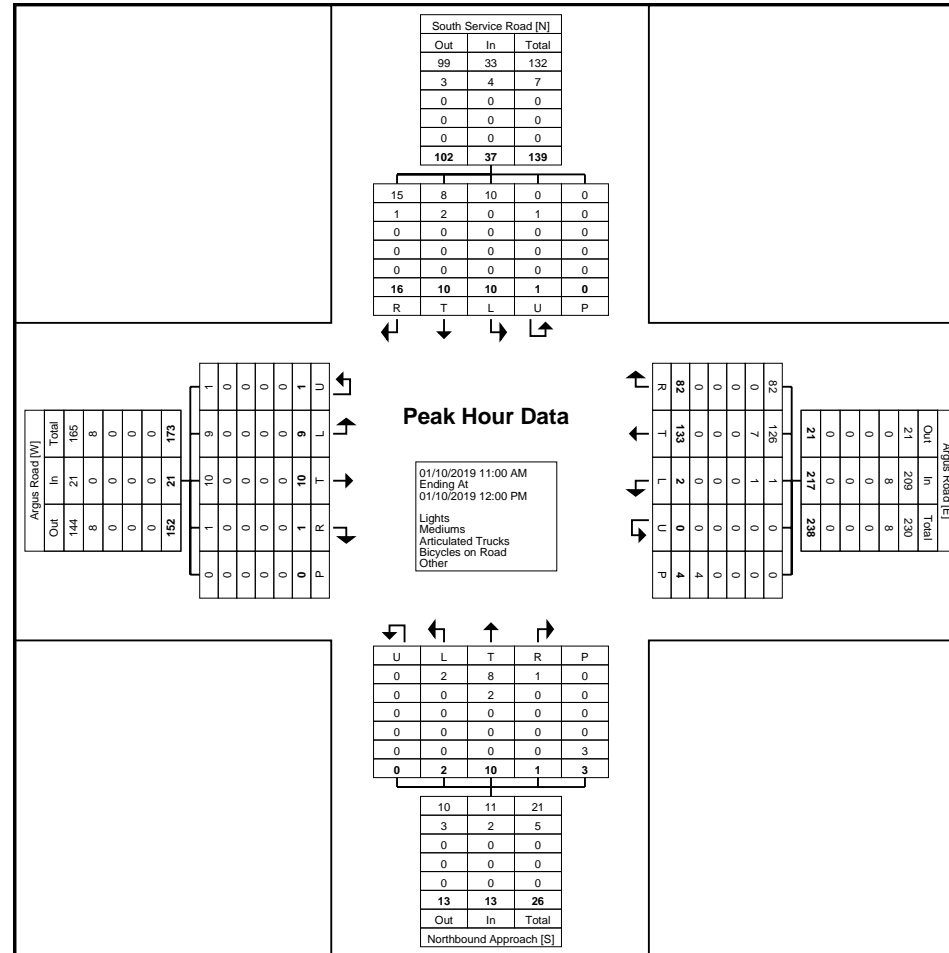
Start Time	Argus Road Eastbound						Argus Road Westbound						Northbound Approach Northbound						South Service Road Southbound						Int. Total
	Left	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	Right	U-Turn	Peds	App. Total	
11:00 AM	2	5	1	1	0	9	0	30	18	0	0	48	1	3	0	0	1	4	4	3	3	1	0	11	72
11:15 AM	2	1	0	0	0	3	1	33	19	0	0	53	0	2	1	0	0	3	2	2	8	0	0	12	71
11:30 AM	4	4	0	0	0	8	1	32	25	0	3	58	0	1	0	0	2	1	2	1	3	0	0	6	73
11:45 AM	1	0	0	0	0	1	0	38	20	0	1	58	1	4	0	0	0	5	2	4	2	0	0	8	72
Total	9	10	1	1	0	21	2	133	82	0	4	217	2	10	1	0	3	13	10	10	16	1	0	37	288
Approach %	42.9	47.6	4.8	4.8	-	-	0.9	61.3	37.8	0.0	-	-	15.4	76.9	7.7	0.0	-	-	27.0	27.0	43.2	2.7	-	-	-
Total %	3.1	3.5	0.3	0.3	-	7.3	0.7	46.2	28.5	0.0	-	75.3	0.7	3.5	0.3	0.0	-	4.5	3.5	3.5	5.6	0.3	-	12.8	-
PHF	0.563	0.500	0.250	0.250	-	0.583	0.500	0.875	0.820	0.000	-	0.935	0.500	0.625	0.250	0.000	-	0.650	0.625	0.625	0.500	0.250	-	0.771	0.986
Lights	9	10	1	1	-	21	1	126	82	0	-	209	2	8	1	0	-	11	10	8	15	0	-	33	274
% Lights	100.0	100.0	100.0	100.0	-	100.0	50.0	94.7	100.0	-	-	96.3	100.0	80.0	100.0	-	-	84.6	100.0	80.0	93.8	0.0	-	89.2	95.1
Mediums	0	0	0	0	-	0	1	7	0	0	-	8	0	2	0	0	-	2	0	2	1	1	-	4	14
% Mediums	0.0	0.0	0.0	0.0	-	0.0	50.0	5.3	0.0	-	-	3.7	0.0	20.0	0.0	-	-	15.4	0.0	20.0	6.3	100.0	-	10.8	4.9
Articulated Trucks	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0
% Articulated Trucks	0.0	0.0	0.0	0.0	-	0.0	0.0	0.0	0.0	-	-	0.0	0.0	0.0	0.0	-	-	0.0	0.0	0.0	0.0	0.0	-	0.0	0.0
Bicycles on Road	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0
% Bicycles on Road	0.0	0.0	0.0	0.0	-	0.0	0.0	0.0	0.0	-	-	0.0	0.0	0.0	0.0	-	-	0.0	0.0	0.0	0.0	0.0	-	0.0	0.0
Bicycles on Crosswalk	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	0	-	-
% Bicycles on Crosswalk	-	-	-	-	-	-	-	-	-	-	0.0	-	-	-	-	-	0.0	-	-	-	-	-	-	-	-
Pedestrians	-	-	-	-	0	-	-	-	-	-	4	-	-	-	-	-	3	-	-	-	-	-	0	-	-
% Pedestrians	-	-	-	-	-	-	-	-	-	-	100.0	-	-	-	-	-	100.0	-	-	-	-	-	-	-	-



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Count Name: Argus Road & South Service Road  
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Turning Movement Peak Hour Data Plot (11:00 AM)



Paradigm Transportation Solutions Limited  
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Count Name: Argus Road & South Service Road  
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### Turning Movement Peak Hour Data (4:00 PM)

Start Time	Argus Road Eastbound						Argus Road Westbound						Northbound Approach Northbound						South Service Road Southbound						Int. Total
	Left	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	Right	U-Turn	Peds	App. Total	
4:00 PM	5	1	0	1	0	7	0	44	12	0	3	56	1	3	0	0	1	4	4	3	2	0	0	9	76
4:15 PM	0	3	0	0	0	3	0	46	19	0	1	65	1	0	0	0	2	1	3	0	2	0	0	5	74
4:30 PM	3	4	0	0	0	7	0	37	9	0	1	46	0	0	0	0	1	0	1	0	4	0	0	5	58
4:45 PM	1	0	0	0	0	1	0	51	12	0	0	63	1	0	0	0	1	1	4	2	5	0	0	11	76
Total	9	8	0	1	0	18	0	178	52	0	5	230	3	3	0	0	5	6	12	5	13	0	0	30	284
Approach %	50.0	44.4	0.0	5.6	-	-	0.0	77.4	22.6	0.0	-	-	50.0	50.0	0.0	0.0	-	-	40.0	16.7	43.3	0.0	-	-	-
Total %	3.2	2.8	0.0	0.4	-	6.3	0.0	62.7	18.3	0.0	-	81.0	1.1	1.1	0.0	0.0	-	2.1	4.2	1.8	4.6	0.0	-	10.6	-
PHF	0.450	0.500	0.000	0.250	-	0.643	0.000	0.873	0.684	0.000	-	0.885	0.750	0.250	0.000	0.000	-	0.375	0.750	0.417	0.650	0.000	-	0.682	0.934
Lights	9	7	0	1	-	17	0	160	52	0	-	212	3	3	0	0	-	6	12	5	13	0	-	30	265
% Lights	100.0	87.5	-	100.0	-	94.4	-	89.9	100.0	-	-	92.2	100.0	100.0	-	-	-	100.0	100.0	100.0	100.0	-	-	100.0	93.3
Mediums	0	1	0	0	-	1	0	18	0	0	-	18	0	0	0	0	-	0	0	0	0	0	-	0	19
% Mediums	0.0	12.5	-	0.0	-	5.6	-	10.1	0.0	-	-	7.8	0.0	0.0	-	-	-	0.0	0.0	0.0	0.0	-	-	0.0	6.7
Articulated Trucks	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0
% Articulated Trucks	0.0	0.0	-	0.0	-	0.0	-	0.0	0.0	-	-	0.0	0.0	0.0	-	-	-	0.0	0.0	0.0	0.0	-	-	0.0	0.0
Bicycles on Road	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0
% Bicycles on Road	0.0	0.0	-	0.0	-	0.0	-	0.0	0.0	-	-	0.0	0.0	0.0	-	-	-	0.0	0.0	0.0	0.0	-	-	0.0	0.0
Bicycles on Crosswalk	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	0	-	-
% Bicycles on Crosswalk	-	-	-	-	-	-	-	-	-	-	0.0	-	-	-	-	-	0.0	-	-	-	-	-	-	-	-
Pedestrians	-	-	-	-	0	-	-	-	-	-	5	-	-	-	-	-	5	-	-	-	-	-	0	-	-
% Pedestrians	-	-	-	-	-	-	-	-	-	-	100.0	-	-	-	-	-	100.0	-	-	-	-	-	-	-	-





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Count Name: Argus Road & South Service Road  
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Paradigm Transportation Solutions Limited  
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Count Name: Cross Avenue & Argus Road  
Site Code:  
Start Date: 01/10/2019  
Page No: 1

### Turning Movement Data

Start Time	Cross Avenue Eastbound						Cross Avenue Westbound						GO Bus Station Northbound						Argus Road Southbound						Int. Total
	Left	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	Right	U-Turn	Peds	App. Total	
7:00 AM	2	75	5	0	1	82	12	134	0	0	1	146	7	0	14	0	0	21	5	2	85	0	1	92	341
7:15 AM	3	82	2	0	3	87	7	154	2	0	10	163	1	0	7	0	0	8	13	2	112	0	2	127	385
7:30 AM	2	90	5	0	1	97	9	131	5	0	8	145	6	0	14	0	1	20	11	3	103	0	0	117	379
7:45 AM	6	87	2	0	1	95	7	157	4	0	3	168	1	0	9	0	0	10	17	6	130	0	0	153	426
Hourly Total	13	334	14	0	6	361	35	576	11	0	22	622	15	0	44	0	1	59	46	13	430	0	3	489	1531
8:00 AM	7	76	5	0	0	88	11	118	7	0	5	136	9	0	16	0	2	25	11	4	110	0	1	125	374
8:15 AM	14	102	1	0	1	117	10	145	6	0	4	161	3	0	6	0	0	9	14	2	120	0	0	136	423
8:30 AM	10	88	6	0	1	104	10	96	6	0	2	112	9	0	13	0	0	22	17	2	39	0	0	58	296
8:45 AM	12	81	2	0	1	95	8	84	8	0	3	100	4	1	3	0	0	8	8	4	57	0	0	69	272
Hourly Total	43	347	14	0	3	404	39	443	27	0	14	509	25	1	38	0	2	64	50	12	326	0	1	388	1365
*** BREAK ***	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
11:00 AM	6	81	4	0	1	91	6	59	7	0	3	72	6	0	8	0	1	14	20	4	22	0	0	46	223
11:15 AM	4	73	2	0	2	79	2	66	10	0	1	78	0	0	3	0	0	3	20	0	30	0	0	50	210
11:30 AM	7	76	7	0	1	90	7	66	7	0	3	80	7	0	11	0	1	18	21	2	33	0	2	56	244
11:45 AM	3	94	1	0	1	98	1	67	8	1	1	77	0	0	3	0	0	3	28	2	24	0	1	54	232
Hourly Total	20	324	14	0	5	358	16	258	32	1	8	307	13	0	25	0	2	38	89	8	109	0	3	206	909
12:00 PM	3	77	4	0	4	84	7	73	6	0	4	86	3	0	4	0	1	7	31	3	20	0	1	54	231
12:15 PM	7	88	1	0	0	96	2	78	4	0	1	84	4	0	7	0	0	11	23	0	37	0	1	60	251
12:30 PM	1	97	4	0	3	102	7	76	9	0	4	92	4	0	8	0	0	12	25	3	19	0	0	47	253
12:45 PM	7	98	1	0	2	106	3	82	7	0	1	92	1	0	3	0	0	4	26	1	21	0	3	48	250
Hourly Total	18	360	10	0	9	388	19	309	26	0	10	354	12	0	22	0	1	34	105	7	97	0	5	209	985
1:00 PM	7	117	4	0	2	128	8	89	10	0	1	107	9	0	9	0	1	18	18	4	19	0	2	41	294
1:15 PM	4	99	1	0	0	104	3	76	9	0	3	88	1	0	2	0	0	3	18	0	25	0	2	43	238
1:30 PM	3	75	6	0	2	84	8	69	4	0	1	81	7	0	8	0	1	15	22	1	23	0	0	46	226
1:45 PM	5	84	1	0	1	90	1	82	13	0	4	96	0	0	4	0	1	4	17	1	18	0	2	36	226
Hourly Total	19	375	12	0	5	406	20	316	36	0	9	372	17	0	23	0	3	40	75	6	85	0	6	166	984
*** BREAK ***	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
3:00 PM	4	92	4	0	4	100	8	82	10	0	10	100	7	0	10	0	6	17	24	4	24	0	0	52	269
3:15 PM	6	108	1	0	0	115	3	83	9	0	6	95	3	0	2	0	0	5	22	4	27	0	1	53	268
3:30 PM	7	101	4	0	5	112	9	63	14	0	8	86	5	0	10	0	1	15	11	3	25	0	10	39	252
3:45 PM	4	84	1	0	1	89	1	87	4	0	2	92	3	0	2	0	0	5	19	3	27	0	3	49	235
Hourly Total	21	385	10	0	10	416	21	315	37	0	26	373	18	0	24	0	7	42	76	14	103	0	14	193	1024
4:00 PM	4	145	4	0	0	153	8	72	6	0	6	86	5	0	11	0	1	16	27	5	34	0	0	66	321
4:15 PM	5	90	2	0	0	97	4	72	4	0	3	80	2	1	7	0	2	10	23	3	33	0	1	59	246
4:30 PM	3	195	4	1	1	203	4	64	10	0	5	78	2	0	13	0	1	15	25	6	36	0	3	67	363
4:45 PM	2	122	5	0	0	129	4	72	5	0	2	81	5	0	10	0	0	15	25	3	37	0	1	65	290
Hourly Total	14	552	15	1	1	582	20	280	25	0	16	325	14	1	41	0	4	56	100	17	140	0	5	257	1220

5:00 PM	2	212	5	0	2	219	9	83	8	0	4	100	3	0	14	0	1	17	39	6	21	0	0	66	402
5:15 PM	4	108	3	0	6	115	8	82	12	0	8	102	4	0	8	0	2	12	32	2	30	0	2	64	293
5:30 PM	2	225	5	0	2	232	8	86	8	0	3	102	2	0	16	0	0	18	24	4	31	0	4	59	411
5:45 PM	5	242	2	0	2	249	9	76	11	0	5	96	5	2	5	0	1	12	39	6	39	0	3	84	441
Hourly Total	13	787	15	0	12	815	34	327	39	0	20	400	14	2	43	0	4	59	134	18	121	0	9	273	1547
Grand Total	161	3464	104	1	51	3730	204	2824	233	1	125	3262	128	4	260	0	24	392	675	95	1411	0	46	2181	9565
Approach %	4.3	92.9	2.8	0.0	-	-	6.3	86.6	7.1	0.0	-	-	32.7	1.0	66.3	0.0	-	-	30.9	4.4	64.7	0.0	-	-	-
Total %	1.7	36.2	1.1	0.0	-	39.0	2.1	29.5	2.4	0.0	-	34.1	1.3	0.0	2.7	0.0	-	4.1	7.1	1.0	14.8	0.0	-	22.8	-
Lights	160	3426	4	1	-	3591	17	2797	228	1	-	3043	8	0	13	0	-	21	665	4	1405	0	-	2074	8729
% Lights	99.4	98.9	3.8	100.0	-	96.3	8.3	99.0	97.9	100.0	-	93.3	6.3	0.0	5.0	-	-	5.4	98.5	4.2	99.6	-	-	95.1	91.3
Mediums	1	32	100	0	-	133	187	25	5	0	-	217	120	4	247	0	-	371	8	91	6	0	-	105	826
% Mediums	0.6	0.9	96.2	0.0	-	3.6	91.7	0.9	2.1	0.0	-	6.7	93.8	100.0	95.0	-	-	94.6	1.2	95.8	0.4	-	-	4.8	8.6
Articulated Trucks	0	6	0	0	-	6	0	1	0	0	-	1	0	0	0	0	-	0	2	0	0	0	-	2	9
% Articulated Trucks	0.0	0.2	0.0	0.0	-	0.2	0.0	0.0	0.0	0.0	-	0.0	0.0	0.0	0.0	-	-	0.0	0.3	0.0	0.0	-	-	0.1	0.1
Bicycles on Road	0	0	0	0	-	0	0	1	0	0	-	1	0	0	0	0	-	0	0	0	0	0	-	0	1
% Bicycles on Road	0.0	0.0	0.0	0.0	-	0.0	0.0	0.0	0.0	0.0	-	0.0	0.0	0.0	0.0	-	-	0.0	0.0	0.0	0.0	-	-	0.0	0.0
Bicycles on Crosswalk	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	3	-	-
% Bicycles on Crosswalk	-	-	-	-	0.0	-	-	-	-	-	0.0	-	-	-	-	-	0.0	-	-	-	-	-	6.5	-	-
Pedestrians	-	-	-	-	51	-	-	-	-	-	125	-	-	-	-	-	24	-	-	-	-	-	43	-	-
% Pedestrians	-	-	-	-	100.0	-	-	-	-	-	100.0	-	-	-	-	-	100.0	-	-	-	-	-	93.5	-	-

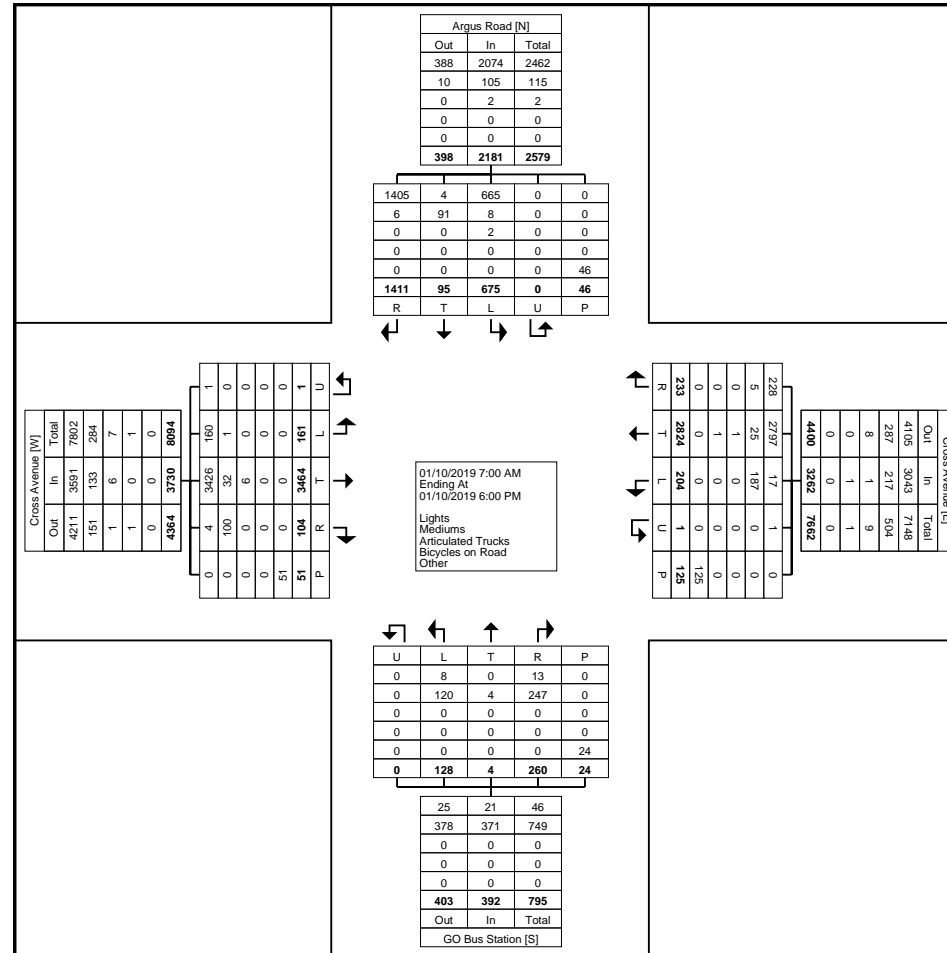




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Count Name: Cross Avenue & Argus Road  
 Site Code:  
 Start Date: 01/10/2019  
 Page No: 3



Turning Movement Data Plot



Paradigm Transportation Solutions Limited  
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Count Name: Cross Avenue & Argus Road  
Site Code:  
Start Date: 01/10/2019  
Page No: 4

### Turning Movement Peak Hour Data (7:30 AM)

Start Time	Cross Avenue Eastbound						Cross Avenue Westbound						GO Bus Station Northbound						Argus Road Southbound						Int. Total
	Left	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	Right	U-Turn	Peds	App. Total	
7:30 AM	2	90	5	0	1	97	9	131	5	0	8	145	6	0	14	0	1	20	11	3	103	0	0	117	379
7:45 AM	6	87	2	0	1	95	7	157	4	0	3	168	1	0	9	0	0	10	17	6	130	0	0	153	426
8:00 AM	7	76	5	0	0	88	11	118	7	0	5	136	9	0	16	0	2	25	11	4	110	0	1	125	374
8:15 AM	14	102	1	0	1	117	10	145	6	0	4	161	3	0	6	0	0	9	14	2	120	0	0	136	423
Total	29	355	13	0	3	397	37	551	22	0	20	610	19	0	45	0	3	64	53	15	463	0	1	531	1602
Approach %	7.3	89.4	3.3	0.0	-	-	6.1	90.3	3.6	0.0	-	-	29.7	0.0	70.3	0.0	-	-	10.0	2.8	87.2	0.0	-	-	-
Total %	1.8	22.2	0.8	0.0	-	24.8	2.3	34.4	1.4	0.0	-	38.1	1.2	0.0	2.8	0.0	-	4.0	3.3	0.9	28.9	0.0	-	33.1	-
PHF	0.518	0.870	0.650	0.000	-	0.848	0.841	0.877	0.786	0.000	-	0.908	0.528	0.000	0.703	0.000	-	0.640	0.779	0.625	0.890	0.000	-	0.868	0.940
Lights	29	350	0	0	-	379	3	548	21	0	-	572	1	0	4	0	-	5	53	1	462	0	-	516	1472
% Lights	100.0	98.6	0.0	-	-	95.5	8.1	99.5	95.5	-	-	93.8	5.3	-	8.9	-	-	7.8	100.0	6.7	99.8	-	-	97.2	91.9
Mediums	0	3	13	0	-	16	34	3	1	0	-	38	18	0	41	0	-	59	0	14	1	0	-	15	128
% Mediums	0.0	0.8	100.0	-	-	4.0	91.9	0.5	4.5	-	-	6.2	94.7	-	91.1	-	-	92.2	0.0	93.3	0.2	-	-	2.8	8.0
Articulated Trucks	0	2	0	0	-	2	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	2
% Articulated Trucks	0.0	0.6	0.0	-	-	0.5	0.0	0.0	0.0	-	-	0.0	0.0	-	0.0	-	-	0.0	0.0	0.0	0.0	-	-	0.0	0.1
Bicycles on Road	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0
% Bicycles on Road	0.0	0.0	0.0	-	-	0.0	0.0	0.0	0.0	-	-	0.0	0.0	-	0.0	-	-	0.0	0.0	0.0	0.0	-	-	0.0	0.0
Bicycles on Crosswalk	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	1	-	-
% Bicycles on Crosswalk	-	-	-	-	0.0	-	-	-	-	-	0.0	-	-	-	-	-	0.0	-	-	-	-	-	100.0	-	-
Pedestrians	-	-	-	-	3	-	-	-	-	-	20	-	-	-	-	-	3	-	-	-	-	-	0	-	-
% Pedestrians	-	-	-	-	100.0	-	-	-	-	-	100.0	-	-	-	-	-	100.0	-	-	-	-	-	0.0	-	-





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Count Name: Cross Avenue & Argus Road  
Site Code:  
Start Date: 01/10/2019  
Page No: 6

### Turning Movement Peak Hour Data (12:15 PM)

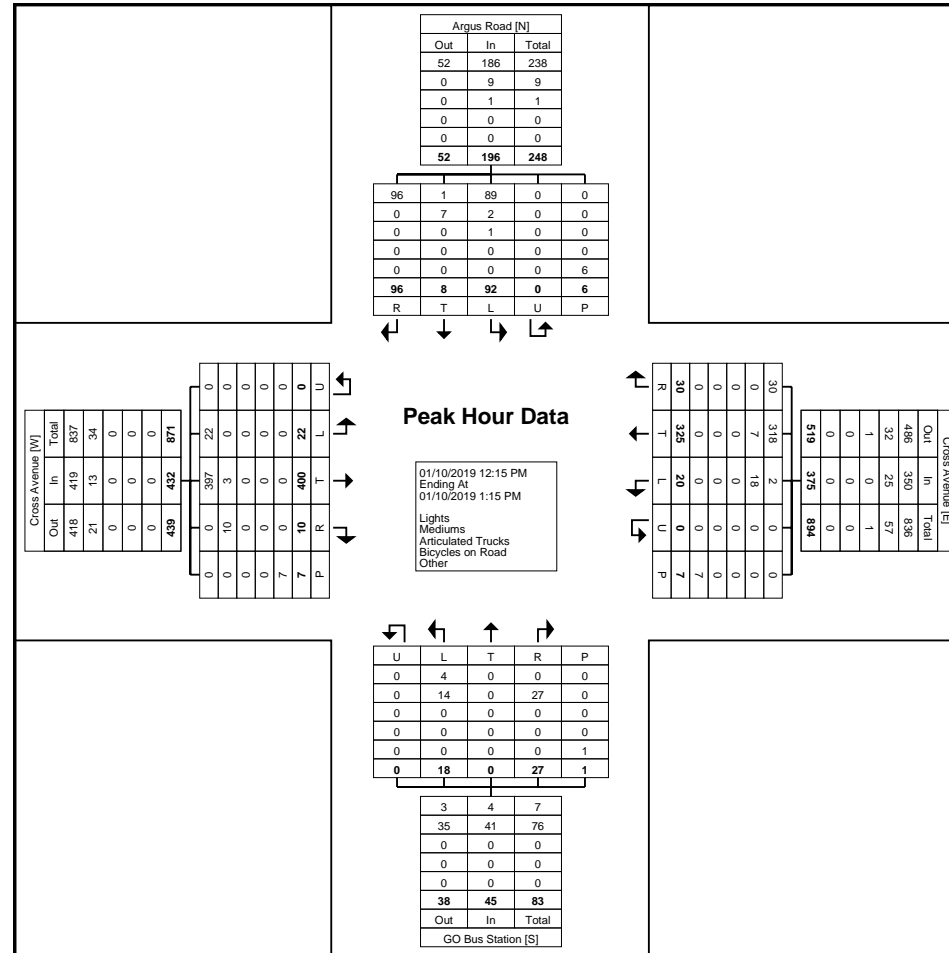
Start Time	Cross Avenue Eastbound						Cross Avenue Westbound						GO Bus Station Northbound						Argus Road Southbound						Int. Total
	Left	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	Right	U-Turn	Peds	App. Total	
12:15 PM	7	88	1	0	0	96	2	78	4	0	1	84	4	0	7	0	0	11	23	0	37	0	1	60	251
12:30 PM	1	97	4	0	3	102	7	76	9	0	4	92	4	0	8	0	0	12	25	3	19	0	0	47	253
12:45 PM	7	98	1	0	2	106	3	82	7	0	1	92	1	0	3	0	0	4	26	1	21	0	3	48	250
1:00 PM	7	117	4	0	2	128	8	89	10	0	1	107	9	0	9	0	1	18	18	4	19	0	2	41	294
Total	22	400	10	0	7	432	20	325	30	0	7	375	18	0	27	0	1	45	92	8	96	0	6	196	1048
Approach %	5.1	92.6	2.3	0.0	-	-	5.3	86.7	8.0	0.0	-	-	40.0	0.0	60.0	0.0	-	-	46.9	4.1	49.0	0.0	-	-	-
Total %	2.1	38.2	1.0	0.0	-	41.2	1.9	31.0	2.9	0.0	-	35.8	1.7	0.0	2.6	0.0	-	4.3	8.8	0.8	9.2	0.0	-	18.7	-
PHF	0.786	0.855	0.625	0.000	-	0.844	0.625	0.913	0.750	0.000	-	0.876	0.500	0.000	0.750	0.000	-	0.625	0.885	0.500	0.649	0.000	-	0.817	0.891
Lights	22	397	0	0	-	419	2	318	30	0	-	350	4	0	0	0	-	4	89	1	96	0	-	186	959
% Lights	100.0	99.3	0.0	-	-	97.0	10.0	97.8	100.0	-	-	93.3	22.2	-	0.0	-	-	8.9	96.7	12.5	100.0	-	-	94.9	91.5
Mediums	0	3	10	0	-	13	18	7	0	0	-	25	14	0	27	0	-	41	2	7	0	0	-	9	88
% Mediums	0.0	0.8	100.0	-	-	3.0	90.0	2.2	0.0	-	-	6.7	77.8	-	100.0	-	-	91.1	2.2	87.5	0.0	-	-	4.6	8.4
Articulated Trucks	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	1	0	0	0	-	1	1
% Articulated Trucks	0.0	0.0	0.0	-	-	0.0	0.0	0.0	0.0	-	-	0.0	0.0	-	0.0	-	-	0.0	1.1	0.0	0.0	-	-	0.5	0.1
Bicycles on Road	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0
% Bicycles on Road	0.0	0.0	0.0	-	-	0.0	0.0	0.0	0.0	-	-	0.0	0.0	-	0.0	-	-	0.0	0.0	0.0	0.0	-	-	0.0	0.0
Bicycles on Crosswalk	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	0	-	-
% Bicycles on Crosswalk	-	-	-	-	0.0	-	-	-	-	-	0.0	-	-	-	-	-	0.0	-	-	-	-	-	0.0	-	-
Pedestrians	-	-	-	-	7	-	-	-	-	-	7	-	-	-	-	-	1	-	-	-	-	-	6	-	-
% Pedestrians	-	-	-	-	100.0	-	-	-	-	-	100.0	-	-	-	-	-	100.0	-	-	-	-	-	100.0	-	-



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Count Name: Cross Avenue & Argus Road  
 Site Code:  
 Start Date: 01/10/2019  
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Turning Movement Peak Hour Data Plot (12:15 PM)



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Count Name: Cross Avenue & Argus Road  
Site Code:  
Start Date: 01/10/2019  
Page No: 8

### Turning Movement Peak Hour Data (5:00 PM)

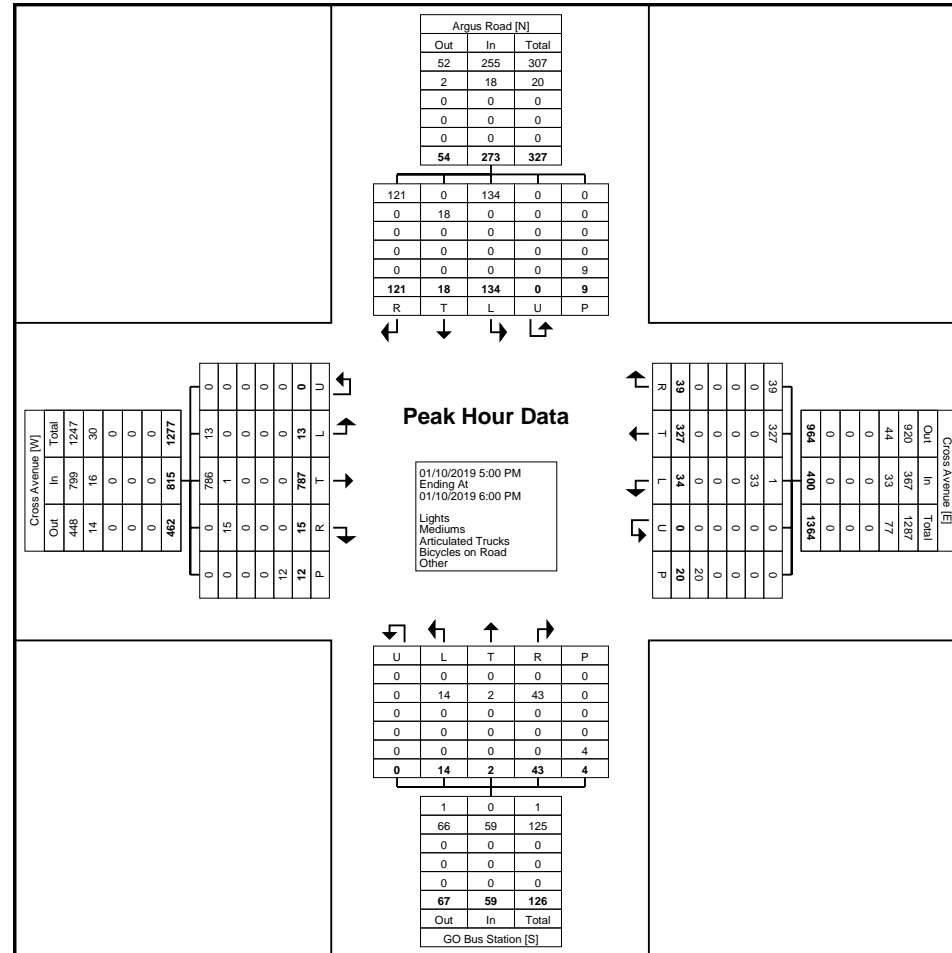
Start Time	Cross Avenue Eastbound						Cross Avenue Westbound						GO Bus Station Northbound						Argus Road Southbound						Int. Total
	Left	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	Right	U-Turn	Peds	App. Total	
5:00 PM	2	212	5	0	2	219	9	83	8	0	4	100	3	0	14	0	1	17	39	6	21	0	0	66	402
5:15 PM	4	108	3	0	6	115	8	82	12	0	8	102	4	0	8	0	2	12	32	2	30	0	2	64	293
5:30 PM	2	225	5	0	2	232	8	86	8	0	3	102	2	0	16	0	0	18	24	4	31	0	4	59	411
5:45 PM	5	242	2	0	2	249	9	76	11	0	5	96	5	2	5	0	1	12	39	6	39	0	3	84	441
<b>Total</b>	<b>13</b>	<b>787</b>	<b>15</b>	<b>0</b>	<b>12</b>	<b>815</b>	<b>34</b>	<b>327</b>	<b>39</b>	<b>0</b>	<b>20</b>	<b>400</b>	<b>14</b>	<b>2</b>	<b>43</b>	<b>0</b>	<b>4</b>	<b>59</b>	<b>134</b>	<b>18</b>	<b>121</b>	<b>0</b>	<b>9</b>	<b>273</b>	<b>1547</b>
Approach %	1.6	96.6	1.8	0.0	-	-	8.5	81.8	9.8	0.0	-	-	23.7	3.4	72.9	0.0	-	-	49.1	6.6	44.3	0.0	-	-	-
Total %	0.8	50.9	1.0	0.0	-	52.7	2.2	21.1	2.5	0.0	-	25.9	0.9	0.1	2.8	0.0	-	3.8	8.7	1.2	7.8	0.0	-	17.6	-
PHF	0.650	0.813	0.750	0.000	-	0.818	0.944	0.951	0.813	0.000	-	0.980	0.700	0.250	0.672	0.000	-	0.819	0.859	0.750	0.776	0.000	-	0.813	0.877
Lights	13	786	0	0	-	799	1	327	39	0	-	367	0	0	0	0	-	0	134	0	121	0	-	255	1421
% Lights	100.0	99.9	0.0	-	-	98.0	2.9	100.0	100.0	-	-	91.8	0.0	0.0	0.0	-	-	0.0	100.0	0.0	100.0	-	-	93.4	91.9
Mediums	0	1	15	0	-	16	33	0	0	0	-	33	14	2	43	0	-	59	0	18	0	0	-	18	126
% Mediums	0.0	0.1	100.0	-	-	2.0	97.1	0.0	0.0	-	-	8.3	100.0	100.0	100.0	-	-	100.0	0.0	100.0	0.0	-	-	6.6	8.1
Articulated Trucks	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0
% Articulated Trucks	0.0	0.0	0.0	-	-	0.0	0.0	0.0	0.0	-	-	0.0	0.0	0.0	0.0	-	-	0.0	0.0	0.0	0.0	-	-	0.0	0.0
Bicycles on Road	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0
% Bicycles on Road	0.0	0.0	0.0	-	-	0.0	0.0	0.0	0.0	-	-	0.0	0.0	0.0	0.0	-	-	0.0	0.0	0.0	0.0	-	-	0.0	0.0
Bicycles on Crosswalk	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	1	-	-
% Bicycles on Crosswalk	-	-	-	-	0.0	-	-	-	-	-	0.0	-	-	-	-	-	0.0	-	-	-	-	-	11.1	-	-
Pedestrians	-	-	-	-	12	-	-	-	-	-	20	-	-	-	-	-	4	-	-	-	-	-	8	-	-
% Pedestrians	-	-	-	-	100.0	-	-	-	-	-	100.0	-	-	-	-	-	100.0	-	-	-	-	-	88.9	-	-



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Count Name: Cross Avenue & Argus Road  
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Turning Movement Peak Hour Data Plot (5:00 PM)



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Count Name: Cross Avenue & Argus Road  
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Paradigm Transportation Solutions Limited  
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Count Name: Cross Avenue & Lyons Lane  
Site Code:  
Start Date: 01/10/2019  
Page No: 1

### Turning Movement Data

Start Time	Cross Avenue Eastbound					Cross Avenue Westbound					Lyons Lane Southbound					Int. Total
	Left	Thru	U-Turn	Peds	App. Total	Thru	Right	U-Turn	Peds	App. Total	Left	Right	U-Turn	Peds	App. Total	
7:00 AM	0	60	0	0	60	19	0	0	0	19	0	1	0	2	1	80
7:15 AM	4	99	0	0	103	22	3	0	1	25	2	1	0	0	3	131
7:30 AM	2	84	0	0	86	32	4	0	0	36	3	2	0	0	5	127
7:45 AM	2	111	0	0	113	36	0	0	3	36	2	2	0	2	4	153
Hourly Total	8	354	0	0	362	109	7	0	4	116	7	6	0	4	13	491
8:00 AM	8	84	0	0	92	43	3	0	1	46	2	0	0	0	2	140
8:15 AM	7	106	0	0	113	45	2	0	1	47	1	1	0	0	2	162
8:30 AM	5	80	0	0	85	49	1	0	2	50	0	1	0	2	1	136
8:45 AM	1	71	0	0	72	38	6	0	0	44	1	1	0	0	2	118
Hourly Total	21	341	0	0	362	175	12	0	4	187	4	3	0	2	7	556
*** BREAK ***	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
11:00 AM	1	54	0	0	55	61	1	0	0	62	3	8	0	2	11	128
11:15 AM	3	43	0	0	46	58	5	0	2	63	1	3	0	1	4	113
11:30 AM	4	50	0	0	54	72	3	0	0	75	7	5	0	0	12	141
11:45 AM	1	42	0	0	43	68	3	0	1	71	5	2	0	0	7	121
Hourly Total	9	189	0	0	198	259	12	0	3	271	16	18	0	3	34	503
12:00 PM	3	53	0	0	56	60	2	0	0	62	10	7	0	2	17	135
12:15 PM	0	55	0	0	55	76	3	0	0	79	0	9	0	1	9	143
12:30 PM	4	59	0	0	63	75	1	0	0	76	5	3	0	1	8	147
12:45 PM	3	47	0	0	50	55	2	0	0	57	3	2	0	0	5	112
Hourly Total	10	214	0	0	224	266	8	0	0	274	18	21	0	4	39	537
1:00 PM	5	50	0	0	55	67	5	0	1	72	4	5	0	1	9	136
1:15 PM	2	49	0	0	51	79	7	0	0	86	2	1	0	2	3	140
1:30 PM	5	53	0	0	58	72	1	0	0	73	1	4	0	2	5	136
1:45 PM	4	48	0	0	52	63	4	0	2	67	5	7	0	1	12	131
Hourly Total	16	200	0	0	216	281	17	0	3	298	12	17	0	6	29	543
*** BREAK ***	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
3:00 PM	1	58	0	0	59	84	3	0	1	87	3	4	0	0	7	153
3:15 PM	1	53	0	0	54	100	4	0	0	104	7	6	0	1	13	171
3:30 PM	1	57	0	0	58	64	2	0	2	66	4	2	0	2	6	130
3:45 PM	2	51	0	0	53	102	4	0	0	106	5	5	0	0	10	169
Hourly Total	5	219	0	0	224	350	13	0	3	363	19	17	0	3	36	623
4:00 PM	2	44	0	0	46	122	0	0	1	122	5	7	0	2	12	180
4:15 PM	3	37	0	0	40	75	1	0	0	76	2	9	0	2	11	127
4:30 PM	1	56	0	0	57	118	2	0	1	120	5	11	0	2	16	193
4:45 PM	1	66	0	0	67	102	0	0	1	102	0	15	0	1	15	184
Hourly Total	7	203	0	0	210	417	3	0	3	420	12	42	0	7	54	684
5:00 PM	3	43	0	0	46	164	2	0	1	166	4	16	0	1	20	232

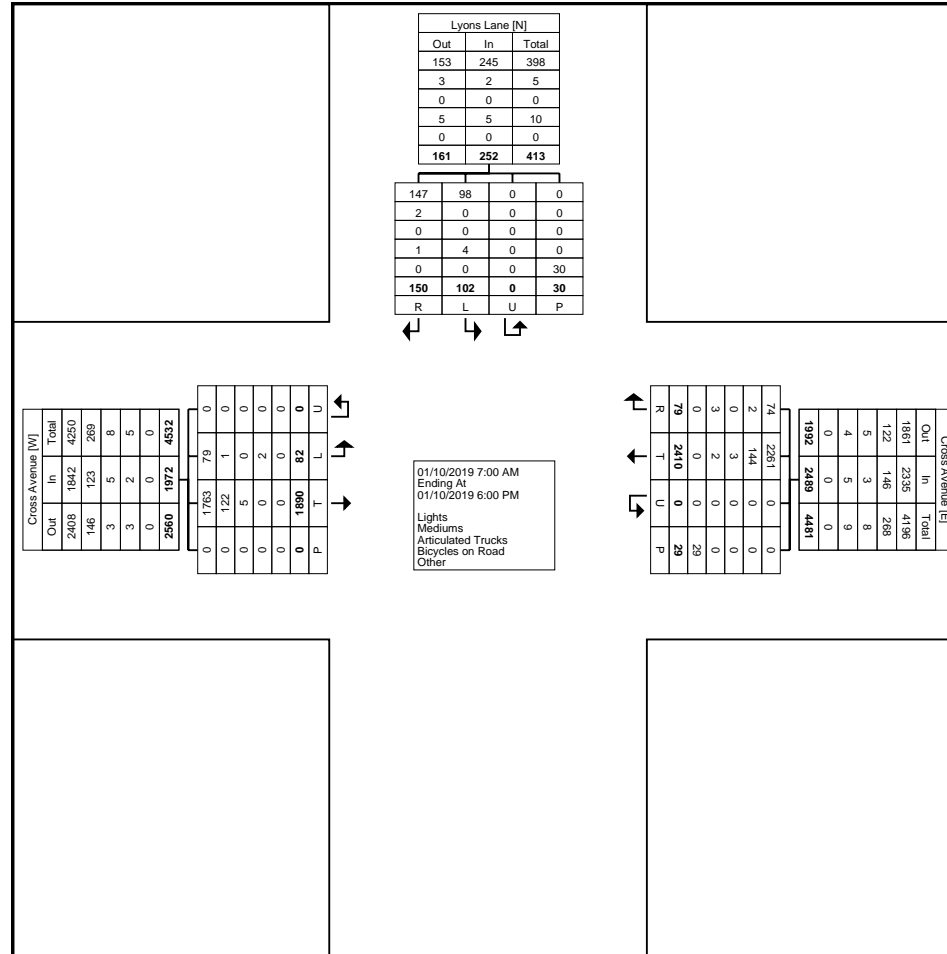
5:15 PM	0	37	0	0	37	104	3	0	4	107	1	3	0	0	4	148
5:30 PM	1	46	0	0	47	140	2	0	1	142	8	4	0	0	12	201
5:45 PM	2	44	0	0	46	145	0	0	3	145	1	3	0	0	4	195
Hourly Total	6	170	0	0	176	553	7	0	9	560	14	26	0	1	40	776
Grand Total	82	1890	0	0	1972	2410	79	0	29	2489	102	150	0	30	252	4713
Approach %	4.2	95.8	0.0	-	-	96.8	3.2	0.0	-	-	40.5	59.5	0.0	-	-	-
Total %	1.7	40.1	0.0	-	41.8	51.1	1.7	0.0	-	52.8	2.2	3.2	0.0	-	5.3	-
Lights	79	1763	0	-	1842	2261	74	0	-	2335	98	147	0	-	245	4422
% Lights	96.3	93.3	-	-	93.4	93.8	93.7	-	-	93.8	96.1	98.0	-	-	97.2	93.8
Mediums	1	122	0	-	123	144	2	0	-	146	0	2	0	-	2	271
% Mediums	1.2	6.5	-	-	6.2	6.0	2.5	-	-	5.9	0.0	1.3	-	-	0.8	5.8
Articulated Trucks	0	5	0	-	5	3	0	0	-	3	0	0	0	-	0	8
% Articulated Trucks	0.0	0.3	-	-	0.3	0.1	0.0	-	-	0.1	0.0	0.0	-	-	0.0	0.2
Bicycles on Road	2	0	0	-	2	2	3	0	-	5	4	1	0	-	5	12
% Bicycles on Road	2.4	0.0	-	-	0.1	0.1	3.8	-	-	0.2	3.9	0.7	-	-	2.0	0.3
Bicycles on Crosswalk	-	-	-	0	-	-	-	-	3	-	-	-	-	1	-	-
% Bicycles on Crosswalk	-	-	-	-	-	-	-	-	10.3	-	-	-	-	3.3	-	-
Pedestrians	-	-	-	0	-	-	-	-	26	-	-	-	-	29	-	-
% Pedestrians	-	-	-	-	-	-	-	-	89.7	-	-	-	-	96.7	-	-



Paradigm Transportation Solutions Limited  
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Count Name: Cross Avenue & Lyons Lane  
Site Code:  
Start Date: 01/10/2019  
Page No: 3



Turning Movement Data Plot



Paradigm Transportation Solutions Limited  
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Count Name: Cross Avenue & Lyons Lane  
Site Code:  
Start Date: 01/10/2019  
Page No: 4

### Turning Movement Peak Hour Data (7:45 AM)

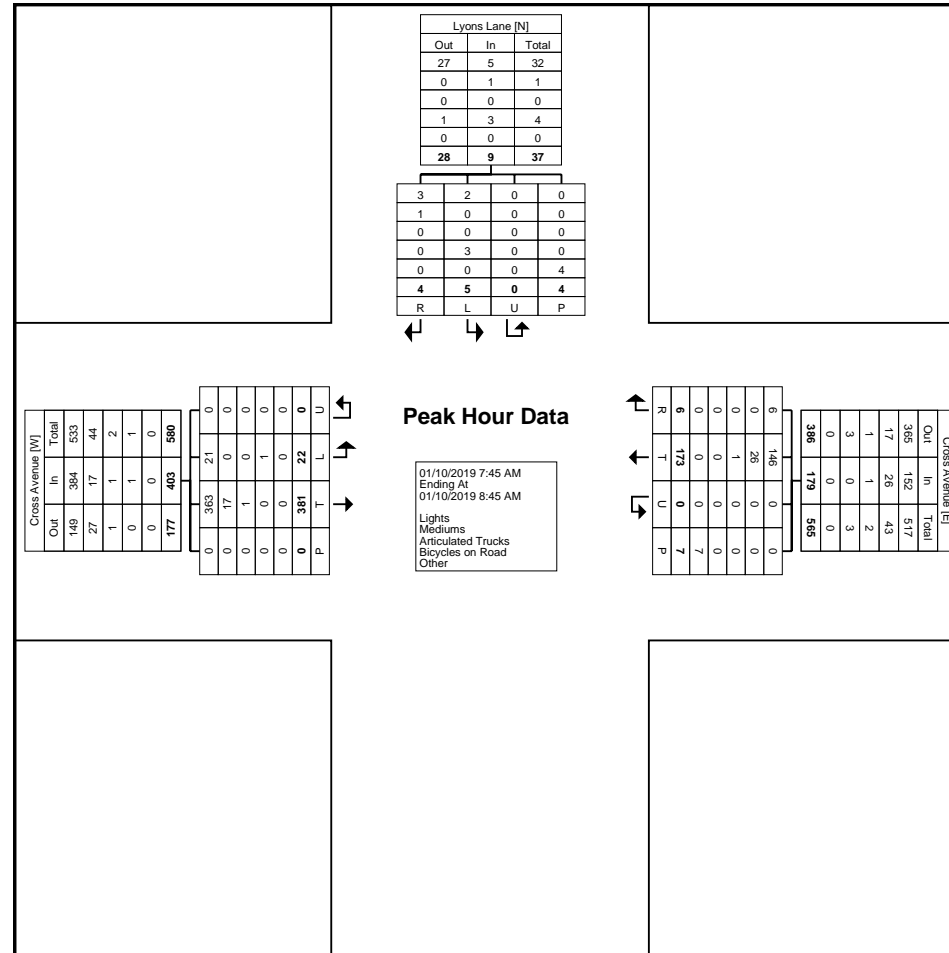
Start Time	Cross Avenue Eastbound					Cross Avenue Westbound					Lyons Lane Southbound					Int. Total
	Left	Thru	U-Turn	Peds	App. Total	Thru	Right	U-Turn	Peds	App. Total	Left	Right	U-Turn	Peds	App. Total	
7:45 AM	2	111	0	0	113	36	0	0	3	36	2	2	0	2	4	153
8:00 AM	8	84	0	0	92	43	3	0	1	46	2	0	0	0	2	140
8:15 AM	7	106	0	0	113	45	2	0	1	47	1	1	0	0	2	162
8:30 AM	5	80	0	0	85	49	1	0	2	50	0	1	0	2	1	136
Total	22	381	0	0	403	173	6	0	7	179	5	4	0	4	9	591
Approach %	5.5	94.5	0.0	-	-	96.6	3.4	0.0	-	-	55.6	44.4	0.0	-	-	-
Total %	3.7	64.5	0.0	-	68.2	29.3	1.0	0.0	-	30.3	0.8	0.7	0.0	-	1.5	-
PHF	0.688	0.858	0.000	-	0.892	0.883	0.500	0.000	-	0.895	0.625	0.500	0.000	-	0.563	0.912
Lights	21	363	0	-	384	146	6	0	-	152	2	3	0	-	5	541
% Lights	95.5	95.3	-	-	95.3	84.4	100.0	-	-	84.9	40.0	75.0	-	-	55.6	91.5
Mediums	0	17	0	-	17	26	0	0	-	26	0	1	0	-	1	44
% Mediums	0.0	4.5	-	-	4.2	15.0	0.0	-	-	14.5	0.0	25.0	-	-	11.1	7.4
Articulated Trucks	0	1	0	-	1	1	0	0	-	1	0	0	0	-	0	2
% Articulated Trucks	0.0	0.3	-	-	0.2	0.6	0.0	-	-	0.6	0.0	0.0	-	-	0.0	0.3
Bicycles on Road	1	0	0	-	1	0	0	0	-	0	3	0	0	-	3	4
% Bicycles on Road	4.5	0.0	-	-	0.2	0.0	0.0	-	-	0.0	60.0	0.0	-	-	33.3	0.7
Bicycles on Crosswalk	-	-	-	0	-	-	-	-	0	-	-	-	-	0	-	-
% Bicycles on Crosswalk	-	-	-	-	-	-	-	-	0.0	-	-	-	-	0.0	-	-
Pedestrians	-	-	-	0	-	-	-	-	7	-	-	-	-	4	-	-
% Pedestrians	-	-	-	-	-	-	-	-	100.0	-	-	-	-	100.0	-	-



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Count Name: Cross Avenue & Lyons Lane  
Site Code:  
Start Date: 01/10/2019  
Page No: 5



Turning Movement Peak Hour Data Plot (7:45 AM)



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Count Name: Cross Avenue & Lyons Lane  
Site Code:  
Start Date: 01/10/2019  
Page No: 6

### Turning Movement Peak Hour Data (11:45 AM)

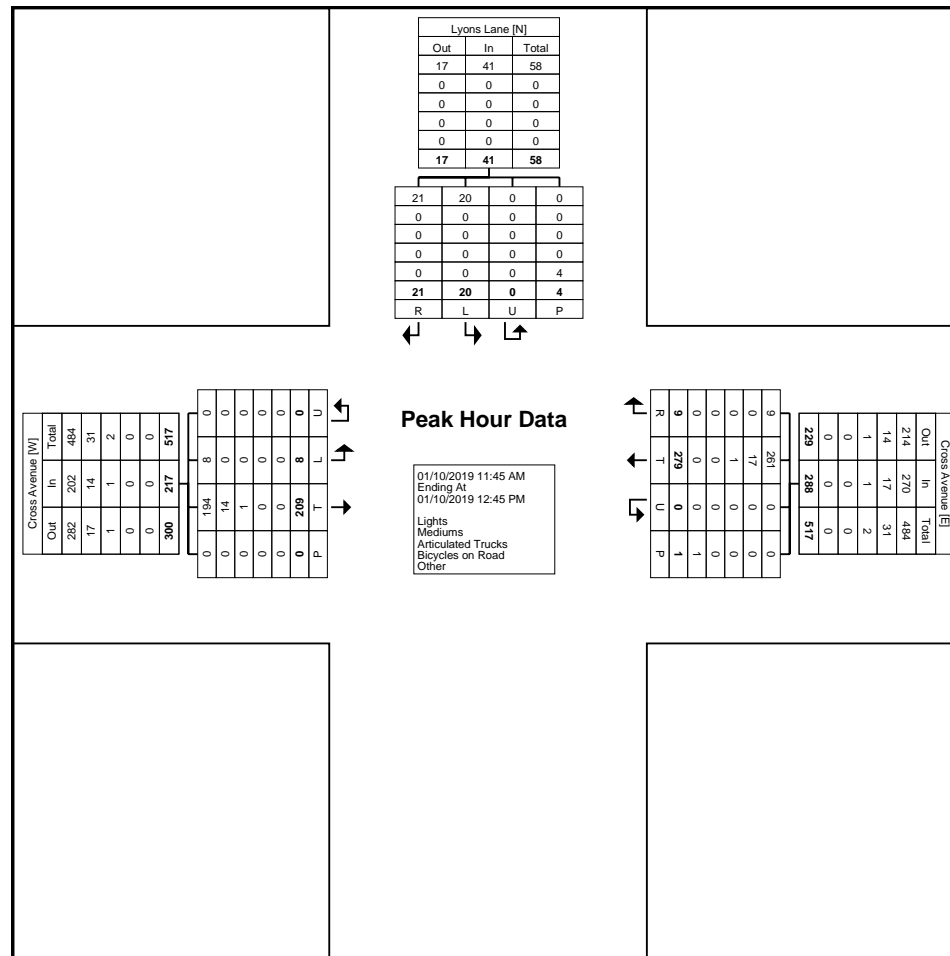
Start Time	Cross Avenue Eastbound					Cross Avenue Westbound					Lyons Lane Southbound					Int. Total
	Left	Thru	U-Turn	Peds	App. Total	Thru	Right	U-Turn	Peds	App. Total	Left	Right	U-Turn	Peds	App. Total	
11:45 AM	1	42	0	0	43	68	3	0	1	71	5	2	0	0	7	121
12:00 PM	3	53	0	0	56	60	2	0	0	62	10	7	0	2	17	135
12:15 PM	0	55	0	0	55	76	3	0	0	79	0	9	0	1	9	143
12:30 PM	4	59	0	0	63	75	1	0	0	76	5	3	0	1	8	147
Total	8	209	0	0	217	279	9	0	1	288	20	21	0	4	41	546
Approach %	3.7	96.3	0.0	-	-	96.9	3.1	0.0	-	-	48.8	51.2	0.0	-	-	-
Total %	1.5	38.3	0.0	-	39.7	51.1	1.6	0.0	-	52.7	3.7	3.8	0.0	-	7.5	-
PHF	0.500	0.886	0.000	-	0.861	0.918	0.750	0.000	-	0.911	0.500	0.583	0.000	-	0.603	0.929
Lights	8	194	0	-	202	261	9	0	-	270	20	21	0	-	41	513
% Lights	100.0	92.8	-	-	93.1	93.5	100.0	-	-	93.8	100.0	100.0	-	-	100.0	94.0
Mediums	0	14	0	-	14	17	0	0	-	17	0	0	0	-	0	31
% Mediums	0.0	6.7	-	-	6.5	6.1	0.0	-	-	5.9	0.0	0.0	-	-	0.0	5.7
Articulated Trucks	0	1	0	-	1	1	0	0	-	1	0	0	0	-	0	2
% Articulated Trucks	0.0	0.5	-	-	0.5	0.4	0.0	-	-	0.3	0.0	0.0	-	-	0.0	0.4
Bicycles on Road	0	0	0	-	0	0	0	0	-	0	0	0	0	-	0	0
% Bicycles on Road	0.0	0.0	-	-	0.0	0.0	0.0	-	-	0.0	0.0	0.0	-	-	0.0	0.0
Bicycles on Crosswalk	-	-	-	0	-	-	-	-	0	-	-	-	-	1	-	-
% Bicycles on Crosswalk	-	-	-	-	-	-	-	-	0.0	-	-	-	-	25.0	-	-
Pedestrians	-	-	-	0	-	-	-	-	1	-	-	-	-	3	-	-
% Pedestrians	-	-	-	-	-	-	-	-	100.0	-	-	-	-	75.0	-	-



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Count Name: Cross Avenue & Lyons Lane  
Site Code:  
Start Date: 01/10/2019  
Page No: 7



Turning Movement Peak Hour Data Plot (11:45 AM)



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Count Name: Cross Avenue & Lyons Lane  
Site Code:  
Start Date: 01/10/2019  
Page No: 8

### Turning Movement Peak Hour Data (5:00 PM)

Start Time	Cross Avenue Eastbound					Cross Avenue Westbound					Lyons Lane Southbound					Int. Total
	Left	Thru	U-Turn	Peds	App. Total	Thru	Right	U-Turn	Peds	App. Total	Left	Right	U-Turn	Peds	App. Total	
5:00 PM	3	43	0	0	46	164	2	0	1	166	4	16	0	1	20	232
5:15 PM	0	37	0	0	37	104	3	0	4	107	1	3	0	0	4	148
5:30 PM	1	46	0	0	47	140	2	0	1	142	8	4	0	0	12	201
5:45 PM	2	44	0	0	46	145	0	0	3	145	1	3	0	0	4	195
Total	6	170	0	0	176	553	7	0	9	560	14	26	0	1	40	776
Approach %	3.4	96.6	0.0	-	-	98.8	1.3	0.0	-	-	35.0	65.0	0.0	-	-	-
Total %	0.8	21.9	0.0	-	22.7	71.3	0.9	0.0	-	72.2	1.8	3.4	0.0	-	5.2	-
PHF	0.500	0.924	0.000	-	0.936	0.843	0.583	0.000	-	0.843	0.438	0.406	0.000	-	0.500	0.836
Lights	5	153	0	-	158	537	5	0	-	542	14	25	0	-	39	739
% Lights	83.3	90.0	-	-	89.8	97.1	71.4	-	-	96.8	100.0	96.2	-	-	97.5	95.2
Mediums	1	17	0	-	18	15	0	0	-	15	0	1	0	-	1	34
% Mediums	16.7	10.0	-	-	10.2	2.7	0.0	-	-	2.7	0.0	3.8	-	-	2.5	4.4
Articulated Trucks	0	0	0	-	0	1	0	0	-	1	0	0	0	-	0	1
% Articulated Trucks	0.0	0.0	-	-	0.0	0.2	0.0	-	-	0.2	0.0	0.0	-	-	0.0	0.1
Bicycles on Road	0	0	0	-	0	0	2	0	-	2	0	0	0	-	0	2
% Bicycles on Road	0.0	0.0	-	-	0.0	0.0	28.6	-	-	0.4	0.0	0.0	-	-	0.0	0.3
Bicycles on Crosswalk	-	-	-	0	-	-	-	-	2	-	-	-	-	0	-	-
% Bicycles on Crosswalk	-	-	-	-	-	-	-	-	22.2	-	-	-	-	0.0	-	-
Pedestrians	-	-	-	0	-	-	-	-	7	-	-	-	-	1	-	-
% Pedestrians	-	-	-	-	-	-	-	-	77.8	-	-	-	-	100.0	-	-

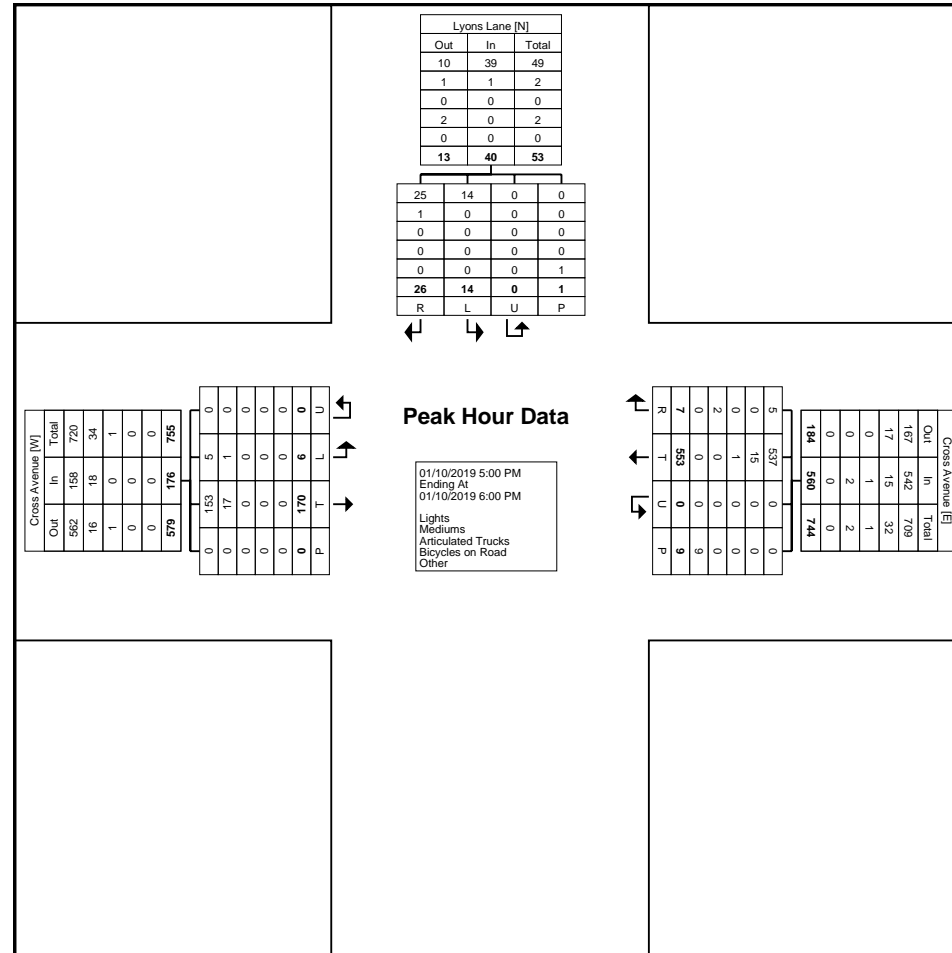




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Count Name: Cross Avenue & Lyons Lane  
Site Code:  
Start Date: 01/10/2019  
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Turning Movement Peak Hour Data Plot (5:00 PM)



Paradigm Transportation Solutions Limited  
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Count Name: Cross Avenue & Lyons Lane  
Site Code:  
Start Date: 01/10/2019  
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Count Name: Cross Avenue & Lyons  
Lane/Commercial Plaza  
Site Code:  
Start Date: 01/10/2019  
Page No: 1

### Turning Movement Data

Start Time	Cross Avenue Eastbound						Cross Avenue Westbound						Lyons Lane Northbound						Plaza Entrance Southbound						Int. Total	
	Left	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	Right	U-Turn	Peds	App. Total		
7:00 AM	7	27	26	0	0	60	29	15	3	0	1	47	2	1	0	0	1	3	3	2	4	0	1	9	119	
7:15 AM	5	50	46	0	2	101	64	20	1	0	1	85	2	0	8	0	0	10	3	2	6	0	2	11	207	
7:30 AM	12	37	42	0	0	91	35	22	3	0	1	60	4	0	3	0	0	7	6	6	8	0	0	20	178	
7:45 AM	8	34	62	0	2	104	46	21	1	0	1	68	2	1	2	0	0	5	1	6	11	0	5	18	195	
Hourly Total	32	148	176	0	4	356	174	78	8	0	4	260	10	2	13	0	1	25	13	16	29	0	8	58	699	
8:00 AM	16	39	43	0	0	98	45	34	1	0	1	80	5	1	2	0	1	8	6	5	9	0	0	20	206	
8:15 AM	17	49	35	0	1	101	77	31	0	0	0	108	5	1	5	0	0	11	2	5	10	0	0	17	237	
8:30 AM	17	41	22	0	2	80	28	35	3	0	0	66	2	1	2	0	0	5	12	4	11	0	1	27	178	
8:45 AM	12	53	7	0	0	72	23	29	5	0	0	57	0	0	1	0	0	1	7	2	15	0	0	24	154	
Hourly Total	62	182	107	0	3	351	173	129	9	0	1	311	12	3	10	0	1	25	27	16	45	0	1	88	775	
*** BREAK ***	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
11:00 AM	19	33	2	0	1	54	0	38	3	0	0	41	0	0	0	0	0	0	8	1	21	0	2	30	125	
11:15 AM	20	34	0	0	1	54	1	46	2	0	0	49	0	1	0	0	1	1	4	0	16	0	0	20	124	
11:30 AM	14	38	1	0	0	53	2	59	7	0	1	68	1	0	1	0	1	2	5	0	23	0	0	28	151	
11:45 AM	16	32	0	0	1	48	1	52	4	0	1	57	2	0	1	0	0	3	3	0	12	0	1	15	123	
Hourly Total	69	137	3	0	3	209	4	195	16	0	2	215	3	1	2	0	2	6	20	1	72	0	3	93	523	
12:00 PM	16	45	0	0	0	61	0	49	8	0	4	57	0	0	0	0	0	0	4	0	18	0	1	22	140	
12:15 PM	17	39	1	0	0	57	0	56	10	0	0	66	3	0	3	0	0	6	9	0	13	0	1	22	151	
12:30 PM	16	47	4	0	0	67	3	47	4	0	1	54	2	0	2	0	0	4	5	0	28	0	1	33	158	
12:45 PM	16	30	0	0	0	46	0	41	6	0	0	47	1	0	3	0	0	4	9	1	18	0	0	28	125	
Hourly Total	65	161	5	0	0	231	3	193	28	0	5	224	6	0	8	0	0	14	27	1	77	0	3	105	574	
1:00 PM	19	33	1	0	1	53	0	51	7	0	1	58	0	1	0	0	0	1	7	0	23	0	0	30	142	
1:15 PM	14	37	0	0	0	51	2	55	6	0	0	63	5	0	0	0	0	5	4	0	27	0	1	31	150	
1:30 PM	12	43	0	0	1	55	1	56	2	0	0	59	0	1	2	0	1	3	7	0	17	0	1	24	141	
1:45 PM	16	37	2	0	1	55	0	42	5	0	0	47	3	1	1	0	0	5	9	0	23	0	1	32	139	
Hourly Total	61	150	3	0	3	214	3	204	20	0	1	227	8	3	3	0	1	14	27	0	90	0	3	117	572	
*** BREAK ***	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
3:00 PM	15	43	1	0	1	59	0	72	4	0	0	76	0	0	1	0	0	1	6	0	20	0	0	26	162	
3:15 PM	11	50	0	0	1	61	0	83	4	0	1	87	2	0	7	0	1	9	6	0	16	0	3	22	179	
3:30 PM	15	42	2	0	1	59	0	53	2	0	0	55	0	0	2	0	1	2	2	0	17	0	0	19	135	
3:45 PM	13	39	3	0	0	55	2	78	5	0	3	85	5	1	4	0	1	10	5	0	19	0	1	24	174	
Hourly Total	54	174	6	0	3	234	2	286	15	0	4	303	7	1	14	0	3	22	19	0	72	0	4	91	650	
4:00 PM	11	38	1	0	0	50	1	93	3	0	0	97	13	1	18	0	0	32	5	0	14	0	2	19	198	
4:15 PM	8	33	2	0	0	43	3	59	1	0	0	63	6	1	5	0	0	12	5	0	14	0	1	19	137	
4:30 PM	14	43	5	0	1	62	4	68	4	0	1	76	26	0	21	0	1	47	3	0	20	0	1	23	208	
4:45 PM	12	46	5	0	3	63	3	72	3	0	0	78	11	1	11	0	1	23	8	0	23	0	0	31	195	
Hourly Total	45	160	13	0	4	218	11	292	11	0	1	314	56	3	55	0	2	114	21	0	71	0	4	92	738	

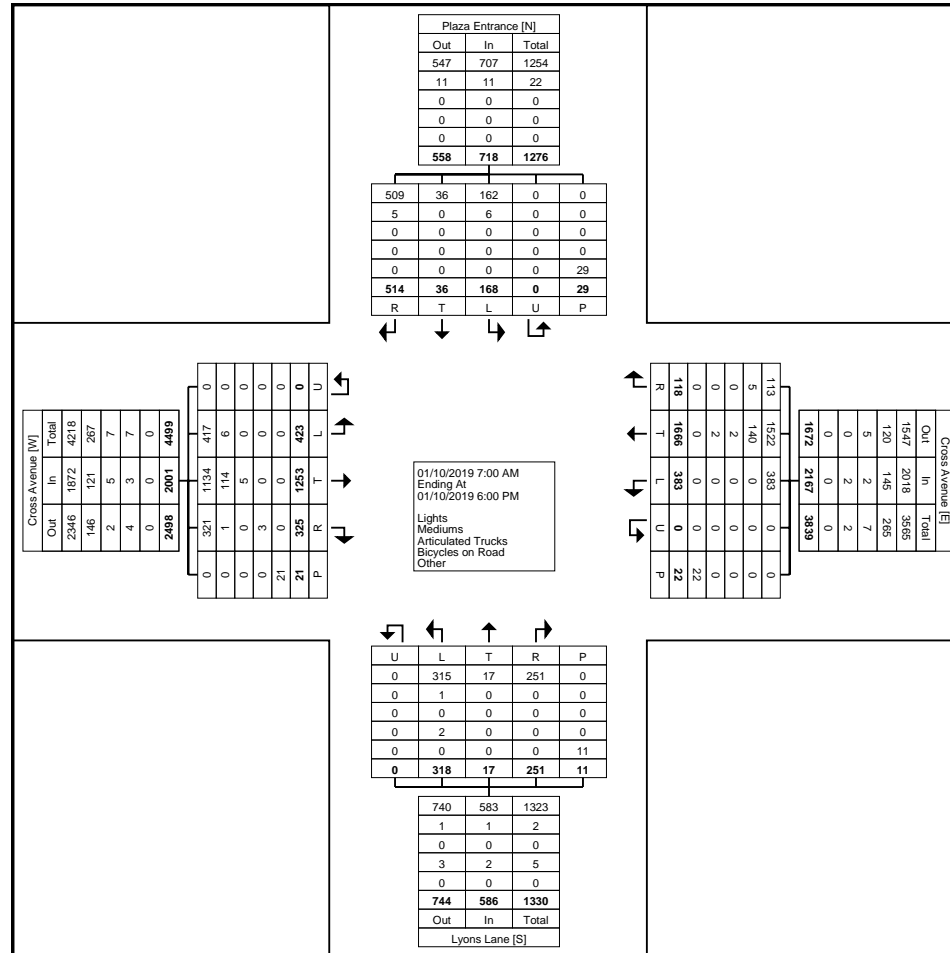
5:00 PM	9	41	0	0	0	50	1	76	2	0	0	79	73	0	32	0	1	105	5	0	24	0	0	29	263
5:15 PM	7	24	4	0	1	35	6	73	3	0	1	82	24	0	24	0	0	48	3	0	14	0	1	17	182
5:30 PM	15	42	4	0	0	61	2	66	5	0	1	73	59	1	46	0	0	106	4	1	10	0	1	15	255
5:45 PM	4	34	4	0	0	42	4	74	1	0	2	79	60	3	44	0	0	107	2	1	10	0	1	13	241
Hourly Total	35	141	12	0	1	188	13	289	11	0	4	313	216	4	146	0	1	366	14	2	58	0	3	74	941
Grand Total	423	1253	325	0	21	2001	383	1666	118	0	22	2167	318	17	251	0	11	586	168	36	514	0	29	718	5472
Approach %	21.1	62.6	16.2	0.0	-	-	17.7	76.9	5.4	0.0	-	-	54.3	2.9	42.8	0.0	-	-	23.4	5.0	71.6	0.0	-	-	-
Total %	7.7	22.9	5.9	0.0	-	36.6	7.0	30.4	2.2	0.0	-	39.6	5.8	0.3	4.6	0.0	-	10.7	3.1	0.7	9.4	0.0	-	13.1	-
Lights	417	1134	321	0	-	1872	383	1522	113	0	-	2018	315	17	251	0	-	583	162	36	509	0	-	707	5180
% Lights	98.6	90.5	98.8	-	-	93.6	100.0	91.4	95.8	-	-	93.1	99.1	100.0	100.0	-	-	99.5	96.4	100.0	99.0	-	-	98.5	94.7
Mediums	6	114	1	0	-	121	0	140	5	0	-	145	1	0	0	0	-	1	6	0	5	0	-	11	278
% Mediums	1.4	9.1	0.3	-	-	6.0	0.0	8.4	4.2	-	-	6.7	0.3	0.0	0.0	-	-	0.2	3.6	0.0	1.0	-	-	1.5	5.1
Articulated Trucks	0	5	0	0	-	5	0	2	0	0	-	2	0	0	0	0	-	0	0	0	0	0	-	0	7
% Articulated Trucks	0.0	0.4	0.0	-	-	0.2	0.0	0.1	0.0	-	-	0.1	0.0	0.0	0.0	-	-	0.0	0.0	0.0	0.0	-	-	0.0	0.1
Bicycles on Road	0	0	3	0	-	3	0	2	0	0	-	2	2	0	0	0	-	2	0	0	0	0	-	0	7
% Bicycles on Road	0.0	0.0	0.9	-	-	0.1	0.0	0.1	0.0	-	-	0.1	0.6	0.0	0.0	-	-	0.3	0.0	0.0	0.0	-	-	0.0	0.1
Bicycles on Crosswalk	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	3	-	-
% Bicycles on Crosswalk	-	-	-	-	0.0	-	-	-	-	-	0.0	-	-	-	-	-	0.0	-	-	-	-	-	10.3	-	-
Pedestrians	-	-	-	-	21	-	-	-	-	-	22	-	-	-	-	-	11	-	-	-	-	-	26	-	-
% Pedestrians	-	-	-	-	100.0	-	-	-	-	-	100.0	-	-	-	-	-	100.0	-	-	-	-	-	89.7	-	-



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Count Name: Cross Avenue & Lyons  
Lane/Commercial Plaza  
Site Code:  
Start Date: 01/10/2019  
Page No: 3



Turning Movement Data Plot



Paradigm Transportation Solutions Limited  
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Count Name: Cross Avenue & Lyons  
Lane/Commercial Plaza  
Site Code:  
Start Date: 01/10/2019  
Page No: 4

### Turning Movement Peak Hour Data (7:30 AM)

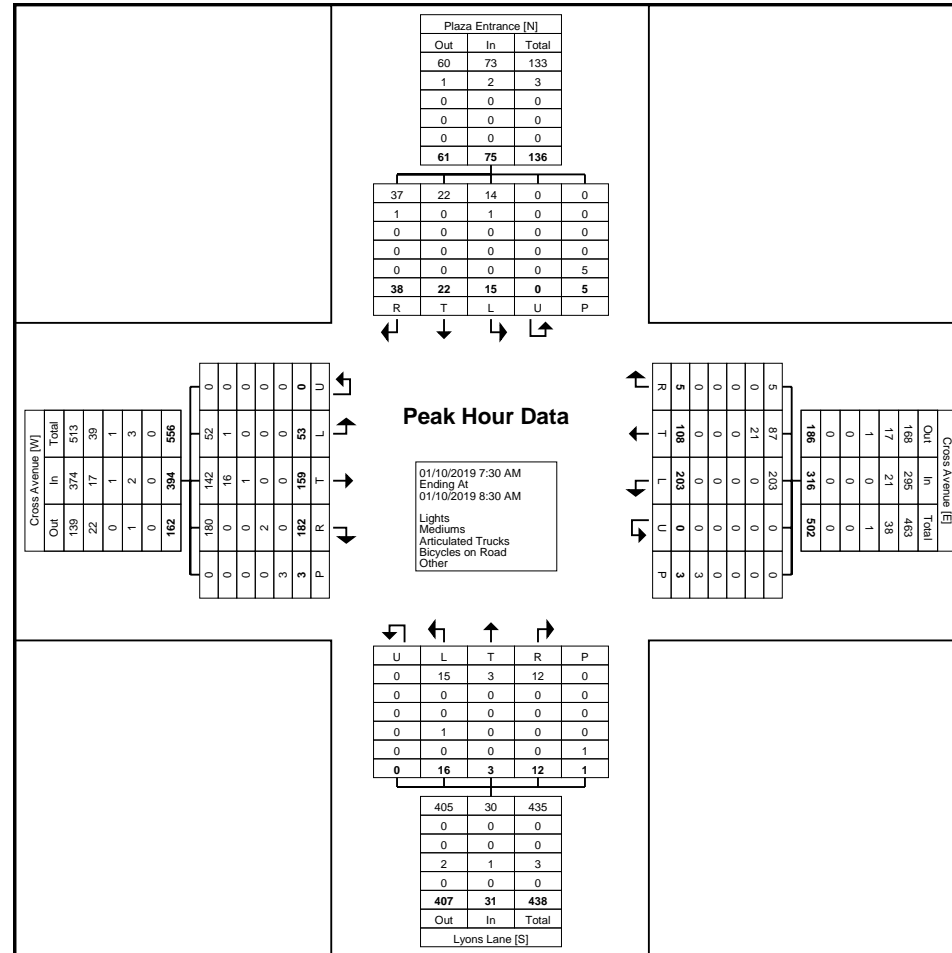
Start Time	Cross Avenue Eastbound						Cross Avenue Westbound						Lyons Lane Northbound						Plaza Entrance Southbound						Int. Total
	Left	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	Right	U-Turn	Peds	App. Total	
7:30 AM	12	37	42	0	0	91	35	22	3	0	1	60	4	0	3	0	0	7	6	6	8	0	0	20	178
7:45 AM	8	34	62	0	2	104	46	21	1	0	1	68	2	1	2	0	0	5	1	6	11	0	5	18	195
8:00 AM	16	39	43	0	0	98	45	34	1	0	1	80	5	1	2	0	1	8	6	5	9	0	0	20	206
8:15 AM	17	49	35	0	1	101	77	31	0	0	0	108	5	1	5	0	0	11	2	5	10	0	0	17	237
<b>Total</b>	<b>53</b>	<b>159</b>	<b>182</b>	<b>0</b>	<b>3</b>	<b>394</b>	<b>203</b>	<b>108</b>	<b>5</b>	<b>0</b>	<b>3</b>	<b>316</b>	<b>16</b>	<b>3</b>	<b>12</b>	<b>0</b>	<b>1</b>	<b>31</b>	<b>15</b>	<b>22</b>	<b>38</b>	<b>0</b>	<b>5</b>	<b>75</b>	<b>816</b>
Approach %	13.5	40.4	46.2	0.0	-	-	64.2	34.2	1.6	0.0	-	-	51.6	9.7	38.7	0.0	-	-	20.0	29.3	50.7	0.0	-	-	-
Total %	6.5	19.5	22.3	0.0	-	48.3	24.9	13.2	0.6	0.0	-	38.7	2.0	0.4	1.5	0.0	-	3.8	1.8	2.7	4.7	0.0	-	9.2	-
PHF	0.779	0.811	0.734	0.000	-	0.947	0.659	0.794	0.417	0.000	-	0.731	0.800	0.750	0.600	0.000	-	0.705	0.625	0.917	0.864	0.000	-	0.938	0.861
Lights	52	142	180	0	-	374	203	87	5	0	-	295	15	3	12	0	-	30	14	22	37	0	-	73	772
% Lights	98.1	89.3	98.9	-	-	94.9	100.0	80.6	100.0	-	-	93.4	93.8	100.0	100.0	-	-	96.8	93.3	100.0	97.4	-	-	97.3	94.6
Mediums	1	16	0	0	-	17	0	21	0	0	-	21	0	0	0	0	-	0	1	0	1	0	-	2	40
% Mediums	1.9	10.1	0.0	-	-	4.3	0.0	19.4	0.0	-	-	6.6	0.0	0.0	0.0	-	-	0.0	6.7	0.0	2.6	-	-	2.7	4.9
Articulated Trucks	0	1	0	0	-	1	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	1
% Articulated Trucks	0.0	0.6	0.0	-	-	0.3	0.0	0.0	0.0	-	-	0.0	0.0	0.0	0.0	-	-	0.0	0.0	0.0	0.0	-	-	0.0	0.1
Bicycles on Road	0	0	2	0	-	2	0	0	0	0	-	0	1	0	0	0	-	1	0	0	0	0	-	0	3
% Bicycles on Road	0.0	0.0	1.1	-	-	0.5	0.0	0.0	0.0	-	-	0.0	6.3	0.0	0.0	-	-	3.2	0.0	0.0	0.0	-	-	0.0	0.4
Bicycles on Crosswalk	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	1	-	-
% Bicycles on Crosswalk	-	-	-	-	0.0	-	-	-	-	-	0.0	-	-	-	-	-	0.0	-	-	-	-	-	20.0	-	-
Pedestrians	-	-	-	-	3	-	-	-	-	-	3	-	-	-	-	-	1	-	-	-	-	-	4	-	-
% Pedestrians	-	-	-	-	100.0	-	-	-	-	-	100.0	-	-	-	-	-	100.0	-	-	-	-	-	80.0	-	-



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Count Name: Cross Avenue & Lyons  
Lane/Commercial Plaza  
Site Code:  
Start Date: 01/10/2019  
Page No: 5



Turning Movement Peak Hour Data Plot (7:30 AM)



Paradigm Transportation Solutions Limited  
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Count Name: Cross Avenue & Lyons  
Lane/Commercial Plaza  
Site Code:  
Start Date: 01/10/2019  
Page No: 6

### Turning Movement Peak Hour Data (12:15 PM)

Start Time	Cross Avenue Eastbound						Cross Avenue Westbound						Lyons Lane Northbound						Plaza Entrance Southbound						Int. Total
	Left	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	Right	U-Turn	Peds	App. Total	
12:15 PM	17	39	1	0	0	57	0	56	10	0	0	66	3	0	3	0	0	6	9	0	13	0	1	22	151
12:30 PM	16	47	4	0	0	67	3	47	4	0	1	54	2	0	2	0	0	4	5	0	28	0	1	33	158
12:45 PM	16	30	0	0	0	46	0	41	6	0	0	47	1	0	3	0	0	4	9	1	18	0	0	28	125
1:00 PM	19	33	1	0	1	53	0	51	7	0	1	58	0	1	0	0	0	1	7	0	23	0	0	30	142
<b>Total</b>	<b>68</b>	<b>149</b>	<b>6</b>	<b>0</b>	<b>1</b>	<b>223</b>	<b>3</b>	<b>195</b>	<b>27</b>	<b>0</b>	<b>2</b>	<b>225</b>	<b>6</b>	<b>1</b>	<b>8</b>	<b>0</b>	<b>0</b>	<b>15</b>	<b>30</b>	<b>1</b>	<b>82</b>	<b>0</b>	<b>2</b>	<b>113</b>	<b>576</b>
Approach %	30.5	66.8	2.7	0.0	-	-	1.3	86.7	12.0	0.0	-	-	40.0	6.7	53.3	0.0	-	-	26.5	0.9	72.6	0.0	-	-	-
Total %	11.8	25.9	1.0	0.0	-	38.7	0.5	33.9	4.7	0.0	-	39.1	1.0	0.2	1.4	0.0	-	2.6	5.2	0.2	14.2	0.0	-	19.6	-
PHF	0.895	0.793	0.375	0.000	-	0.832	0.250	0.871	0.675	0.000	-	0.852	0.500	0.250	0.667	0.000	-	0.625	0.833	0.250	0.732	0.000	-	0.856	0.911
Lights	66	138	6	0	-	210	3	177	26	0	-	206	6	1	8	0	-	15	30	1	81	0	-	112	543
% Lights	97.1	92.6	100.0	-	-	94.2	100.0	90.8	96.3	-	-	91.6	100.0	100.0	100.0	-	-	100.0	100.0	100.0	98.8	-	-	99.1	94.3
Mediums	2	11	0	0	-	13	0	18	1	0	-	19	0	0	0	0	-	0	0	0	1	0	-	1	33
% Mediums	2.9	7.4	0.0	-	-	5.8	0.0	9.2	3.7	-	-	8.4	0.0	0.0	0.0	-	-	0.0	0.0	0.0	1.2	-	-	0.9	5.7
Articulated Trucks	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0
% Articulated Trucks	0.0	0.0	0.0	-	-	0.0	0.0	0.0	0.0	-	-	0.0	0.0	0.0	0.0	-	-	0.0	0.0	0.0	0.0	-	-	0.0	0.0
Bicycles on Road	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0
% Bicycles on Road	0.0	0.0	0.0	-	-	0.0	0.0	0.0	0.0	-	-	0.0	0.0	0.0	0.0	-	-	0.0	0.0	0.0	0.0	-	-	0.0	0.0
Bicycles on Crosswalk	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	0	-	-
% Bicycles on Crosswalk	-	-	-	-	0.0	-	-	-	-	-	0.0	-	-	-	-	-	-	-	-	-	-	-	0.0	-	-
Pedestrians	-	-	-	-	1	-	-	-	-	-	2	-	-	-	-	-	0	-	-	-	-	-	2	-	-
% Pedestrians	-	-	-	-	100.0	-	-	-	-	-	100.0	-	-	-	-	-	-	-	-	-	-	-	100.0	-	-

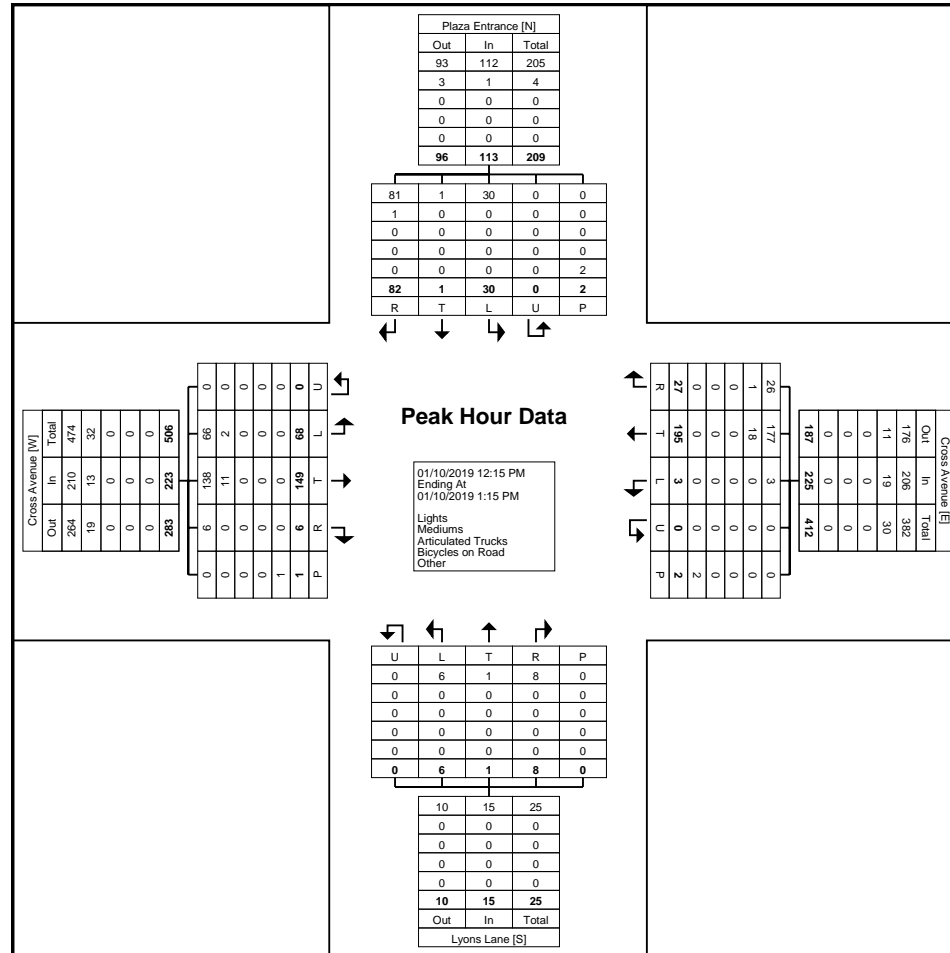




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Count Name: Cross Avenue & Lyons  
Lane/Commercial Plaza  
Site Code:  
Start Date: 01/10/2019  
Page No: 7



Turning Movement Peak Hour Data Plot (12:15 PM)



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Count Name: Cross Avenue & Lyons  
Lane/Commercial Plaza  
Site Code:  
Start Date: 01/10/2019  
Page No: 8

### Turning Movement Peak Hour Data (5:00 PM)

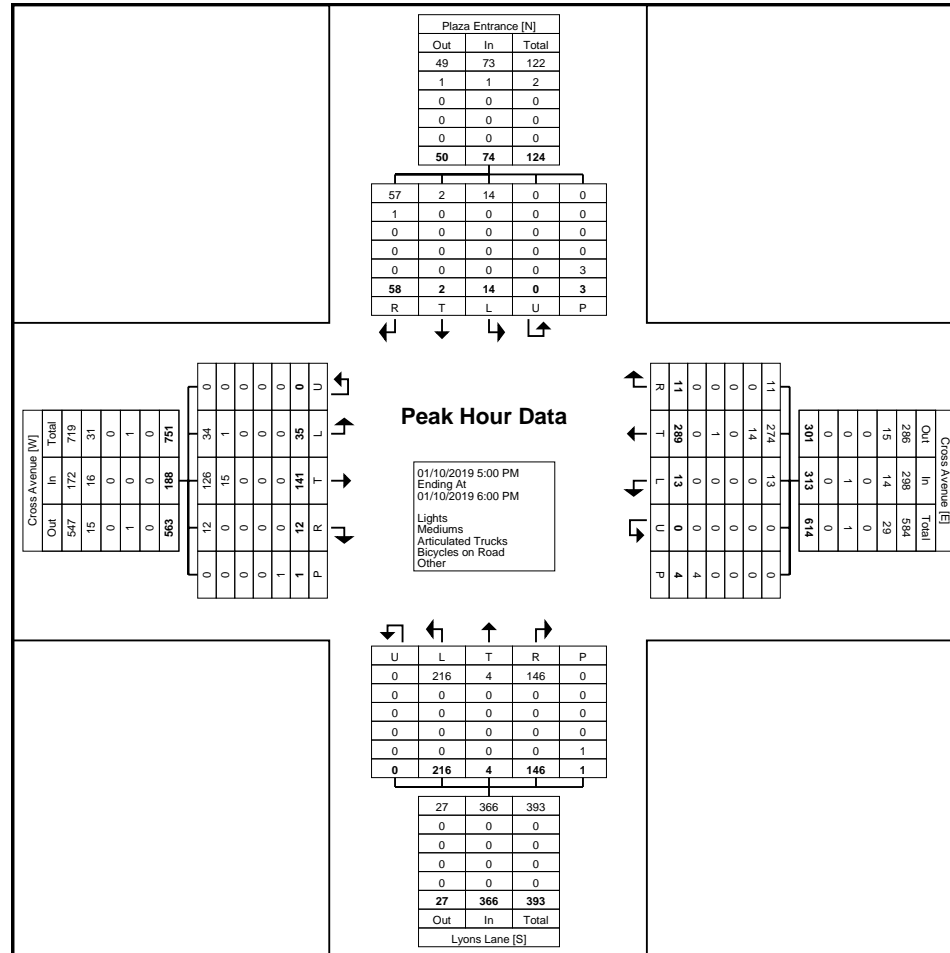
Start Time	Cross Avenue Eastbound						Cross Avenue Westbound						Lyons Lane Northbound						Plaza Entrance Southbound						Int. Total
	Left	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	Right	U-Turn	Peds	App. Total	
5:00 PM	9	41	0	0	0	50	1	76	2	0	0	79	73	0	32	0	1	105	5	0	24	0	0	29	263
5:15 PM	7	24	4	0	1	35	6	73	3	0	1	82	24	0	24	0	0	48	3	0	14	0	1	17	182
5:30 PM	15	42	4	0	0	61	2	66	5	0	1	73	59	1	46	0	0	106	4	1	10	0	1	15	255
5:45 PM	4	34	4	0	0	42	4	74	1	0	2	79	60	3	44	0	0	107	2	1	10	0	1	13	241
<b>Total</b>	<b>35</b>	<b>141</b>	<b>12</b>	<b>0</b>	<b>1</b>	<b>188</b>	<b>13</b>	<b>289</b>	<b>11</b>	<b>0</b>	<b>4</b>	<b>313</b>	<b>216</b>	<b>4</b>	<b>146</b>	<b>0</b>	<b>1</b>	<b>366</b>	<b>14</b>	<b>2</b>	<b>58</b>	<b>0</b>	<b>3</b>	<b>74</b>	<b>941</b>
Approach %	18.6	75.0	6.4	0.0	-	-	4.2	92.3	3.5	0.0	-	-	59.0	1.1	39.9	0.0	-	-	18.9	2.7	78.4	0.0	-	-	-
Total %	3.7	15.0	1.3	0.0	-	20.0	1.4	30.7	1.2	0.0	-	33.3	23.0	0.4	15.5	0.0	-	38.9	1.5	0.2	6.2	0.0	-	7.9	-
PHF	0.583	0.839	0.750	0.000	-	0.770	0.542	0.951	0.550	0.000	-	0.954	0.740	0.333	0.793	0.000	-	0.855	0.700	0.500	0.604	0.000	-	0.638	0.894
Lights	34	126	12	0	-	172	13	274	11	0	-	298	216	4	146	0	-	366	14	2	57	0	-	73	909
% Lights	97.1	89.4	100.0	-	-	91.5	100.0	94.8	100.0	-	-	95.2	100.0	100.0	100.0	-	-	100.0	100.0	100.0	98.3	-	-	98.6	96.6
Mediums	1	15	0	0	-	16	0	14	0	0	-	14	0	0	0	0	-	0	0	0	1	0	-	1	31
% Mediums	2.9	10.6	0.0	-	-	8.5	0.0	4.8	0.0	-	-	4.5	0.0	0.0	0.0	-	-	0.0	0.0	0.0	1.7	-	-	1.4	3.3
Articulated Trucks	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0
% Articulated Trucks	0.0	0.0	0.0	-	-	0.0	0.0	0.0	0.0	-	-	0.0	0.0	0.0	0.0	-	-	0.0	0.0	0.0	0.0	-	-	0.0	0.0
Bicycles on Road	0	0	0	0	-	0	0	1	0	0	-	1	0	0	0	0	-	0	0	0	0	0	-	0	1
% Bicycles on Road	0.0	0.0	0.0	-	-	0.0	0.0	0.3	0.0	-	-	0.3	0.0	0.0	0.0	-	-	0.0	0.0	0.0	0.0	-	-	0.0	0.1
Bicycles on Crosswalk	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	1	-	-
% Bicycles on Crosswalk	-	-	-	-	0.0	-	-	-	-	-	0.0	-	-	-	-	-	0.0	-	-	-	-	-	33.3	-	-
Pedestrians	-	-	-	-	1	-	-	-	-	-	4	-	-	-	-	-	1	-	-	-	-	-	2	-	-
% Pedestrians	-	-	-	-	100.0	-	-	-	-	-	100.0	-	-	-	-	-	100.0	-	-	-	-	-	66.7	-	-



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Count Name: Cross Avenue & Lyons  
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Site Code:  
Start Date: 01/10/2019  
Page No: 9



Turning Movement Peak Hour Data Plot (5:00 PM)



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Count Name: Cross Avenue & Lyons  
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Paradigm Transportation Solutions Limited  
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Count Name: South Service Road & Lyons Lane  
Site Code:  
Start Date: 01/10/2019  
Page No: 1

### Turning Movement Data

Start Time	South Service Road Westbound					Lyons Lane Northbound					Lyons Lane Southbound					Int. Total
	Left	Right	U-Turn	Peds	App. Total	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	U-Turn	Peds	App. Total	
7:00 AM	1	1	0	0	2	0	0	0	0	0	0	0	0	0	0	2
7:15 AM	1	0	0	0	1	0	1	0	0	1	0	1	0	0	1	3
7:30 AM	0	0	0	0	0	1	0	0	0	1	0	0	0	0	0	1
7:45 AM	2	0	0	3	2	1	0	0	0	1	0	2	0	0	2	5
Hourly Total	4	1	0	3	5	2	1	0	0	3	0	3	0	0	3	11
8:00 AM	1	0	2	2	3	1	1	0	0	2	0	1	0	0	1	6
8:15 AM	0	0	0	1	0	1	1	0	0	2	0	1	0	0	1	3
8:30 AM	0	1	0	0	1	0	1	0	1	1	0	0	0	0	0	2
8:45 AM	1	0	0	0	1	0	0	0	0	0	1	0	0	0	1	2
Hourly Total	2	1	2	3	5	2	3	0	1	5	1	2	0	0	3	13
*** BREAK ***	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
11:00 AM	4	2	2	2	8	0	0	0	0	0	2	0	0	0	2	10
11:15 AM	1	0	2	2	3	0	1	0	0	1	0	0	0	0	0	4
11:30 AM	2	0	0	1	2	0	0	0	0	0	0	0	0	0	0	2
11:45 AM	2	1	1	0	4	0	2	0	0	2	1	1	0	0	2	8
Hourly Total	9	3	5	5	17	0	3	0	0	3	3	1	0	0	4	24
12:00 PM	3	1	1	3	5	0	2	0	0	2	1	1	0	0	2	9
12:15 PM	3	1	1	0	5	1	0	0	0	1	1	0	0	0	1	7
12:30 PM	1	0	0	0	1	3	1	0	0	4	0	0	0	0	0	5
12:45 PM	2	2	0	0	4	1	1	0	0	2	2	2	0	0	4	10
Hourly Total	9	4	2	3	15	5	4	0	0	9	4	3	0	0	7	31
1:00 PM	2	0	0	1	2	0	3	0	0	3	0	1	0	0	1	6
1:15 PM	0	0	0	3	0	0	1	0	0	1	0	0	0	2	0	1
1:30 PM	1	0	0	1	1	1	1	0	0	2	0	0	0	0	0	3
1:45 PM	3	2	0	3	5	0	0	0	0	0	1	1	0	0	2	7
Hourly Total	6	2	0	8	8	1	5	0	0	6	1	2	0	2	3	17
*** BREAK ***	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
3:00 PM	1	1	0	0	2	2	0	0	0	2	0	1	0	0	1	5
3:15 PM	2	2	0	2	4	0	1	0	0	1	1	2	0	0	3	8
3:30 PM	2	0	1	3	3	0	1	0	0	1	0	1	0	0	1	5
3:45 PM	2	0	2	2	4	1	2	0	0	3	0	1	0	0	1	8
Hourly Total	7	3	3	7	13	3	4	0	0	7	1	5	0	0	6	26
4:00 PM	6	3	0	0	9	0	1	0	0	1	2	1	0	0	3	13
4:15 PM	3	0	0	0	3	0	0	0	0	0	0	0	0	0	0	3
4:30 PM	4	1	0	0	5	0	0	0	0	0	0	2	0	0	2	7
4:45 PM	2	0	0	4	2	0	0	0	0	0	0	0	0	0	0	2
Hourly Total	15	4	0	4	19	0	1	0	0	1	2	3	0	0	5	25
5:00 PM	2	1	0	0	3	1	2	0	0	3	1	1	0	0	2	8

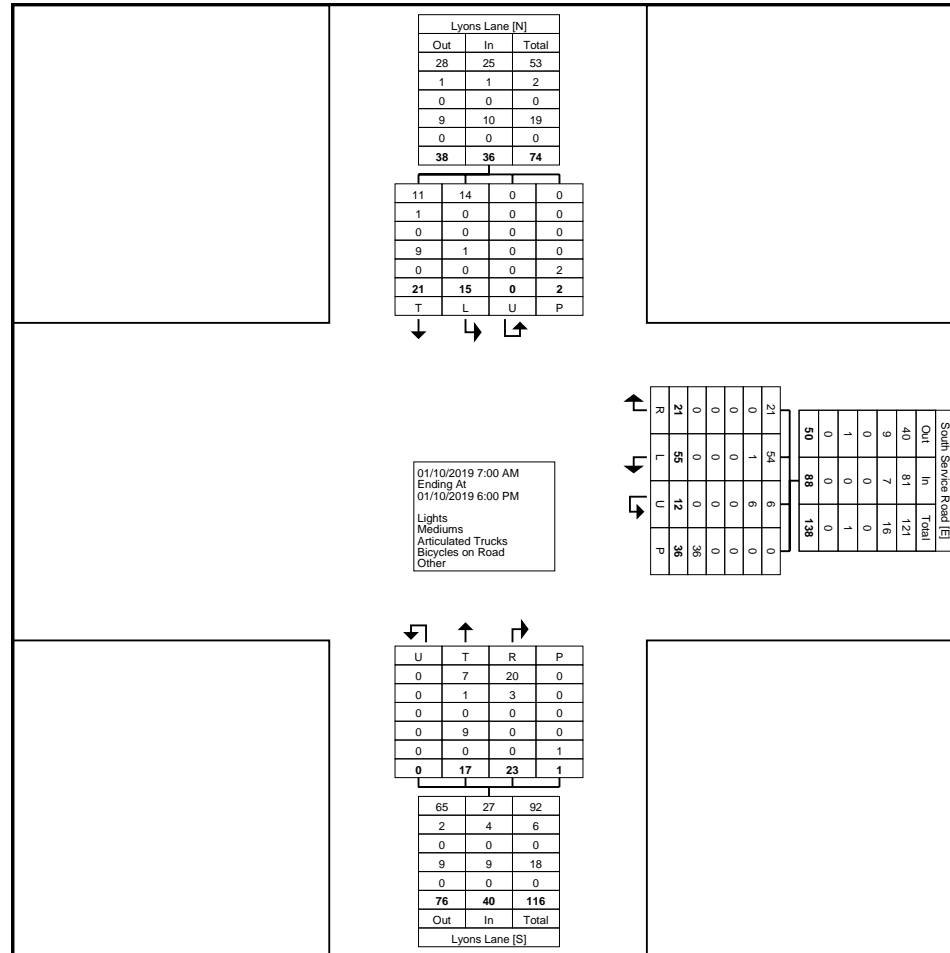
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5:30 PM	0	0	0	0	0	1	0	0	0	1	0	1	0	0	1	2
5:45 PM	0	0	0	3	0	1	0	0	0	1	0	0	0	0	0	1
Hourly Total	3	3	0	3	6	4	2	0	0	6	3	2	0	0	5	17
Grand Total	55	21	12	36	88	17	23	0	1	40	15	21	0	2	36	164
Approach %	62.5	23.9	13.6	-	-	42.5	57.5	0.0	-	-	41.7	58.3	0.0	-	-	-
Total %	33.5	12.8	7.3	-	53.7	10.4	14.0	0.0	-	24.4	9.1	12.8	0.0	-	22.0	-
Lights	54	21	6	-	81	7	20	0	-	27	14	11	0	-	25	133
% Lights	98.2	100.0	50.0	-	92.0	41.2	87.0	-	-	67.5	93.3	52.4	-	-	69.4	81.1
Mediums	1	0	6	-	7	1	3	0	-	4	0	1	0	-	1	12
% Mediums	1.8	0.0	50.0	-	8.0	5.9	13.0	-	-	10.0	0.0	4.8	-	-	2.8	7.3
Articulated Trucks	0	0	0	-	0	0	0	0	-	0	0	0	0	-	0	0
% Articulated Trucks	0.0	0.0	0.0	-	0.0	0.0	0.0	-	-	0.0	0.0	0.0	-	-	0.0	0.0
Bicycles on Road	0	0	0	-	0	9	0	0	-	9	1	9	0	-	10	19
% Bicycles on Road	0.0	0.0	0.0	-	0.0	52.9	0.0	-	-	22.5	6.7	42.9	-	-	27.8	11.6
Bicycles on Crosswalk	-	-	-	0	-	-	-	-	0	-	-	-	-	0	-	-
% Bicycles on Crosswalk	-	-	-	0.0	-	-	-	-	0.0	-	-	-	-	0.0	-	-
Pedestrians	-	-	-	36	-	-	-	-	1	-	-	-	-	2	-	-
% Pedestrians	-	-	-	100.0	-	-	-	-	100.0	-	-	-	-	100.0	-	-



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Count Name: South Service Road & Lyons Lane  
Site Code:  
Start Date: 01/10/2019  
Page No: 3



Turning Movement Data Plot



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Count Name: South Service Road & Lyons Lane  
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Page No: 4

### Turning Movement Peak Hour Data (7:45 AM)

Start Time	South Service Road Westbound					Lyons Lane Northbound					Lyons Lane Southbound					Int. Total
	Left	Right	U-Turn	Peds	App. Total	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	U-Turn	Peds	App. Total	
7:45 AM	2	0	0	3	2	1	0	0	0	1	0	2	0	0	2	5
8:00 AM	1	0	2	2	3	1	1	0	0	2	0	1	0	0	1	6
8:15 AM	0	0	0	1	0	1	1	0	0	2	0	1	0	0	1	3
8:30 AM	0	1	0	0	1	0	1	0	1	1	0	0	0	0	0	2
Total	3	1	2	6	6	3	3	0	1	6	0	4	0	0	4	16
Approach %	50.0	16.7	33.3	-	-	50.0	50.0	0.0	-	-	0.0	100.0	0.0	-	-	-
Total %	18.8	6.3	12.5	-	37.5	18.8	18.8	0.0	-	37.5	0.0	25.0	0.0	-	25.0	-
PHF	0.375	0.250	0.250	-	0.500	0.750	0.750	0.000	-	0.750	0.000	0.500	0.000	-	0.500	0.667
Lights	2	1	2	-	5	2	3	0	-	5	0	1	0	-	1	11
% Lights	66.7	100.0	100.0	-	83.3	66.7	100.0	-	-	83.3	-	25.0	-	-	25.0	68.8
Mediums	1	0	0	-	1	0	0	0	-	0	0	0	0	-	0	1
% Mediums	33.3	0.0	0.0	-	16.7	0.0	0.0	-	-	0.0	-	0.0	-	-	0.0	6.3
Articulated Trucks	0	0	0	-	0	0	0	0	-	0	0	0	0	-	0	0
% Articulated Trucks	0.0	0.0	0.0	-	0.0	0.0	0.0	-	-	0.0	-	0.0	-	-	0.0	0.0
Bicycles on Road	0	0	0	-	0	1	0	0	-	1	0	3	0	-	3	4
% Bicycles on Road	0.0	0.0	0.0	-	0.0	33.3	0.0	-	-	16.7	-	75.0	-	-	75.0	25.0
Bicycles on Crosswalk	-	-	-	0	-	-	-	-	0	-	-	-	-	0	-	-
% Bicycles on Crosswalk	-	-	-	0.0	-	-	-	-	0.0	-	-	-	-	-	-	-
Pedestrians	-	-	-	6	-	-	-	-	1	-	-	-	-	0	-	-
% Pedestrians	-	-	-	100.0	-	-	-	-	100.0	-	-	-	-	-	-	-

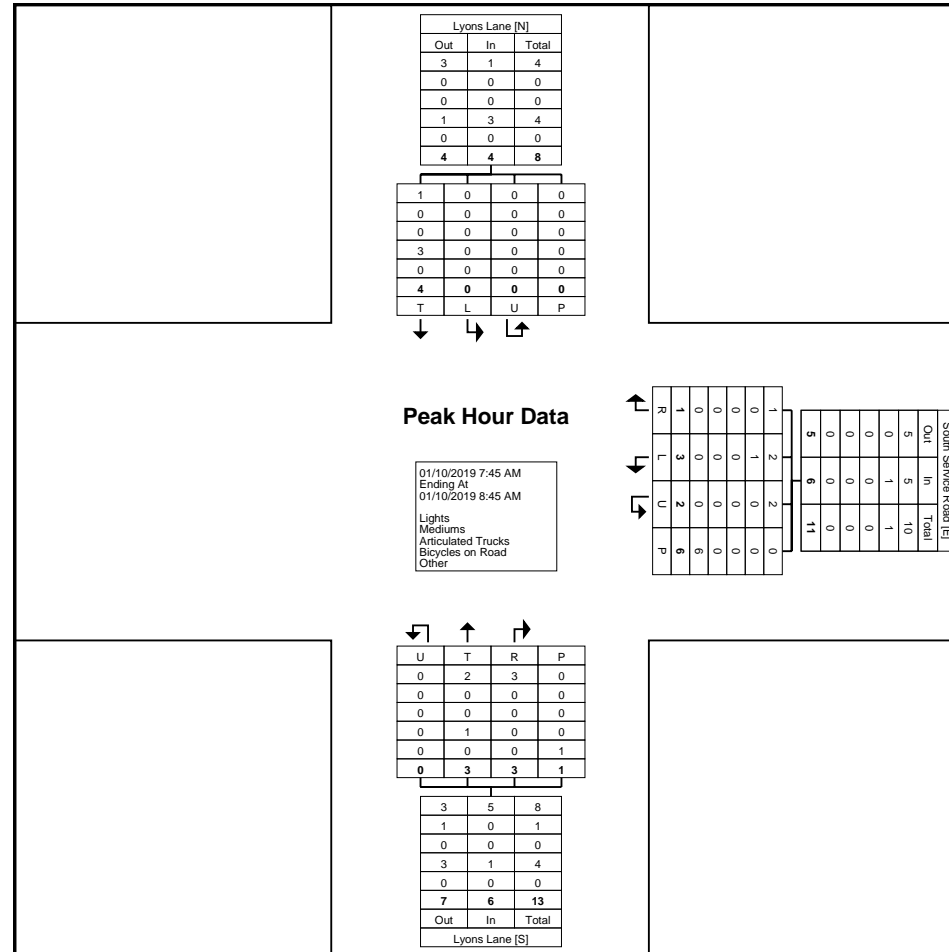




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Turning Movement Peak Hour Data Plot (7:45 AM)

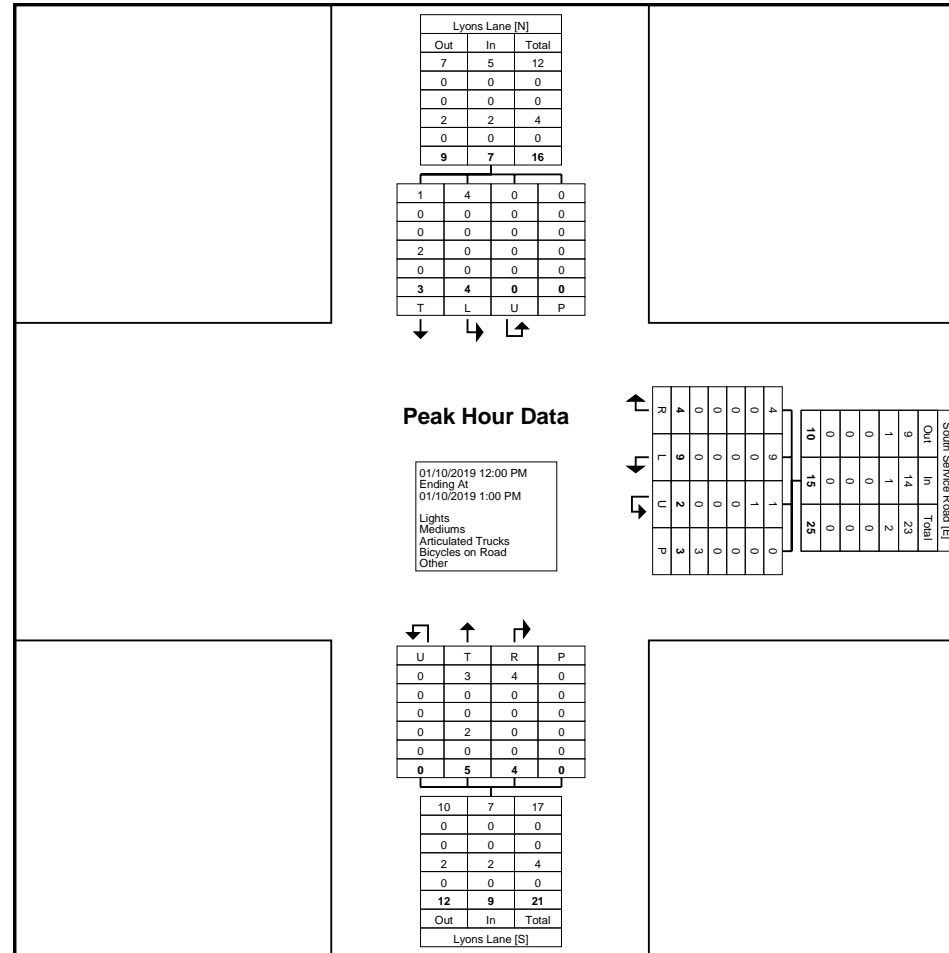




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Turning Movement Peak Hour Data Plot (12:00 PM)

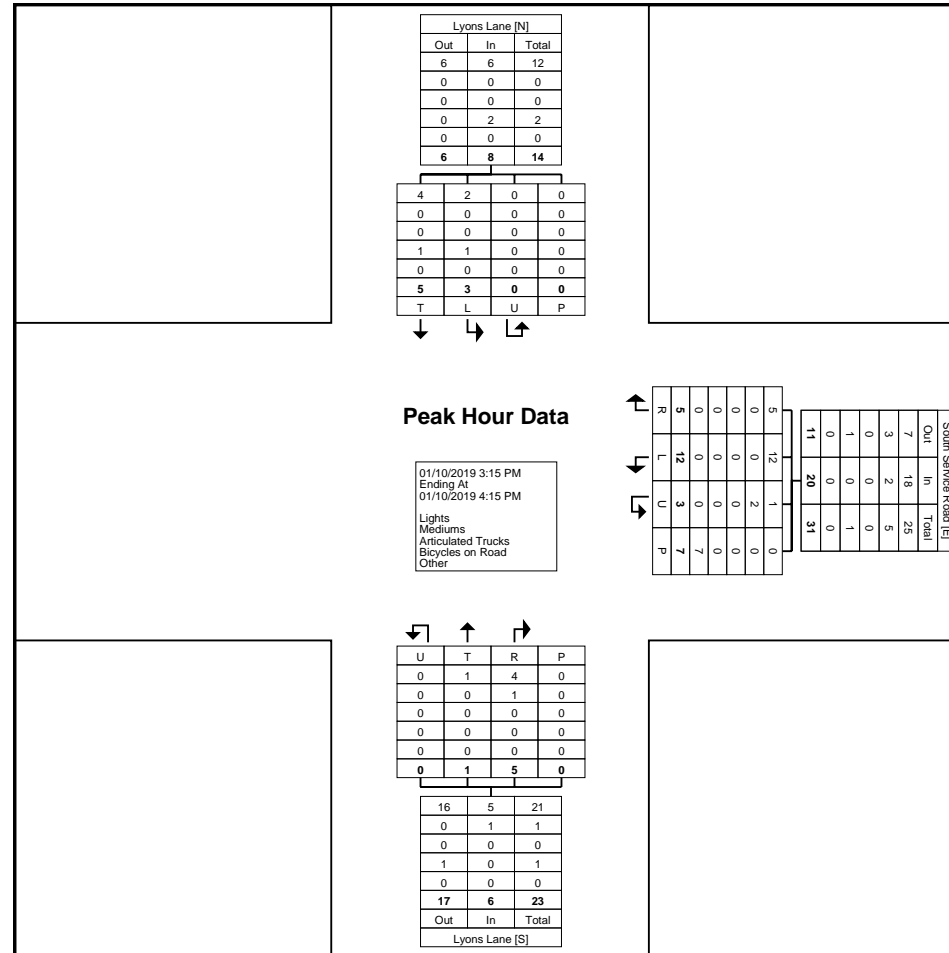




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Turning Movement Peak Hour Data Plot (3:15 PM)



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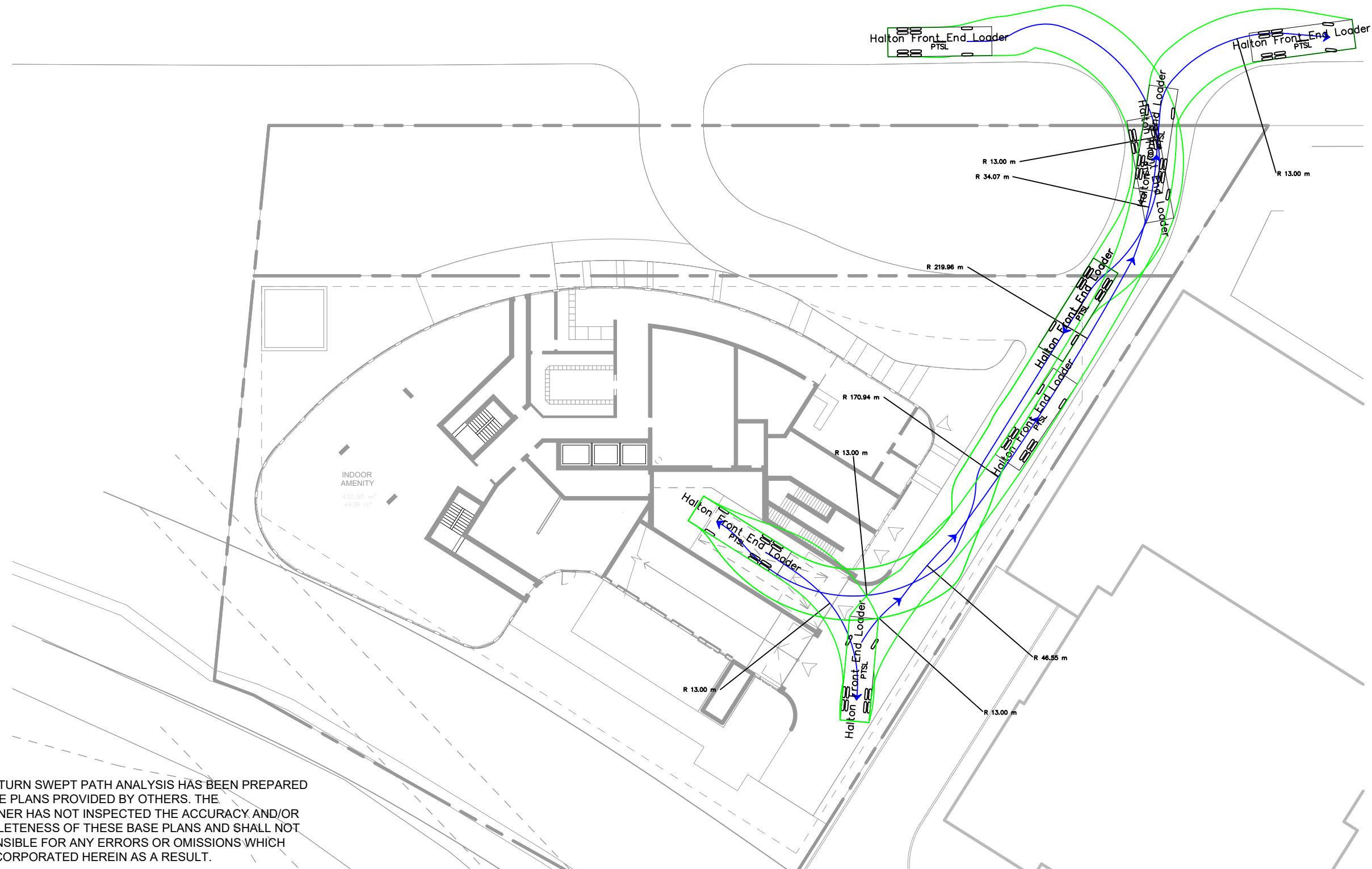
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519-896-3163 cbowness@ptsl.com

Count Name: South Service Road & Lyons Lane  
Site Code:  
Start Date: 01/10/2019  
Page No: 10

# Appendix D

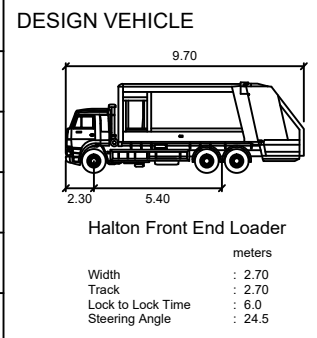
## AutoTURN Assessment





THIS AUTOTURN SWEEP PATH ANALYSIS HAS BEEN PREPARED USING BASE PLANS PROVIDED BY OTHERS. THE PRACTITIONER HAS NOT INSPECTED THE ACCURACY AND/OR THE COMPLETENESS OF THESE BASE PLANS AND SHALL NOT BE RESPONSIBLE FOR ANY ERRORS OR OMISSIONS WHICH MAY BE INCORPORATED HEREIN AS A RESULT.

NO.	DATE	INITIAL	REVISION DETAIL

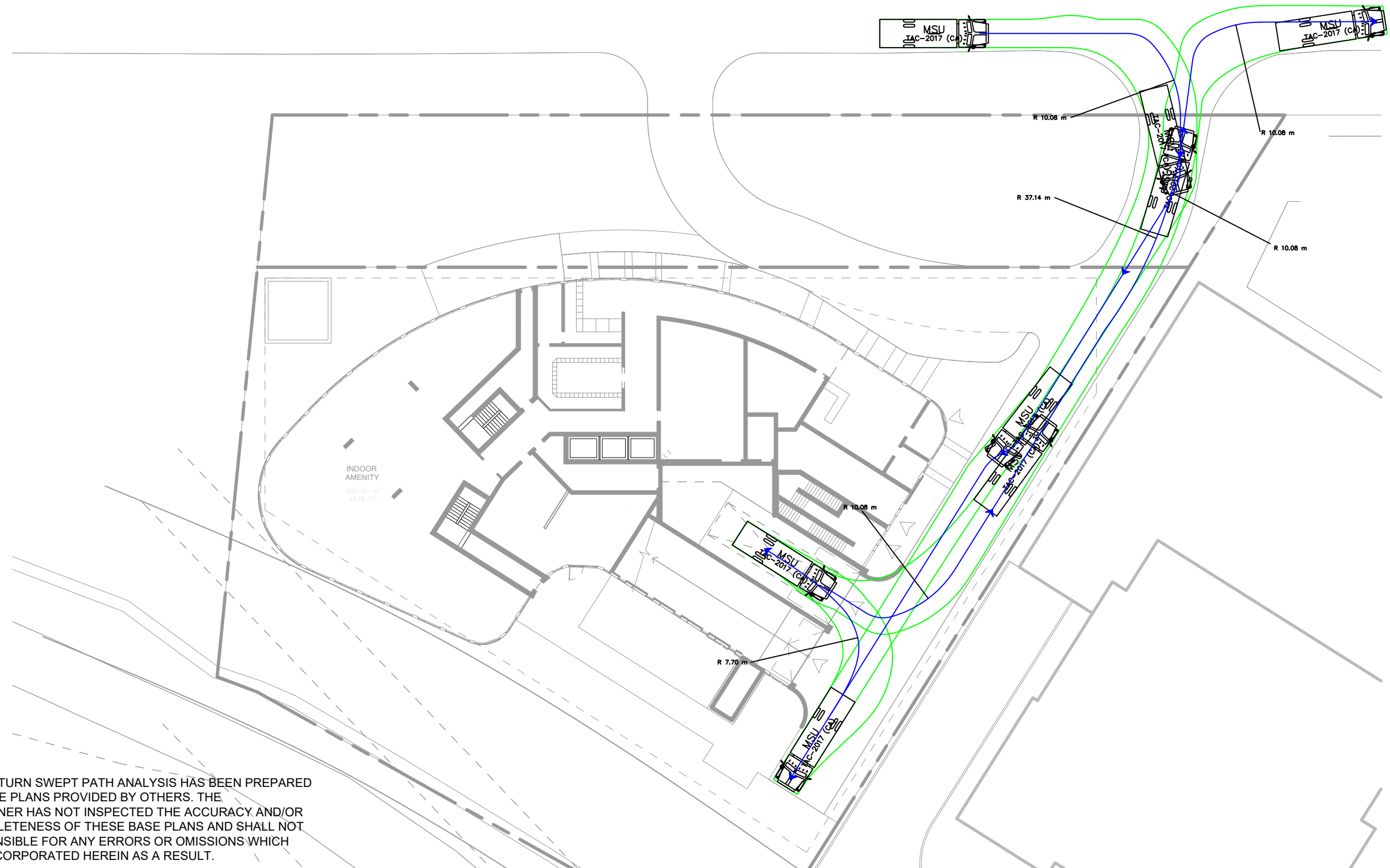


## AUTOTURN ASSESSMENT 627 LYONS LANE, TOWN OF OAKVILLE



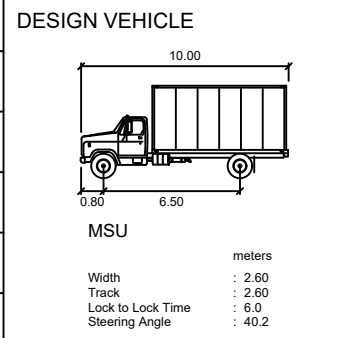
PROJECT NO.: 210060	DATE: MAY 2022	SCALE: 1:400	DRAWING NO.: <b>001</b>
DRAWN: SC	DESIGN: SC	CHECK: AMa	





THIS AUTOTURN SWEEP PATH ANALYSIS HAS BEEN PREPARED USING BASE PLANS PROVIDED BY OTHERS. THE PRACTITIONER HAS NOT INSPECTED THE ACCURACY AND/OR THE COMPLETENESS OF THESE BASE PLANS AND SHALL NOT BE RESPONSIBLE FOR ANY ERRORS OR OMISSIONS WHICH MAY BE INCORPORATED HEREIN AS A RESULT.

NO.	DATE	INITIAL	REVISION DETAIL



## AUTOTURN ASSESSMENT 627 LYONS LANE, TOWN OF OAKVILLE

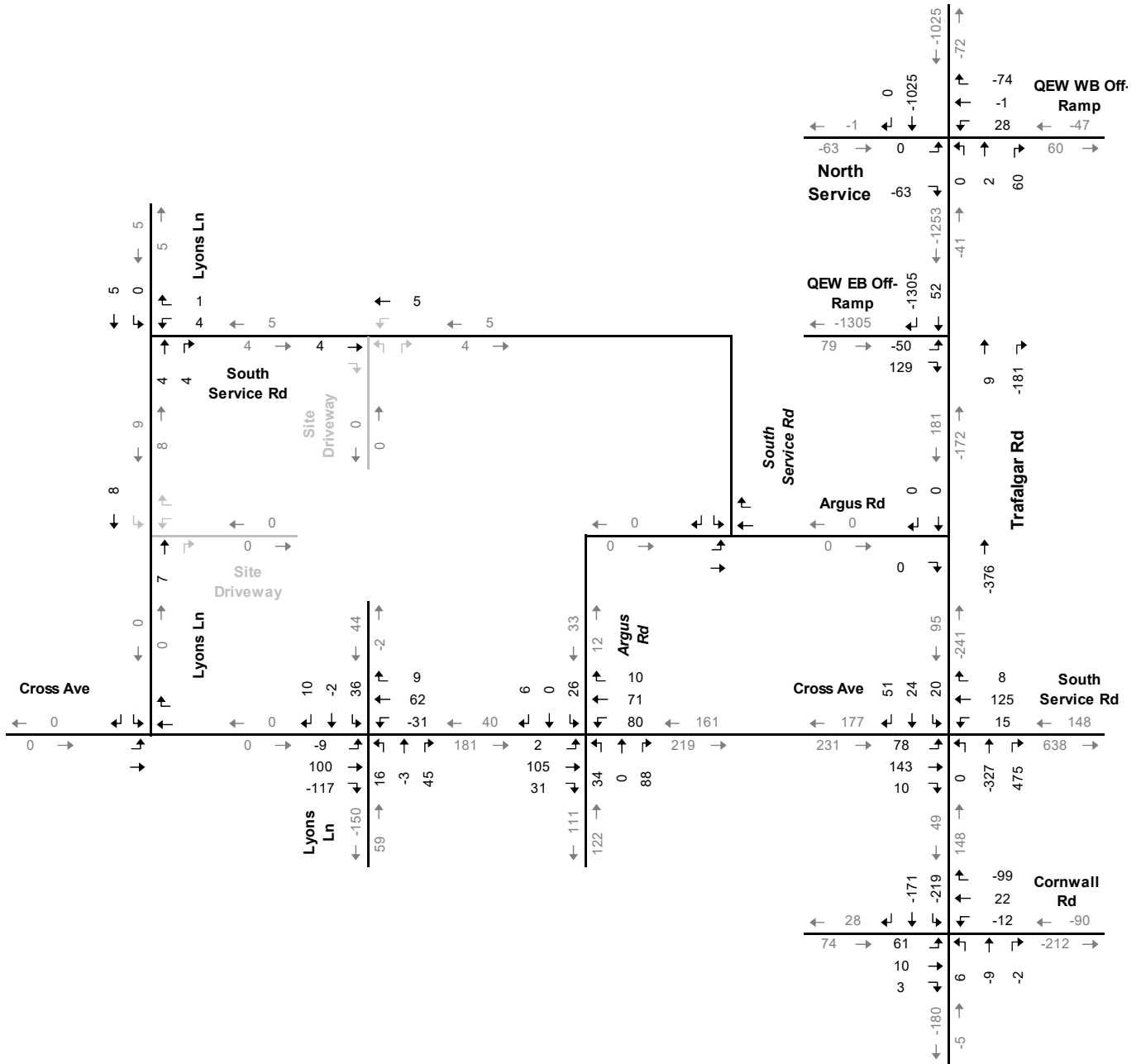


PROJECT NO.: 210060	DATE: MAY 2022	SCALE: 1:400	DRAWING NO.: <b>001</b>
DRAWN: SC	DESIGN: SC	CHECK: AMa	

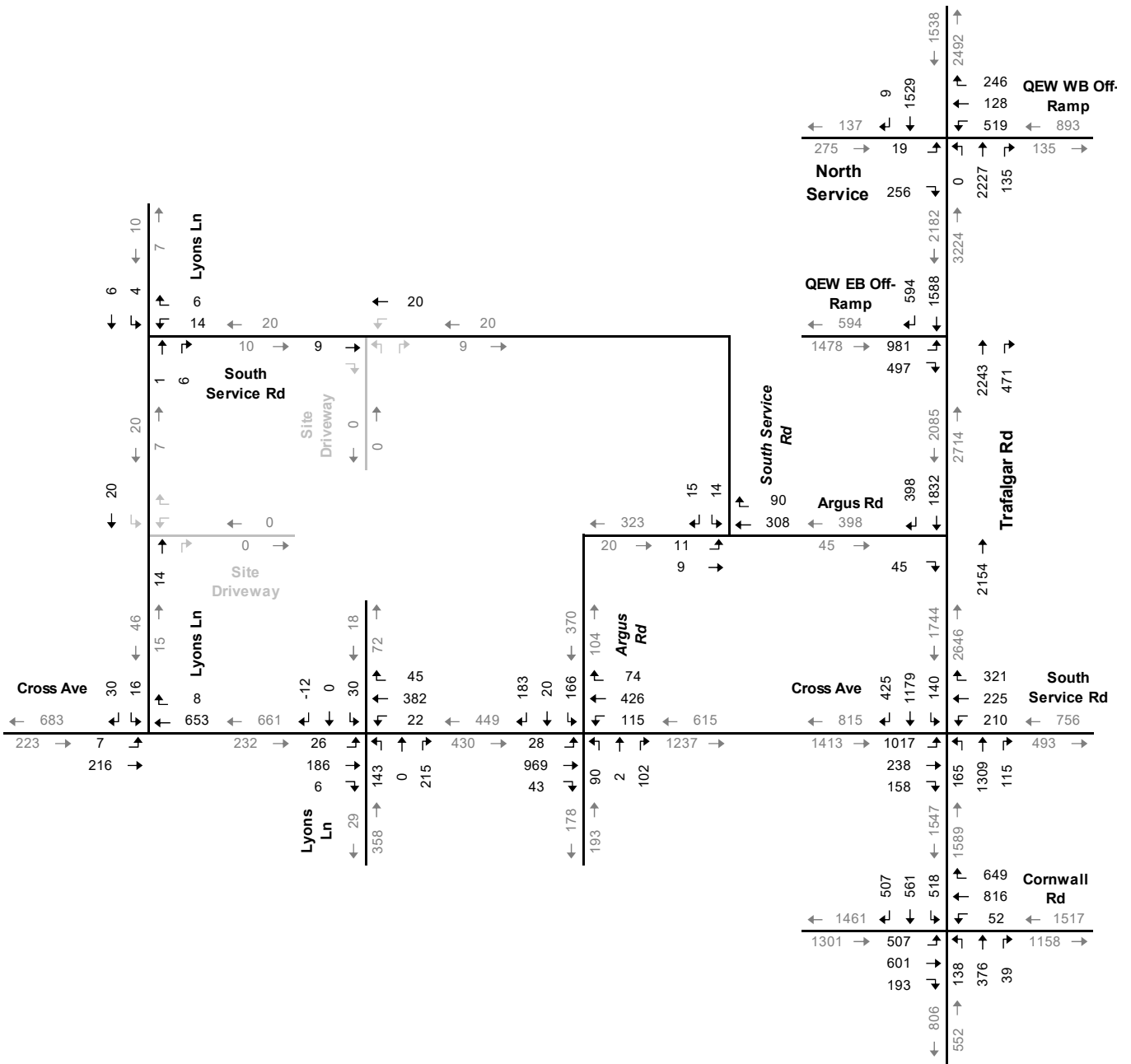
# Appendix E

## Site-Specific Projections

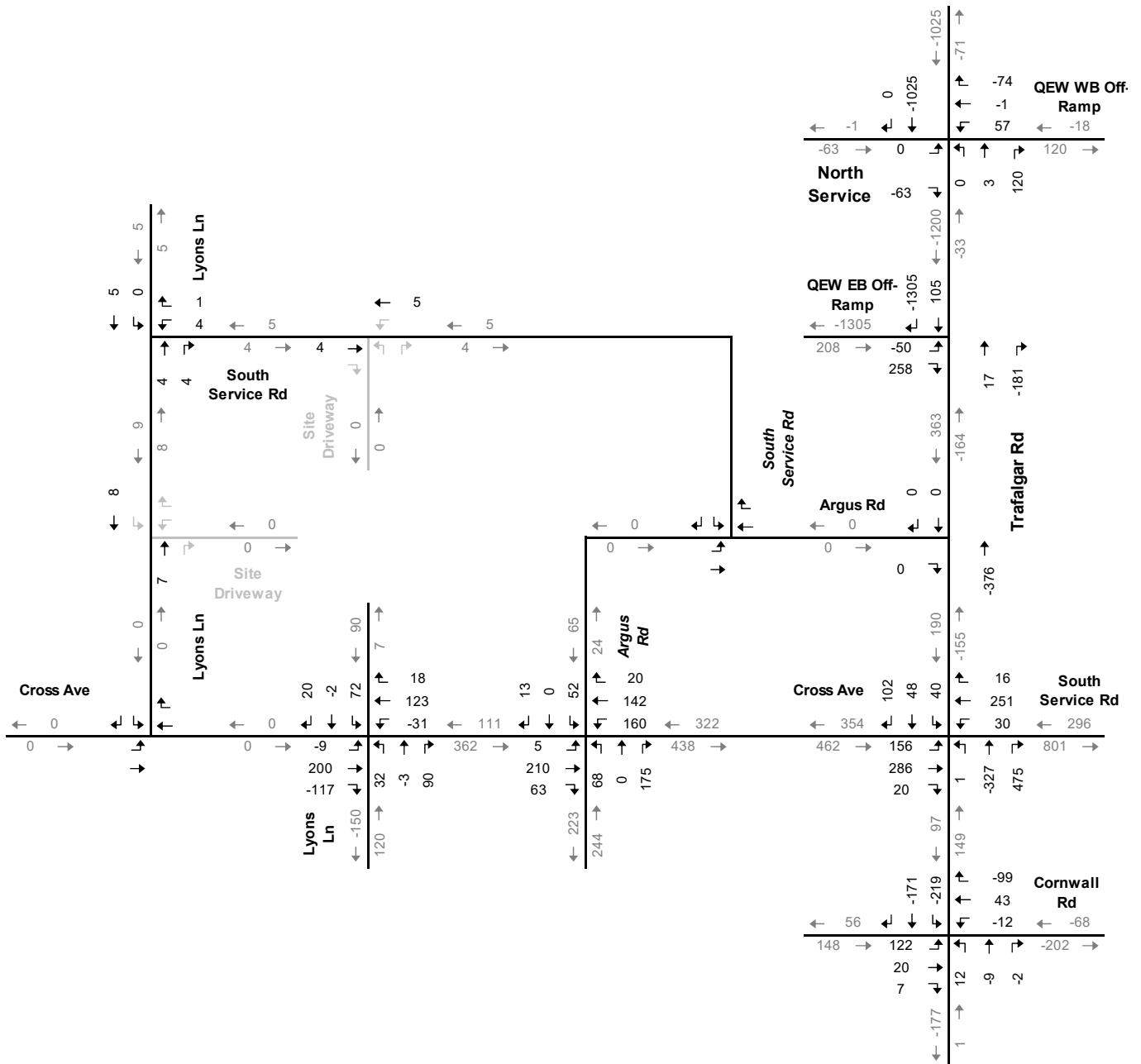




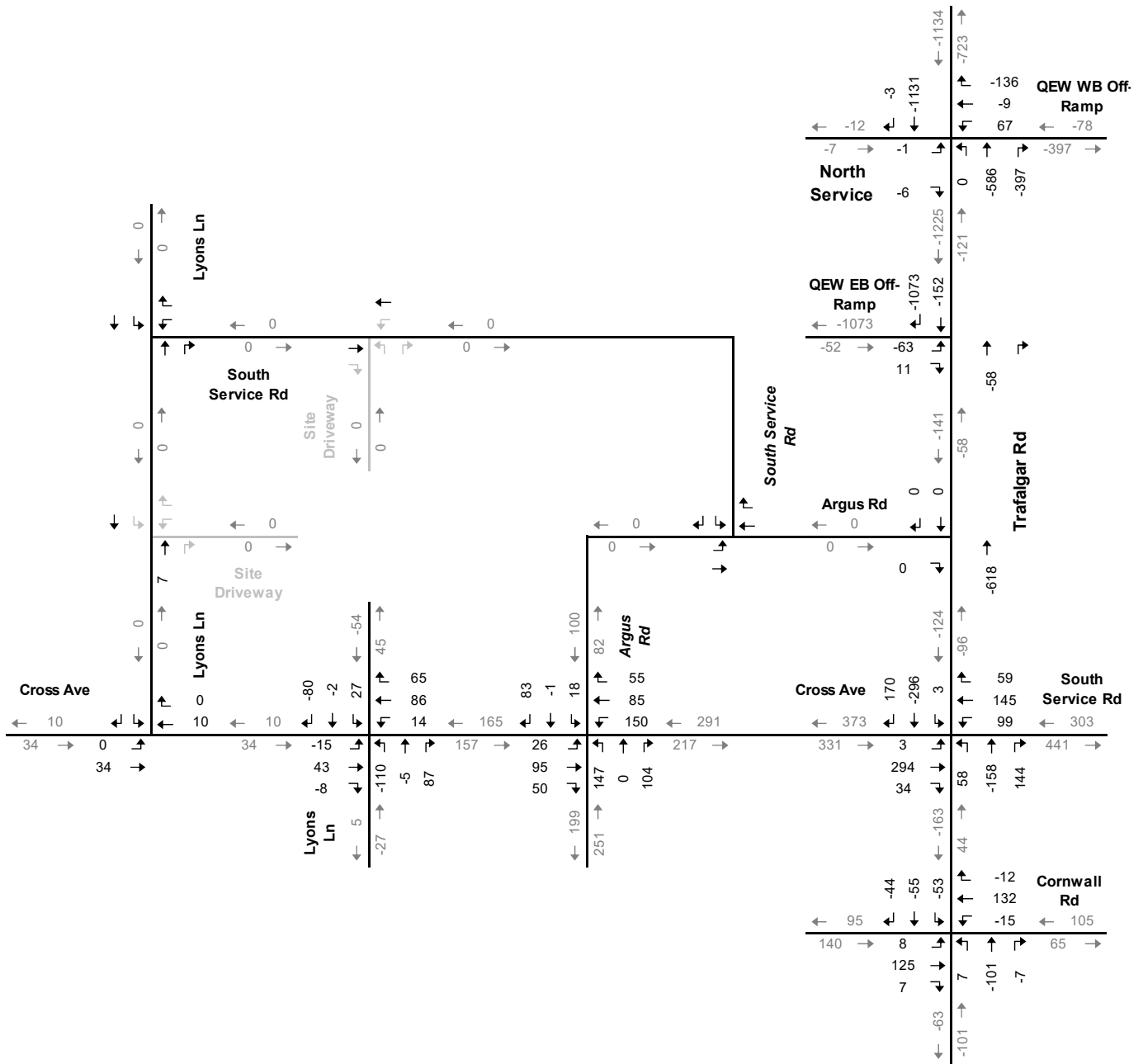
# MOEA Traffic Volumes (20%) AM Peak Hour



# MOEA Traffic Volumes (20%) PM Peak Hour



# MOEA Traffic Volumes (40%) AM Peak Hour



## MOEA Traffic Volumes (40%) PM Peak Hour

# Appendix F

## Operations Reports







Lanes, Volumes, Timings  
1: Trafalgar Road & North Service Road/QEW WB Off Ramp

2022 Existing AM Peak Hour.syn  
04-27-2022

	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Turn Type	Prot	Perm	Perm	Perm	Perm	Perm	NA	NA	NA	NA	NA	Perm
Protected Phases	7						2					6
Permitted Phases		4	8	8	8	8	2	2	2	2	2	6
Detector Phase	7	4	8	8	8	8	2	2	2	2	2	6
Switch Phase												
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Minimum Split (s)	23.0	38.0	38.0	38.0	38.0	38.0	35.0	35.0	35.0	35.0	35.0	35.0
Total Split (s)	23.0	61.0	38.0	38.0	38.0	38.0	79.0	79.0	79.0	79.0	79.0	79.0
Total Split (%)	16.4%	43.6%	27.1%	27.1%	27.1%	27.1%	56.4%	56.4%	56.4%	56.4%	56.4%	56.4%
Maximum Green (s)	18.0	54.0	31.0	31.0	31.0	31.0	72.0	72.0	72.0	72.0	72.0	72.0
Yellow Time (s)	3.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
All-Red Time (s)	2.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Lost Time Adjust (s)	-1.0	-3.0	-3.0	-3.0	-3.0	-3.0	-3.0	-3.0	-3.0	-3.0	-3.0	-3.0
Total Lost Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lead/Lag	Lead	Lag	Lag	Lag	Lag	Lag	Lead	Lead	Lead	Lead	Lead	Lead
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes						
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	None	None	None	None	None	None	C-Max	C-Max	C-Max	C-Max	C-Max	C-Max
Walk Time (s)		7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0
Flash Dont Walk (s)		11.0	11.0	11.0	11.0	11.0	11.0	11.0	11.0	11.0	11.0	11.0
Pedestrian Calls (#/hr)		0	0	0	0	0	0	0	0	0	0	0
Act Effct Green (s)	6.7	31.6	29.3	29.3	29.3	29.3	100.4	100.4	100.4	100.4	100.4	100.4
Actuated g/C Ratio	0.05	0.23	0.21	0.21	0.21	0.21	0.72	0.72	0.72	0.72	0.72	0.72
v/c Ratio	0.01	0.58	0.69	0.71	0.55	0.55	0.55	0.29	1.02	0.01	0.01	0.01
Control Delay	64.0	45.5	61.1	62.1	14.1	14.1	10.5	3.4	42.3	0.0	0.0	0.0
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	64.0	45.5	61.1	62.1	14.1	14.1	10.5	3.4	42.3	0.0	0.0	0.0
LOS	E	D	E	E	B	B	B	A	A	D	D	A
Approach Delay	45.6			44.3			9.5			42.2		
Approach LOS	D			D			A			D		
Queue Length 50th (m)	0.3	49.5	67.3	67.8	12.0	12.0	70.9	2.1	~369.3	0.0		
Queue Length 95th (m)	2.3	63.7	91.7	92.5	37.3	37.3	126.3	m15.8	#510.5	0.0		
Internal Link Dist (m)		96.4		141.1			121.2		185.2			
Turn Bay Length (m)	50.0						70.0					
Base Capacity (vph)	244	656	406	400	546	546	3544	1131	3613	1119		
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0		
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0		
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0		
Reduced v/c Ratio	0.00	0.33	0.58	0.59	0.50	0.50	0.55	0.29	1.02	0.01		
Intersection Summary	Other											
Area Type:	Other											
Control Type:	Unsignalized											
Intersection Capacity Utilization	20.4%											
Analysis Period (min)	15											
ICU Level of Service A												

Lanes, Volumes, Timings  
12: Site Driveway

2035 Total PM Peak Hour.syn  
05-04-2022

	EBT	EBR	WBL	WBT	NBL	NBR
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	10	22	45	23	4	39
Future Volume (vph)	10	22	45	23	4	39
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
FrT	0.907				0.877	
Flt Protected					0.968	0.996
Satd. Flow (prot)	1690	0	0	1803	1627	0
Flt Permitted					0.968	0.996
Satd. Flow (perm)	1690	0	0	1803	1627	0
Link Speed (k/h)	50			50	50	
Link Distance (m)	110.8			715.0	42.7	
Travel Time (s)	8.0			51.5	3.1	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	11	24	49	25	4	42
Shared Lane Traffic (%)						
Lane Group Flow (vph)	35	0	0	74	46	0
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Left	Left	Right
Median Width (m)	0.0	0.0	0.0	0.0	3.6	0.0
Link Offset (m)	0.0	0.0	0.0	0.0	0.0	0.0
Crosswalk Width (m)	4.8			4.8	4.8	
Two way Left Turn Lane						
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (k/h)		15	25		25	15
Sign Control	Free	Free	Free	Free	Stop	Stop
Intersection Summary	Other					
Area Type:	Other					
Control Type:	Unsignalized					
Intersection Capacity Utilization	20.4%					
Analysis Period (min)	15					
ICU Level of Service A						

HCM Unsignalized Intersection Capacity Analysis  
 11: Argus Rd & South Service Rd

2035 Total PM Peak Hour.syn  
 05-04-2022



Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	4	4	1	1	1	1
Traffic Volume (veh/h)	13	10	361	151	28	46
Future Volume (Veh/h)	13	10	361	151	28	46
Sign Control	Free	Free	Free	Stop	Stop	Stop
Grade	0%	0%	0%	0%	0%	0%
Peak Hour Factor	0.45	0.50	0.87	0.68	0.75	0.65
Hourly flow rate (vph)	29	20	415	222	37	71
Pedestrians			5			
Lane Width (m)			3.6			
Walking Speed (m/s)			1.2			
Percent Blockage			0			
Right turn flare (veh)			None	None		
Median type			None	None		
Median storage (veh)						
Upstream signal (m)			358			
pX, platoon unblocked						
vC, conflicting volume			637		609	526
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol			637		609	526
tC, single (s)			4.1		6.4	6.2
tC, 2 stage (s)						
tF (s)			2.2		3.5	3.3
p0 queue free %			97		92	87
cM capacity (veh/h)			956		446	556
Direction_Lane #	EB1	WB1	SB1			
Volume Total	49	637	108			
Volume Left	29	0	37			
Volume Right	0	222	71			
GSH	956	1700	512			
Volume to Capacity	0.03	0.37	0.21			
Queue Length 95th (m)	0.7	0.0	6.3			
Control Delay (s)	5.4	0.0	13.9			
Lane LOS	A	B	B			
Approach Delay (s)	5.4	0.0	13.9			
Approach LOS	B	B	B			
Intersection Summary						
Average Delay			2.2			
Intersection Capacity Utilization			42.9%		ICU Level of Service	A
Analysis Period (min)			15			

Lanes, Volumes, Timings  
 1: Trafalgar Road & North Service Road/QEW WB Off Ramp

2022 Existing AM Peak Hour.syn  
 04-27-2022

Intersection Capacity Utilization	100.2%	ICU Level of Service	G
Analysis Period (min)	15		
~ Volume exceeds capacity, queue is theoretically infinite.			
Queue shown is maximum after two cycles.			
#	95th percentile volume exceeds capacity, queue may be longer.		
Queue shown is maximum after two cycles.			
m	Volume for 95th percentile queue is metered by upstream signal.		



HCM Signalized Intersection Capacity Analysis  
 1: Trafalgar Road & North Service Road/QEW WB Off Ramp

Lanes, Volumes, Timings  
 11: Argus Rd & South Service Rd

2022 Existing AM Peak Hour.syn  
 04-27-2022

2035 Total PM Peak Hour.syn  
 05-04-2022

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	1	0	200	402	33	250	0	1788	305	0	3406	6
Traffic Volume (vph)	1	0	200	402	33	250	0	1788	305	0	3406	6
Future Volume (vph)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Ideal Flow (vphpl)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Total Lost time (s)	1.00	1.00	0.95	1.00	1.00	0.91	1.00	0.91	1.00	1.00	0.91	1.00
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.96	1.00	1.00	0.95	1.00
Frb, ped/bikes	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frb, ped/bikes	1.00	0.85	1.00	1.00	0.85	1.00	0.85	1.00	0.85	1.00	0.85	1.00
Flt Protected	0.95	1.00	0.95	0.96	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Satd. Flow (prot)	1805	1568	1633	1607	1524	4940	1478	5036	1533	5036	1533	1533
Flt Permitted	0.95	1.00	0.95	0.96	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Satd. Flow (perm)	1805	1568	1633	1607	1524	4940	1478	5036	1533	5036	1533	1533
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	1	0	217	437	36	272	0	1943	332	0	3702	7
RTOR Reduction (vph)	0	0	23	0	0	176	0	78	0	0	0	2
Lane Group Flow (vph)	1	0	194	236	237	96	0	1943	254	0	3702	5
Conf. Peds. (#/hr)	0%	0%	3%	5%	23%	6%	0%	5%	5%	0%	3%	0%
Heavy Vehicles (%)	Prot	Perm	Perm	NA	Perm	NA	Perm	NA	Perm	NA	Perm	NA
Turn Type	7	8	8	8	8	8	2	2	2	6	6	6
Protected Phases	1.2	32.5	26.3	26.3	26.3	26.3	93.5	93.5	93.5	93.5	93.5	93.5
Actuated Green, G (s)	2.2	35.5	29.3	29.3	29.3	29.3	96.5	96.5	96.5	96.5	96.5	96.5
Effective Green, g (s)	0.02	0.25	0.21	0.21	0.21	0.21	0.69	0.69	0.69	0.69	0.69	0.69
Actuated g/C Ratio	5.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0
Clearance Time (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Vehicle Extension (s)	28	397	341	336	318	3405	1018	3471	1056	3471	1056	1056
Lane Grp. Cap (vph)	0.00	c0.12	0.14	0.15	0.06	0.39	0.39	c0.74	0.17	0.25	1.07	0.00
v/s Ratio Prot	0.04	0.49	0.69	0.71	0.30	0.57	0.25	0.57	0.25	1.07	0.00	0.00
v/s Ratio Perm	67.9	44.5	51.2	51.3	46.7	11.1	8.2	21.8	6.8	21.8	6.8	6.8
Uniform Delay, d1	1.00	1.00	1.00	1.00	1.00	1.00	0.96	1.19	1.00	1.00	1.00	1.00
Progression Factor	0.5	0.9	6.0	6.6	0.5	0.3	0.3	0.3	0.3	36.7	0.0	0.0
Incremental Delay, d2	68.4	45.5	57.1	57.9	47.3	11.0	10.0	58.5	6.8	58.5	6.8	6.8
Delay (s)	E	D	E	E	D	B	B	E	A	E	A	A
Level of Service	E	D	E	E	D	B	B	E	A	E	A	A
Approach Delay (s)	45.6	53.8	53.8	53.8	53.8	10.8	10.8	58.4	6.8	58.4	6.8	6.8
Approach LOS	D	D	D	D	D	B	B	E	A	E	A	A
Intersection Summary												
HCM 2000 Control Delay	41.9 HCM 2000 Level of Service D											
HCM 2000 Volume to Capacity ratio	0.98											
Actuated Cycle Length (s)	140.0 Sum of lost time (s) 12.0											
Intersection Capacity Utilization	100.2% ICU Level of Service G											
Analysis Period (min)	15											
c Critical Lane Group												

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	13	10	361	151	28	46						
Traffic Volume (vph)	13	10	361	151	28	46						
Future Volume (vph)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Ideal Flow (vphpl)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Lane Util. Factor	0.971	0.971	0.971	0.971	0.971	0.971	0.971	0.971	0.971	0.971	0.971	0.971
Flt Protected	0	1577	1530	0	1531	0						
Satd. Flow (prot)	0	1577	1530	0	1531	0						
Flt Permitted	0	1577	1530	0	1531	0						
Satd. Flow (perm)	0	1577	1530	0	1531	0						
Link Speed (k/h)	50	50	50	50	50	50						
Link Distance (m)	1777.7	165.2	103.5									
Travel Time (s)	12.8	11.9	7.5									
Conf. Peds. (#/hr)	5											
Peak Hour Factor	0.45	0.50	0.87	0.68	0.75	0.65						
Heavy Vehicles (%)	0%	13%	10%	0%	0%	0%						
Adj. Flow (vph)	29	20	415	222	37	71						
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	49	637	0	108	0						
Enter Blocked Intersection	No	No	No	No	No	No						
Lane Alignment	Left	Left	Left	Right	Left	Right						
Median Width(m)	0.0	0.0	0.0	3.6	0.0	0.0						
Link Offset(m)	0.0	0.0	0.0	0.0	0.0	0.0						
Crosswalk Width(m)	4.8	4.8	4.8	4.8	4.8	4.8						
Two way Left Turn Lane												
Headway Factor	1.14	1.14	1.14	1.14	1.14	1.14						
Turning Speed (k/h)	24	Free	Free	Free	Free	Free						
Sign Control												
Intersection Summary												
Area Type:	CBD											
Control Type:	Unsignalized											
Intersection Capacity Utilization	42.9% ICU Level of Service A											
Analysis Period (min)	15											

HCM Unsignalized Intersection Capacity Analysis  
 10: Lyons Lane & South Service Road

2022 Existing AM Peak Hour.syn  
 2: Trafalgar Road & QEW SB On Ramps

05-04-2022

→ ← → ← → ← → ←

Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	4	6	1	28	21	6
Traffic Volume (veh/h)	4	6	1	28	21	6
Future Volume (Veh/h)	4	6	1	28	21	6
Sign Control	Free	Free	0%	Stop	0%	0%
Grade	0%	0%	0%	0%	0%	0%
Peak Hour Factor	0.38	0.62	0.25	0.62	0.50	0.42
Hourly flow rate (vph)	11	10	4	45	42	14
Pedestrians						
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)						
Median type						
Median storage (veh)						
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume						
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
tC, single (s)						
tC, 2 stage (s)						
tF (s)						
p0 queue free %						
cM capacity (veh/h)						
Direction, Lane #	EB1	WB1	SB1			
Volume Total	21	49	56			
Volume Left	11	0	42			
Volume Right	0	45	14			
GSH	1553	1700	957			
Volume to Capacity	0.01	0.03	0.06			
Queue Length 95th (m)	0.2	0.0	1.5			
Control Delay (s)	3.9	0.0	9.0			
Lane LOS	A	A	A			
Approach Delay (s)	3.9	0.0	9.0			
Approach LOS	A	A	A			
Intersection Summary						
Average Delay				4.6		
Intersection Capacity Utilization				15.4%		ICU Level of Service A
Analysis Period (min)				15		

2022 Existing AM Peak Hour.syn  
 2: Trafalgar Road & QEW SB On Ramps

04-27-2022

→ ← → ← → ← → ←

Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Volume (vph)	0	0	0	2081	2259	1749
Future Volume (vph)	0	0	0	2081	2259	1749
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	0.91	0.91	1.00
FrT						
Flt Protected						
Satd. Flow (prot)	0	0	0	5085	5085	1583
Flt Permitted						
Satd. Flow (perm)	0	0	0	5085	5085	1583
Link Speed (k/h)	50			50	50	
Link Distance (m)	159.6			186.0	145.2	
Travel Time (s)	11.5			13.4	10.5	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	0	0	0	2262	2455	1901
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	0	0	2262	2455	1901
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Left	Left	Right
Median Width(m)	0.0			0.0	0.0	
Link Offset(m)	0.0			0.0	0.0	
Crosswalk Width(m)	4.8			4.8	4.8	
Two way Left Turn Lane						
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (k/h)	25	15	25	Free	Free	15
Sign Control	Stop	Stop	Free	Free	Free	Free
Intersection Summary						
Area Type:						Other
Control Type:						Unsignalized
Intersection Capacity Utilization						111.6%
Analysis Period (min)						15
ICU Level of Service						H

HCM Unsignalized Intersection Capacity Analysis  
 2: Trafalgar Road & QEW SB On Ramps

Lanes, Volumes, Timings  
 10: Lyons Lane & South Service Road

2022 Existing AM Peak Hour.syn  
 04-27-2022

2035 Total PM Peak Hour.syn  
 05-04-2022

Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations			↑↑↑	↑↑↑	↑↑↑	↑
Traffic Volume (veh/h)	0	0	0	2081	2259	1749
Future Volume (Veh/h)	0	0	0	2081	2259	1749
Sign Control	Stop	Free	Free	Free	Free	Free
Grade	0%	0%	0%	0%	0%	0%
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	0	0	0	2262	2455	1901
Pedestrians						
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)						
Median type			None	None	None	None
Median storage (veh)						
Upstream signal (m)			186	145		
pX, platoon unblocked	0.60	0.42	0.42			
vC, conflicting volume	3209	818	2455			
vC1, stage 1 conf/vol						
vC2, stage 2 conf/vol						
vCu, unblocked vol	0	0	0			
tC, single (s)	6.8	6.9	4.1			
tC, 2 stage (s)						
tF (s)	3.5	3.3	2.2			
p0 queue free %	100	100	100			
cM capacity (veh/h)	617	463	678			
Direction_Lane #	NB.1	NB.2	NB.3	SB.1	SB.2	SB.3
Volume Total	754	754	818	818	818	1901
Volume Left	0	0	0	0	0	0
Volume Right	0	0	0	0	0	1901
GSH	1700	1700	1700	1700	1700	1700
Volume to Capacity	0.44	0.44	0.44	0.48	0.48	0.48
Queue Length 95th (m)	0.0	0.0	0.0	0.0	0.0	0.0
Control Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0
Lane LOS						
Approach Delay (s)	0.0			0.0		
Approach LOS						
Intersection Summary						
Average Delay	0.0					
Intersection Capacity Utilization	111.6%					
ICU Level of Service	H					
Analysis Period (min)	15					

Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	←	←	←	←	←	←
Traffic Volume (vph)	4	6	1	28	21	6
Future Volume (vph)	4	6	1	28	21	6
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor						
Ft		0.876			0.966	
Ft Protected		0.974			0.964	
Satd. Flow (prot)		1666		1246	1592	
Ft Permitted		0.974			0.964	
Satd. Flow (perm)		1666		1246	1592	
Link Speed (k/h)		50		50	50	
Link Distance (m)		60.5		89.6	37.6	
Travel Time (s)		4.4		6.5	2.7	
Conf. Peds. (#/hr)		7		7		
Peak Hour Factor	0.38	0.62	0.25	0.62	0.50	0.42
Heavy Vehicles (%)	0%	0%	0%	22%	0%	0%
Adj. Flow (vph)	11	10	4	45	42	14
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	21	49	0	56	0
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Left	Left	Right	Left	Right
Median Width(m)		0.0	0.0		3.6	
Link Offset(m)		0.0	0.0		0.0	
Crosswalk Width(m)		4.8	4.8		4.8	
Two way Left Turn Lane						
Headway Factor	1.14	1.14	1.14	1.14	1.14	1.14
Turning Speed (k/h)	24			14	24	14
Sign Control		Free	Free		Stop	
Intersection Summary						
Area Type:	CBD					
Control Type:	Unsignalized					
Intersection Capacity Utilization	15.4%					
ICU Level of Service A						
Analysis Period (min)	15					



Lanes, Volumes, Timings  
3: Trafalgar Road & QEW EB Off Ramp

Lanes, Volumes, Timings  
8: Cross Ave & Lyons Lane

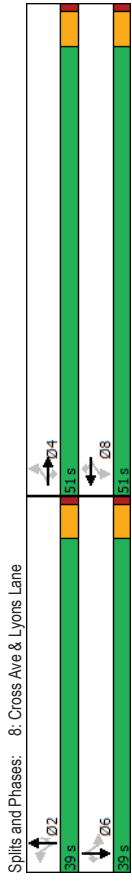
2022 Existing AM Peak Hour.syn  
04-27-2022

2035 Total PM Peak Hour.syn  
05-04-2022



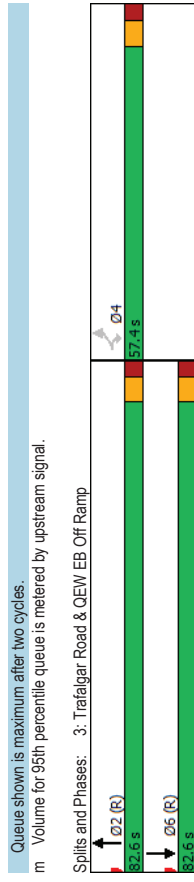
Analysis Period (min) 15

Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Minimum Initial (s)	10.0	10.0		29.0	29.0	
Minimum Split (s)	38.0	38.0		36.0	36.0	
Total Split (s)	57.4	57.4		82.6	82.6	
Total Split (%)	41.0%	41.0%		59.0%	59.0%	
Maximum Green (s)	50.4	50.4		75.6	75.6	
Yellow Time (s)	4.0	4.0		4.0	4.0	
All-Red Time (s)	3.0	3.0		3.0	3.0	
Lost Time Adjust (s)	-3.0	-3.0		-3.0	-3.0	
Total Lost Time (s)	4.0	4.0		4.0	4.0	
Lead/Lag						
Lead/Lag Optimize?						
Vehicle Extension (s)	3.0	3.0		3.0	3.0	
Recall Mode	None	None		C-Max	C-Max	
Walk Time (s)	7.0	7.0		7.0	7.0	
Flash Dont Walk (s)	11.0	11.0		11.0	11.0	
Pedestrian Calls (#/hr)	0	0		0	0	
Act Effct Green (s)	53.4	53.4		78.6	78.6	
Actuated g/C Ratio	0.38	0.38		0.56	0.56	
v/c Ratio	0.76	1.12		0.89	0.46	
Control Delay	42.4	110.1		42.4	15.2	
Queue Delay	0.0	0.0		0.0	0.0	
Total Delay	42.4	110.1		42.4	15.2	
LOS	D	F		D	B	
Approach Delay	70.9			42.4	15.2	
Approach LOS	E			D	B	
Queue Length 50th (m)	128.2	~230.7		234.0	60.9	
Queue Length 95th (m)	154.9	#310.5		275.3	m50.8	
Internal Link Dist (m)	47.3			26.7	162.0	
Turn Bay Length (m)						
Base Capacity (vph)	1296	642		2747	2800	
Starvation Cap Reductn	0	0		0	0	
Spillback Cap Reductn	0	0		0	0	
Storage Cap Reductn	0	0		0	0	
Reduced v/c Ratio	0.76	1.12		0.89	0.46	
Intersection Summary						
Area Type:	Other					
Cycle Length:	140					
Actuated Cycle Length:	140					
Offset:	0 (0%). Referenced to phase 2:NBT and 6:SBT, Start of Green					
Natural Cycle:	100					
Control Type:	Actuated-Coordinated					
Maximum v/c Ratio:	1.12					
Intersection Signal Delay:	44.9					
Intersection Capacity Utilization:	111.6%					
Analysis Period (min):	15					
~ Volume exceeds capacity, queue is theoretically infinite.						
# 95th percentile volume exceeds capacity, queue may be longer.						



Splits and Phases: 8: Cross Ave & Lyons Lane

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Perm	NA	NA	NA	Perm	NA	Perm	Perm	Perm	Perm	Perm	NA	Perm
Protected Phases	4	4	4	8	8	8	2	2	2	2	6	6
Detector Phase	4	4	4	8	8	8	2	2	2	2	6	6
Switch Phase												
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Minimum Split (s)	22.5	22.5	22.5	22.5	22.5	22.5	22.5	22.5	22.5	22.5	22.5	22.5
Total Split (s)	51.0	51.0	51.0	51.0	51.0	51.0	39.0	39.0	39.0	39.0	39.0	39.0
Total Split (%)	56.7%	56.7%	56.7%	56.7%	56.7%	56.7%	43.3%	43.3%	43.3%	43.3%	43.3%	43.3%
Maximum Green (s)	46.5	46.5	46.5	46.5	46.5	46.5	34.5	34.5	34.5	34.5	34.5	34.5
Yellow Time (s)	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5
LeadLag												
LeadLag Optimize?												
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	None	None	None	None	None	None	Max	Max	Max	Max	Max	Max
Walk Time (s)	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0
Flash Dont Walk (s)	11.0	11.0	11.0	11.0	11.0	11.0	11.0	11.0	11.0	11.0	11.0	11.0
Pedestrian Calls (#/hr)	0	0	0	0	0	0	0	0	0	0	0	0
Act Effct Green (s)	20.9	20.9	20.9	20.9	20.9	20.9	34.7	34.7	34.7	34.7	34.7	34.7
Actuated g/C Ratio	0.32	0.32	0.32	0.32	0.32	0.32	0.54	0.54	0.54	0.54	0.54	0.54
v/c Ratio	0.75	0.26	0.02	0.11	0.70	0.32	0.30	0.30	0.22	0.00	0.12	0.12
Control Delay	56.7	16.4	4.7	15.3	22.9	4.2	11.2	4.2	0.7	10.4	9.0	3.1
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	56.7	16.4	4.7	15.3	22.9	4.2	11.2	4.2	0.7	10.4	9.0	3.1
LOS	E	B	A	B	C	A	B	A	B	A	B	A
Approach Delay	26.4			18.9			4.8				7.6	
Approach LOS	C			B			A				A	
Queue Length 50th (m)	9.6	11.7	0.0	3.0	40.5	0.0	12.8	0.0	0.0	9.2	0.1	0.0
Queue Length 95th (m)	11.2	19.2	2.0	8.6	51.7	1.1	32.5	0.0	0.0	10.5	0.9	0.0
Internal Link Dist (m)	257.3			16.1			148.6				17.7	
Turn Bay Length (m)	60.0	60.0	60.0	60.0	60.0	60.0	60.0	60.0	60.0	60.0	60.0	60.0
Base Capacity (vph)	266	2137	1036	728	2282	1080	681	1040	685	899	793	793
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.33	0.11	0.01	0.05	0.31	0.17	0.30	0.30	0.22	0.00	0.12	0.12









HCM Unsignalized Intersection Capacity Analysis  
 4: Trafalgar Road & Argus Road/QEW EB On Ramp

2022 Existing AM Peak Hour.syn  
 04-27-2022

	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Movement												
Lane Configurations	0	0	17	0	0	0	0	1174	587	0	2134	713
Traffic Volume (veh/h)	0	0	17	0	0	0	0	1174	587	0	2134	713
Future Volume (Veh/h)	0	0	17	0	0	0	0	1174	587	0	2134	713
Sign Control	Stop	Stop	Stop	0%	0%	0%	Free	Free	Free	Free	Free	Free
Grade	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Peak Hour Factor	0	0	18	0	0	0	0	1276	638	0	2320	775
Hourly flow rate (vph)	0	0	18	0	0	0	0	1276	638	0	2320	775
Pedestrians	24											
Lane Width (m)	3.6											
Walking Speed (m/s)	1.2											
Percent Blockage	2											
Right turn flare (veh)												
Median type							None	None	None	None	None	None
Median storage (veh)												
Upstream signal (m)							271					51
pX, platoon unblocked	0.91	0.91	0.85	0.91	0.91	0.89	0.85			0.89		
vC, conflicting volume	3157	4008	1185	2049	4395	425	3119			1276		
vC1, stage 1 conf vol												
vC2, stage 2 conf vol	2172	3110	603	952	3536	0	2877			861		
vCu, unblocked vol	7.5	6.5	7.1	7.5	6.5	6.9	4.1			4.1		
tC, single (s)	3.5	4.0	3.4	3.5	4.0	3.3	2.2			2.2		
tC, 2 stage (s)	100	100	95	100	100	100	100			100		
tF (s)	23	10	356	184	5	967	109			699		
cM capacity (veh/h)												
Direction_Lane #	EB 1	NB 1	NB 2	NB 3	NB 4	SB 1	SB 2	SB 3				
Volume Total	18	425	425	425	638	928	928	1239				
Volume Left	0	0	0	0	0	0	0	0				
Volume Right	18	0	0	0	638	0	0	775				
GSH	356	1700	1700	1700	1700	1700	1700	1700				
Volume to Capacity	0.05	0.25	0.25	0.25	0.38	0.55	0.55	0.73				
Queue Length 95th (m)	1.3	0.0	0.0	0.0	0.0	0.0	0.0	0.0				
Control Delay (s)	15.6	0.0	0.0	0.0	0.0	0.0	0.0	0.0				
Lane LOS	C											
Approach Delay (s)	15.6	0.0										
Approach LOS	C											
Intersection Summary												
Average Delay	0.1											
Intersection Capacity Utilization	67.7%											
ICU Level of Service	C											
Analysis Period (min)	15											

Lanes, Volumes, Timings  
 7: GO Bus Terminal/Argus Rd & Cross Ave

2035 Total PM Peak Hour.syn  
 05-04-2022

	ICU Level of Service F
Intersection Capacity Utilization	91.4%
Analysis Period (min)	15
~ Volume exceeds capacity, queue is theoretically infinite.	
# 95th percentile volume exceeds capacity, queue may be longer.	
Queue shown is maximum after two cycles.	
Splits and Phases: 7: GO Bus Terminal/Argus Rd & Cross Ave	





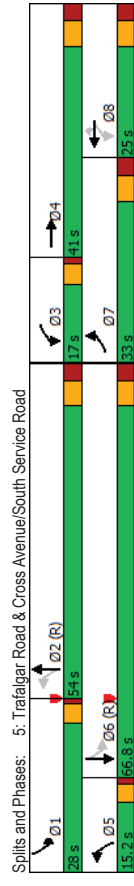
HCM Signalized Intersection Capacity Analysis  
6: Trafalgar Road

2035 Total PM Peak Hour.syn  
05-04-2022

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations	←	←	←	←	←	←	←	←	←	←	←	←	
Traffic Volume (vph)	621	781	239	66	1047	804	170	486	49	650	701	627	
Future Volume (vph)	621	781	239	66	1047	804	170	486	49	650	701	627	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	
Total Lost time (s)	4.0	2.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	
Lane Util. Factor	0.97	0.95	1.00	0.95	1.00	1.00	1.00	0.95	1.00	0.97	1.00	1.00	
Frb, ped/bikes	1.00	0.98	1.00	1.00	0.94	1.00	1.00	1.00	0.97	1.00	1.00	0.95	
Frb, ped/bikes	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Frt	1.00	0.96	1.00	1.00	0.85	1.00	1.00	0.85	1.00	0.85	1.00	0.85	
Flt Protected	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	
Satd. Flow (prot)	3467	3339	1805	3574	1503	1770	3610	1559	3433	1900	1503	1503	
Flt Permitted	0.95	1.00	0.13	1.00	0.12	1.00	1.00	0.12	1.00	0.95	1.00	1.00	
Satd. Flow (perm)	3467	3339	241	3574	1503	219	3610	1559	3433	1900	1503	1503	
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	
Adj. Flow (vph)	675	849	260	72	1138	874	185	528	53	707	762	682	
RTOR Reduction (vph)	0	20	0	0	216	0	0	39	0	0	0	186	
Lane Group Flow (vph)	675	1089	0	72	1138	658	185	528	14	707	762	496	
Conf. Peds. (#/hr)	21	14	14	14	21	17	17	10	10	10	10	17	
Heavy Vehicles (%)	1%	3%	1%	0%	1%	1%	2%	0%	0%	2%	0%	2%	
Turn Type	Prot	NA	pm+pt	NA	pm	pm+pt	NA	pm	Prot	NA	Prot	NA	
Protected Phases	7	4		3	8		5	2		1		6	
Permitted Phases				8		8	2		2			6	
Actuated Green, G (s)	21.0	57.4	45.6	40.0	40.0	40.0	33.0	33.0	33.0	22.0	48.0	48.0	
Effective Green, g (s)	22.0	60.4	47.6	43.0	43.0	42.0	36.0	36.0	36.0	23.0	51.0	51.0	
Actuated g/C Ratio	0.16	0.43	0.34	0.31	0.31	0.30	0.26	0.26	0.26	0.16	0.36	0.36	
Clearance Time (s)	5.0	5.0	5.0	7.0	7.0	5.0	7.0	7.0	7.0	5.0	7.0	7.0	
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	
Lane Grp. Cap (vph)	544	1440	155	1097	461	154	928	400	563	692	547	547	
v/s Ratio Prot	c0.19	0.33	0.02	0.32	0.07	0.15			c0.21	c0.40			
v/s Ratio Perm			0.14	0.46	0.44	0.29	0.01					0.33	
vic Ratio	1.24	0.76	0.46	1.04	1.43	1.20	0.57	0.03	1.26	1.10	0.91	0.91	
Uniform Delay, d1	59.0	33.6	33.6	48.5	48.5	44.4	45.2	39.0	58.5	44.5	42.3	42.3	
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.32	1.24	1.82	1.82	
Incremental Delay, d2	123.3	2.3	2.2	37.3	204.4	136.8	2.5	0.2	120.4	53.9	9.3	9.3	
Delay (s)	182.3	35.9	35.8	85.8	252.9	181.1	47.8	39.1	197.7	109.0	86.3	86.3	
Level of Service	F	D	D	F	F	F	D	D	D	F	F	F	
Approach Delay (s)			91.3		154.2		79.4				131.0		
Approach LOS			F		F		E				F		
Intersection Summary													
HCM 2000 Control Delay	121.8											HCM 2000 Level of Service	F
HCM 2000 Volume to Capacity ratio	1.29												
Actuated Cycle Length (s)	140.0											Sum of lost time (s)	16.0
Intersection Capacity Utilization	106.3%											ICU Level of Service	G
Analysis Period (min)	15												
c Critical Lane Group													

Lanes, Volumes, Timings  
5: Trafalgar Road & Cross Avenue/South Service Road  
2022 Existing AM Peak Hour.syn  
04-27-2022

Intersection Capacity Utilization 77.8% ICU Level of Service D  
Analysis Period (min) 15  
# 95th percentile volume exceeds capacity, queue may be longer.  
Queue shown is maximum after two cycles.  
m Volume for 95th percentile queue is metered by upstream signal.

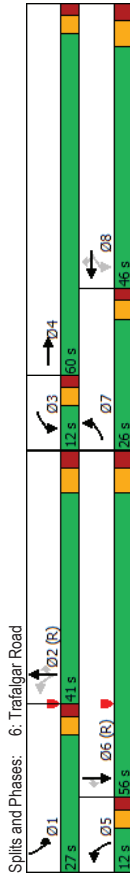


HCM Signalized Intersection Capacity Analysis  
 5: Trafalgar Road & Cross Avenue/South Service Road  
 2022 Existing AM Peak Hour.syn  
 04-27-2022

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations	←	←	←	←	←	←	←	←	←	←	←	←	
Traffic Volume (vph)	408	85	118	44	88	126	118	1227	56	270	1401	434	
Future Volume (vph)	408	85	118	44	88	126	118	1227	56	270	1401	434	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	
Total Lost time (s)	4.0	4.0	1.5	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	
Lane Util. Factor	0.97	1.00	1.00	1.00	1.00	1.00	1.00	0.91	1.00	0.91	1.00	0.91	
Frbp, ped/bikes	1.00	0.99	1.00	1.00	1.00	0.99	1.00	0.99	1.00	0.99	1.00	0.99	
Frbp, ped/bikes	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Frt	1.00	0.91	1.00	1.00	0.85	1.00	0.99	1.00	0.99	1.00	0.96	1.00	
Flt Protected	0.95	1.00	0.95	1.00	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.95	
Satd. Flow (prot)	3213	1564	1745	1759	1531	1641	4961	1752	4806	1752	4806	1752	
Flt Permitted	0.95	1.00	0.62	1.00	1.00	0.07	1.00	1.00	0.08	1.00	1.00	0.08	
Satd. Flow (perm)	3213	1564	1140	1759	1531	125	4961	151	4806	151	4806	151	
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	
Adj. Flow (vph)	443	92	128	48	96	137	128	1334	61	293	1523	472	
RTOR Reduction (vph)	0	37	0	0	0	120	0	3	0	0	36	0	
Lane Group Flow (vph)	443	183	0	48	96	17	128	1392	0	293	1959	0	
Conf. Peds. (#/hr)	1	4	4	4	4	1	10	52	52	52	10	10	
Heavy Vehicles (%)	9%	6%	12%	3%	8%	4%	10%	3%	0%	3%	3%	4%	
Turn Type	Prot	NA	pm+pt	NA	pm+pt	NA	pm+pt	NA	pm+pt	NA	pm+pt	NA	
Protected Phases	7	4		3	8		5	2		1	6		
Permitted Phases				8		8	2			6			
Actuated Green, G (s)	23.4	30.5	23.8	14.2	14.2	65.8	55.1	81.4	66.7	81.4	66.7	66.7	
Effective Green, g (s)	26.4	33.5	29.8	17.2	17.2	68.8	58.1	81.4	69.7	81.4	69.7	69.7	
Actuated g/C Ratio	0.19	0.24	0.21	0.12	0.12	0.47	0.42	0.58	0.50	0.58	0.50	0.50	
Clearance Time (s)	7.0	7.0	4.5	7.0	7.0	4.0	7.0	4.0	7.0	4.0	7.0	7.0	
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	
Lane Grp. Cap (vph)	605	374	297	216	188	174	2058	342	2392	342	2392	2392	
v/s Ratio Prot	c0.14	c0.12	0.01	0.05	0.06	0.28	c0.14	c0.41	c0.14	c0.41	c0.41	c0.41	
v/s Ratio Perm			0.02	0.01	0.29			0.36			0.36	0.36	
v/c Ratio	0.73	0.49	0.16	0.44	0.09	0.74	0.68	0.86	0.86	0.86	0.82	0.82	
Uniform Delay, d1	53.5	45.9	44.6	57.0	54.5	28.4	33.3	39.3	29.8	39.3	29.8	29.8	
Progression Factor	1.00	1.00	1.00	1.00	1.00	0.97	1.19	0.88	1.04	0.88	1.04	1.04	
Incremental Delay, d2	4.6	1.0	0.3	1.5	0.2	9.8	1.1	17.2	3.0	17.2	3.0	3.0	
Delay (s)	58.0	46.9	44.9	58.4	54.7	37.2	40.6	51.8	33.8	51.8	33.8	33.8	
Level of Service	E	D	D	E	D	D	D	D	D	D	C	C	
Approach Delay (s)	54.3		54.3		54.3		40.3		36.1		36.1		
Approach LOS	D		D		D		D		D		D		
Intersection Summary													
HCM 2000 Control Delay	41.1											HCM 2000 Level of Service	D
HCM 2000 Volume to Capacity ratio	0.79												
Actuated Cycle Length (s)	140.0											Sum of lost time (s)	16.0
Intersection Capacity Utilization	77.8%											ICU Level of Service	D
Analysis Period (min)	15												
c Critical Lane Group													

Lanes, Volumes, Timings  
 6: Trafalgar Road  
 2035 Total PM Peak Hour.syn  
 05-04-2022

Intersection Capacity Utilization	106.3%	ICU Level of Service	G
Analysis Period (min)	15		
~ Volume exceeds capacity, queue is theoretically infinite.			
Queue shown is maximum after two cycles.			
# 95th percentile volume exceeds capacity, queue may be longer.			
Queue shown is maximum after two cycles.			
m Volume for 95th percentile queue is metered by upstream signal.			









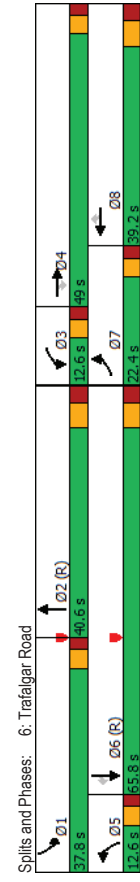
HCM Signalized Intersection Capacity Analysis  
 5: Trafalgar Road & Cross Avenue/South Service Road  
 2035 Total PM Peak Hour.syn  
 05-04-2022

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	
Traffic Volume (vph)	1268	407	205	294	334	413	240	1474	197	172	1481	584	
Future Volume (vph)	1268	407	205	294	334	413	240	1474	197	172	1481	584	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	
Total Lost time (s)	4.0	4.0	7.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	
Lane Util. Factor	0.97	0.95	1.00	0.97	0.95	1.00	1.00	0.91	1.00	1.00	0.91	1.00	
Future Volume (vph)	1.00	1.00	0.95	1.00	1.00	1.00	1.00	1.00	0.88	1.00	1.00	0.95	
Frb, ped/bikes	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Frt	1.00	1.00	0.85	1.00	1.00	0.85	1.00	1.00	0.85	1.00	1.00	0.85	
Flt Protected	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00	
Satd. Flow (prot)	3400	3343	1413	3433	3471	1599	1719	5136	1422	1805	5136	1485	
Flt Permitted	0.95	1.00	1.00	0.95	1.00	1.00	0.10	1.00	1.00	0.11	1.00	1.00	
Satd. Flow (perm)	3400	3343	1413	3433	3471	1599	181	5136	1422	201	5136	1485	
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	
Adj. Flow (vph)	1378	442	223	320	363	449	261	1602	214	187	1610	635	
RTOR Reduction (vph)	0	0	124	0	0	93	0	85	0	85	0	236	
Lane Group Flow (vph)	1378	442	99	320	363	356	261	1602	129	187	1610	399	
Conf. Peds. (#/hr)	3%	8%	9%	2%	4%	1%	5%	1%	0%	0%	1%	3%	
Heavy Vehicles (%)	Prot	NA	Perm	Prot	NA	pm+ov	pm+pt	NA	pm+ov	pm+pt	NA	Perm	
Turn Type	7	4		3	3	8	1	5	2	3	1	6	
Protected Phases													
Permitted Phases			4			8	2		2	6		6	
Actuated Green, G (s)	46.0	48.8	17.6	17.4	28.9	53.8	40.1	57.7	49.4	37.9	37.9	48.0	
Effective Green, g (s)	49.0	51.8	48.8	17.6	20.4	28.9	53.8	43.1	57.7	49.4	40.9	40.9	
Actuated g/C Ratio	0.35	0.37	0.35	0.13	0.15	0.21	0.38	0.31	0.41	0.35	0.29	0.29	
Clearance Time (s)	7.0	7.0	4.0	7.0	4.0	7.0	4.0	7.0	4.0	7.0	4.0	7.0	
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	
Lane Grp Cap (vph)	1190	1236	492	431	505	330	220	1581	586	202	1500	433	
v/s Ratio Prot	c0.41	0.13		0.09	0.10	c0.09	c0.12	0.31	0.03	0.08	0.31		
v/s Ratio Perm			0.07			0.13	c0.34		0.06	0.25		0.27	
vic Ratio	1.16	0.36	0.20	0.74	0.72	1.08	1.19	1.01	0.22	0.93	1.07	0.92	
Uniform Delay, d1	45.5	32.0	32.0	59.0	57.1	55.5	40.8	48.4	26.6	37.3	49.5	48.0	
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.34	1.06	2.18	1.05	0.74	0.47	
Incremental Delay, d2	80.8	0.2	0.2	6.8	4.9	72.3	88.3	10.4	0.0	25.0	39.6	15.4	
Delay (s)	126.3	32.2	32.2	65.8	61.9	127.9	143.2	62.0	58.1	64.0	76.2	37.9	
Level of Service	F	C	C	E	E	F	F	E	E	E	E	D	
Approach Delay (s)			95.7			89.2		71.8			65.2		
Approach LOS			F			F		E			E		
Intersection Summary													
HCM 2000 Control Delay	78.6											HCM 2000 Level of Service	E
HCM 2000 Volume to Capacity ratio	1.12												
Actuated Cycle Length (s)	140.0											Sum of lost time (s)	16.0
Intersection Capacity Utilization	100.7%											ICU Level of Service	G
Analysis Period (min)	15												

c Critical Lane Group

Lanes, Volumes, Timings  
 6: Trafalgar Road  
 2022 Existing AM Peak Hour.syn  
 04-27-2022

Intersection Capacity Utilization	77.9%
Analysis Period (min)	15
# 95th percentile volume exceeds capacity, queue may be longer.	
Queue shown is maximum after two cycles.	
m Volume for 95th percentile queue is metered by upstream signal.	



HCM Signalized Intersection Capacity Analysis  
6: Trafalgar Road

2022 Existing AM Peak Hour.syn  
04-27-2022

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	381	510	79	20	417	615	53	351	50	565	520	352
Traffic Volume (vph)	381	510	79	20	417	615	53	351	50	565	520	352
Future Volume (vph)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Ideal Flow (vphpl)	4.0	2.0	2.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Total Lost time (s)	0.97	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	0.97	1.00	1.00
Lane Util. Factor	1.00	1.00	0.95	1.00	0.93	1.00	0.99	1.00	1.00	0.99	1.00	0.97
Frb, ped/bikes	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frb, ped/bikes	1.00	1.00	0.85	1.00	1.00	0.85	1.00	0.98	1.00	1.00	1.00	0.85
Frt	0.95	1.00	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.85
Flt Protected	3433	3471	1524	1703	3505	1460	1770	3489	3367	1863	1474	1474
Satd. Flow (prot)	0.95	1.00	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.85
Flt Permitted	3433	3471	1524	1703	3505	1460	1770	3489	3367	1863	1474	1474
Satd. Flow (perm)	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Peak-hour factor, PHF	414	564	86	22	453	668	56	382	54	614	565	383
Adj. Flow (vph)	0	0	56	0	357	0	8	0	0	0	186	0
RTOR Reduction (vph)	414	564	30	22	453	311	58	428	0	614	565	197
Lane Group Flow (vph)	25	25	7	7	25	9	9	18	18	18	18	9
Conf. Peds. (#/hr)	2%	4%	1%	6%	3%	3%	2%	1%	0%	4%	2%	6%
Heavy Vehicles (%)	Prot	NA	Perm	Prot	NA	Perm	Prot	NA	Prot	NA	Perm	Prot
Turn Type	7	4	3	3	8	8	5	2	1	6	6	6
Protected Phases	17.4	46.2	46.2	4.5	31.3	31.3	6.2	37.7	29.6	61.1	61.1	61.1
Permitted Phases	18.4	49.2	49.2	5.5	34.3	34.3	7.2	40.7	30.6	64.1	64.1	64.1
Actuated Green, G (s)	0.13	0.35	0.35	0.04	0.24	0.24	0.05	0.29	0.22	0.46	0.46	0.46
Effective Green, g (s)	5.0	5.0	5.0	7.0	7.0	7.0	5.0	7.0	5.0	7.0	7.0	7.0
Actuated g/C Ratio	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Clearance Time (s)	451	1219	535	66	858	357	91	1014	735	852	674	674
Vehicle Extension (s)	c0.12	0.16	0.01	0.13	0.13	0.03	0.12	0.12	c0.18	c0.30	0.13	0.13
Lane Grp Cap (vph)	0.92	0.45	0.06	0.33	0.53	0.87	0.64	0.42	0.84	0.66	0.29	0.29
v/s Ratio Prot	60.1	35.0	30.0	65.5	45.8	50.7	65.1	40.1	52.3	29.5	23.8	23.8
v/s Ratio Perm	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.47	0.92	2.14	2.14
Uniform Delay, d1	23.4	0.3	0.0	3.0	0.6	20.0	13.7	1.3	5.2	2.5	0.7	0.7
Progression Factor	83.5	35.3	30.1	68.4	46.4	70.7	78.8	41.4	82.1	29.6	51.5	51.5
Incremental Delay, d2	F	D	C	E	D	E	E	D	F	C	D	D
Delay (s)	53.8	61.1	61.1	45.8	45.8	55.6	55.6	55.6	55.6	55.6	55.6	55.6
Level of Service	D	D	C	E	D	E	E	D	F	C	D	D
Approach Delay (s)	D	D	C	E	D	E	E	D	F	C	D	D
Approach LOS	D	D	C	E	D	E	E	D	F	C	D	D
Intersection Summary	Intersection Summary											
HCM 2000 Control Delay	55.5 HCM 2000 Level of Service E											
HCM 2000 Volume to Capacity ratio	0.81											
Actuated Cycle Length (s)	140.0 Sum of lost time (s) 16.0											
Intersection Capacity Utilization	77.9% ICU Level of Service D											
Analysis Period (min)	15											
c Critical Lane Group												

Lanes, Volumes, Timings  
5: Trafalgar Road & Cross Avenue/South Service Road

2035 Total PM Peak Hour.syn  
05-04-2022

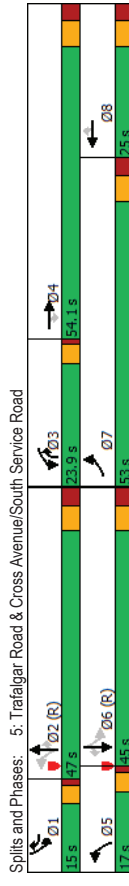
Analysis Period (min)	ICU Level of Service
15	G

Intersection Capacity Utilization 100.7%

~ Volume exceeds capacity, queue is theoretically infinite.  
Queue shown is maximum after two cycles.

# 95th percentile volume exceeds capacity, queue may be longer.  
Queue shown is maximum after two cycles.

m Volume for 95th percentile queue is metered by upstream signal.







HCM Unsignalized Intersection Capacity Analysis  
 4: Trafalgar Road & Argus Road/QEW EB On Ramp

2035 Total PM Peak Hour.syn  
 05-04-2022

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	0	0	65	0	0	0	0	2210	944	0	2165	530
Future Volume (Veh/h)	0	0	65	0	0	0	0	2210	944	0	2165	530
Sign Control	Stop			Stop			Free			Free		
Grade	0%			0%			0%			0%		
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	0	0	71	0	0	0	0	2402	1026	0	2353	576
Pedestrians	24											
Lane Width (m)	3.6											
Walking Speed (m/s)	1.2											
Percent Blockage	2											
Right turn flare (veh)	None											
Median type	None											
Median storage (veh)	None											
Upstream signal (m)	271											
pX, platoon unblocked	0.72	0.72	0.59	0.72	0.72	0.75	0.59			0.75		0.75
vC, conflicting volume	3466	5067	1096	3186	5355	801	2953			2402		2402
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	735	2967	0	346	3368	0	1896			1714		1714
tC, single (s)	7.5	6.5	7.1	7.5	6.5	6.9	4.1			4.1		4.1
tC, 2 stage (s)												
tF (s)	3.5	4.0	3.4	3.5	4.0	3.3	2.2			2.2		2.2
p0 queue free %	100	100	89	100	100	100	100			100		100
cM capacity (veh/h)	215	10	620	369	5	821	186			282		282
Direction_Lane #	EB 1	NB 1	NB 2	NB 3	NB 4	SB 1	SB 2	SB 3	SB 3			
Volume Total	71	801	801	801	1026	941	941	1047				
Volume Left	0	0	0	0	0	0	0	0				
Volume Right	71	0	0	0	1026	0	0	576				
GSH	620	1700	1700	1700	1700	1700	1700	1700				
Volume to Capacity	0.11	0.47	0.47	0.47	0.60	0.55	0.55	0.62				
Queue Length 95th (m)	3.1	0.0	0.0	0.0	0.0	0.0	0.0	0.0				
Control Delay (s)	11.6	0.0	0.0	0.0	0.0	0.0	0.0	0.0				
Lane LOS	B	B	B	B	B	B	B	B				
Approach Delay (s)	11.6	0.0				0.0						
Approach LOS	B	B				B						
Intersection Summary												
Average Delay	0.1											
Intersection Capacity Utilization	64.8%											
ICU Level of Service	C											
Analysis Period (min)	15											

Lanes, Volumes, Timings  
 7: GO Bus Terminal/Argus Rd & Cross Ave

2022 Existing AM Peak Hour.syn  
 04-27-2022

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	0	0	65	0	0	0	0	2210	944	0	2165	530
Future Volume (Veh/h)	0	0	65	0	0	0	0	2210	944	0	2165	530
Sign Control	Stop			Stop			Free			Free		
Grade	0%			0%			0%			0%		
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	0	0	71	0	0	0	0	2402	1026	0	2353	576
Pedestrians	24											
Lane Width (m)	3.6											
Walking Speed (m/s)	1.2											
Percent Blockage	2											
Right turn flare (veh)	None											
Median type	None											
Median storage (veh)	None											
Upstream signal (m)	271											
pX, platoon unblocked	0.72	0.72	0.59	0.72	0.72	0.75	0.59			0.75		0.75
vC, conflicting volume	3466	5067	1096	3186	5355	801	2953			2402		2402
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	735	2967	0	346	3368	0	1896			1714		1714
tC, single (s)	7.5	6.5	7.1	7.5	6.5	6.9	4.1			4.1		4.1
tC, 2 stage (s)												
tF (s)	3.5	4.0	3.4	3.5	4.0	3.3	2.2			2.2		2.2
p0 queue free %	100	100	89	100	100	100	100			100		100
cM capacity (veh/h)	215	10	620	369	5	821	186			282		282
Direction_Lane #	EB 1	NB 1	NB 2	NB 3	NB 4	SB 1	SB 2	SB 3	SB 3			
Volume Total	71	801	801	801	1026	941	941	1047				
Volume Left	0	0	0	0	0	0	0	0				
Volume Right	71	0	0	0	1026	0	0	576				
GSH	620	1700	1700	1700	1700	1700	1700	1700				
Volume to Capacity	0.11	0.47	0.47	0.47	0.60	0.55	0.55	0.62				
Queue Length 95th (m)	3.1	0.0	0.0	0.0	0.0	0.0	0.0	0.0				
Control Delay (s)	11.6	0.0	0.0	0.0	0.0	0.0	0.0	0.0				
Lane LOS	B	B	B	B	B	B	B	B				
Approach Delay (s)	11.6	0.0				0.0						
Approach LOS	B	B				B						
Intersection Summary												
Average Delay	0.1											
Intersection Capacity Utilization	64.8%											
ICU Level of Service	C											
Analysis Period (min)	15											

HCM Signalized Intersection Capacity Analysis  
 7: GO Bus Terminal/Argus Rd & Cross Ave  
 2022 Existing AM Peak Hour.syn  
 04-27-2022

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	31	377	14	39	585	23	20	0	48	56	16	491
Traffic Volume (vph)	31	377	14	39	585	23	20	0	48	56	16	491
Future Volume (vph)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Ideal Flow (vphpl)	3.3	3.6	3.6	3.3	3.6	3.6	3.3	3.6	3.6	3.3	3.6	3.6
Lane Width	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Total Lost time (s)	1.00	0.95	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frpb, ped/bikes	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Flpb, ped/bikes	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Fit Protected	0.95	1.00	0.95	1.00	1.00	0.95	1.00	0.95	1.00	0.95	1.00	1.00
Satd. Flow (prot)	1569	3045	817	3189	805	723	805	723	1527	1387	1387	1387
Fit Permitted	0.39	1.00	0.36	1.00	0.16	1.00	0.16	1.00	0.71	1.00	1.00	1.00
Satd. Flow (perm)	641	3045	310	3189	136	723	136	723	1145	1387	1387	1387
Peak-hour factor, PHF	0.52	0.87	0.65	0.84	0.88	0.79	0.53	0.25	0.70	0.78	0.62	0.89
Adj. Flow (vph)	60	433	22	46	665	29	38	0	69	72	26	552
RTOR Reduction (vph)	0	5	0	0	4	0	0	44	0	0	0	177
Lane Group Flow (vph)	60	450	0	46	690	0	38	25	0	72	401	0
Conf. Peds. (#/hr)	1	3	3	3	3	1	3	3	20	20	3	3
Heavy Vehicles (%)	0%	1%	100%	92%	1%	5%	95%	0%	91%	0%	93%	0%
Turn Type	Perm	NA	NA	pm-pt	NA	Perm	NA	NA	NA	Perm	NA	NA
Protected Phases	2	1	6	8	8	8	8	8	8	8	4	4
Permitted Phases	2	6	6	8	8	8	8	8	8	8	4	4
Actuated Green, G (s)	22.3	22.3	34.4	34.4	34.4	23.0	23.0	23.0	23.0	23.0	23.0	23.0
Effective Green, g (s)	24.3	24.3	34.4	36.4	36.4	25.0	25.0	25.0	25.0	25.0	25.0	25.0
Actuated g/C Ratio	0.35	0.35	0.50	0.52	0.52	0.36	0.36	0.36	0.36	0.36	0.36	0.36
Clearance Time (s)	6.0	6.0	4.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0
Vehicle Extension (s)	5.0	5.0	2.5	5.0	5.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lane Grp Cap (vph)	224	1066	212	1672	48	260	412	499				
v/s Ratio Prot	0.15	0.03	0.22	0.03	0.03	0.28	0.06	0.06				
v/s Ratio Perm	0.09	0.42	0.22	0.41	0.79	0.10	0.17	0.80				
Uniform Delay, d1	16.2	17.2	9.7	10.0	19.9	14.7	15.2	20.0				
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00				
Incremental Delay, d2	1.3	0.6	0.4	0.3	61.0	0.2	0.3	9.6				
Delay (s)	17.5	17.8	10.1	10.4	80.8	14.9	15.4	29.6				
Level of Service	B	B	B	B	F	B	B	C				
Approach Delay (s)	17.7		10.3		38.3		28.0					
Approach LOS	B		B		D		C					
Intersection Summary												
HCM 2000 Control Delay	19.4											
HCM 2000 Level of Service	B											
HCM 2000 Volume to Capacity ratio	0.61											
Actuated Cycle Length (s)	69.4											
Intersection Capacity Utilization	77.7%											
ICU Level of Service	D											
Analysis Period (min)	15											
Critical Lane Group	C											

Lanes, Volumes, Timings  
 4: Trafalgar Road & Argus Road/QEW EB On Ramp  
 2035 Total PM Peak Hour.syn  
 05-04-2022

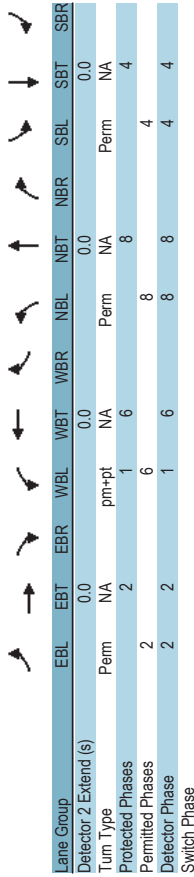
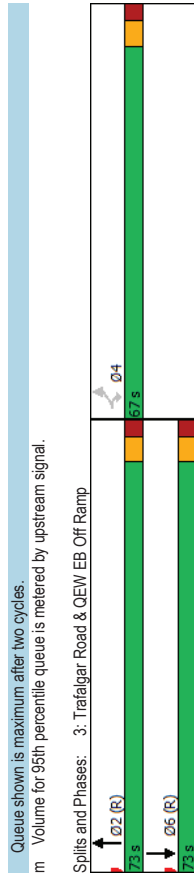
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	0	0	65	0	0	0	0	2210	944	0	2165	530
Traffic Volume (vph)	0	0	65	0	0	0	0	2210	944	0	2165	530
Future Volume (vph)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Ideal Flow (vphpl)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Storage Length (m)	0	0	1	0	0	0	0	1	0	0	0	0
Taper Length (m)	7.5	7.5	7.5	7.5	7.5	7.5	7.5	7.5	7.5	7.5	7.5	7.5
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor	0.865											
Fit Protected	0.850											
Satd. Flow (prot)	0	0	1522	0	0	0	0	5085	1568	0	4957	0
Fit Permitted	0											
Satd. Flow (perm)	0	0	1522	0	0	0	0	5085	1568	0	4957	0
Link Speed (k/h)	50											
Link Distance (m)	165.2											
Travel Time (s)	11.9											
Confl. Peds. (#/hr)	14.1											
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	0%	0%	8%	0%	2%	0%	0%	2%	3%	0%	2%	0%
Adj. Flow (vph)	0	0	71	0	0	0	0	2402	1026	0	2353	576
Shared Lane Traffic (%)	0											
Lane Group Flow (vph)	0	0	71	0	0	0	0	2402	1026	0	2929	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	3.6	3.6	0.0	3.6	0.0
Link Offset(m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Crosswalk Width(m)	4.8											
Two way Left Turn Lane	0											
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (k/h)	25	15	25	25	25	15	25	25	15	25	25	15
Sign Control	Stop											
Intersection Summary												
Area Type:	Other											
Control Type:	Unsignalized											
Intersection Capacity Utilization	64.8%											
ICU Level of Service	C											
Analysis Period (min)	15											

HCM Signalized Intersection Capacity Analysis  
3: Trafalgar Road & QEW EB Off Ramp

Lanes, Volumes, Timings  
8: Lyons Lane/Commercial Driveway & Cross Ave

Movement	EBL	EBR	NBL	NBT	SBT	SBR	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑↑	↑↑	↑↑↑↑	↑↑↑↑	↑↑↑↑	↑↑	↑↑	↑↑	↑↑	↑↑	↑↑	↑↑	↑↑	↑↑	↑↑	↑↑	↑↑	↑↑
Traffic Volume (vph)	1210	623	0	2210	2072	0	56	169	193	215	115	5	17	3	13	16	23	40
Future Volume (vph)	1210	623	0	2210	2072	0	56	169	193	215	115	5	17	3	13	16	23	40
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0	3.3	3.6	3.6	3.3	3.6	3.6	3.3	3.6	3.6	3.3	3.6	3.6
Lane Util. Factor	0.97	1.00	0.91	0.91	0.91	0.91	1.00	0.95	0.95	1.00	0.95	0.95	1.00	1.00	1.00	1.00	1.00	1.00
Frb, ped/bikes	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt	1.00	0.85	1.00	1.00	1.00	1.00	0.99	0.99	1.00	1.00	1.00	1.00	0.99	0.99	1.00	0.99	0.99	0.99
Flt Protected	3502	1573	5085	5085	5085	0	0.950	0.916	0.989	0.989	0.989	0.989	0.950	0.916	0.989	0.950	0.916	0.989
Flt Permitted	3502	1573	5085	5085	5085	0	0.950	0.916	0.989	0.989	0.989	0.989	0.950	0.916	0.989	0.950	0.916	0.989
Satd. Flow (perm)	0.92	0.92	0.92	0.92	0.92	0.92	1540	2803	0	1570	2727	0	1570	1473	0	1468	1497	0
Peak-hour factor, PHF	1315	677	0	2402	2252	0	0.652	0.416	0.710	0.710	0.710	0.710	0.652	0.416	0.710	0.652	0.416	0.710
Adj. Flow (vph)	0	1	0	0	0	0	1048	2803	0	687	2727	0	1170	1473	0	1139	1497	0
RTOR Reduction (vph)	1315	676	0	2402	2252	0	264	264	12	12	12	12	22	22	22	22	22	47
Lane Group Flow (vph)	1315	676	0	2402	2252	0	50	50	50	50	50	50	50	50	50	50	50	50
Conf. Peds. (#/hr)	0%	1%	0%	2%	2%	0%	40.1	40.1	374.0	374.0	374.0	374.0	69.1	69.1	69.1	70.9	70.9	70.9
Heavy Vehicles (%)	Perm	Perm	0%	2%	2%	0%	2.9	2.9	26.9	26.9	26.9	26.9	5.0	5.0	5.0	5.1	5.1	5.1
Turn Type	Perm	Perm	NA	NA	NA	NA	5	5	1	1	1	1	5	3	3	3	3	3
Protected Phases	4	4	2	2	6	6	0.78	0.81	0.73	0.66	0.79	0.42	0.80	0.75	0.60	0.62	0.92	0.86
Permitted Phases	59.7	59.7	66.3	66.3	66.3	2%	11%	0%	19%	0%	0%	0%	0%	0%	0%	7%	0%	3%
Actuated Green, G (s)	62.7	62.7	69.3	69.3	69.3	72	209	264	326	146	12	21	4	22	26	25	25	47
Effective Green, g (s)	0.45	0.45	0.49	0.49	0.49	72	473	0	326	158	0	21	26	0	26	26	72	0
Actuated g/C Ratio	7.0	7.0	7.0	7.0	7.0	7.0	No	No	No	No	No	No	No	No	No	No	No	No
Clearance Time (s)	3.0	3.0	3.0	3.0	3.0	3.0	Left	Left	Right	Left	Right	Left	Left	Left	Left	Left	Left	Right
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.3	3.3	3.3	3.3	3.3	3.3	3.3	3.3	3.3	3.3	3.3	3.3
Lane Grp. Cap (vph)	1568	704	2517	2517	2517	2517	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
v/s Ratio Prot	0.38	0.43	0.47	0.44	0.44	0.44	4.8	4.8	4.8	4.8	4.8	4.8	4.8	4.8	4.8	4.8	4.8	4.8
v/s Ratio Perm	0.84	0.96	0.95	0.89	0.89	0.89	1.19	1.14	1.14	1.14	1.14	1.14	1.19	1.14	1.14	1.19	1.14	1.14
Uniform Delay, d1	34.2	37.4	33.8	32.0	32.0	32.0	24	24	24	24	24	24	24	24	24	24	24	24
Progression Factor	1.00	1.00	1.36	0.87	0.87	0.87	1	2	1	1	2	1	1	2	1	2	1	2
Incremental Delay, d2	4.1	24.4	1.3	2.9	2.9	2.9	Left	Thru	Left	Thru	Left	Thru	Left	Thru	Left	Thru	Left	Thru
Delay (s)	38.3	61.8	47.4	30.6	30.6	30.6	2.0	10.0	2.0	10.0	2.0	10.0	2.0	10.0	2.0	10.0	2.0	10.0
Level of Service	D	E	D	C	C	C	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Approach Delay (s)	46.3		47.4	30.6	30.6	30.6	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Approach LOS	D		D	C	C	C	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex
Intersection Summary																		
HCM 2000 Control Delay																		
HCM 2000 Volume to Capacity ratio																		
Actuated Cycle Length (s)																		
Intersection Capacity Utilization																		
Analysis Period (min)																		
c Critical Lane Group																		





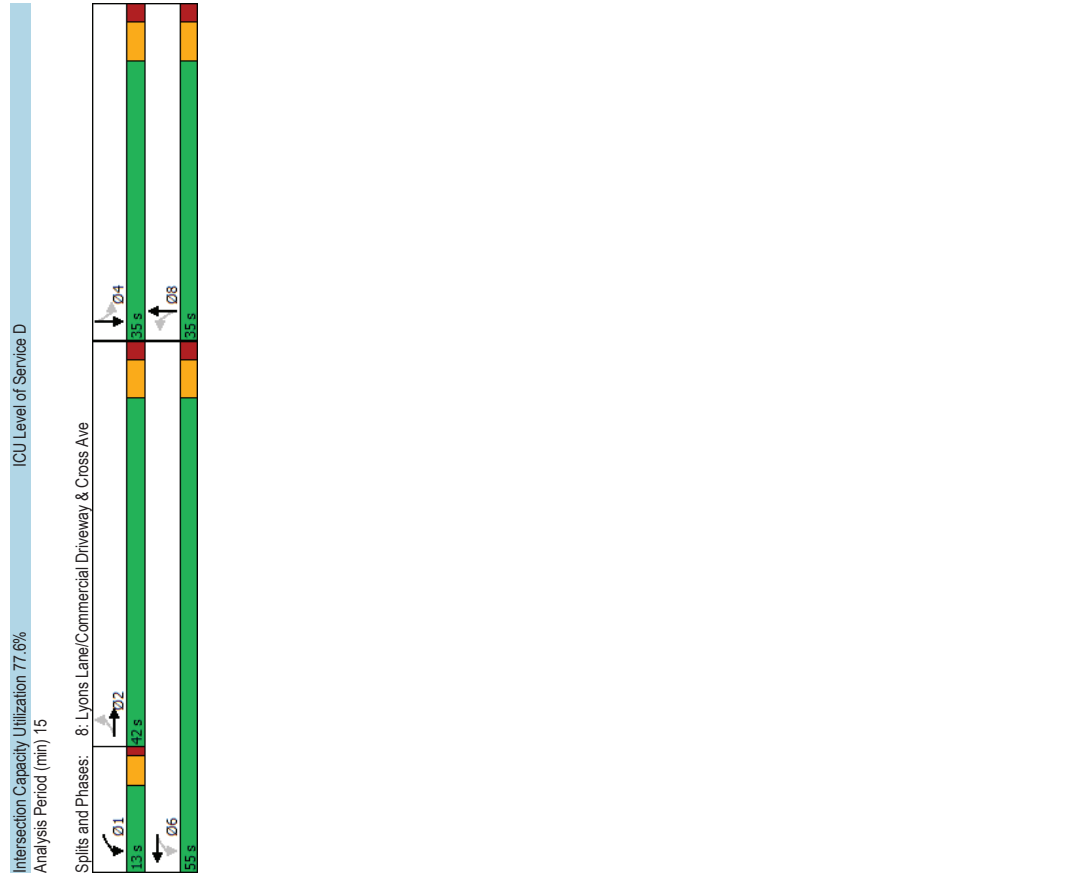
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Detector 2 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Turn Type	Perm	NA	pm-pt	NA	Perm	NA	Perm	NA	Perm	NA	NA	NA
Permitted Phases	2	2	6	1	6	8	8	8	8	4	4	4
Detector Phase	2	2	1	6	6	8	8	8	8	4	4	4
Switch Phase												
Minimum Initial (s)	35.0	35.0	8.0	35.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0
Minimum Split (s)	41.0	41.0	12.5	41.0	28.0	28.0	28.0	28.0	28.0	28.0	28.0	28.0
Total Split (s)	42.0	42.0	13.0	55.0	35.0	35.0	35.0	35.0	35.0	35.0	35.0	35.0
Total Split (%)	46.7%	46.7%	14.4%	61.1%	38.9%	38.9%	38.9%	38.9%	38.9%	38.9%	38.9%	38.9%
Maximum Green (s)	36.0	36.0	9.0	49.0	29.0	29.0	29.0	29.0	29.0	29.0	29.0	29.0
Yellow Time (s)	4.0	4.0	3.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
All-Red Time (s)	2.0	2.0	1.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
Lost Time Adjust (s)	-2.0	-2.0	0.0	-2.0	-2.0	-2.0	-2.0	-2.0	-2.0	-2.0	-2.0	-2.0
Total Lost Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lead/Lag	Lag	Lag	Lead	Lead	Lead	Lead	Lead	Lead	Lead	Lead	Lead	Lead
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Vehicle Extension (s)	5.0	5.0	2.5	5.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Recall Mode	Min	Min	Min	Min	Min	Min	Min	Min	Min	Min	Min	Min
Walk Time (s)	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0
Flash Dont Walk (s)	15.0	15.0	15.0	15.0	15.0	15.0	15.0	15.0	15.0	15.0	15.0	15.0
Pedestrian Calls (#/hr)	0	0	0	0	0	0	0	0	0	0	0	0
Act Effct Green (s)	37.0	37.0	49.8	12.2	12.2	12.2	12.2	12.2	12.2	12.2	12.2	12.2
Actuated g/C Ratio	0.53	0.53	0.71	0.71	0.17	0.17	0.17	0.17	0.17	0.17	0.17	0.17
v/c Ratio	0.13	0.29	0.54	0.08	0.10	0.09	0.13	0.24	0.13	0.24	0.13	0.24
Control Delay	9.3	4.4	7.4	3.0	25.9	13.4	26.4	14.7	26.4	14.7	26.4	14.7
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	9.3	4.4	7.4	3.0	25.9	13.4	26.4	14.7	26.4	14.7	26.4	14.7
LOS	A	A	A	A	C	B	C	B	C	B	C	B
Approach Delay	5.1	5.1	5.9	19.0	19.0	19.0	19.0	19.0	19.0	19.0	19.0	19.0
Approach LOS	A	A	A	B	B	B	B	B	B	B	B	B
Queue Length 50th (m)	4.7	7.1	12.8	2.5	2.5	0.5	3.1	3.0	3.1	3.0	3.1	3.0
Queue Length 95th (m)	9.5	12.0	15.1	4.3	7.3	5.1	6.4	13.5	6.4	13.5	6.4	13.5
Internal Link Dist (m)	16.1	16.1	350.0	45.1	45.1	45.1	45.1	45.1	45.1	45.1	45.1	45.1
Turn Bay Length (m)	25.0	25.0	25.0	20.0	20.0	20.0	20.0	20.0	20.0	20.0	20.0	20.0
Base Capacity (vph)	569	1643	602	1991	518	664	504	689	504	689	504	689
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Spillover Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.13	0.29	0.54	0.08	0.04	0.04	0.05	0.10	0.05	0.10	0.05	0.10

Intersection Summary	
Area Type:	CBD
Cycle Length:	90
Actuated Cycle Length:	70
Natural Cycle:	85
Control Type:	Semi Act-Uncoord
Maximum v/c Ratio:	0.54
Intersection Signal Delay:	7.0
Intersection LOS:	A

Lanes, Volumes, Timings  
3: Trafalgar Road & QEW EB Off Ramp

Lanes, Volumes, Timings  
8: Lyons Lane/Commercial Driveway & Cross Ave

Detector Phase	EBL	EBR	NBL	NBT	SBT	SBR
Switch Phase	4	4	2	2	6	6
Minimum Initial (s)	10.0	10.0	29.0	29.0	29.0	29.0
Minimum Split (s)	38.0	38.0	36.0	36.0	36.0	36.0
Total Split (s)	67.0	67.0	73.0	73.0	73.0	73.0
Total Split (%)	47.9%	47.9%	52.1%	52.1%	52.1%	52.1%
Maximum Green (s)	60.0	60.0	66.0	66.0	66.0	66.0
Yellow Time (s)	4.0	4.0	4.0	4.0	4.0	4.0
All-Red Time (s)	3.0	3.0	3.0	3.0	3.0	3.0
Lost Time Adjust (s)	-3.0	-3.0	-3.0	-3.0	-3.0	-3.0
Total Lost Time (s)	4.0	4.0	4.0	4.0	4.0	4.0
Lead/Lag						
Lead-Lag Optimize?						
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	None	None	C-Max	C-Max	C-Max	C-Max
Walk Time (s)	7.0	7.0	7.0	7.0	7.0	7.0
Flash Dont Walk (s)	11.0	11.0	11.0	11.0	11.0	11.0
Pedestrian Calls (#/hr)	0	0	0	0	0	0
Act Effct Green (s)	62.7	62.7	69.3	69.3	69.3	69.3
Actuated g/C Ratio	0.45	0.45	0.50	0.50	0.50	0.50
v/c Ratio	0.84	0.96	0.95	0.89	0.89	0.89
Control Delay	40.1	63.1	47.2	31.0	31.0	31.0
Queue Delay	0.0	0.0	8.2	0.0	0.0	0.0
Total Delay	40.1	63.1	55.4	31.0	31.0	31.0
LOS	D	E	E	C	C	C
Approach Delay	47.9	70.8	55.4	31.0	31.0	31.0
Approach LOS	D	E	E	C	C	C
Queue Length 50th (m)	171.1	187.0	266.1	185.4	185.4	185.4
Queue Length 95th (m)	202.5	#272.9	m247.2	228.1	228.1	228.1
Internal Link Dist (m)	47.3		26.7	162.0	162.0	162.0
Turn Bay Length (m)						
Base Capacity (vph)	1575	708	2517	2517	2517	2517
Starvation Cap Reductn	0	0	0	0	0	0
Spillback Cap Reductn	0	0	133	0	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	0.83	0.96	1.01	0.89	0.89	0.89
<b>Intersection Summary</b>						
Area Type:	Other					
Cycle Length:	140					
Actuated Cycle Length:	140					
Offset: Q (0%), Referenced to phase 2:NBT and 6:SBT, Start of Green						
Natural Cycle:	90					
Control Type:	Actuated-Coordinated					
Maximum v/c Ratio:	0.96					
Intersection Signal Delay:	44.9					
Intersection Capacity Utilization:	85.5%					
Analysis Period (min):	15					
# 95th percentile volume exceeds capacity, queue may be longer.						



HCM Signalized Intersection Capacity Analysis  
 8: Lyons Lane/Commercial Driveway & Cross Ave

2022 Existing AM Peak Hour.syn  
 04-27-2022

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↔	↕	↔	↔	↕	↔	↔	↕	↔	↔	↕	↔
Traffic Volume (vph)	56	169	193	215	115	5	17	3	13	16	23	40
Future Volume (vph)	56	169	193	215	115	5	17	3	13	16	23	40
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width	3.3	3.6	3.6	3.3	3.6	3.6	3.3	3.6	3.6	3.3	3.6	3.6
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lane Util. Factor	1.00	0.95	1.00	0.95	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frbp, ped/bikes	1.00	0.99	1.00	1.00	1.00	1.00	0.99	1.00	1.00	1.00	0.99	1.00
Fipb, ped/bikes	0.99	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Flt	1.00	0.92	1.00	0.99	1.00	0.87	1.00	0.90	1.00	0.90	1.00	0.90
Flt Protected	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00
Satd. Flow (prot)	1530	2805	1570	2727	1566	1474	1566	1474	1464	1464	1498	1498
Flt Permitted	0.65	1.00	0.42	1.00	0.71	1.00	0.74	1.00	0.74	1.00	0.74	1.00
Satd. Flow (perm)	1050	2805	687	2727	1171	1474	1141	1498	1141	1498	1141	1498
Peak-hour factor, PHF	0.78	0.81	0.73	0.66	0.79	0.42	0.80	0.75	0.60	0.62	0.92	0.86
Adj. Flow (vph)	72	209	264	326	146	12	21	4	22	26	25	47
RTOR Reduction (vph)	0	124	0	0	3	0	0	18	0	0	0	39
Lane Group Flow (vph)	72	349	0	326	155	0	21	8	0	26	33	0
Conf. Peds. (#/hr)	5	0	1	0	0	5	3	0	3	3	3	0
Heavy Vehicles (%)	2%	11%	0%	0%	19%	0%	0%	0%	0%	7%	0%	3%
Turn Type	Perm	NA	pm-pt	NA	NA	Perm	NA	NA	0%	0%	7%	0%
Permitted Phases	2		1	6		8		8		Perm	NA	4
Actuated Green, G (s)	35.0	35.0	47.8	47.8	10.2	10.2	10.2	10.2	10.2	10.2	10.2	10.2
Effective Green, g (s)	37.0	37.0	47.8	49.8	12.2	12.2	12.2	12.2	12.2	12.2	12.2	12.2
Actuated g/C Ratio	0.53	0.63	0.68	0.71	0.17	0.17	0.17	0.17	0.17	0.17	0.17	0.17
Clearance Time (s)	6.0	6.0	4.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0
Vehicle Extension (s)	5.0	5.0	2.5	5.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lane Grp Cap (vph)	555	1482	580	1940	204	256	198	261	198	261	261	261
v/s Ratio Prot	0.12		0.07	0.06		0.01		0.01			0.02	
v/s Ratio Perm	0.07		0.31		0.02		0.02		0.02		0.02	
w/c Ratio	0.13	0.24	0.56	0.08	0.10	0.03	0.13	0.13	0.13	0.13	0.13	0.13
Uniform Delay, d1	8.4	8.9	4.8	3.1	24.3	24.0	24.4	24.4	24.4	24.4	24.4	24.4
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	0.2	0.2	1.0	0.0	0.3	0.1	0.4	0.3	0.4	0.3	0.3	0.3
Delay (s)	8.6	9.1	5.8	3.1	24.6	24.1	24.8	24.7	24.8	24.7	24.7	24.7
Level of Service	A	A	A	A	A	C	C	C	C	C	C	C
Approach Delay (s)	9.0		4.9		24.3		24.7		24.7		24.7	
Approach LOS	A		A		C		C		C		C	
Intersection Summary												
HCM 2000 Control Delay	9.2				HCM 2000 Level of Service		A					
HCM 2000 Volume to Capacity ratio	0.48								12.0			
Actuated Cycle Length (s)	70.0				Sum of lost time (s)							
Intersection Capacity Utilization	77.6%				ICU Level of Service		D					
Analysis Period (min)	15											
Critical Lane Group	C											

Baseline

Lanes, Volumes, Timings  
 3: Trafalgar Road & QEW EB Off Ramp

2035 Total PM Peak Hour.syn  
 05-04-2022

Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	↔	↔	↔	↕	↕	↕
Traffic Volume (vph)	1210	623	0	2210	2072	0
Future Volume (vph)	1210	623	0	2210	2072	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	0.97	1.00	1.00	0.91	0.91	1.00
Ped Bike Factor	0.98					
Flt	0.850					
Flt Protected	0.950					
Satd. Flow (prot)	3502	1599	0	5085	5085	0
Flt Permitted	0.950					
Satd. Flow (perm)	3502	1573	0	5085	5085	0
Right Turn on Red	Yes				Yes	
Satd. Flow (RTOR)	2					
Link Speed (k/h)	50			50	50	
Link Distance (m)	71.3			50.7	186.0	
Travel Time (s)	5.1			3.7	13.4	
Conf. Peds. (#/hr)	2					
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	0%	1%	0%	2%	2%	0%
Adj. Flow (vph)	1315	677	0	2402	2252	0
Shared Lane Traffic (%)						
Lane Group Flow (vph)	1315	677	0	2402	2252	0
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Left	Left	Right
Median Width(m)	7.2			0.0	0.0	0.0
Link Offset(m)	0.0			0.0	0.0	0.0
Crosswalk Width(m)	4.8			4.8	4.8	
Two way Left Turn Lane						
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (k/h)	25	15	25			15
Number of Detectors	1	1		2	2	
Detector Template	Left	Right	Thru	Thru	Thru	
Leading Detector (m)	2.0	2.0	10.0	10.0	10.0	
Trailing Detector (m)	0.0	0.0	0.0	0.0	0.0	
Detector 1 Position(m)	0.0	0.0	0.0	0.0	0.0	
Detector 1 Size(m)	2.0	2.0	0.6	0.6	0.6	
Detector 1 Type	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	
Detector 1 Channel	0.0	0.0	0.0	0.0	0.0	
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	
Detector 2 Position(m)				9.4	9.4	
Detector 2 Size(m)				0.6	0.6	
Detector 2 Type				Ch+Ex	Ch+Ex	
Detector 2 Channel				0.0	0.0	
Detector 2 Extend (s)				0.0	0.0	
Turn Type	Perm	Perm	NA	NA	NA	
Permitted Phases	4	4	2	6		

Baseline

HCM Unsignalized Intersection Capacity Analysis  
 2: Trafalgar Road & QEW SB On Ramps

Lanes, Volumes, Timings  
 9: Cross Ave & Lyons Lane

2035 Total PM Peak Hour.syn  
 05-04-2022

2022 Existing AM Peak Hour.syn  
 04-27-2022



Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations				↑↑↑	↑↑↑	↑
Traffic Volume (veh/h)	0	0	0	3420	2072	959
Future Volume (Veh/h)	0	0	0	3420	2072	959
Sign Control	Stop	Free	Free	Free	Free	Free
Grade	0%	0%	0%	0%	0%	0%
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	0	0	0	3717	2252	1042
Pedestrians						
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)				None	None	
Median type				None	None	
Median storage (veh)				186	145	
Upstream signal (m)	0.25					
pX, platoon unblocked	3491	751	2252			
vC, conflicting volume						
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCU, unblocked vol	401	751	2252			
tC, single (s)	6.8	6.9	4.1			
tC, 2 stage (s)						
tF (s)	3.5	3.3	2.2			
p0 queue free %	100	100	100			
cM capacity (veh/h)	142	353	225			
Direction_Lane #	NB.1	NB.2	NB.3	SB.1	SB.2	SB.3
Volume Total	1239	1239	1239	751	751	751
Volume Left	0	0	0	0	0	0
Volume Right	0	0	0	0	0	0
GSH	1700	1700	1700	1700	1700	1700
Volume to Capacity	0.73	0.73	0.73	0.44	0.44	0.44
Queue Length 95th (m)	0.0	0.0	0.0	0.0	0.0	0.0
Control Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0
Lane LOS						
Approach Delay (s)	0.0			0.0		
Approach LOS						
Intersection Summary						
Average Delay	0.0					
Intersection Capacity Utilization	85.5%					
Analysis Period (min)	15					
ICU Level of Service	E					

Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	↑	↑↑	↑↑	↑↑	↑	↑
Traffic Volume (vph)	23	404	184	6	5	4
Future Volume (vph)	23	404	184	6	5	4
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (m)	5.0					
Storage Lanes	1					
Taper Length (m)	7.5					
Lane Util. Factor	1.00	0.95	0.95	0.95	1.00	1.00
Ped Bike Factor						
Fit		0.992			0.932	
Fit Protected		0.950			0.976	
Satd. Flow (prot)		1624	3094	2799	0	1383
Fit Permitted		0.950			0.976	
Satd. Flow (perm)		1624	3094	2799	0	1383
Link Speed (k/h)		50	50	50	50	50
Link Distance (m)		281.3	40.1	41.7		
Travel Time (s)		20.3	2.9	3.0		
Confl. Peds. (#/hr)	4			4	7	
Peak Hour Factor	0.69	0.58	0.88	0.50	0.62	0.50
Heavy Vehicles (%)	0%	5%	16%	0%	0%	25%
Adj. Flow (vph)	33	697	209	12	8	8
Shared Lane Traffic (%)						
Lane Group Flow (vph)	33	697	221	0	16	0
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Left	Left	Right	Left	Right
Median Width(m)		3.6	3.6		3.6	
Link Offset(m)		0.0	0.0		0.0	
Crosswalk Width(m)		4.8	4.8		4.8	
Two way Left Turn Lane						
Headway Factor	1.14	1.14	1.14	1.14	1.14	1.14
Turning Speed (k/h)	25			15	25	15
Sign Control		Free	Free	Free	Stop	Stop
Intersection Summary						
Area Type:	CBD					
Control Type:	Unsignalized					
Intersection Capacity Utilization	23.5%					
Analysis Period (min)	15					
ICU Level of Service A						

HCM Unsignalized Intersection Capacity Analysis  
 9: Cross Ave & Lyons Lane  
 2022 Existing AM Peak Hour.syn  
 04-27-2022

Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	↔	↔	↔	↔	↔	↔
Traffic Volume (veh/h)	23	404	184	6	5	4
Future Volume (Veh/h)	23	404	184	6	5	4
Sign Control	Free	Free	Free	Stop	0%	0%
Grade	0%	0%	0%	0%	0%	0%
Peak Hour Factor	0.69	0.88	0.88	0.50	0.62	0.50
Hourly flow rate (vph)	33	697	209	12	8	8
Pedestrians						
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)						
Median type						
Median storage (veh)						
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume						
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
tC, single (s)						
tC, 2 stage (s)						
tF (s)						
p0 queue free %						
cM capacity (veh/h)						
Direction_Lane #	EB1	EB2	EB3	WB1	WB2	SB1
Volume Total	33	348	348	139	82	16
Volume Left	33	0	0	0	0	8
Volume Right	0	0	0	0	12	8
GSH	1351	1700	1700	1700	1700	541
Volume to Capacity	0.02	0.20	0.20	0.08	0.05	0.03
Queue Length 95th (m)	0.6	0.0	0.0	0.0	0.0	0.7
Control Delay (s)	7.7	0.0	0.0	0.0	0.0	11.9
Lane LOS	A	A	A	B	B	B
Approach Delay (s)	0.3			0.0	11.9	
Approach LOS						
Intersection Summary						
Average Delay	0.5					
Intersection Capacity Utilization	23.5% ICU Level of Service A					
Analysis Period (min)	15					

Lanes, Volumes, Timings  
 2: Trafalgar Road & QEW SB On Ramps  
 2035 Total PM Peak Hour.syn  
 05-04-2022

Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	↔	↔	↔	↔	↔	↔
Traffic Volume (vph)	0	0	0	3420	2072	959
Future Volume (vph)	0	0	0	3420	2072	959
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	0.91	0.91	1.00
FrT						0.850
FrT Protected						
Satd. Flow (prot)	0	0	0	5085	5085	1583
FrT Permitted						
Satd. Flow (perm)	0	0	0	5085	5085	1583
Link Speed (k/h)	50			50	50	
Link Distance (m)	159.6			186.0	145.2	
Travel Time (s)	11.5			13.4	10.5	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	0	0	0	3717	2252	1042
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	0	0	3717	2252	1042
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Left	Left	Right
Median Width (m)	0.0			0.0	0.0	
Link Offset (m)	0.0			0.0	0.0	
Crosswalk Width (m)	4.8			4.8	4.8	
Two way Left Turn Lane						
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (k/h)	25	15	25			15
Sign Control	Stop	Stop	Free	Free	Free	Free
Intersection Summary						
Area Type:	Other					
Control Type:	Unsignalized					
Intersection Capacity Utilization	85.5% ICU Level of Service E					
Analysis Period (min)	15					

HCM Signalized Intersection Capacity Analysis  
1: Trafalgar Road & North Service Road/QEW WB Off Ramp

2035 Total PM Peak Hour.syn  
05-04-2022

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	24	0	317	664	158	330	0	2861	559	0	2050	11
Traffic Volume (vph)	24	0	317	664	158	330	0	2861	559	0	2050	11
Future Volume (vph)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	3.5	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lane Util. Factor	1.00	1.00	0.95	0.95	1.00	0.91	1.00	0.91	1.00	0.91	1.00	0.91
Frpb, ped/bikes	1.00	1.00	1.00	1.00	0.98	1.00	1.00	0.93	1.00	0.93	1.00	0.93
Flpb, ped/bikes	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt	1.00	0.85	1.00	1.00	0.85	1.00	1.00	0.85	1.00	0.85	1.00	0.85
Flt Permitted	0.95	1.00	0.95	0.97	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Satd. Flow (prot)	1805	1615	1681	1723	1557	5085	1482	4217	1497	4217	1497	1497
Flt Permitted	0.95	1.00	0.95	0.97	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Satd. Flow (perm)	1805	1615	1681	1723	1557	5085	1482	4217	1497	4217	1497	1497
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	26	0	345	722	172	359	0	3110	608	0	2228	12
RTOR Reduction (vph)	0	0	21	0	0	47	0	138	0	0	0	5
Lane Group Flow (vph)	26	0	324	440	454	312	0	3110	470	0	2228	7
Conf. Peds. (#/hr)	2	0	0	0	0	2	14	14	14	14	14	14
Heavy Vehicles (%)	0%	0%	0%	2%	1%	2%	0%	2%	1%	0%	23%	0%
Turn Type	Prot	Perm	Perm	NA	Perm	NA	Perm	NA	Perm	NA	Perm	NA
Protected Phases	7			8			2			6		
Permitted Phases			4	8		8		2		6		
Actuated Green, G (s)	3.0	44.3	36.8	36.8	36.8	81.7	81.7	81.7	81.7	81.7	81.7	81.7
Effective Green, g (s)	4.0	47.3	39.8	39.8	39.8	84.7	84.7	84.7	84.7	84.7	84.7	84.7
Actuated g/C Ratio	0.03	0.34	0.28	0.28	0.28	0.61	0.61	0.61	0.61	0.61	0.61	0.61
Clearance Time (s)	4.5	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Lane Grp. Cap (vph)	51	545	477	489	442	3076	896	2551	905	2551	905	905
v/s Ratio Prot	0.01					c0.61				0.53		
v/s Ratio Perm		c0.20	0.26	0.26	0.20	0.32	0.32	0.32	0.32	0.32	0.32	0.32
vic Ratio	0.51	0.60	0.92	0.93	0.71	1.01	0.52	0.87	0.01	0.87	0.01	0.01
Uniform Delay, d1	67.0	38.4	48.6	48.7	44.9	27.6	16.0	23.2	11.0	23.2	11.0	11.0
Progression Factor	1.00	1.00	1.00	1.00	1.00	0.63	0.60	1.00	1.00	0.63	0.60	1.00
Incremental Delay, d2	7.8	1.8	23.4	23.9	5.1	13.0	0.9	4.5	0.0	4.5	0.0	0.0
Delay (s)	74.8	40.2	72.0	72.6	49.9	30.4	10.5	27.7	11.0	27.7	11.0	11.0
Level of Service	E	D	E	E	D	C	B	C	B	C	B	B
Approach Delay (s)		42.6			65.9		27.1		27.6		27.6	
Approach LOS		D			E		C		C		C	
Intersection Summary												
HCM 2000 Control Delay												C
HCM 2000 Volume to Capacity ratio	34.4											C
Actuated Cycle Length (s)	140.0											11.5
Intersection Capacity Utilization	91.8%											F
Analysis Period (min)	15											
c Critical Lane Group												

Lanes, Volumes, Timings  
10: Lyons Lane & South Service Road

2022 Existing AM Peak Hour.syn  
04-27-2022

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	0	4	3	3	3	3	3	3	3	3	3	1
Traffic Volume (vph)	0	4	3	3	3	3	3	3	3	3	3	1
Future Volume (vph)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor												
Frt												
Satd. Flow (prot)	0	1710	1594	0	1296	0	0	0	0	0	0	0
Flt Permitted	0	1710	1594	0	1296	0	0	0	0	0	0	0
Satd. Flow (perm)	0	1710	1594	0	1296	0	0	0	0	0	0	0
Link Speed (k/h)		50	50		50					50		
Link Distance (m)		60.5	89.6		37.6							
Travel Time (s)		4.4	6.5		2.7							
Conf. Peds. (#/hr)	6											
Peak Hour Factor	0.25	0.50	0.75	0.75	0.38	0.25						
Heavy Vehicles (%)	0%	0%	0%	0%	33%	0%						
Adj. Flow (vph)	0	8	4	4	8	4						
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	8	8	0	12	0						
Enter Blocked Intersection	No	No	No	No	No	No						
Lane Alignment	Left	Left	Left	Right	Left	Right						
Median Width(m)		0.0	0.0		0.0							
Link Offset(m)		0.0	0.0		0.0							
Crosswalk Width(m)		4.8	4.8		4.8							
Two way Left Turn Lane												
Headway Factor	1.14	1.14	1.14	1.14	1.14	1.14						
Turning Speed (k/h)	24				Free					Stop		
Sign Control					Free					Stop		
Intersection Summary												
Area Type:												CBD
Control Type:												Unsignalized
Intersection Capacity Utilization												15.1%
Analysis Period (min)												15
ICU Level of Service A												

HCM Unsignalized Intersection Capacity Analysis  
 10: Lyons Lane & South Service Road

2022 Existing AM Peak Hour.syn  
 04-27-2022

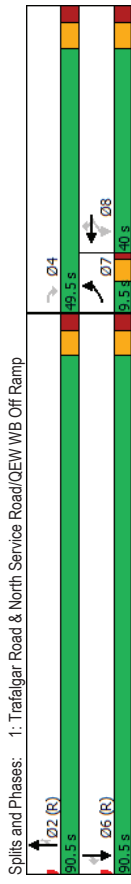


Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	0	4	3	3	3	1
Traffic Volume (veh/h)	0	4	3	3	3	1
Future Volume (Veh/h)	0	4	3	3	3	1
Sign Control	Free	Free	Stop	Stop	0%	0%
Grade	0%	0%	0%	0%	0%	0%
Peak Hour Factor	0.25	0.50	0.75	0.75	0.38	0.25
Hourly flow rate (vph)	0	8	4	4	8	4
Pedestrians	0	1	1	1	6	4
Lane Width (m)	3.6	3.6	3.6	3.6	3.6	3.6
Walking Speed (m/s)	1.2	1.2	1.2	1.2	1.2	1.2
Percent Blockage	0	0	0	0	1	1
Right turn flare (veh)	None	None	None	None	None	None
Median type	None	None	None	None	None	None
Median storage (veh)						
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume	14				21	12
vC1, stage 1 conf vol						
vC2, stage 2 conf vol	14				21	12
vCu, unblocked vol	4.1				6.7	6.2
tC, single (s)						
tC, 2 stage (s)						
tF (s)	2.2				3.8	3.3
p0 queue free %	100				99	100
cM capacity (veh/h)	1609				916	1069
Direction_Lane #	EB 1	WB 1	SB 1			
Volume Total	8	8	12			
Volume Left	0	0	8			
Volume Right	0	4	4			
GSH	1609	1700	962			
Volume to Capacity	0.00	0.00	0.01			
Queue Length 95th (m)	0.0	0.0	0.3			
Control Delay (s)	0.0	0.0	8.8			
Lane LOS	A	A	A			
Approach Delay (s)	0.0	0.0	8.8			
Approach LOS	A	A	A			
<b>Intersection Summary</b>						
Average Delay					3.8	
Intersection Capacity Utilization					15.1%	ICU Level of Service A
Analysis Period (min)					15	

Lanes, Volumes, Timings  
 1: Trafalgar Road & North Service Road/QEW WB Off Ramp

2035 Total PM Peak Hour.syn  
 05-04-2022

Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	0	4	3	3	3	1
Traffic Volume (veh/h)	0	4	3	3	3	1
Future Volume (Veh/h)	0	4	3	3	3	1
Sign Control	Free	Free	Stop	Stop	0%	0%
Grade	0%	0%	0%	0%	0%	0%
Peak Hour Factor	0.25	0.50	0.75	0.75	0.38	0.25
Hourly flow rate (vph)	0	8	4	4	8	4
Pedestrians	0	1	1	1	6	4
Lane Width (m)	3.6	3.6	3.6	3.6	3.6	3.6
Walking Speed (m/s)	1.2	1.2	1.2	1.2	1.2	1.2
Percent Blockage	0	0	0	0	1	1
Right turn flare (veh)	None	None	None	None	None	None
Median type	None	None	None	None	None	None
Median storage (veh)						
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume	14				21	12
vC1, stage 1 conf vol						
vC2, stage 2 conf vol	14				21	12
vCu, unblocked vol	4.1				6.7	6.2
tC, single (s)						
tC, 2 stage (s)						
tF (s)	2.2				3.8	3.3
p0 queue free %	100				99	100
cM capacity (veh/h)	1609				916	1069



Intersection Capacity Utilization 91.8%  
 ICU Level of Service F  
 Analysis Period (min) 15  
 ~ Volume exceeds capacity, queue is theoretically infinite.  
 Queue shown is maximum after two cycles.  
 # 95th percentile volume exceeds capacity, queue may be longer.  
 Queue shown is maximum after two cycles.  
 m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 1: Trafalgar Road & North Service Road/QEW WB Off Ramp

Lanes, Volumes, Timings  
 1: Trafalgar Road & North Service Road/QEW WB Off Ramp

Lanes, Volumes, Timings  
 11: Argus Rd & South Service Rd

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Turn Type	7											
Protected Phases												
Permitted Phases	4	8	8	8	8	8	2	2	2	2	6	6
Detector Phase	7	4	8	8	8	8	2	2	2	2	6	6
Switch Phase												
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Minimum Split (s)	9.5	38.0	38.0	38.0	38.0	38.0	35.0	35.0	35.0	35.0	35.0	35.0
Total Split (s)	9.5	49.5	40.0	40.0	40.0	40.0	90.5	90.5	90.5	90.5	90.5	90.5
Total Split (%)	6.8%	35.4%	28.6%	28.6%	28.6%	28.6%	64.6%	64.6%	64.6%	64.6%	64.6%	64.6%
Maximum Green (s)	5.0	42.5	33.0	33.0	33.0	33.0	83.5	83.5	83.5	83.5	83.5	83.5
Yellow Time (s)	3.5	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
All-Red Time (s)	1.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Lost Time Adjust (s)	-1.0	-3.0	-3.0	-3.0	-3.0	-3.0	-3.0	-3.0	-3.0	-3.0	-3.0	-3.0
Total Lost Time (s)	3.5	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lead/Lag	Lead	Lag	Lag	Lag	Lag	Lag	Lag	Lag	Lag	Lag	Lag	Lag
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	None	None	None	None	None	None	C-Max	C-Max	C-Max	C-Max	C-Max	C-Max
Walk Time (s)	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0
Flash Dont Walk (s)	11.0	11.0	11.0	11.0	11.0	11.0	11.0	11.0	11.0	11.0	11.0	11.0
Pedestrian Calls (#/hr)	0	0	0	0	0	0	0	0	0	0	0	0
Act Effct Green (s)	6.0	45.5	39.8	39.8	39.8	39.8	86.5	86.5	86.5	86.5	86.5	86.5
Actuated g/C Ratio	0.04	0.32	0.28	0.28	0.28	0.28	0.62	0.62	0.62	0.62	0.62	0.62
v/c Ratio	0.34	0.63	0.92	0.93	0.73	0.39	0.99	0.58	0.58	0.86	0.01	0.01
Control Delay	77.2	42.5	74.7	75.3	47.6	25.3	4.5	4.5	4.5	26.0	0.0	0.0
Queue Delay	0.0	0.0	0.0	0.0	0.0	2.3	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	77.2	42.5	74.7	75.3	47.6	27.6	4.5	4.5	4.5	26.0	0.0	0.0
LOS	E	D	E	E	D	C	A	A	A	C	C	A
Approach Delay	44.9			67.1		23.8				25.8		
Approach LOS	D			E		C				C		
Queue Length 50th (m)	7.5	78.0	~142.5	~148.0	81.8	210.5	22.5	22.5	22.5	182.8	0.0	0.0
Queue Length 95th (m)	18.3	113.6	#215.0	#221.1	#130.2	#362.0	m23.2	m23.2	m23.2	207.3	0.0	0.0
Internal Link Dist (m)	96.4			141.1		121.2				185.2		
Turn Bay Length (m)	50.0						70.0					
Base Capacity (vph)	77	545	478	489	489	3141	1049	1049	1049	2605	950	950
Starvation Cap Reductn	0	0	0	0	0	35	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.34	0.63	0.92	0.93	0.73	1.00	0.58	0.58	0.58	0.86	0.01	0.01
Intersection Summary												
Area Type:	Other											
Actuated Cycle Length: 140												
Offset: 0 (0%); Referenced to phase 2:NBT and 6:SBT; Start of Green												
Natural Cycle: 135												
Control Type: Actuated-Coordinated												
Maximum v/c Ratio: 0.99												
Intersection Signal Delay: 32.6	Intersection LOS: C											

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	1	4		1	4					1	4	3
Traffic Volume (vph)	5	572	120	4	3					120	4	3
Future Volume (vph)	1	5	572	120	4	3				120	4	3
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor												
Ft		0.973				0.942						
Flt Protected		0.988				0.972						
Satd. Flow (prot)	0	1352	1625	0	1566	0						
Flt Permitted		0.988				0.972						
Satd. Flow (perm)	0	1352	1625	0	1566	0						
Link Speed (k/h)		50	50		50					50		
Link Distance (m)		177.7	165.2		103.5							
Travel Time (s)		12.8	11.9		7.5							
Conf. Peds. (#/hr)	1			1						5		1
Peak Hour Factor	0.25	0.42	0.86	0.72	0.25	0.25				0.25		0.25
Heavy Vehicles (%)	100%	0%	3%	0%	0%	0%				0%		0%
Adj. Flow (vph)	4	12	665	167	16	12						
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	16	832	0	28	0						
Enter Blocked Intersection	No	No	No	No	No	No				No	No	No
Lane Alignment	Left	Left	Left	Right	Left	Right				Left	Right	
Median Width(m)		0.0	0.0		3.6							
Link Offset(m)		0.0	0.0		0.0					0.0		
Crosswalk Width(m)		4.8	4.8		4.8							
Two way Left Turn Lane												
Headway Factor	1.14	1.14	1.14	1.14	1.14	1.14				1.14		1.14
Turning Speed (k/h)	24									24		14
Sign Control		Free	Free		Free	Stop						
Intersection Summary												
Area Type:	CBD											
Control Type: Unsignalized												
Intersection Capacity Utilization 51.9%												
Analysis Period (min) 15	ICU Level of Service A											



HCM Unsignalized Intersection Capacity Analysis  
 11: Argus Rd & South Service Rd

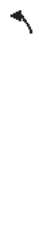
2022 Existing AM Peak Hour.syn  
 04-27-2022



Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	1	4	1	1	1	1
Traffic Volume (veh/h)	1	5	572	120	4	3
Future Volume (Veh/h)	1	5	572	120	4	3
Sign Control	Free	Free	Free	Stop	Stop	0%
Grade	0%	0%	0%	0%	0%	0%
Peak Hour Factor	0.25	0.42	0.86	0.72	0.25	0.25
Hourly flow rate (vph)	4	12	665	167	16	12
Pedestrians	1	5	3.6	3.6	1	1
Lane Width (m)	1.2	1.2	1.2	1.2	1.2	1.2
Percent Blockage	0	0	0	0	0	0
Right turn flare (veh)	None	None	None	None	None	None
Median storage (veh)	368					
Upstream signal (m)	833				774	750
pX, platoon unblocked						
vC, conflicting volume						
vC1, stage 1 conf/vol						
vC2, stage 2 conf/vol						
tC, single (s)	5.1	0.42	0.86	0.72	0.25	0.25
tC, 2 stage (s)	3.1					
p0 queue free %	99				96	97
cM capacity (veh/h)	500				365	414
Direction, Lane #	EB 1	WB 1	SB 1			
Volume Total	16	832	28			
Volume Left	4	0	16			
Volume Right	0	167	12			
GSH	500	1700	384			
Volume to Capacity	0.01	0.49	0.07			
Queue Length 95th (m)	0.2	0.0	1.9			
Control Delay (s)	3.1	0.0	15.1			
Lane LOS	A	C	C			
Approach Delay (s)	3.1	0.0	15.1			
Approach LOS	C					
Intersection Summary						
Average Delay	0.5					
Intersection Capacity Utilization	51.9%					
Analysis Period (min)	15					

Lanes, Volumes, Timings  
 1: Trafalgar Road & North Service Road/QEW WB Off Ramp

2035 Total PM Peak Hour.syn  
 05-04-2022



Lane Group	EBL	EBT	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	1	1	1	1	1	1	1	1	1	1	1
Traffic Volume (vph)	24	0	317	664	158	330	0	2861	559	0	2050
Future Volume (vph)	24	0	317	664	158	330	0	2861	559	0	2050
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (m)	50.0	0.0	0.0	0.0	0.0	0.0	0.0	70.0	0.0	0.0	0.0
Storage Lanes	1	1	1	1	1	1	1	1	1	1	1
Taper Length (m)	7.5	1.0	7.5	0.95	1.00	1.00	0.91	1.00	1.00	0.91	1.00
Lane Util. Factor	1.00	1.00	1.00	0.95	1.00	1.00	0.91	1.00	1.00	0.91	1.00
Ped Bike Factor	1.00	0.850	0.850	0.970	0.98	0.850	0.93	0.850	0.850	0.93	0.850
Flt Protected	0.950	0.950	0.950	0.970	0.98	0.850	0.93	0.850	0.850	0.93	0.850
Satd. Flow (prot)	1805	0	1615	1681	1723	1583	0	5085	1599	0	4217
Flt Permitted	0.950	0.950	0.950	0.970	0.98	0.850	0.93	0.850	0.850	0.93	0.850
Satd. Flow (perm)	1801	0	1615	1681	1723	1557	0	5085	1482	0	4217
Right Turn on Red	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Satd. Flow (RTOR)	31	31	31	31	31	31	31	31	31	31	31
Link Speed (k/h)	50	50	50	50	50	50	50	50	50	50	50
Link Distance (m)	120.4	120.4	165.1	111.9	105.5	105.5	105.5	105.5	105.5	105.5	105.5
Travel Time (s)	8.7	8.7	11.9	7.5	7.5	7.5	7.5	7.5	7.5	7.5	7.5
Conf. Peds. (#/hr)	2	2	2	2	2	2	2	2	2	2	2
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%
Adj. Flow (vph)	26	0	345	722	172	359	0	3110	608	0	2228
Shared Lane Traffic (%)	39%	39%	39%	39%	39%	39%	39%	39%	39%	39%	39%
Lane Group Flow (vph)	26	0	345	440	454	359	0	3110	608	0	2228
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Left	Left	Left	Left	Left	Left	Left	Left	Left
Median Width (m)	3.6	3.6	3.6	3.6	3.6	3.6	3.6	3.6	3.6	3.6	3.6
Link Offset (m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Crosswalk Width (m)	4.8	4.8	4.8	4.8	4.8	4.8	4.8	4.8	4.8	4.8	4.8
Two way Left Turn Lane											
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (k/h)	25	15	25	15	25	15	25	15	25	15	25
Number of Detectors	1	1	1	2	1	2	2	1	2	1	2
Detector Template	Left	Right	Left	Thru	Right	Thru	Right	Thru	Right	Thru	Right
Leading Detector (m)	2.0	2.0	2.0	10.0	2.0	10.0	2.0	10.0	2.0	10.0	2.0
Trailing Detector (m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Position (m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Size (m)	2.0	2.0	2.0	0.6	2.0	0.6	2.0	0.6	2.0	0.6	2.0
Detector 1 Type	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex
Detector 1 Channel											
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 2 Position (m)				9.4		9.4		9.4		9.4	
Detector 2 Size (m)				0.6		0.6		0.6		0.6	
Detector 2 Type				Ch+Ex		Ch+Ex		Ch+Ex		Ch+Ex	
Detector 2 Channel											
Detector 2 Extend (s)				0.0		0.0		0.0		0.0	

2035 Total AM Peak Hour.syn  
05-04-2022

HCM Unsignalized Intersection Capacity Analysis  
12: Site Driveway

Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	←	←	←	←	←	←
Traffic Volume (veh/h)	8	8	18	10	8	78
Future Volume (Veh/h)	8	8	18	10	8	78
Sign Control	Free	Free	Free	Stop	Stop	Stop
Grade	0%	0%	0%	0%	0%	0%
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	9	9	20	11	9	85
Pedestrians						
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)	None					
Median type	None					
Median storage (veh)						
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume						
vC1, stage 1 conf vol		18			64	14
vC2, stage 2 conf vol						
tC, single (s)		18			64	14
tC, 2 stage (s)		4.1			6.4	6.2
tF (s)		2.2			3.5	3.3
p0 queue free %		99			99	92
cM capacity (veh/h)		1599			929	1067
Direction_Lane #	EB 1	WB 1	NB 1			
Volume Total	18	31	94			
Volume Left	0	20	9			
Volume Right	9	0	85			
GSH	1700	1599	1052			
Volume to Capacity	0.01	0.01	0.09			
Queue Length 95th (m)	0.0	0.3	2.3			
Control Delay (s)	0.0	4.7	8.8			
Lane LOS	A	A	A			
Approach Delay (s)	0.0	4.7	8.8			
Approach LOS	A	A	A			
Intersection Summary						
Average Delay		6.8				A
Intersection Capacity Utilization		20.1%			ICU Level of Service	A
Analysis Period (min)		15				

2022 Existing PM Peak Hour.syn  
05-04-2022

Lanes, Volumes, Timings  
1: Trafalgar Road & North Service Road/QEW WB Off Ramp

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	←	←	←	←	←	←	←	←	←	←	←	←
Traffic Volume (vph)	19	0	247	457	129	360	0	2651	501	0	2506	11
Future Volume (vph)	19	0	247	457	129	360	0	2651	501	0	2506	11
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (m)	50.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	70.0	0.0	0.0	0.0
Storage Lanes	1		1	1	1	1	0	1	1	0	1	1
Taper Length (m)	7.5		7.5	7.5	7.5	7.5		7.5		7.5		
Lane Util. Factor	1.00	1.00	1.00	0.95	0.95	1.00	1.00	0.91	1.00	1.00	0.91	1.00
Ped Bike Factor	1.00		0.850		0.99	0.99		0.850	0.97		0.850	0.93
Flt			0.950	0.950	0.972			0.950	0.850		0.950	0.850
Satd. Flow (prot)	1805	0	1615	1681	1727	1583	0	5085	1599	0	4217	1615
Flt Permitted	0.950		0.950	0.972				0.950	0.850		0.950	0.850
Satd. Flow (perm)	1800	0	1615	1681	1727	1563	0	5085	1556	0	4217	1497
Right Turn on Red	Yes		Yes		Yes			Yes	Yes		Yes	Yes
Satd. Flow (RTOR)		31							129			70
Link Speed (k/h)		50			50			50				50
Link Distance (m)		120.4			165.1			145.2				209.2
Travel Time (s)		8.7			11.9			10.5				15.1
Conf. Peds. (#/hr)	2				2		14		14			14
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	0%	0%	0%	2%	1%	2%	0%	2%	1%	0%	23%	0%
Adj. Flow (vph)	21	0	268	497	140	391	0	2882	545	0	2724	12
Shared Lane Traffic (%)			37%									
Lane Group Flow (vph)	21	0	268	313	324	391	0	2882	545	0	2724	12
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Right	Right
Median Width(m)		3.6			3.6			0.0			0.0	0.0
Link Offset(m)		0.0			0.0			0.0			0.0	0.0
Crosswalk Width(m)		4.8			4.8			4.8			4.8	4.8
Two way Left Turn Lane												
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (k/h)	25	15	25	15	25	15	25	15	25	15	25	15
Number of Detectors	1	1	1	2	1	2	2	1	1	2	1	1
Detector Template	Left	Right	Left	Thru	Right	Thru	Right	Thru	Right	Thru	Right	Right
Leading Detector (m)	2.0	2.0	2.0	10.0	2.0	10.0	2.0	10.0	2.0	10.0	2.0	2.0
Trailing Detector (m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Position(m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Size(m)	2.0	2.0	2.0	0.6	2.0	0.6	2.0	0.6	2.0	0.6	2.0	0.6
Detector 1 Type	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 2 Position(m)				9.4		9.4		9.4		9.4		9.4
Detector 2 Size(m)				0.6		0.6		0.6		0.6		0.6
Detector 2 Type				Ch+Ex		Ch+Ex		Ch+Ex		Ch+Ex		Ch+Ex
Detector 2 Channel												
Detector 2 Extend (s)				0.0		0.0		0.0		0.0		0.0

Lanes, Volumes, Timings  
1: Trafalgar Road & North Service Road/QEW WB Off Ramp

Lanes, Volumes, Timings  
12: Site Driveway

2022 Existing PM Peak Hour.syn  
05-04-2022

2035 Total AM Peak Hour.syn  
05-04-2022

	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Group	Prot	7									NA	Perm
Protected Phases											6	6
Permitted Phases												
Detector Phase	7	4	4	8	8	Free	2	2	Free	Free	6	6
Switch Phase												
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Minimum Split (s)	23.0	38.0	38.0	38.0	38.0	35.0	35.0	35.0	35.0	35.0	35.0	35.0
Total Split (s)	23.0	61.0	38.0	38.0	38.0	79.0	79.0	79.0	79.0	79.0	79.0	79.0
Total Split (%)	16.4%	43.6%	27.1%	27.1%	27.1%	56.4%	56.4%	56.4%	56.4%	56.4%	56.4%	56.4%
Maximum Green (s)	18.0	54.0	31.0	31.0	31.0	72.0	72.0	72.0	72.0	72.0	72.0	72.0
Yellow Time (s)	3.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
All-Red Time (s)	2.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Lost Time Adjust (s)	-1.0	-3.0	-3.0	-3.0	-3.0	-3.0	-3.0	-3.0	-3.0	-3.0	-3.0	-3.0
Total Lost Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lead/Lag	Lead	Lag	Lag	Lag	Lag	Lag	Lag	Lag	Lag	Lag	Lag	Lag
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	None	None	None	None	None	C-Max	C-Max	C-Max	C-Max	C-Max	C-Max	C-Max
Walk Time (s)		7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0
Flash Dont Walk (s)		11.0	11.0	11.0	11.0	11.0	11.0	11.0	11.0	11.0	11.0	11.0
Pedestrian Calls (#/hr)		0	0	0	0	0	0	0	0	0	0	0
Act Effct Green (s)	8.2	42.4	34.5	34.5	34.5	140.0	89.6	140.0	140.0	89.6	89.6	89.6
Actuated g/C Ratio	0.06	0.30	0.25	0.25	0.25	1.00	0.64	1.00	0.64	0.64	0.64	0.64
v/c Ratio	0.20	0.53	0.76	0.76	0.76	0.25	0.89	0.35	0.35	1.01	0.01	0.01
Control Delay	66.5	37.4	60.3	60.4	0.4	19.5	0.4	19.5	0.4	45.6	0.0	0.0
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	66.5	37.4	60.3	60.4	0.4	19.5	0.4	19.5	0.4	45.6	0.0	0.0
LOS	E	D	E	E	A	B	B	A	A	D	D	A
Approach Delay		39.5		37.6		16.5		16.5		45.4		45.4
Approach LOS		D		D		B		B		D		D
Queue Length 50th (m)	6.0	54.7	89.4	92.7	0.0	150.4	0.0	150.4	0.0	~318.4	0.0	0.0
Queue Length 95th (m)	15.0	73.3	117.6	121.2	0.0	#360.3	m0.0	#360.3	m0.0	#379.5	0.0	0.0
Internal Link Dist (m)		96.4		141.1		121.2		121.2		185.2		185.2
Turn Bay Length (m)	50.0					70.0		70.0				
Base Capacity (vph)	244	675	442	454	1563	3255	1556	3255	1556	2700	983	983
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.09	0.40	0.71	0.71	0.25	0.89	0.35	0.89	0.35	1.01	0.01	0.01
Intersection Summary	Other											
Area Type:	Other											
Control Type:	Unsignalized											
Intersection Capacity Utilization	20.1%											
Analysis Period (min)	15											
ICU Level of Service A												

	EBT	EBR	WBL	WBT	NBL	NBR
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	8	8	18	10	8	78
Future Volume (vph)	8	8	18	10	8	78
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt	0.932				0.878	
Flt Protected					0.969	0.995
Satd. Flow (prot)	1736	0	0	1805	1627	0
Flt Permitted					0.969	0.995
Satd. Flow (perm)	1736	0	0	1805	1627	0
Link Speed (k/h)	50				50	50
Link Distance (m)	103.3				722.4	44.4
Travel Time (s)	7.4				52.0	3.2
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	9	9	20	11	9	85
Shared Lane Traffic (%)						
Lane Group Flow (vph)	18	0	0	31	94	0
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Left	Left	Right
Median Width (m)	0.0	0.0	0.0	0.0	3.6	0.0
Link Offset (m)	0.0	0.0	0.0	0.0	0.0	0.0
Crosswalk Width (m)	4.8				4.8	4.8
Two way Left Turn Lane						
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (k/h)	Free	15	25	Free	25	15
Sign Control	Free	Free	Free	Free	Stop	Stop
Intersection Summary	Other					
Area Type:	Other					
Control Type:	Unsignalized					
Intersection Capacity Utilization	20.1%					
Analysis Period (min)	15					
ICU Level of Service A						

HCM Unsignalized Intersection Capacity Analysis  
 11: Argus Rd & South Service Rd

2035 Total AM Peak Hour.syn  
 05-04-2022

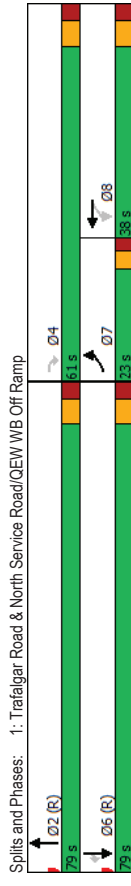


Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	1	6	740	173	19	59
Traffic Volume (veh/h)	1	6	740	173	19	59
Future Volume (Veh/h)	1	6	740	173	19	59
Sign Control	Free	Free	0%	0%	Stop	0%
Grade	0.25	0.42	0.86	0.72	0.25	0.25
Peak Hour Factor	4	14	860	240	76	236
Hourly flow rate (vph)	1	5	5	1	1	1
Pedestrians	3.6	3.6	3.6	3.6	3.6	3.6
Lane Width (m)	1.2	1.2	1.2	1.2	1.2	1.2
Walking Speed (m/s)	0	0	0	0	0	0
Percent Blockage	None	None	None	None	None	None
Right turn flare (veh)	None	None	None	None	None	None
Median type	None	None	None	None	None	None
Median storage (veh)	None	None	None	None	None	None
Upstream signal (m)	358	358	358	358	358	358
pX, platoon unblocked	1101	1101	1101	1101	1008	982
vC, conflicting volume	1101	1101	1101	1101	1008	982
vC1, stage 1 conf vol	1101	1101	1101	1101	1008	982
vC2, stage 2 conf vol	5.1	5.1	5.1	5.1	6.4	6.2
tC, single (s)	3.1	3.1	3.1	3.1	3.5	3.3
tC, 2 stage (s)	99	99	99	99	71	22
tf (s)	377	377	377	377	265	304
p0 queue free %	EB 1	WB 1	SB 1			
cM capacity (veh/h)	18	1100	312			
Direction_Lane #	4	0	76			
Volume Total	0	240	236			
Volume Left	377	1700	294			
Volume Right	0.01	0.65	1.06			
GSH	0.3	0.0	96.2			
Volume to Capacity	3.4	0.0	109.1			
Queue Length 95th (m)	A	F	F			
Control Delay (s)	3.4	0.0	109.1			
Lane LOS	A	F	F			
Approach Delay (s)	3.4	0.0	109.1			
Approach LOS	F	F	F			
Intersection Summary						
Average Delay			23.8			
Intersection Capacity Utilization			67.2%	ICU Level of Service		C
Analysis Period (min)			15			

Lanes, Volumes, Timings  
 1: Trafalgar Road & North Service Road/QEW WB Off Ramp

2022 Existing PM Peak Hour.syn  
 05-04-2022

Intersection Capacity Utilization 89.8%  
 ICU Level of Service E  
 Analysis Period (min) 15  
 ~ Volume exceeds capacity, queue is theoretically infinite.  
 Queue shown is maximum after two cycles.  
 # 95th percentile volume exceeds capacity, queue may be longer.  
 Queue shown is maximum after two cycles.  
 m Volume for 95th percentile queue is metered by upstream signal.



Splits and Phases: 1: Trafalgar Road & North Service Road/QEW WB Off Ramp

HCM Signalized Intersection Capacity Analysis  
 1: Trafalgar Road & North Service Road/QEW WB Off Ramp  
 2022 Existing PM Peak Hour.syn  
 05-04-2022

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	19	0	247	457	129	360	0	2651	501	0	2506	11
Traffic Volume (vph)	19	0	247	457	129	360	0	2651	501	0	2506	11
Future Volume (vph)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Ideal Flow (vphpl)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Total Lost time (s)	1.00	1.00	0.95	0.95	1.00	1.00	0.91	1.00	0.91	1.00	0.91	1.00
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.99	1.00	0.97	1.00	0.93
Frpb, ped/bikes	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Flpb, ped/bikes	1.00	0.85	1.00	1.00	0.85	1.00	0.85	1.00	0.85	1.00	0.85	1.00
Flt Protected	0.95	1.00	0.95	0.97	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Satd. Flow (prot)	1805	1615	1681	1728	1563	5085	5085	1556	4217	1497	4217	1497
Flt Permitted	0.95	1.00	0.95	0.97	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Satd. Flow (perm)	1805	1615	1681	1728	1563	5085	5085	1556	4217	1497	4217	1497
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	21	0	268	497	140	391	0	2882	545	0	2724	12
RTOR Reduction (vph)	0	0	21	0	0	0	0	0	0	0	0	4
Lane Group Flow (vph)	21	0	247	313	324	391	0	2882	545	0	2724	8
Conf. Peds. (#/hr)	2	0	247	313	324	391	2	14	14	14	14	14
Heavy Vehicles (%)	0%	0%	0%	2%	1%	2%	0%	2%	1%	0%	23%	0%
Turn Type	Prot	Perm	Perm	NA	Free	NA	Free	NA	Free	NA	NA	Perm
Protected Phases	7			8			2				6	
Permitted Phases			4	8		Free	Free	Free	Free		84.6	84.6
Actuated Green, G (s)	4.9	41.4	31.5	31.5	140.0	84.6	140.0	84.6	140.0	84.6	84.6	84.6
Effective Green, g (s)	5.9	44.4	34.5	34.5	140.0	87.6	140.0	87.6	140.0	87.6	87.6	87.6
Actuated g/C Ratio	0.04	0.32	0.25	0.25	1.00	0.63	1.00	0.63	1.00	0.63	0.63	0.63
Clearance Time (s)	5.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Lane Grp Cap (vph)	76	512	414	425	1563	3181	1556	2638	936			
v/s Ratio Prot	0.01					0.57						
v/s Ratio Perm	0.28	0.48	0.76	0.76	0.25	0.91	0.35	1.03	0.01			
Uniform Delay, d1	65.0	38.5	48.9	48.9	0.0	22.6	0.0	26.2	9.9			
Progression Factor	1.00	1.00	1.00	1.00	1.00	0.71	1.00	1.00	1.00			
Incremental Delay, d2	2.0	0.7	7.7	7.9	0.4	2.9	0.4	2.6	0.0			
Delay (s)	67.0	39.2	56.5	56.8	0.4	18.9	0.4	52.8	9.9			
Level of Service	E	D	E	E	A	B	A	D	A			
Approach Delay (s)		41.3		35.3		15.9		52.6				
Approach LOS		D		D		B		D				
Intersection Summary												
HCM 2000 Control Delay	33.0 HCM 2000 Level of Service C											
HCM 2000 Volume to Capacity ratio	0.94											
Actuated Cycle Length (s)	140.0 Sum of lost time (s)											
Intersection Capacity Utilization	89.8% ICU Level of Service E											
Analysis Period (min)	15											
c Critical Lane Group												

Lanes, Volumes, Timings  
 11: Argus Rd & South Service Rd  
 2035 Total AM Peak Hour.syn  
 05-04-2022

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	SBL	SBR
Lane Configurations	1	4	4	1	1	1	1	1
Traffic Volume (vph)	1	6	740	173	19	59	19	59
Future Volume (vph)	1	6	740	173	19	59	19	59
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor			0.971			0.988		
Flt Protected			0.989			0.988		
Satd. Flow (prot)	0	1384	1622	0	1517	0		
Flt Permitted			0.989			0.988		
Satd. Flow (perm)	0	1384	1622	0	1517	0		
Link Speed (k/h)		50	50		50		50	
Link Distance (m)		177.7	165.2		103.5		7.5	
Travel Time (s)		12.8	11.9		7.5		1	
Conf. Peds. (#/hr)	1			1		5		1
Peak Hour Factor	0.25	0.42	0.86	0.72	0.25	0.25	0.25	0.25
Heavy Vehicles (%)	100%	0%	3%	0%	0%	0%	0%	0%
Adj. Flow (vph)	4	14	860	240	76	236		
Shared Lane Traffic (%)								
Lane Group Flow (vph)	0	18	1100	0	312	0		
Enter Blocked Intersection	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Left	Right	Left	Right	Left	Right
Median Width(m)		0.0	0.0		0.0		3.6	
Link Offset(m)		0.0	0.0		0.0		0.0	
Crosswalk Width(m)		4.8	4.8		4.8		4.8	
Two way Left Turn Lane								
Headway Factor	1.14	1.14	1.14	1.14	1.14	1.14	1.14	1.14
Turning Speed (k/h)	24		Free	Free	Free	Stop		
Sign Control			Free	Free	Free	Stop		
Intersection Summary								
Area Type:	CBD							
Control Type:	Unsignalized							
Intersection Capacity Utilization	67.2%							
Analysis Period (min)	15							
ICU Level of Service C								

HCM Unsignalized Intersection Capacity Analysis  
 10: Lyons Lane & South Service Road

2035 Total AM Peak Hour.syn  
 05-04-2022



Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	0	10	8	16	16	2
Traffic Volume (veh/h)	0	10	8	16	16	2
Future Volume (Veh/h)	0	10	8	16	16	2
Sign Control	Free	Free	Free	Free	Stop	Stop
Grade	0%	0%	0%	0%	0%	0%
Peak Hour Factor	0.25	0.50	0.75	0.75	0.38	0.25
Hourly flow rate (vph)	0	20	11	21	42	8
Pedestrians						
Lane Width (m)			3.6	3.6		
Walking Speed (m/s)			1.2	1.2		
Percent Blockage			0		1	
Right turn flare (veh)			None	None		
Median type						
Median storage (veh)						
Upstream signal (m)			393			
pX, platoon unblocked					48	28
vC, conflicting volume						
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
tC, single (s)					48	28
tC, 2 stage (s)					6.7	6.2
tF (s)					3.8	3.3
p0 queue free %					95	99
cM capacity (veh/h)					883	1048
Direction, Lane #	EB 1	WB 1	SB 1			
Volume Total	20	32	50			
Volume Left	0	0	42			
Volume Right	0	21	8			
GSH	1577	1700	906			
Volume to Capacity	0.00	0.02	0.06			
Queue Length 95th (m)	0.0	0.0	1.4			
Control Delay (s)	0.0	0.0	9.2			
Lane LOS	A	A	A			
Approach Delay (s)	0.0	0.0	9.2			
Approach LOS	A	A	A			
Intersection Summary						
Average Delay	4.5					
Intersection Capacity Utilization	15.1%					
Analysis Period (min)	15					
	ICU Level of Service A					

Lanes, Volumes, Timings  
 2: Trafalgar Road & QEW SB On Ramps

2022 Existing PM Peak Hour.syn  
 05-04-2022



Lane Group	EBL	EBR	NBL	NBT	SBL	SBR
Lane Configurations						
Traffic Volume (vph)	0	0	0	3152	3152	1571
Future Volume (vph)	0	0	0	3152	3152	1571
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	0.91	0.91	1.00
Flt Protected						0.850
Satd. Flow (prot)	0	0	0	5085	5085	1583
Flt Permitted						
Satd. Flow (perm)	0	0	0	5085	5085	1583
Link Speed (k/h)	50			50		50
Link Distance (m)	159.6			186.0		145.2
Travel Time (s)	11.5			13.4		10.5
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	0	0	0	3426	3426	1708
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	0	0	3426	3426	1708
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Left	Left	Right
Median Width(m)	0.0			0.0		0.0
Link Offset(m)	0.0			0.0		0.0
Crosswalk Width(m)	4.8			4.8		4.8
Two way Left Turn Lane						
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (k/h)	25	15	25			15
Sign Control	Stop			Free	Free	Free
Intersection Summary						
Area Type:	Other					
Control Type:	Unsignalized					
Intersection Capacity Utilization	100.6%					
Analysis Period (min)	15					
	ICU Level of Service G					

HCM Unsignalized Intersection Capacity Analysis  
 2: Trafalgar Road & QEW SB On Ramps

Lanes, Volumes, Timings  
 10: Lyons Lane & South Service Road

2022 Existing PM Peak Hour.syn  
 05-04-2022

2035 Total AM Peak Hour.syn  
 05-04-2022



Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations			↑↑↑	↑↑↑	↑↑↑	↑
Traffic Volume (veh/h)	0	0	0	3152	3152	1571
Future Volume (Veh/h)	0	0	0	3152	3152	1571
Sign Control	Stop	Free	Free	Free	Free	Free
Grade	0%	0%	0%	0%	0%	0%
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	0	0	0	3426	3426	1708
Pedestrians						
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)			None	None	None	None
Median type			None	None	None	None
Median storage (veh)						
Upstream signal (m)			186	145		
pX, platoon unblocked	0.39					
vC, conflicting volume	4568	1142	3426			
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCU, unblocked vol	4674	1142	3426			
tC, single (s)	6.8	6.9	4.1			
tC, 2 stage (s)						
tF (s)	3.5	3.3	2.2			
p0 queue free %	100	100	100			
cM capacity (veh/h)	0	194	76			
Direction_Lane #	NB.1	NB.2	NB.3	SB.1	SB.2	SB.3
Volume Total	1142	1142	1142	1142	1142	1708
Volume Left	0	0	0	0	0	0
Volume Right	0	0	0	0	0	1708
GSH	1700	1700	1700	1700	1700	1700
Volume to Capacity	0.67	0.67	0.67	0.67	0.67	1.00
Queue Length 95th (m)	0.0	0.0	0.0	0.0	0.0	0.0
Control Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0
Lane LOS						
Approach Delay (s)	0.0			0.0		
Approach LOS						
Intersection Summary						
Average Delay	0.0					
Intersection Capacity Utilization	100.6%					
ICU Level of Service	G					
Analysis Period (min)	15					

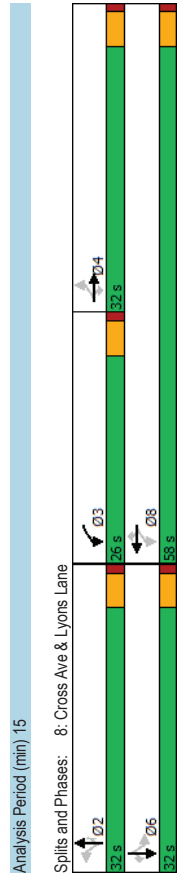
Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		←	←	←	←	←
Traffic Volume (vph)	0	10	8	16	16	2
Future Volume (vph)	0	10	8	16	16	2
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor						
Flt Protected		0.911			0.978	
Satd. Flow (prot)		1710	1558		1257	
Flt Permitted					0.960	
Satd. Flow (perm)		1710	1558		1257	
Link Speed (k/h)		50	50		50	
Link Distance (m)		60.5	89.6		37.6	
Travel Time (s)		4.4	6.5		2.7	
Conf. Peds. (#/hr)		6			6	
Peak Hour Factor		0.25	0.50		0.75	
Heavy Vehicles (%)		0%	0%		0%	
Adj. Flow (vph)		0	20		11	
Shared Lane Traffic (%)						
Lane Group Flow (vph)		0	20		32	
Enter Blocked Intersection		No	No		No	
Lane Alignment		Left	Left		Right	
Median Width(m)		0.0	0.0		3.6	
Link Offset(m)		0.0	0.0		0.0	
Crosswalk Width(m)		4.8	4.8		4.8	
Two way Left Turn Lane						
Headway Factor		1.14	1.14		1.14	
Turning Speed (k/h)		24			14	
Sign Control		Free	Free		Stop	
Intersection Summary						
Area Type:	CBD					
Control Type:	Unsignalized					
Intersection Capacity Utilization	15.1%					
ICU Level of Service A						
Analysis Period (min)	15					







Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Detector Phase	4	4		2	6	6
Switch Phase						
Minimum Initial (s)	10.0	10.0	29.0	29.0	29.0	29.0
Minimum Split (s)	38.0	38.0	36.0	36.0	36.0	36.0
Total Split (s)	56.0	56.0	84.0	84.0	84.0	84.0
Total Split (%)	40.0%	40.0%	60.0%	60.0%	60.0%	60.0%
Maximum Green (s)	49.0	49.0	77.0	77.0	77.0	77.0
Yellow Time (s)	4.0	4.0	4.0	4.0	4.0	4.0
All-Red Time (s)	3.0	3.0	3.0	3.0	3.0	3.0
Lost Time Adjust (s)	-3.0	-3.0	-3.0	-3.0	-3.0	-3.0
Total Lost Time (s)	4.0	4.0	4.0	4.0	4.0	4.0
Lead/Lag						
Lead-Lag Optimize?						
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	None	None	C-Max	C-Max	C-Max	C-Max
Walk Time (s)	7.0	7.0	7.0	7.0	7.0	7.0
Flash Dont Walk (s)	11.0	11.0	11.0	11.0	11.0	11.0
Pedestrian Calls (#/hr)	0	0	0	0	0	0
Act Effct Green (s)	50.1	50.1	81.9	81.9	81.9	81.9
Actuated g/C Ratio	0.36	0.36	0.58	0.58	0.58	0.58
v/c Ratio	0.85	0.88	0.79	0.60	0.60	0.60
Control Delay	49.3	58.5	33.0	17.1	17.1	17.1
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	49.3	58.5	33.0	17.1	17.1	17.1
LOS	D	E	C	C	B	B
Approach Delay	52.2		33.0	17.1		
Approach LOS	D		C	C	B	B
Queue Length 50th (m)	144.8	130.5	212.4	92.8		
Queue Length 95th (m)	173.4	#193.3	m218.4	m95.0		
Internal Link Dist (m)	47.3		26.7	162.0		
Turn Bay Length (m)						
Base Capacity (vph)	1300	593	2975	2975	2975	2975
Starvation Cap Reductn	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	0.82	0.85	0.79	0.60	0.60	0.60
<b>Intersection Summary</b>						
Area Type:	Other					
Cycle Length:	140					
Actuated Cycle Length:	140					
Offset: Q (0%), Referenced to phase 2:NBT and 6:SBT, Start of Green						
Natural Cycle:	80					
Control Type:	Actuated-Coordinated					
Maximum w/c Ratio:	0.88					
Intersection Signal Delay:	33.4			Intersection LOS: C		
Intersection Capacity Utilization	100.6%			ICU Level of Service G		
Analysis Period (min)	15					
# 95th percentile volume exceeds capacity, queue may be longer.						



Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Detector Phase	4	4		2	6	6
Switch Phase						
Minimum Initial (s)	10.0	10.0	29.0	29.0	29.0	29.0
Minimum Split (s)	38.0	38.0	36.0	36.0	36.0	36.0
Total Split (s)	56.0	56.0	84.0	84.0	84.0	84.0
Total Split (%)	40.0%	40.0%	60.0%	60.0%	60.0%	60.0%
Maximum Green (s)	49.0	49.0	77.0	77.0	77.0	77.0
Yellow Time (s)	4.0	4.0	4.0	4.0	4.0	4.0
All-Red Time (s)	3.0	3.0	3.0	3.0	3.0	3.0
Lost Time Adjust (s)	-3.0	-3.0	-3.0	-3.0	-3.0	-3.0
Total Lost Time (s)	4.0	4.0	4.0	4.0	4.0	4.0
Lead/Lag						
Lead-Lag Optimize?						
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	None	None	C-Max	C-Max	C-Max	C-Max
Walk Time (s)	7.0	7.0	7.0	7.0	7.0	7.0
Flash Dont Walk (s)	11.0	11.0	11.0	11.0	11.0	11.0
Pedestrian Calls (#/hr)	0	0	0	0	0	0
Act Effct Green (s)	50.1	50.1	81.9	81.9	81.9	81.9
Actuated g/C Ratio	0.36	0.36	0.58	0.58	0.58	0.58
v/c Ratio	0.85	0.88	0.79	0.60	0.60	0.60
Control Delay	49.3	58.5	33.0	17.1	17.1	17.1
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	49.3	58.5	33.0	17.1	17.1	17.1
LOS	D	E	C	C	B	B
Approach Delay	52.2		33.0	17.1		
Approach LOS	D		C	C	B	B
Queue Length 50th (m)	144.8	130.5	212.4	92.8		
Queue Length 95th (m)	173.4	#193.3	m218.4	m95.0		
Internal Link Dist (m)	47.3		26.7	162.0		
Turn Bay Length (m)						
Base Capacity (vph)	1300	593	2975	2975	2975	2975
Starvation Cap Reductn	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	0.82	0.85	0.79	0.60	0.60	0.60
<b>Intersection Summary</b>						
Area Type:	Other					
Cycle Length:	140					
Actuated Cycle Length:	140					
Offset: Q (0%), Referenced to phase 2:NBT and 6:SBT, Start of Green						
Natural Cycle:	80					
Control Type:	Actuated-Coordinated					
Maximum w/c Ratio:	0.88					
Intersection Signal Delay:	33.4			Intersection LOS: C		
Intersection Capacity Utilization	100.6%			ICU Level of Service G		
Analysis Period (min)	15					
# 95th percentile volume exceeds capacity, queue may be longer.						

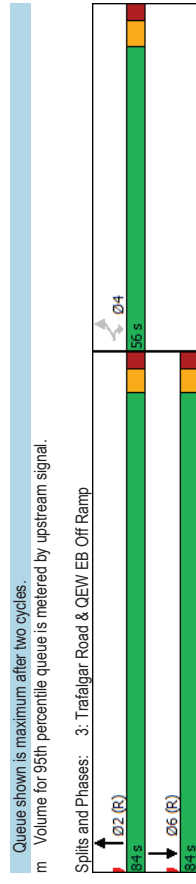
Lanes, Volumes, Timings  
8: Cross Ave & Lyons Lane

2035 Total AM Peak Hour.syn  
05-04-2022

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Perm	NA	Perm	pmrpt	NA	Perm	NA	Perm	NA	Perm	NA	Perm	NA
Protected Phases	4	4	4	8	8	8	2	2	2	6	6	6
Detector Phase	4	4	4	3	8	8	2	2	2	6	6	6
Switch Phase												
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Minimum Split (s)	22.5	22.5	22.5	9.5	22.5	22.5	22.5	22.5	22.5	22.5	22.5	22.5
Total Split (s)	32.0	32.0	32.0	26.0	58.0	58.0	32.0	32.0	32.0	32.0	32.0	32.0
Total Split (%)	35.6%	35.6%	35.6%	28.9%	64.4%	64.4%	35.6%	35.6%	35.6%	35.6%	35.6%	35.6%
Maximum Green (s)	27.5	27.5	27.5	21.5	53.5	53.5	27.5	27.5	27.5	27.5	27.5	27.5
Yellow Time (s)	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5
LeadLag	Lag	Lag	Lag	Lead	Lead	Lead	Lag	Lag	Lag	Lag	Lag	Lag
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	None	None	None	None	None	None	Max	Max	Max	Max	Max	Max
Walk Time (s)	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0
Flash Dont Walk (s)	11.0	11.0	11.0	11.0	11.0	11.0	11.0	11.0	11.0	11.0	11.0	11.0
Pedestrian Calls (#/hr)	0	0	0	0	0	0	0	0	0	0	0	0
Act Effct Green (s)	19.4	19.4	19.4	36.8	36.8	36.8	27.8	27.8	27.8	27.8	27.8	27.8
Actuated g/C Ratio	0.26	0.26	0.26	0.50	0.50	0.50	0.38	0.38	0.38	0.38	0.38	0.38
v/c Ratio	0.46	0.69	0.30	0.65	0.09	0.11	0.13	0.13	0.34	0.05	0.30	0.30
Control Delay	28.4	29.5	5.9	18.7	9.2	2.6	19.0	0.3	21.5	18.2	5.4	5.4
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	28.4	29.5	5.9	18.7	9.2	2.6	19.0	0.3	21.5	18.2	5.4	5.4
LOS	C	C	A	B	A	A	B	A	C	B	A	A
Approach Delay	25.3			13.5			6.6			13.9		
Approach LOS	C			B			A			B		
Queue Length 50th (m)	16.4	38.5	0.0	21.8	4.7	0.0	5.4	0.0	16.7	2.7	0.0	0.0
Queue Length 95th (m)	24.9	34.3	12.8	36.0	8.4	0.6	16.6	0.0	25.8	9.7	0.0	0.0
Internal Link Dist (m)	257.3			16.1			125.2			17.7		
Turn Bay Length (m)	60.0	60.0	60.0	60.0	60.0	60.0	60.0	60.0	60.0	60.0	60.0	60.0
Base Capacity (vph)	429	1168	628	553	2058	1056	467	868	483	632	540	540
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.32	0.48	0.23	0.48	0.06	0.07	0.13	0.13	0.34	0.05	0.30	0.30
Intersection Summary												
Area Type:	CBD											
Cycle Length:	90											
Actuated Cycle Length:	73.7											
Natural Cycle:	60											
Control Type:	Semi Act-Uncoordinated											
Maximum v/c Ratio:	0.69											
Intersection Signal Delay:	18.3											
Intersection Capacity Utilization:	49.5%											

Lanes, Volumes, Timings  
3: Trafalgar Road & QEW EB Off Ramp

2022 Existing PM Peak Hour.syn  
05-04-2022







HCM Unsignalized Intersection Capacity Analysis  
 4: Trafalgar Road & Argus Road/QEW EB On Ramp

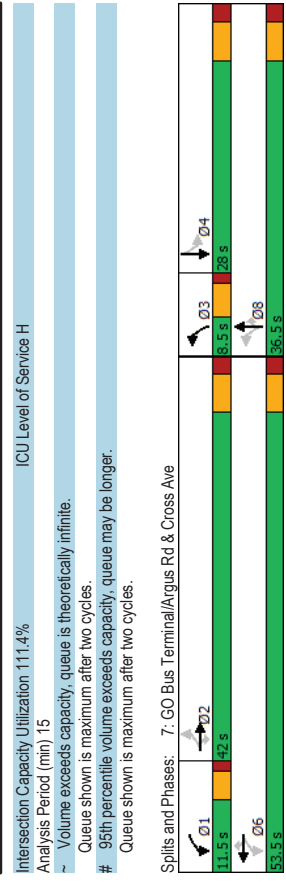
2022 Existing PM Peak Hour.syn  
 05-04-2022

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	0	0	42	0	0	0	0	2168	444	0	1727	375
Traffic Volume (veh/h)	0	0	42	0	0	0	0	2168	444	0	1727	375
Future Volume (Veh/h)	0	0	42	0	0	0	0	2168	444	0	1727	375
Sign Control	Stop	Stop	Stop	0%	0%	0%	Free	Free	Free	Free	Free	Free
Grade	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Peak Hour Factor	0	0	46	0	0	0	0	2357	483	0	1877	408
Hourly flow rate (vph)	0	0	46	0	0	0	0	2357	483	0	1877	408
Pedestrians	24											
Lane Width (m)	3.6											
Walking Speed (m/s)	1.2											
Percent Blockage	2											
Right turn flare (veh)												
Median type							None	None	None	None	None	None
Median storage (veh)												
Upstream signal (m)							271					51
pX, platoon unblocked	0.88	0.88	0.78	0.88	0.88	0.79	0.78					0.79
vC, conflicting volume	2891	4462	854	2983	4666	786	2309					2357
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	1059	2844	0	1163	3076	0	1673					1786
tC, single (s)	7.5	6.5	7.1	7.5	6.5	6.9	4.1					4.1
tC, 2 stage (s)												
tF (s)	3.5	4.0	3.4	3.5	4.0	3.3	2.2					2.2
p0 queue free %	100	100	94	100	100	100	100					100
cM capacity (veh/h)	154	15	809	124	10	861	295					278
Direction_Lane #	EB.1	NB.1	NB.2	NB.3	NB.4	SB.1	SB.2	SB.3	SB.3			
Volume Total	46	786	786	786	483	751	751	763				
Volume Left	0	0	0	0	0	0	0	0				
Volume Right	46	0	0	0	483	0	0	408				
GSH	809	1700	1700	1700	1700	1700	1700	1700				
Volume to Capacity	0.06	0.46	0.46	0.46	0.28	0.44	0.44	0.46				
Queue Length 95th (m)	1.4	0.0	0.0	0.0	0.0	0.0	0.0	0.0				
Control Delay (s)	9.7	0.0	0.0	0.0	0.0	0.0	0.0	0.0				
Lane LOS	A											
Approach Delay (s)	9.7	0.0				0.0						
Approach LOS	A											
Intersection Summary												
Average Delay	0.1											
Intersection Capacity Utilization	52.1%											
ICU Level of Service	A											
Analysis Period (min)	15											

Lanes, Volumes, Timings  
 7: GO Bus Terminal/Argus Rd & Cross Ave

2035 Total AM Peak Hour.syn  
 05-04-2022

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	0	0	42	0	0	0	0	2168	444	0	1727	375
Traffic Volume (veh/h)	0	0	42	0	0	0	0	2168	444	0	1727	375
Future Volume (Veh/h)	0	0	42	0	0	0	0	2168	444	0	1727	375
Sign Control	Stop	Stop	Stop	0%	0%	0%	Free	Free	Free	Free	Free	Free
Grade	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Peak Hour Factor	0	0	46	0	0	0	0	2357	483	0	1877	408
Hourly flow rate (vph)	0	0	46	0	0	0	0	2357	483	0	1877	408
Pedestrians	24											
Lane Width (m)	3.6											
Walking Speed (m/s)	1.2											
Percent Blockage	2											
Right turn flare (veh)												
Median type							None	None	None	None	None	None
Median storage (veh)												
Upstream signal (m)							271					51
pX, platoon unblocked	0.88	0.88	0.78	0.88	0.88	0.79	0.78					0.79
vC, conflicting volume	2891	4462	854	2983	4666	786	2309					2357
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	1059	2844	0	1163	3076	0	1673					1786
tC, single (s)	7.5	6.5	7.1	7.5	6.5	6.9	4.1					4.1
tC, 2 stage (s)												
tF (s)	3.5	4.0	3.4	3.5	4.0	3.3	2.2					2.2
p0 queue free %	100	100	94	100	100	100	100					100
cM capacity (veh/h)	154	15	809	124	10	861	295					278



Intersection Capacity Utilization 111.4% ICU Level of Service H  
 Analysis Period (min) 15  
 ~ Volume exceeds capacity, queue is theoretically infinite.  
 Queue shown is maximum after two cycles.  
 # 95th percentile volume exceeds capacity, queue may be longer.  
 Queue shown is maximum after two cycles.





HCM Signalized Intersection Capacity Analysis  
6: Trafalgar Road

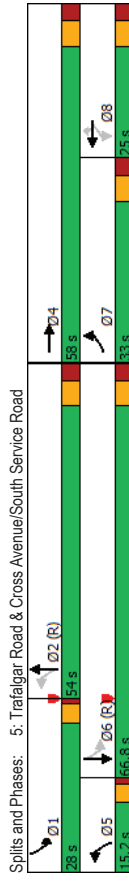
2035 Total AM Peak Hour.syn  
05-04-2022

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	
Traffic Volume (vph)	615	680	109	14	582	701	81	447	63	525	508	455	
Future Volume (vph)	615	680	109	14	582	701	81	447	63	525	508	455	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	
Total Lost time (s)	4.0	2.0	4.0	4.0	4.0	4.0	4.0	4.0	7.0	4.0	4.0	4.0	
Lane Util. Factor	0.97	0.95	1.00	0.95	1.00	1.00	1.00	0.95	1.00	0.97	1.00	1.00	
Frpb, ped/bikes	1.00	0.99	1.00	1.00	0.93	1.00	1.00	0.95	1.00	0.95	1.00	0.97	
Flpb, ped/bikes	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Frt	1.00	0.98	1.00	1.00	0.85	1.00	1.00	0.85	1.00	0.85	1.00	0.85	
Flt Protected	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	
Satd. Flow (prot)	3433	3391	1703	3505	1460	1770	3574	1529	3367	1863	1474	1474	
Flt Permitted	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	
Satd. Flow (perm)	3433	3391	1703	3505	1460	1770	3574	1529	3367	1863	1474	1474	
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	
Adj. Flow (vph)	668	739	118	15	633	762	88	486	68	571	552	495	
RTOR Reduction (vph)	0	8	0	0	181	0	0	53	0	237	0	237	
Lane Group Flow (vph)	668	849	0	15	633	581	88	486	15	571	552	258	
Conf. Peds. (#/hr)	25	25	7	7	25	9	9	18	18	18	18	9	
Heavy Vehicles (%)	2%	4%	1%	6%	3%	3%	2%	1%	0%	4%	2%	6%	
Turn Type	Prot	NA	Prot	NA	Prot	NA	Prot	NA	Prot	NA	Prot	NA	
Protected Phases	7	4		3	8		5	2		1		6	
Permitted Phases						8			2			6	
Actuated Green, G (s)	23.0	66.2	2.8	44.0	44.0	7.0	30.0	30.0	19.0	42.0	42.0	42.0	
Effective Green, g (s)	24.0	69.2	3.8	47.0	47.0	8.0	33.0	33.0	20.0	45.0	45.0	45.0	
Actuated g/C Ratio	0.17	0.49	0.03	0.34	0.34	0.06	0.24	0.21	0.14	0.32	0.32	0.32	
Clearance Time (s)	5.0	5.0	5.0	7.0	7.0	5.0	7.0	7.0	5.0	7.0	7.0	7.0	
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	
Lane Grp Cap (vph)	588	1676	46	1176	490	101	842	327	481	598	473	473	
v/s Ratio Prot	c0.19	0.25	0.01	0.18	0.05	0.14			c0.17	c0.30			
v/s Ratio Perm													
v/c Ratio	1.14	0.51	0.33	0.54	1.19	0.87	0.58	0.04	1.19	0.92	0.55	0.55	
Uniform Delay, d1	58.0	23.9	66.8	37.7	46.5	65.5	47.3	43.6	60.0	45.8	39.1	39.1	
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Incremental Delay, d2	80.6	0.2	4.1	0.5	102.4	50.9	2.9	0.3	99.8	18.4	3.5	3.5	
Delay (s)	138.6	24.1	71.0	38.2	148.9	116.4	50.2	43.9	197.1	38.8	29.0	29.0	
Level of Service	F	C	E	D	F	F	D	D	F	D	F	C	
Approach Delay (s)	74.3		98.4		58.6				91.7				
Approach LOS	E		F		E				F			F	
Intersection Summary													
HCM 2000 Control Delay	84.3											HCM 2000 Level of Service	F
HCM 2000 Volume to Capacity ratio	1.12												
Actuated Cycle Length (s)	140.0											Sum of lost time (s)	16.0
Intersection Capacity Utilization	89.9%											ICU Level of Service	E
Analysis Period (min)	15												
c Critical Lane Group													

Lanes, Volumes, Timings  
5: Trafalgar Road & Cross Avenue/South Service Road

2022 Existing PM Peak Hour.syn  
05-04-2022

Intersection Capacity Utilization 90.4%	ICU Level of Service E
Analysis Period (min) 15	
~ Volume exceeds capacity, queue is theoretically infinite.	
Queue shown is maximum after two cycles.	
# 95th percentile volume exceeds capacity, queue may be longer.	
Queue shown is maximum after two cycles.	
m Volume for 95th percentile queue is metered by upstream signal.	





HCM Signalized Intersection Capacity Analysis  
 5: Trafalgar Road & Cross Avenue/South Service Road

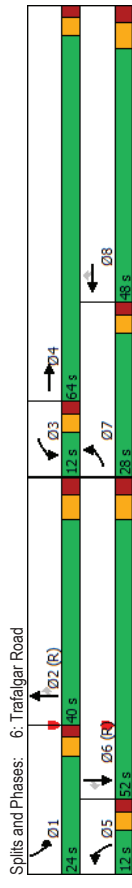
2022 Existing PM Peak Hour.syn  
 05-04-2022

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	
Traffic Volume (vph)	956	86	132	151	144	274	128	1382	41	131	1390	320	
Future Volume (vph)	956	86	132	151	144	274	128	1382	41	131	1390	320	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	
Lane Util. Factor	0.97	1.00	1.00	1.00	1.00	1.00	1.00	0.91	1.00	1.00	0.91	1.00	
Frb, ped/bikes	1.00	0.97	1.00	1.00	1.00	1.00	1.00	0.99	1.00	1.00	0.99	1.00	
Frb, ped/bikes	1.00	1.00	0.97	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Frt	1.00	0.91	1.00	1.00	1.00	0.85	1.00	1.00	1.00	1.00	0.97	1.00	
Flt Protected	0.95	1.00	0.95	1.00	1.00	0.95	1.00	0.95	1.00	0.95	1.00	1.00	
Satd. Flow (prot)	3400	1546	1725	1827	1599	1719	5072	1805	4924	1805	4924	1805	
Flt Permitted	0.95	1.00	0.61	1.00	1.00	0.07	1.00	1.00	0.07	1.00	0.07	1.00	
Satd. Flow (perm)	3400	1546	1111	1827	1599	122	5072	133	4924	133	4924	133	
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	
Adj. Flow (vph)	1039	93	143	164	157	298	139	1502	45	142	1511	348	
RTOR Reduction (vph)	0	39	0	0	0	201	0	2	0	0	27	0	
Lane Group Flow (vph)	1039	197	0	164	157	97	139	1545	0	142	1832	0	
Conf. Peds. (#/hr)	15	15	15	15	15	18	18	70	70	70	70	18	
Heavy Vehicles (%)	3%	8%	9%	2%	4%	1%	5%	1%	0%	0%	1%	3%	
Turn Type	Prot	NA	Perm	NA	Perm	pm+pt	NA	pm+pt	NA	pm+pt	NA	NA	
Protected Phases	7	4			8		5	2		1		6	
Permitted Phases				8		2			6				
Actuated Green, G (s)	26.0	51.0	18.0	18.0	18.0	69.6	59.2	72.4	60.6	72.4	60.6	60.6	
Effective Green, g (s)	29.0	54.0	21.0	21.0	21.0	69.6	62.2	72.4	63.6	72.4	63.6	63.6	
Actuated g/C Ratio	0.21	0.39	0.15	0.15	0.15	0.50	0.44	0.52	0.45	0.52	0.45	0.45	
Clearance Time (s)	7.0	7.0	7.0	7.0	7.0	4.0	7.0	4.0	7.0	4.0	7.0	7.0	
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	
Lane Grp. Cap (vph)	704	596	166	274	239	179	2253	209	2336	209	2336	2336	
v/s Ratio Prot	e0.31	0.13		0.09		e0.06	0.30		e0.37		e0.37		
v/s Ratio Perm			e0.15		0.06	0.33		0.29		0.29		0.29	
v/c Ratio	1.48	0.33	0.99	0.57	0.41	0.78	0.69	0.68	0.82	0.68	0.82	0.82	
Uniform Delay, d1	55.5	30.3	59.4	55.3	53.9	32.3	31.1	27.3	33.2	27.3	33.2	33.2	
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.28	1.26	1.06	0.85	1.06	0.85	0.85	
Incremental Delay, d2	221.8	0.3	65.7	2.9	1.1	2.0	0.2	6.8	2.8	6.8	2.8	2.8	
Delay (s)	277.3	30.6	125.0	58.2	55.0	43.3	39.4	35.6	30.9	35.6	30.9	30.9	
Level of Service	F	C	F	E	E	D	D	D	C	D	C	C	
Approach Delay (s)	231.6		74.4			39.7			31.3			31.3	
Approach LOS	F		E			D			C			C	
Intersection Summary													
HCM 2000 Control Delay	84.4											HCM 2000 Level of Service	F
HCM 2000 Volume to Capacity ratio	1.00												
Actuated Cycle Length (s)	140.0											Sum of lost time (s)	16.0
Intersection Capacity Utilization	90.4%											ICU Level of Service	E
Analysis Period (min)	15												
c Critical Lane Group													

Lanes, Volumes, Timings  
 6: Trafalgar Road

2035 Total AM Peak Hour.syn  
 05-04-2022

Intersection Capacity Utilization	89.9%
Analysis Period (min)	15
ICU Level of Service	E
~ Volume exceeds capacity, queue is theoretically infinite.	
Queue shown is maximum after two cycles.	
# 95th percentile volume exceeds capacity, queue may be longer.	
Queue shown is maximum after two cycles.	







HCM Signalized Intersection Capacity Analysis  
 5: Trafalgar Road & Cross Avenue/South Service Road

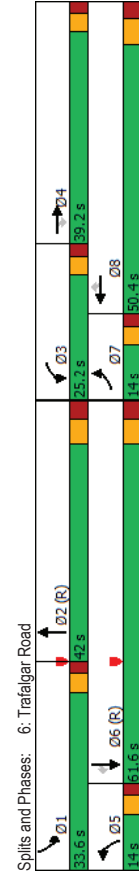
Lanes, Volumes, Timings  
 6: Trafalgar Road

2035 Total AM Peak Hour.syn  
 05-04-2022

2022 Existing PM Peak Hour.syn  
 05-04-2022

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	
Traffic Volume (vph)	743	400	173	87	366	179	160	1062	547	389	1244	663	
Future Volume (vph)	743	400	173	87	366	179	160	1062	547	389	1244	663	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	
Lane Util. Factor	0.97	0.95	1.00	0.97	0.95	1.00	1.00	0.91	1.00	0.91	1.00	0.91	
Frb, ped/bikes	1.00	1.00	0.98	1.00	0.99	1.00	1.00	0.92	1.00	0.92	1.00	0.97	
Frb, ped/bikes	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Frt	1.00	1.00	0.85	1.00	1.00	0.85	1.00	0.85	1.00	0.85	1.00	0.85	
Flt Protected	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	0.95	1.00	0.95	1.00	
Satd. Flow (prot)	3213	3406	1411	3400	3343	1545	1641	5036	1485	1752	5036	1499	
Flt Permitted	0.95	1.00	1.00	0.95	1.00	1.00	0.13	1.00	1.00	0.11	1.00	1.00	
Satd. Flow (perm)	3213	3406	1411	3400	3343	1545	219	5036	1485	195	5036	1499	
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	
Adj. Flow (vph)	808	435	188	95	398	195	174	1154	595	423	1352	721	
RTOR Reduction (vph)	0	0	140	0	0	41	0	0	57	0	0	314	
Lane Group Flow (vph)	808	435	48	95	398	154	174	1154	538	423	1352	407	
Conf. Peds. (#/hr)	1	1	4	4	4	1	10	10	52	52	52	10	
Heavy Vehicles (%)	9%	6%	12%	3%	8%	4%	10%	3%	0%	3%	3%	4%	
Turn Type	Prot	NA	Perm	Prot	NA	pm+ov	pm+pt	NA	pm+ov	pm+pt	NA	Perm	
Protected Phases	7	4	3	8	1	5	2	3	1	6	6	6	
Permitted Phases	4	4	4	8	2	2	2	2	2	2	6	6	
Actuated Green, G (s)	33.0	32.6	21.1	17.7	48.1	47.0	33.9	55.0	66.3	51.2	51.2	51.2	
Effective Green, g (s)	36.0	35.6	21.1	20.7	48.1	47.0	36.9	55.0	66.3	54.2	54.2	54.2	
Actuated g/C Ratio	0.26	0.25	0.25	0.15	0.34	0.34	0.26	0.39	0.49	0.39	0.39	0.39	
Clearance Time (s)	7.0	7.0	4.0	7.0	4.0	7.0	4.0	7.0	4.0	7.0	4.0	7.0	
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	
Lane Grp Cap (vph)	826	866	358	512	494	530	206	1327	583	433	1949	580	
v/s Ratio Prot	c0.25	0.13	0.03	c0.12	0.06	0.08	0.23	0.14	c0.21	0.27	0.27	0.27	
v/s Ratio Perm	0.98	0.50	0.13	0.19	0.81	0.29	0.84	0.87	0.92	0.98	0.69	0.70	
Uniform Delay, d1	51.6	44.6	40.3	51.9	57.7	33.5	49.3	40.5	43.2	35.9	36.1	36.1	
Progression Factor	1.00	1.00	1.00	1.00	1.00	0.93	1.05	1.24	0.99	0.69	0.35	0.35	
Incremental Delay, d2	25.7	0.5	0.2	0.2	9.3	0.3	7.5	2.0	6.3	8.4	0.2	0.7	
Delay (s)	77.3	45.1	40.5	52.1	67.0	33.8	40.4	53.8	56.4	51.2	24.9	13.2	
Level of Service	E	D	D	D	E	C	D	D	E	D	C	B	
Approach Delay (s)	62.7			55.5			53.4				26.0		
Approach LOS	E			E			D				C		
Intersection Summary													
HCM 2000 Control Delay	45.2											HCM 2000 Level of Service	D
HCM 2000 Volume to Capacity ratio	0.95												
Actuated Cycle Length (s)	140.0											Sum of lost time (s)	16.0
Intersection Capacity Utilization	88.9%											ICU Level of Service	E
Analysis Period (min)	15												
c Critical Lane Group													

Intersection Capacity Utilization 84.0% ICU Level of Service E  
 Analysis Period (min) 15  
 ~ Volume exceeds capacity, queue is theoretically infinite.  
 Queue shown is maximum after two cycles.  
 # 95th percentile volume exceeds capacity, queue may be longer.  
 Queue shown is maximum after two cycles.  
 m Volume for 95th percentile queue is metered by upstream signal.



HCM Signalized Intersection Capacity Analysis  
 6: Trafalgar Road

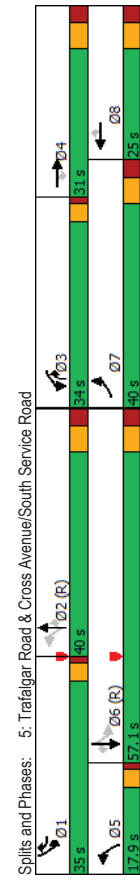
Lanes, Volumes, Timings  
 5: Trafalgar Road & Cross Avenue/South Service Road

2022 Existing PM Peak Hour.syn  
 05-04-2022

2035 Total AM Peak Hour.syn  
 05-04-2022

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	←	←	←	←	←	←	←	←	←	←	←	←
Traffic Volume (vph)	474	507	179	63	707	623	126	449	43	538	581	519
Future Volume (vph)	474	507	179	63	707	623	126	449	43	538	581	519
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	2.0	5.0	4.0	4.0	7.0	4.0	4.0	4.0	4.0	4.0	4.0
Lane Util. Factor	0.97	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	0.97	1.00	1.00
Frb, ped/bikes	1.00	1.00	0.93	1.00	1.00	0.94	1.00	1.00	1.00	1.00	1.00	0.95
Frb, ped/bikes	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt	1.00	1.00	0.85	1.00	1.00	0.85	1.00	0.99	1.00	0.95	1.00	0.85
Flt Protected	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	0.95	1.00	0.95	1.00
Satd. Flow (prot)	3467	3505	1482	1805	3574	1503	1770	3552	3433	1900	1503	1503
Flt Permitted	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	0.95	1.00	0.95	1.00
Satd. Flow (perm)	3467	3505	1482	1805	3574	1503	1770	3552	3433	1900	1503	1503
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	515	551	195	68	768	677	137	488	47	585	632	564
RTOR Reduction (vph)	0	0	131	0	0	305	0	5	0	0	0	127
Lane Group Flow (vph)	515	551	64	68	768	372	137	530	0	585	632	437
Conf. Peds. (#/hr)	21	3%	1%	0%	1%	1%	2%	0%	2%	0%	2%	0%
Heavy Vehicles (%)	1%	3%	1%	0%	1%	1%	2%	0%	2%	0%	2%	0%
Turn Type	Prot	NA	Perm	Prot	NA	Perm	Prot	NA	Perm	Prot	NA	Perm
Protected Phases	7	4		3	8		5	2		1		6
Permitted Phases			4			8						6
Actuated Green, G (s)	9.0	41.8	41.8	9.3	40.1	40.1	12.8	39.9	27.0	54.1	54.1	54.1
Effective Green, g (s)	10.0	44.8	41.8	10.3	43.1	40.1	13.8	42.9	28.0	57.1	57.1	57.1
Actuated g/C Ratio	0.07	0.32	0.30	0.07	0.31	0.29	0.10	0.31	0.20	0.41	0.41	0.41
Clearance Time (s)	5.0	5.0	5.0	5.0	7.0	7.0	5.0	7.0	5.0	7.0	7.0	7.0
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Lane Grp. Cap (vph)	247	1121	442	132	1100	430	174	1088	686	774	613	613
v/s Ratio Prot	c0.15	0.16	0.04	0.04	0.21	0.08	0.15	c0.17	c0.33			
v/s Ratio Perm			0.04			c0.25						0.29
v/c Ratio	2.09	0.49	0.14	0.52	0.70	0.87	0.79	0.49	0.85	0.82	0.71	0.71
Uniform Delay, d1	65.0	38.4	36.0	62.4	42.7	47.4	61.7	39.6	54.0	36.8	34.6	34.6
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.32	0.85	0.86	0.86
Incremental Delay, d2	501.9	0.3	0.2	3.4	2.0	16.5	20.6	1.6	6.1	5.6	4.0	4.0
Delay (s)	566.9	38.8	36.1	65.8	44.7	63.9	82.2	41.1	77.2	36.9	33.7	33.7
Level of Service	F	D	D	E	D	E	F	D	E	D	C	C
Approach Delay (s)	254.0			54.2			49.5				49.1	
Approach LOS	F			D			D				D	
Intersection Summary												
HCM 2000 Control Delay	100.1 HCM 2000 Level of Service F											
HCM 2000 Volume to Capacity ratio	0.94											
Actuated Cycle Length (s)	140.0 Sum of lost time (s) 16.0											
Intersection Capacity Utilization	84.0% ICU Level of Service E											
Analysis Period (min)	15											
c Critical Lane Group												

Intersection Capacity Utilization 88.9% ICU Level of Service E  
 Analysis Period (min) 15  
 # 95th percentile volume exceeds capacity, queue may be longer.  
 Queue shown is maximum after two cycles.  
 m Volume for 95th percentile queue is metered by upstream signal.







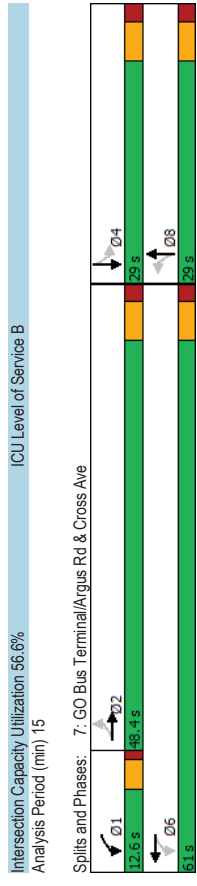
HCM Unsignalized Intersection Capacity Analysis  
 4: Trafalgar Road & Argus Road/QEW EB On Ramp

2035 Total AM Peak Hour.syn  
 05-04-2022

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	0	0	41	0	0	0	0	1401	584	0	2273	940
Traffic Volume (veh/h)	0	0	41	0	0	0	0	1401	584	0	2273	940
Future Volume (Veh/h)	0	0	41	0	0	0	0	1401	584	0	2273	940
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
Grade	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	0	0	45	0	0	0	0	1523	635	0	2471	1022
Pedestrians	24											
Lane Width (m)	3.6											
Walking Speed (m/s)	1.2											
Percent Blockage	2											
Right turn flare (veh)												
Median type							None	None	None			
Median storage (veh)												
Upstream signal (m)							271					51
pX, platoon unblocked	0.71	0.71	0.63	0.71	0.71	0.85	0.63				0.85	
vC, conflicting volume	3514	4529	1359	2347	5040	508	3517				1523	
vC1, stage 1 conf vol												
vC2, stage 2 conf vol	1645	3075	0	1	3795	0	2946				981	
vCu, unblocked vol	7.5	6.5	7.1	7.5	6.5	6.9	4.1				4.1	
tC, single (s)												
tC, 2 stage (s)	3.5	4.0	3.4	3.5	4.0	3.3	2.2				2.2	
tF (s)	100	100	93	100	100	100	100				100	
p0 queue free %												
cM capacity (veh/h)	46	9	660	669	3	923	76				602	
Direction_Lane #	EB 1	NB 1	NB 2	NB 3	NB 4	SB 1	SB 2	SB 3				
Volume Total	45	508	508	508	635	988	988	1516				
Volume Left	0	0	0	0	0	0	0	0				
Volume Right	45	0	0	0	635	0	0	1022				
GSH	660	1700	1700	1700	1700	1700	1700	1700				
Volume to Capacity	0.07	0.30	0.30	0.30	0.37	0.58	0.58	0.89				
Queue Length 95th (m)	1.8	0.0	0.0	0.0	0.0	0.0	0.0	0.0				
Control Delay (s)	10.8	0.0	0.0	0.0	0.0	0.0	0.0	0.0				
Lane LOS	B											
Approach Delay (s)	10.8	0.0										
Approach LOS	B											
Intersection Summary												
Average Delay	0.1											
Intersection Capacity Utilization	75.6% ICU Level of Service											
Analysis Period (min)	15											

Lanes, Volumes, Timings  
 7: GO Bus Terminal/Argus Rd & Cross Ave

2022 Existing PM Peak Hour.syn  
 05-04-2022





HCM Signalized Intersection Capacity Analysis  
 7: GO Bus Terminal/Argus Rd & Cross Ave

2022 Existing PM Peak Hour.syn  
 05-04-2022

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	14	835	16	36	347	41	15	2	46	142	19	128
Traffic Volume (vph)	14	835	16	36	347	41	15	2	46	142	19	128
Future Volume (vph)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Ideal Flow (vphpl)	3.3	3.6	3.6	3.3	3.6	3.6	3.3	3.6	3.6	3.3	3.6	3.6
Lane Width	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Total Lost time (s)	1.00	0.95	1.00	1.00	0.95	1.00	1.00	1.00	0.95	1.00	1.00	1.00
Lane Util. Factor	0.99	1.00	1.00	1.00	1.00	1.00	0.99	1.00	0.97	1.00	0.97	1.00
Frbp, ped/bikes	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	1.00	0.95	1.00
Fit Protected	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00
Satd. Flow (prot)	1557	3174	797	3176	778	705	778	705	1524	1287	1524	1287
Fit Permitted	0.51	1.00	0.51	1.00	0.55	1.00	0.55	1.00	0.71	1.00	0.71	1.00
Satd. Flow (perm)	833	3174	125	3176	450	705	450	705	1134	1287	1134	1287
Peak-hour factor, PHF	0.66	0.81	0.75	0.94	0.95	0.81	0.70	0.25	0.67	0.86	0.75	0.78
Adj. Flow (vph)	21	1031	21	38	365	51	21	8	69	165	25	164
RTOR Reduction (vph)	0	2	0	0	12	0	0	52	0	0	123	0
Lane Group Flow (vph)	21	1060	0	38	404	0	21	25	0	165	66	0
Confl. Peds. (#/hr)	9	4	4	4	9	12	12	20	20	20	12	12
Heavy Vehicles (%)	0%	0%	100%	97%	0%	0%	100%	100%	100%	0%	100%	0%
Turn Type	Perm	NA	NA	pm-pt	NA	Perm	NA	NA	NA	Perm	NA	NA
Protected Phases	2	2	1	6	6	8	8	8	8	4	4	4
Permitted Phases	2	2	1	6	6	8	8	8	8	4	4	4
Actuated Green, G (s)	34.4	34.4	46.7	46.7	46.7	16.9	16.9	16.9	16.9	16.9	16.9	16.9
Effective Green, g (s)	36.4	36.4	46.7	46.7	48.7	18.9	18.9	18.9	18.9	18.9	18.9	18.9
Actuated g/C Ratio	0.48	0.48	0.62	0.64	0.25	0.25	0.25	0.25	0.25	0.25	0.25	0.25
Clearance Time (s)	6.0	6.0	4.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0
Vehicle Extension (s)	5.0	5.0	2.5	5.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lane Grp Cap (vph)	401	1528	150	2045	112	176	283	321	283	321	283	321
v/s Ratio Prot	0.033	0.033	0.03	0.13	0.04	0.04	0.04	0.04	0.04	0.05	0.05	0.05
v/s Ratio Perm	0.03	0.03	0.03	0.13	0.04	0.04	0.04	0.04	0.04	0.05	0.05	0.05
v/c Ratio	0.05	0.69	0.25	0.20	0.19	0.14	0.14	0.14	0.14	0.14	0.14	0.14
Uniform Delay, d1	10.4	15.2	8.2	5.5	22.3	22.1	22.1	22.1	22.1	24.9	22.4	22.4
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	0.1	1.7	0.7	0.1	1.1	0.5	0.5	0.5	0.5	3.6	0.4	0.4
Delay (s)	10.5	16.9	8.9	5.6	23.4	22.6	22.6	22.6	22.6	28.5	22.8	22.8
Level of Service	B	B	A	A	A	C	C	C	C	C	C	C
Approach Delay (s)	16.7	16.7	5.9	5.9	22.7	22.7	22.7	22.7	22.7	25.5	25.5	25.5
Approach LOS	B	B	A	A	A	C	C	C	C	C	C	C
Intersection Summary	Other											
HCM 2000 Control Delay	16.1 HCM 2000 Level of Service B											
HCM 2000 Volume to Capacity ratio	0.60											
Actuated Cycle Length (s)	75.6 Sum of lost time (s) 12.0											
Intersection Capacity Utilization	56.6% ICU Level of Service B											
Analysis Period (min)	15											
Critical Lane Group	C Critical Lane Group											

Lanes, Volumes, Timings  
 4: Trafalgar Road & Argus Road/QEW EB On Ramp

2035 Total AM Peak Hour.syn  
 05-04-2022

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	0	0	41	0	0	0	0	0	1401	584	0	2273
Traffic Volume (vph)	0	0	41	0	0	0	0	0	1401	584	0	2273
Future Volume (vph)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Ideal Flow (vphpl)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	70.0	0.0	0.0	0.0
Storage Length (m)	0	0	0	0	0	0	0	0	1	0	0	0
Taper Length (m)	7.5	7.5	7.5	7.5	7.5	7.5	7.5	7.5	7.5	7.5	7.5	7.5
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.91	1.00	0.91	0.91
Ped Bike Factor	0.865											
Fit Protected	0.865											
Satd. Flow (prot)	0	0	1522	0	0	0	0	0	5085	1568	0	4890
Fit Permitted	0.865											
Satd. Flow (perm)	0	0	1522	0	0	0	0	0	5085	1568	0	4890
Link Speed (k/h)	50											
Link Distance (m)	165.2											
Travel Time (s)	11.9											
Confl. Peds. (#/hr)	14.1											
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	0%	0%	8%	0%	2%	0%	0%	0%	2%	3%	0%	2%
Adj. Flow (vph)	0	0	45	0	0	0	0	0	1523	635	0	2471
Shared Lane Traffic (%)	0											
Lane Group Flow (vph)	0	0	45	0	0	0	0	0	1523	635	0	3493
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(m)	0.0											
Link Offset(m)	0.0											
Crosswalk Width(m)	4.8											
Two way Left Turn Lane	0											
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (k/h)	25	25	15	25	25	15	25	25	15	25	25	15
Sign Control	Stop											
Intersection Summary	Stop											
Area Type:	Other											
Control Type:	Unsignalized											
Intersection Capacity Utilization	75.6% ICU Level of Service D											
Analysis Period (min)	15											

HCM Signalized Intersection Capacity Analysis  
 3: Trafalgar Road & QEW EB Off Ramp

2035 Total AM Peak Hour.syn  
 05-04-2022

Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	1	1	1	1	1	1
Traffic Volume (vph)	1123	1119	0	1401	2093	0
Future Volume (vph)	1123	1119	0	1401	2093	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	0.0	4.0	4.0	0.0
Lane Util. Factor	0.97	1.00	0.91	0.91	1.00	1.00
Fr	1.00	0.85	1.00	1.00	1.00	1.00
Flt Protected	0.95	1.00	1.00	1.00	1.00	1.00
Satd. Flow (prot)	3400	1599	4893	4988	4988	4988
Flt Permitted	0.95	1.00	1.00	1.00	1.00	1.00
Satd. Flow (perm)	3400	1599	4893	4988	4988	4988
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	1221	1216	0	1523	2275	0
RTOR Reduction (vph)	0	0	0	0	0	0
Lane Group Flow (vph)	1221	1216	0	1523	2275	0
Heavy Vehicles (%)	3%	1%	0%	6%	4%	0%
Turn Type	Perm	Perm	NA	NA	NA	NA
Protected Phases	4	4	2	6		
Permitted Phases	77.0	77.0	49.0	49.0		
Actuated Green, G (s)	80.0	80.0	52.0	52.0		
Effective Green, g (s)	0.57	0.57	0.37	0.37		
Actuated g/C Ratio	7.0	7.0	7.0	7.0		
Clearance Time (s)	3.0	3.0	3.0	3.0		
Vehicle Extension (s)	1942	913	1817	1852		
Lane Grp Cap (vph)	0.36	c0.76	0.31	c0.46		
v/s Ratio Prot	0.63	1.33	0.84	1.23		
v/s Ratio Perm	20.1	30.0	40.2	44.0		
Uniform Delay, d1	1.00	1.00	1.21	1.08		
Progression Factor	0.6	156.9	2.4	104.1		
Incremental Delay, d2	20.7	186.9	51.1	151.5		
Level of Service	C	F	D	F		
Approach Delay (s)	103.6	F	51.1	151.5		
Approach LOS	F		D	F		
Intersection Summary	HCM 2000 Level of Service F					
HCM 2000 Control Delay	108.3 HCM 2000 Level of Service F					
HCM 2000 Volume to Capacity ratio	1.32					
Actuated Cycle Length (s)	140.0 Sum of lost time (s)					
Intersection Capacity Utilization	116.4% ICU Level of Service H					
Analysis Period (min)	15					
C Critical Lane Group						

Lanes, Volumes, Timings  
 8: Lyons Lane/Commercial Driveway & Cross Ave

2022 Existing PM Peak Hour.syn  
 05-04-2022

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	1	1	1	1	1	1	1	1	1	1	1	1
Traffic Volume (vph)	37	150	13	14	307	12	229	4	155	15	2	62
Future Volume (vph)	37	150	13	14	307	12	229	4	155	15	2	62
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (m)	3.3	3.6	3.6	3.3	3.6	3.6	3.3	3.6	3.6	3.3	3.6	3.6
Storage Length (m)	0.0	0.0	25.0	0.0	20.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Storage Lanes	1	0	1	0	1	0	1	0	1	0	1	0
Taper Length (m)	7.5	7.5	7.5	7.5	7.5	7.5	7.5	7.5	7.5	7.5	7.5	7.5
Lane Util. Factor	1.00	0.95	0.95	1.00	0.95	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor	1.00	1.00	1.00	1.00	1.00	0.98	1.00	0.98	1.00	0.99	1.00	0.99
Fr	0.987	0.987	0.987	0.987	0.987	0.987	0.987	0.987	0.987	0.987	0.987	0.987
Flt Protected	0.950	0.950	0.950	0.950	0.950	0.950	0.950	0.950	0.950	0.950	0.950	0.950
Satd. Flow (prot)	1525	2908	0	1570	3068	0	1570	1446	0	1570	1418	0
Flt Permitted	0.545	0.545	0.545	0.545	0.545	0.545	0.545	0.545	0.545	0.545	0.545	0.545
Satd. Flow (perm)	871	2908	0	931	3068	0	1136	1446	0	906	1418	0
Right Turn on Red	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Satd. Flow (RTOR)	14	14	13	13	13	196	196	196	196	103	103	103
Link Speed (k/h)	50	50	50	50	50	50	50	50	50	50	50	50
Link Distance (m)	40.1	374.0	374.0	374.0	69.1	69.1	69.1	69.1	69.1	70.9	70.9	70.9
Travel Time (s)	2.9	2.9	26.9	26.9	5.0	5.0	5.0	5.0	5.0	5.1	5.1	5.1
Conf. Peds. (#/hr)	3	1	1	1	3	1	4	4	4	4	4	4
Peak Hour Factor	0.58	0.84	0.75	0.54	0.95	0.55	0.74	0.33	0.79	0.70	0.50	0.60
Heavy Vehicles (%)	3%	11%	0%	0%	5%	0%	0%	0%	0%	0%	0%	2%
Adj. Flow (vph)	64	179	17	26	323	22	309	12	196	21	4	103
Shared Lane Traffic (%)	14	14	13	13	14	14	14	14	14	14	14	14
Lane Group Flow (vph)	64	196	0	26	345	0	309	208	0	21	107	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(m)	3.3	3.3	3.3	3.3	3.3	3.3	3.3	3.3	3.3	3.3	3.3	3.3
Link Offset(m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Crosswalk Width(m)	4.8	4.8	4.8	4.8	4.8	4.8	4.8	4.8	4.8	4.8	4.8	4.8
Two way Left Turn Lane	1.19	1.14	1.14	1.19	1.14	1.14	1.19	1.14	1.14	1.19	1.14	1.14
Headway Factor	24	24	14	24	14	24	14	24	14	24	24	14
Turning Speed (k/h)	1	2	1	2	1	2	1	2	1	2	1	2
Number of Detectors	1	2	1	2	1	2	1	2	1	2	1	2
Detector Template	Left	Thru	Left	Thru	Left	Thru	Left	Thru	Left	Thru	Left	Thru
Leading Detector (m)	2.0	10.0	2.0	10.0	2.0	10.0	2.0	10.0	2.0	10.0	2.0	10.0
Trailing Detector (m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Position(m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Size(m)	2.0	0.6	2.0	0.6	2.0	0.6	2.0	0.6	2.0	0.6	2.0	0.6
Detector 1 Type	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex
Detector 1 Channel	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 2 Position(m)	9.4	9.4	9.4	9.4	9.4	9.4	9.4	9.4	9.4	9.4	9.4	9.4
Detector 2 Size(m)	0.6	0.6	0.6	0.6	0.6	0.6	0.6	0.6	0.6	0.6	0.6	0.6
Detector 2 Type	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex
Detector 2 Channel	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0

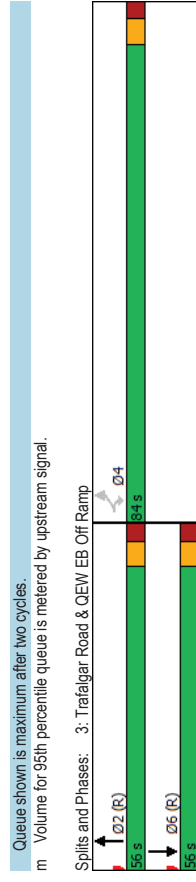
Lanes, Volumes, Timings  
8: Lyons Lane/Commercial Driveway & Cross Ave

Lanes, Volumes, Timings  
3: Trafalgar Road & QEW EB Off Ramp

2022 Existing PM Peak Hour.syn  
05-04-2022

2035 Total AM Peak Hour.syn  
05-04-2022

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Detector 2 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Turn Type	Perm	NA	NA	pm+pt	NA	NA	Perm	NA	NA	Perm	NA	NA
Permitted Phases	2	2	6	1	6	8	8	8	8	4	4	4
Detector Phase	2	2	1	6	6	8	8	8	8	4	4	4
Switch Phase												
Minimum Initial (s)	35.0	35.0	8.0	35.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0
Minimum Split (s)	41.0	41.0	12.5	41.0	28.0	28.0	28.0	28.0	28.0	28.0	28.0	28.0
Total Split (s)	42.0	42.0	13.0	55.0	35.0	35.0	35.0	35.0	35.0	35.0	35.0	35.0
Total Split (%)	46.7%	46.7%	14.4%	61.1%	38.9%	38.9%	38.9%	38.9%	38.9%	38.9%	38.9%	38.9%
Maximum Green (s)	36.0	36.0	9.0	49.0	29.0	29.0	29.0	29.0	29.0	29.0	29.0	29.0
Yellow Time (s)	4.0	4.0	3.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
All-Red Time (s)	2.0	2.0	1.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
Lost Time Adjust (s)	-2.0	-2.0	0.0	-2.0	-2.0	-2.0	-2.0	-2.0	-2.0	-2.0	-2.0	-2.0
Total Lost Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lead/Lag	Lag	Lag	Lead	Lead	Lead	Lead	Lead	Lead	Lead	Lead	Lead	Lead
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Vehicle Extension (s)	5.0	5.0	2.5	5.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Recall Mode	Min	Min	Min	Min	Min	Min	Min	Min	Min	Min	Min	Min
Walk Time (s)	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0
Flash Dont Walk (s)	15.0	15.0	15.0	15.0	15.0	15.0	15.0	15.0	15.0	15.0	15.0	15.0
Pedestrian Calls (#/hr)	0	0	0	0	0	0	0	0	0	0	0	0
Act Effct Green (s)	37.1	37.1	49.1	49.1	28.0	28.0	28.0	28.0	28.0	28.0	28.0	28.0
Actuated g/C Ratio	0.44	0.44	0.58	0.58	0.33	0.33	0.33	0.33	0.33	0.33	0.33	0.33
v/c Ratio	0.17	0.15	0.04	0.19	0.83	0.34	0.07	0.20	0.07	0.20	0.07	0.20
Control Delay	17.4	14.6	9.0	9.1	46.3	5.5	19.9	5.7	19.9	5.7	19.9	5.7
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	17.4	14.6	9.0	9.1	46.3	5.5	19.9	5.7	19.9	5.7	19.9	5.7
LOS	B	B	A	A	D	A	A	B	A	B	A	A
Approach Delay	15.3	15.3	9.1	29.8	8.0	8.0	8.0	8.0	8.0	8.0	8.0	8.0
Approach LOS	B	B	A	C	A	A	A	A	A	A	A	A
Queue Length 50th (m)	6.9	10.2	2.0	14.4	48.1	1.4	2.5	0.5	2.5	0.5	2.5	0.5
Queue Length 95th (m)	9.6	15.9	3.2	21.6	61.2	0.0	5.8	0.6	5.8	0.6	5.8	0.6
Internal Link Dist (m)	16.1	16.1	350.0	45.1	45.1	45.1	45.1	45.1	45.1	45.1	45.1	45.1
Turn Bay Length (m)	25.0	25.0	25.0	20.0	20.0	20.0	20.0	20.0	20.0	20.0	20.0	20.0
Base Capacity (vph)	389	1308	604	1847	414	652	330	582	330	582	330	582
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Spillover Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.16	0.15	0.04	0.19	0.75	0.32	0.06	0.18	0.06	0.18	0.06	0.18
Intersection Summary												
Area Type:	CBD											
Cycle Length:	90											
Actuated Cycle Length:	85.2											
Natural Cycle:	85											
Control Type:	Semi Act-Uncoord											
Maximum v/c Ratio:	0.83											
Intersection Signal Delay:	18.7											





Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Minimum Initial (s)	10.0	10.0	29.0	29.0	29.0	29.0
Minimum Split (s)	38.0	38.0	36.0	36.0	36.0	36.0
Total Split (s)	84.0	84.0	56.0	56.0	56.0	56.0
Total Split (%)	60.0%	60.0%	40.0%	40.0%	40.0%	40.0%
Maximum Green (s)	77.0	77.0	49.0	49.0	49.0	49.0
Yellow Time (s)	4.0	4.0	4.0	4.0	4.0	4.0
All-Red Time (s)	3.0	3.0	3.0	3.0	3.0	3.0
Lost Time Adjust (s)	-3.0	-3.0	-3.0	-3.0	-3.0	-3.0
Total Lost Time (s)	4.0	4.0	4.0	4.0	4.0	4.0
Lead/Lag						
Lead/Lag Optimize?						
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	None	None	C-Max	C-Max	C-Max	C-Max
Walk Time (s)	7.0	7.0	7.0	7.0	7.0	7.0
Flash Dont Walk (s)	11.0	11.0	11.0	11.0	11.0	11.0
Pedestrian Calls (#/hr)	0	0	0	0	0	0
Act Effct Green (s)	80.0	80.0	52.0	52.0	52.0	52.0
Actuated g/C Ratio	0.57	0.57	0.37	0.37	0.37	0.37
v/c Ratio	0.63	1.33	0.84	0.84	1.23	1.23
Control Delay	21.9	185.0	51.5	144.6	144.6	144.6
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	21.9	185.0	51.5	144.6	144.6	144.6
LOS	C	F	D	F	F	F
Approach Delay	103.3		51.5	144.6		
Approach LOS	F		D	F		
Queue Length 50th (m)	117.8	~469.0	172.0	~295.4		
Queue Length 95th (m)	140.4	#543.7	m183.6	m#268.4		
Internal Link Dist (m)	47.3		26.7	162.0		
Turn Bay Length (m)						
Base Capacity (vph)	1942	913	1817	1852		
Starvation Cap Reductn	0	0	0	0		
Spillback Cap Reductn	0	0	0	0		
Storage Cap Reductn	0	0	0	0		
Reduced v/c Ratio	0.63	1.33	0.84	1.23		
<b>Intersection Summary</b>						
Area Type:	Other					
Cycle Length:	140					
Actuated Cycle Length:	140					
Offset:	0 (0%). Referenced to phase 2:NBT and 6:SBT, Start of Green					
Natural Cycle:	75					
Control Type:	Actuated-Coordinated					
Maximum v/c Ratio:	1.33					
Intersection Signal Delay:	105.7					
Intersection Capacity Utilization:	116.4%					
Analysis Period (min):	15					
~ Volume exceeds capacity, queue is theoretically infinite.						
~ 95th percentile volume exceeds capacity, queue may be longer.						

Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Minimum Initial (s)	10.0	10.0	29.0	29.0	29.0	29.0
Minimum Split (s)	38.0	38.0	36.0	36.0	36.0	36.0
Total Split (s)	84.0	84.0	56.0	56.0	56.0	56.0
Total Split (%)	60.0%	60.0%	40.0%	40.0%	40.0%	40.0%
Maximum Green (s)	77.0	77.0	49.0	49.0	49.0	49.0
Yellow Time (s)	4.0	4.0	4.0	4.0	4.0	4.0
All-Red Time (s)	3.0	3.0	3.0	3.0	3.0	3.0
Lost Time Adjust (s)	-3.0	-3.0	-3.0	-3.0	-3.0	-3.0
Total Lost Time (s)	4.0	4.0	4.0	4.0	4.0	4.0
Lead/Lag						
Lead/Lag Optimize?						
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	None	None	C-Max	C-Max	C-Max	C-Max
Walk Time (s)	7.0	7.0	7.0	7.0	7.0	7.0
Flash Dont Walk (s)	11.0	11.0	11.0	11.0	11.0	11.0
Pedestrian Calls (#/hr)	0	0	0	0	0	0
Act Effct Green (s)	80.0	80.0	52.0	52.0	52.0	52.0
Actuated g/C Ratio	0.57	0.57	0.37	0.37	0.37	0.37
v/c Ratio	0.63	1.33	0.84	0.84	1.23	1.23
Control Delay	21.9	185.0	51.5	144.6	144.6	144.6
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	21.9	185.0	51.5	144.6	144.6	144.6
LOS	C	F	D	F	F	F
Approach Delay	103.3		51.5	144.6		
Approach LOS	F		D	F		
Queue Length 50th (m)	117.8	~469.0	172.0	~295.4		
Queue Length 95th (m)	140.4	#543.7	m183.6	m#268.4		
Internal Link Dist (m)	47.3		26.7	162.0		
Turn Bay Length (m)						
Base Capacity (vph)	1942	913	1817	1852		
Starvation Cap Reductn	0	0	0	0		
Spillback Cap Reductn	0	0	0	0		
Storage Cap Reductn	0	0	0	0		
Reduced v/c Ratio	0.63	1.33	0.84	1.23		
<b>Intersection Summary</b>						
Area Type:	Other					
Cycle Length:	140					
Actuated Cycle Length:	140					
Offset:	0 (0%). Referenced to phase 2:NBT and 6:SBT, Start of Green					
Natural Cycle:	75					
Control Type:	Actuated-Coordinated					
Maximum v/c Ratio:	1.33					
Intersection Signal Delay:	105.7					
Intersection Capacity Utilization:	116.4%					
Analysis Period (min):	15					
~ Volume exceeds capacity, queue is theoretically infinite.						
~ 95th percentile volume exceeds capacity, queue may be longer.						

### HCM Signalized Intersection Capacity Analysis 8: Lyons Lane/Commercial Driveway & Cross Ave

2022 Existing PM Peak Hour.syn  
05-04-2022

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	37	150	13	14	307	12	229	4	155	15	2	62
Traffic Volume (vph)	37	150	13	14	307	12	229	4	155	15	2	62
Future Volume (vph)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Ideal Flow (vphpl)	3.3	3.6	3.6	3.3	3.6	3.6	3.3	3.6	3.6	3.3	3.6	3.6
Lane Width	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Total Lost time (s)	1.00	0.95	1.00	1.00	0.95	1.00	1.00	1.00	0.98	1.00	0.99	1.00
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frbp, ped/bikes	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Flt Protected	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00
Satd. Flow (prot)	1519	2908	1569	3069	1569	3069	1569	1445	1565	1417	1565	1417
Flt Permitted	0.54	1.00	0.56	1.00	0.69	1.00	0.69	1.00	0.55	1.00	0.55	1.00
Satd. Flow (perm)	871	2908	932	3069	1136	1445	1136	1445	906	1417	906	1417
Peak-hour factor, PHF	0.58	0.84	0.75	0.54	0.95	0.55	0.74	0.33	0.79	0.70	0.50	0.60
Adj. Flow (vph)	64	179	17	26	323	22	309	12	196	21	4	103
RTOR Reduction (vph)	0	8	0	0	5	0	0	132	0	0	0	69
Lane Group Flow (vph)	64	188	0	26	340	0	309	76	0	21	38	0
Conf. Peds. (#/hr)	3	1	1	0	3	1	0	1	4	4	4	1
Heavy Vehicles (%)	3%	11%	0%	0%	5%	0%	0%	0%	0%	0%	0%	2%
Turn Type	Perm	NA	NA	pm-pt	NA	NA	Perm	NA	NA	Perm	NA	NA
Protected Phases	2			6			8					4
Permitted Phases	2			6			8					4
Actuated Green, G (s)	35.1	35.1	47.1	47.1	47.1	26.0	26.0	26.0	26.0	26.0	26.0	26.0
Effective Green, g (s)	37.1	37.1	47.1	49.1	47.1	28.0	28.0	28.0	28.0	28.0	28.0	28.0
Actuated g/C Ratio	0.44	0.44	0.55	0.58	0.33	0.33	0.33	0.33	0.33	0.33	0.33	0.33
Clearance Time (s)	6.0	6.0	4.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0
Vehicle Extension (s)	5.0	5.0	2.5	5.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lane Grp Cap (vph)	379	1267	575	1770	373	475	373	475	298	466	373	466
v/s Ratio Prot	0.06	0.00	0.00	0.11	0.00	0.05	0.05	0.05	0.02	0.02	0.03	0.03
v/s Ratio Perm	0.07	0.17	0.15	0.05	0.19	0.83	0.16	0.07	0.07	0.07	0.08	0.08
Uniform Delay, d1	14.6	14.5	8.7	8.6	26.3	20.2	19.6	19.7	19.6	19.7	19.7	19.7
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	0.4	0.1	0.0	0.1	14.6	0.2	0.1	0.1	0.1	0.1	0.1	0.1
Delay (s)	15.1	14.6	8.7	8.7	41.0	20.4	19.7	19.8	19.7	19.8	19.8	19.8
Level of Service	B	B	B	A	A	D	C	C	B	B	B	B
Approach Delay (s)	14.7			8.7			32.7				19.8	
Approach LOS	B			A			C				B	
Intersection Summary												
HCM 2000 Control Delay	20.8 HCM 2000 Level of Service C											
HCM 2000 Volume to Capacity ratio	0.45											
Actuated Cycle Length (s)	85.1 Sum of lost time (s) 12.0											
Intersection Capacity Utilization	61.6% ICU Level of Service B											
Analysis Period (min)	15											
c Critical Lane Group												

### Lanes, Volumes, Timings 3: Trafalgar Road & QEW EB Off Ramp

2035 Total AM Peak Hour.syn  
05-04-2022

Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	↑↑	↑		↑↑↑	↑↑↑	↑
Traffic Volume (vph)	1123	1119	0	1401	2093	0
Future Volume (vph)	1123	1119	0	1401	2093	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	0.97	1.00	1.00	0.91	0.91	1.00
Flt Protected	0.950					
Satd. Flow (prot)	3400	1599	0	4893	4988	0
Flt Permitted	0.950					
Satd. Flow (perm)	3400	1599	0	4893	4988	0
Right Turn on Red		Yes			Yes	
Satd. Flow (RTOR)	50		50		50	
Link Distance (m)	71.3		50.7		186.0	
Travel Time (s)	5.1		3.7		13.4	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	3%	1%	0%	6%	4%	0%
Adj. Flow (vph)	1221	1216	0	1523	2275	0
Shared Lane Traffic (%)						
Lane Group Flow (vph)	1221	1216	0	1523	2275	0
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Left	Left	Right
Median Width (m)	7.2		0.0		0.0	
Link Offset (m)	0.0		0.0		0.0	
Crosswalk Width (m)	4.8		4.8		4.8	
Two way Left Turn Lane						
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (k/h)	25	15	25		15	
Number of Detectors	1		1		2	
Detector Template	Left	Right	Thru		Thru	
Leading Detector (m)	2.0	2.0	10.0		10.0	
Trailing Detector (m)	0.0	0.0	0.0		0.0	
Detector 1 Position (m)	0.0	0.0	0.0		0.0	
Detector 1 Size (m)	2.0	2.0	0.6		0.6	
Detector 1 Type	Ch+Ex	Ch+Ex	Ch+Ex		Ch+Ex	
Detector 1 Channel						
Detector 1 Extend (s)	0.0	0.0	0.0		0.0	
Detector 1 Queue (s)	0.0	0.0	0.0		0.0	
Detector 1 Delay (s)	0.0	0.0	0.0		0.0	
Detector 2 Position (m)			9.4		9.4	
Detector 2 Size (m)			0.6		0.6	
Detector 2 Type			Ch+Ex		Ch+Ex	
Detector 2 Channel						
Detector 2 Extend (s)			0.0		0.0	
Turn Type	Perm	Perm	NA		NA	
Protected Phases	4	4	2		6	
Permitted Phases	4	4	2		6	
Switch Phase						

HCM Unsignalized Intersection Capacity Analysis  
 2: Trafalgar Road & QEW SB On Ramps

2035 Total AM Peak Hour.syn  
 05-04-2022

Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations				↑↑↑	↑↑↑	↑
Traffic Volume (veh/h)	0	0	0	2524	2093	958
Future Volume (Veh/h)	0	0	0	2524	2093	958
Sign Control	Stop	Free	Free	Free	Free	Free
Grade	0%	0%	0%	0%	0%	0%
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	0	0	0	2743	2275	1041
Pedestrians						
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)						
Median type			None	None	None	
Median storage (veh)						
Upstream signal (m)			186	145		
pX, platoon unblocked	0.73	0.61	0.61			
vC, conflicting volume	3189	758	2275			
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	0	0	824			
tC, single (s)	6.8	6.9	4.1			
tC, 2 stage (s)						
tF (s)	3.5	3.3	2.2			
p0 queue free %	100	100	100			
cM capacity (veh/h)	748	666	485			
Direction, Lane #	NB.1	NB.2	NB.3	SB.1	SB.2	SB.3 SB.4
Volume Total	914	914	914	758	758	758 1041
Volume Left	0	0	0	0	0	0
Volume Right	0	0	0	0	0	0
Volume 1041						
GSH	1700	1700	1700	1700	1700	1700
Volume to Capacity	0.54	0.54	0.54	0.45	0.45	0.61
Queue Length 95th (m)	0.0	0.0	0.0	0.0	0.0	0.0
Control Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0
Lane LOS						
Approach Delay (s)	0.0			0.0		
Approach LOS						
Intersection Summary	CBD					
Average Delay	0.0					
Intersection Capacity Utilization	116.4% ICU Level of Service H					
Analysis Period (min)	15					

Lanes, Volumes, Timings  
 9: Cross Ave & Lyons Lane

2022 Existing PM Peak Hour.syn  
 05-04-2022

Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	↔	↔	↔	↔	↔	↔
Traffic Volume (vph)	6	180	587	7	15	28
Future Volume (vph)	6	180	587	7	15	28
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (m)	5.0	0.0	0.0	0.0	0.0	0.0
Storage Lanes	1	0	0	1	0	0
Taper Length (m)	7.5	0	0	0	7.5	0
Lane Util. Factor	1.00	0.95	0.95	0.95	1.00	1.00
Ped Bike Factor						
Fit		0.997			0.910	
Fit Protected		0.950			0.984	
Satd. Flow (prot)		1388	2954		0	1491
Fit Permitted		0.950			0.984	
Satd. Flow (perm)		1388	2954		0	1491
Link Speed (k/h)		50	50		50	
Link Distance (m)		281.3	40.1		41.7	
Travel Time (s)		20.3	2.9		3.0	
Confl. Peds. (#/hr)	1			1		9
Peak Hour Factor	0.50	0.92	0.84	0.58	0.44	0.41
Heavy Vehicles (%)	17%	10%	3%	0%	0%	4%
Adj. Flow (vph)	12	196	689	12	34	68
Shared Lane Traffic (%)						
Lane Group Flow (vph)	12	196	711	0	102	0
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Left	Left	Right	Left	Right
Median Width(m)	3.6	3.6	3.6	3.6	3.6	3.6
Link Offset(m)	0.0	0.0	0.0	0.0	0.0	0.0
Crosswalk Width(m)	4.8	4.8	4.8	4.8	4.8	4.8
Two way Left Turn Lane						
Headway Factor	1.14	1.14	1.14	1.14	1.14	1.14
Turning Speed (k/h)	25	15	15	25	15	15
Sign Control	Free	Free	Free	Free	Stop	Stop
Intersection Summary	CBD					
Area Type:	Unsignalized					
Intersection Capacity Utilization	28.3% ICU Level of Service A					
Analysis Period (min)	15					

HCM Unsignalized Intersection Capacity Analysis  
 9: Cross Ave & Lyons Lane  
 2022 Existing PM Peak Hour.syn  
 05-04-2022

Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	↔	↔	↔	↔	↔	↔
Traffic Volume (veh/h)	6	180	587	7	15	28
Future Volume (Veh/h)	6	180	587	7	15	28
Sign Control	Free	Free	Free	Stop	Stop	0%
Grade	0%	0%	0%	0%	0%	0%
Peak Hour Factor	0.50	0.92	0.84	0.58	0.44	0.41
Hourly flow rate (vph)	12	196	699	12	34	68
Pedestrians			9		1	
Lane Width (m)			3.6		3.6	
Walking Speed (m/s)			1.2		1.2	
Percent Blockage			1		0	
Right turn flare (veh)			None		None	
Median type			None		None	
Median storage (veh)			40		0.96	0.96
Upstream signal (m)					837	356
pX, platoon unblocked						
vC, conflicting volume						
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol					745	244
tC, single (s)					6.8	7.0
tC, 2 stage (s)						
tF (s)					3.5	3.3
p0 queue free %					90	91
cM capacity (veh/h)					332	719
Direction_Lane #	EB1	EB2	EB3	WB1	WB2	SB1
Volume Total	12	98	98	466	245	102
Volume Left	12	0	0	0	0	34
Volume Right	0	0	0	0	12	68
GSH	829	1700	1700	1700	1700	518
Volume to Capacity	0.01	0.06	0.06	0.27	0.14	0.20
Queue Length 95th (m)	0.4	0.0	0.0	0.0	0.0	5.8
Control Delay (s)	9.4	0.0	0.0	0.0	0.0	13.7
Lane LOS	A					B
Approach Delay (s)	0.5			0.0		13.7
Approach LOS						B
Intersection Summary						
Average Delay	1.5					
Intersection Capacity Utilization	28.3%					
Analysis Period (min)	15					
	ICU Level of Service A					

Lanes, Volumes, Timings  
 2: Trafalgar Road & QEW SB On Ramps  
 2035 Total AM Peak Hour.syn  
 05-04-2022

Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	↔	↔	↔	↔	↔	↔
Traffic Volume (vph)	0	0	0	2524	2093	958
Future Volume (vph)	0	0	0	2524	2093	958
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	0.91	0.91	1.00
FrT						0.850
FrT Protected						
Satd. Flow (prot)	0	0	0	5085	5085	1583
FrT Permitted						
Satd. Flow (perm)	0	0	0	5085	5085	1583
Link Speed (k/h)	50			50	50	
Link Distance (m)	159.6			186.0	145.2	
Travel Time (s)	11.5			13.4	10.5	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	0	0	0	2743	2275	1041
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	0	0	2743	2275	1041
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Left	Left	Right
Median Width (m)	0.0			0.0	0.0	
Link Offset (m)	0.0			0.0	0.0	
Crosswalk Width (m)	4.8			4.8	4.8	
Two way Left Turn Lane						
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (k/h)	25	15	25			15
Sign Control	Stop			Free	Free	
Intersection Summary						
Area Type:	Other					
Control Type:	Unsignalized					
Intersection Capacity Utilization	116.4%					
Analysis Period (min)	15					
	ICU Level of Service H					

HCM Signalized Intersection Capacity Analysis  
 1: Trafalgar Road & North Service Road/QEW WB Off Ramp

Lanes, Volumes, Timings  
 10: Lyons Lane & South Service Road

2035 Total AM Peak Hour.syn  
 05-04-2022

2022 Existing PM Peak Hour.syn  
 05-04-2022

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	1	0	196	579	42	249	0	1989	535	0	2276	8
Traffic Volume (vph)	1	0	196	579	42	249	0	1989	535	0	2276	8
Future Volume (vph)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Ideal Flow (vphpl)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Total Lost time (s)	1.00	1.00	1.00	0.95	0.95	0.95	0.91	1.00	0.91	1.00	0.91	1.00
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.96	1.00	0.95	1.00	0.95
Frb, ped/bikes	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frb, ped/bikes	1.00	0.85	1.00	0.89	0.85	1.00	0.85	1.00	0.85	1.00	0.85	1.00
Flt Protected	0.95	1.00	0.95	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Satd. Flow (prot)	1805	1568	1719	1454	1447	4940	1478	5036	1533	5036	1533	1533
Flt Permitted	0.95	1.00	0.95	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Satd. Flow (perm)	1805	1568	1719	1454	1447	4940	1478	5036	1533	5036	1533	1533
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	1	0	213	629	46	271	0	2162	582	0	2474	9
RTOR Reduction (vph)	0	0	15	0	51	80	0	192	0	0	0	5
Lane Group Flow (vph)	1	0	198	629	112	74	0	2162	390	0	2474	4
Conf. Peds. (#/hr)	0%	0%	3%	5%	23%	6%	0%	5%	5%	0%	3%	0%
Heavy Vehicles (%)	Prot	Perm	Perm	NA	Perm	NA	Perm	NA	Perm	NA	Perm	NA
Turn Type	7		4	8	8	2	2	2	2	6	6	6
Protected Phases	7		4	8	8	2	2	2	2	6	6	6
Permitted Phases	1.2	70.4	64.2	64.2	64.2	55.6	55.6	55.6	55.6	55.6	55.6	55.6
Actuated Green, G (s)	2.2	73.4	67.2	67.2	67.2	58.6	58.6	58.6	58.6	58.6	58.6	58.6
Effective Green, g (s)	0.02	0.52	0.48	0.48	0.48	0.42	0.42	0.42	0.42	0.42	0.42	0.42
Actuated g/C Ratio	5.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0
Clearance Time (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Vehicle Extension (s)	28	822	825	697	694	2067	618	2107	641	2107	641	641
Lane Grp Cap (vph)	0.00		c0.13	c0.37	0.05	0.26	0.44	c0.49				
v/s Ratio Prot	0.04	0.24	0.76	0.16	0.11	1.05	0.63	1.17	0.01	1.17	0.01	0.01
v/s Ratio Perm	67.9	18.1	29.9	20.5	19.9	40.7	32.1	40.7	23.7	40.7	23.7	23.7
Uniform Delay, d1	1.00	1.00	1.00	1.00	1.00	0.74	0.80	1.00	1.00	1.00	1.00	1.00
Progression Factor	0.5	0.2	4.2	0.1	0.1	29.6	3.2	83.8	0.0	83.8	0.0	0.0
Incremental Delay, d2	68.4	18.3	34.1	20.6	20.0	59.9	28.8	124.5	23.7	124.5	23.7	23.7
Delay (s)	E	B	C	C	C	E	C	F	C	F	C	C
Level of Service	E	B	C	C	C	E	C	F	C	F	C	C
Approach Delay (s)	18.5		29.5			53.3		124.1		124.1		
Approach LOS	B		C			D		F		F		
Intersection Summary												
HCM 2000 Control Delay	76.1 HCM 2000 Level of Service E											
HCM 2000 Volume to Capacity ratio	0.94											
Actuated Cycle Length (s)	140.0 Sum of lost time (s)											
Intersection Capacity Utilization	98.2% ICU Level of Service F											
Analysis Period (min)	15											
c Critical Lane Group												

Baseline

Syncho 10 Report  
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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	4	1	1	1	1	1	1	1	1	1	1	1
Traffic Volume (vph)	3	5	1	5	13	5	13	5	13	5	13	5
Future Volume (vph)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Ideal Flow (vphpl)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Lane Util. Factor	0.976	0.976	0.976	0.976	0.976	0.976	0.976	0.976	0.976	0.976	0.976	0.976
Frb, ped/bikes	0	1669	1357	0	1582	0	1582	0	1582	0	1582	0
Flt Permitted	0	1669	1357	0	1582	0	1582	0	1582	0	1582	0
Satd. Flow (perm)	60.5	89.6	37.6	60.5	89.6	37.6	60.5	89.6	37.6	60.5	89.6	37.6
Link Speed (k/h)	4.4	6.5	2.7	4.4	6.5	2.7	4.4	6.5	2.7	4.4	6.5	2.7
Travel Time (s)	7		7	7		7	7		7	7		7
Conf. Peds. (#/hr)	0.38	0.62	0.25	0.62	0.50	0.42	0.38	0.62	0.25	0.62	0.50	0.42
Peak Hour Factor	0%	0%	0%	0%	22%	0%	0%	22%	0%	22%	0%	0%
Heavy Vehicles (%)	8	8	4	8	4	8	8	4	8	4	8	4
Adj. Flow (vph)	0	16	12	0	38	0	0	16	12	0	38	0
Shared Lane Traffic (%)	0	16	12	0	38	0	0	16	12	0	38	0
Lane Group Flow (vph)	No	No	No	No	No	No	No	No	No	No	No	No
Enter Blocked Intersection	Left	Left	Left	Left	Left	Left	Left	Left	Left	Left	Left	Left
Lane Alignment	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Median Width(m)	4.8	4.8	4.8	4.8	4.8	4.8	4.8	4.8	4.8	4.8	4.8	4.8
Link Offset(m)	1.14	1.14	1.14	1.14	1.14	1.14	1.14	1.14	1.14	1.14	1.14	1.14
Crosswalk Width(m)	24		Free	Free	Free	Free	Free	Free	Free	Free	Free	Free
Two way Left Turn Lane	1.14	1.14	1.14	1.14	1.14	1.14	1.14	1.14	1.14	1.14	1.14	1.14
Headway Factor	24		Free	Free	Free	Free	Free	Free	Free	Free	Free	Free
Turning Speed (k/h)	Free	Free	Free	Free	Free	Free	Free	Free	Free	Free	Free	Free
Sign Control			Stop			Stop			Stop			Stop
Intersection Summary												
Area Type:	CBD											
Control Type:	Unsignalized											
Intersection Capacity Utilization	15.4%											
Analysis Period (min)	15											
ICU Level of Service A												

Baseline

Syncho 10 Report  
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HCM Unsignalized Intersection Capacity Analysis  
 10: Lyons Lane & South Service Road

2022 Existing PM Peak Hour.syn  
 05-04-2022



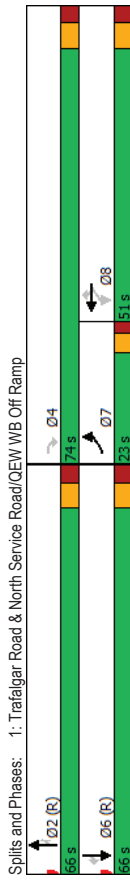
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	3	5	1	5	13	5
Traffic Volume (veh/h)	3	5	1	5	13	5
Future Volume (Veh/h)	3	5	1	5	13	5
Sign Control	Free	Free	Stop	Stop	0%	0%
Grade	0%	0%	0%	0%	0%	0%
Peak Hour Factor	0.38	0.62	0.25	0.62	0.50	0.42
Hourly flow rate (vph)	8	8	4	8	26	12
Pedestrians					7	
Lane Width (m)					3.6	
Walking Speed (m/s)					1.2	
Percent Blockage					1	
Right turn flare (veh)						
Median type	None	None				
Median storage (veh)						
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume	19				39	15
vC1, stage 1 conf vol						
vC2, stage 2 conf vol	19				39	15
tC, single (s)	4.1				6.4	6.2
tC, 2 stage (s)						
tF (s)	2.2				3.5	3.3
p0 queue free %	100				97	99
cM capacity (veh/h)	1601				967	1064
Direction_Lane #	EB 1	WB 1	SB 1			
Volume Total	16	12	38			
Volume Left	8	0	26			
Volume Right	0	8	12			
GSH	1601	1700	996			
Volume to Capacity	0.00	0.01	0.04			
Queue Length 95th (m)	0.1	0.0	1.0			
Control Delay (s)	3.6	0.0	8.8			
Lane LOS	A	A	A			
Approach Delay (s)	3.6	0.0	8.8			
Approach LOS	A	A	A			
Intersection Summary						
Average Delay			5.9			
Intersection Capacity Utilization			15.4%		ICU Level of Service	A
Analysis Period (min)			15			

Baseline

Lanes, Volumes, Timings  
 1: Trafalgar Road & North Service Road/QEW WB Off Ramp

2035 Total AM Peak Hour.syn  
 05-04-2022

Intersection Capacity Utilization 98.2% ICU Level of Service F  
 Analysis Period (min) 15  
 ~ Volume exceeds capacity, queue is theoretically infinite.  
 Queue shown is maximum after two cycles.  
 # 95th percentile volume exceeds capacity, queue may be longer.  
 Queue shown is maximum after two cycles.  
 m Volume for 95th percentile queue is metered by upstream signal.



Splits and Phases: 1: Trafalgar Road & North Service Road/QEW WB Off Ramp

Baseline

Lanes, Volumes, Timings  
1: Trafalgar Road & North Service Road/QEW WB Off Ramp

Lanes, Volumes, Timings  
11: Argus Rd & South Service Rd

2035 Total AM Peak Hour.syn  
05-04-2022

2022 Existing PM Peak Hour.syn  
05-04-2022

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Turn Type	Prot	Perm	Perm	Perm	Perm	Perm	NA	NA	NA	NA	NA	Perm
Protected Phases	7						2					6
Permitted Phases		4	8	8	8	8			2	2		6
Detector Phase	7	4	8	8	8	8	2	2	2	2	6	6
Switch Phase												
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Minimum Split (s)	23.0	38.0	38.0	38.0	38.0	38.0	35.0	35.0	35.0	35.0	35.0	35.0
Total Split (s)	23.0	74.0	51.0	51.0	51.0	51.0	66.0	66.0	66.0	66.0	66.0	66.0
Total Split (%)	16.4%	52.9%	36.4%	36.4%	36.4%	36.4%	47.1%	47.1%	47.1%	47.1%	47.1%	47.1%
Maximum Green (s)	18.0	67.0	44.0	44.0	44.0	44.0	59.0	59.0	59.0	59.0	59.0	59.0
Yellow Time (s)	3.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
All-Red Time (s)	2.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Lost Time Adjust (s)	-1.0	-3.0	-3.0	-3.0	-3.0	-3.0	-3.0	-3.0	-3.0	-3.0	-3.0	-3.0
Total Lost Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
LeadLag	Lead	Lag	Lag	Lag	Lag	Lag						
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes						
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	None	None	None	None	None	None	C-Max	C-Max	C-Max	C-Max	C-Max	C-Max
Walk Time (s)		7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0
Flash Dont Walk (s)		11.0	11.0	11.0	11.0	11.0	11.0	11.0	11.0	11.0	11.0	11.0
Pedestrian Calls (#/hr)		0	0	0	0	0	0	0	0	0	0	0
Act Effct Green (s)	6.7	69.4	67.2	67.2	67.2	67.2	62.6	62.6	62.6	62.6	62.6	62.6
Actuated g/C Ratio	0.05	0.50	0.48	0.48	0.48	0.48	0.45	0.45	0.45	0.45	0.45	0.45
v/c Ratio	0.01	0.27	0.76	0.22	0.20	0.20	0.98	0.69	1.10	0.01	0.01	0.01
Control Delay	64.0	18.4	38.0	10.0	4.0	4.0	40.0	13.1	88.9	0.0	0.0	0.0
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	64.0	18.4	38.0	10.0	4.0	4.0	40.0	13.1	88.9	0.0	0.0	0.0
LOS	E	B	D	B	A	A	D	D	B	F	F	A
Approach Delay	18.6			27.6			34.3			88.6		
Approach LOS	B			C			C			F		
Queue Length 50th (m)	0.3	30.3	144.9	10.4	0.0	0.0	126.7	31.1	-294.2	0.0		
Queue Length 95th (m)	2.3	47.2	#237.3	28.7	14.5		#262.8	m66.5	#328.9	0.0		
Internal Link Dist (m)		96.4		141.1			121.2		185.2			
Turn Bay Length (m)	50.0						70.0					
Base Capacity (vph)	244	799	824	748	774	2208	843	2252	724			
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.00	0.27	0.76	0.22	0.20	0.20	0.98	0.69	1.10	0.01		
Intersection Summary												
Area Type:	Other											
Actuated Cycle Length:	140											
Offset: 0 (0%):	Referenced to phase 2:NBT and 6:SBT, Start of Green											
Natural Cycle:	150											
Control Type:	Actuated-Coordinated											
Maximum v/c Ratio:	1.10											
Intersection Signal Delay:	53.9											
Intersection LOS:	D											

Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	10	8	279	82	13	14
Traffic Volume (vph)	10	8	279	82	13	14
Future Volume (vph)	1900	1900	1900	1900	1900	1900
Ideal Flow (vphpl)	1.00	1.00	1.00	1.00	1.00	1.00
Lane Util. Factor			0.963		0.924	
Ped Bike Factor						
Flt Protected		0.972		0.979		
Satd. Flow (prot)	0	1576	1535	0	1547	0
Flt Permitted		0.972		0.979		
Satd. Flow (perm)	0	1576	1535	0	1547	0
Link Speed (k/h)		50	50		50	
Link Distance (m)		177.7	165.2		103.5	
Travel Time (s)		12.8	11.9		7.5	
Conf. Peds. (#/hr)					5	
Peak Hour Factor	0.45	0.50	0.87	0.68	0.75	0.65
Heavy Vehicles (%)	0%	13%	10%	0%	0%	0%
Adj. Flow (vph)	22	16	321	121	17	22
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	38	442	0	39	0
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Right	Right
Median Width(m)		0.0	0.0		3.6	
Link Offset(m)		0.0	0.0		0.0	
Crosswalk Width(m)		4.8	4.8		4.8	
Two way Left Turn Lane						
Headway Factor	1.14	1.14	1.14	1.14	1.14	1.14
Turning Speed (k/h)	24		Free		Stop	
Sign Control			Free		Stop	
Intersection Summary						
Area Type:	CBD					
Control Type:	Unsignalized					
Intersection Capacity Utilization	31.9%					
Analysis Period (min)	15					
ICU Level of Service A						

HCM Unsignalized Intersection Capacity Analysis  
 11: Argus Rd & South Service Rd

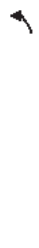
2022 Existing PM Peak Hour.syn  
 05-04-2022



Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	10	8	279	82	13	14
Traffic Volume (veh/h)	10	8	279	82	13	14
Future Volume (Veh/h)	Free	Free	Free	Stop	0%	0%
Sign Control	0%	0%	0%	0%	0%	0%
Grade	0.45	0.50	0.87	0.68	0.75	0.65
Peak Hour Factor	22	16	321	121	17	22
Hourly flow rate (vph)	5	3.6	1.2	0	0	0
Lane Width (m)	3.6	1.2	0	0	0	0
Walking Speed (m/s)	1.2	0	0	0	0	0
Percent Blockage	None	None	None	None	None	None
Right turn flare (veh)	358	442	446	382	446	382
Median storage (veh)	442	442	446	382	446	382
Upstream signal (m)	4.1	4.1	6.4	6.2	6.4	6.2
pX, platoon unblocked	2.2	2.2	3.5	3.3	3.5	3.3
vC, conflicting volume	1129	98	97	97	560	670
vC1, stage 1 conf/vol	EB1	WB1	SB1			
vC2, stage 2 conf/vol	38	442	39			
Volume Total	22	0	17			
Volume Left	0	121	22			
Volume Right	1129	1700	617			
GSH	0.02	0.26	0.06			
Volume to Capacity	0.5	0.0	1.6			
Queue Length 95th (m)	4.8	0.0	11.2			
Control Delay (s)	A	B	B			
Lane LOS	4.8	0.0	11.2			
Approach Delay (s)	B					
Approach LOS	1.2					
Intersection Summary	31.9%	ICU Level of Service	A			
Average Delay	15					
Intersection Capacity Utilization						
Analysis Period (min)						

Lanes, Volumes, Timings  
 1: Trafalgar Road & North Service Road/QEW WB Off Ramp

2035 Total AM Peak Hour.syn  
 05-04-2022



Lane Group	EBL	EBT	WBL	WBT	SBL	SBR
Lane Configurations	1	0	196	579	42	249
Traffic Volume (vph)	1	0	196	579	42	249
Future Volume (vph)	1900	1900	1900	1900	1900	1900
Ideal Flow (vphpl)	50.0	0.0	0.0	0.0	0.0	0.0
Storage Length (m)	1	1	1	1	1	1
Storage Lanes	7.5	7.5	7.5	7.5	7.5	7.5
Taper Length (m)	1.00	1.00	1.00	0.95	1.00	0.91
Lane Util. Factor	0.850	0.850	0.850	0.850	0.850	0.850
Ped Bike Factor	0.950	0.950	0.950	0.950	0.950	0.950
Flt Protected	1805	0	1568	1719	1453	1447
Satd. Flow (prot)	0.950	0.950	0.950	0.950	0.950	0.950
Flt Permitted	1805	0	1568	1719	1453	1447
Satd. Flow (perm)	Yes	Yes	Yes	Yes	Yes	Yes
Right Turn on Red	31	98	154	331	50	50
Satd. Flow (RTOR)	50	50	50	50	50	50
Link Speed (k/h)	120.4	165.1	145.2	145.2	209.2	15.1
Travel Time (s)	8.7	11.9	10.5	10.5	15.1	15.1
Confli. Peds. (#/hr)	0.92	0.92	0.92	0.92	0.92	0.92
Peak Hour Factor	0%	3%	5%	23%	6%	0%
Heavy Vehicles (%)	1	0	213	629	46	271
Adj. Flow (vph)	0	0	0	0	0	0
Shared Lane Traffic (%)	43%	43%	43%	43%	43%	43%
Lane Group Flow (vph)	1	0	213	629	163	154
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Left	Left	Left	Left	Left
Median Width (m)	3.6	3.6	3.6	3.6	3.6	3.6
Link Offset (m)	0.0	0.0	0.0	0.0	0.0	0.0
Crosswalk Width (m)	4.8	4.8	4.8	4.8	4.8	4.8
Two way Left Turn Lane	1.00	1.00	1.00	1.00	1.00	1.00
Headway Factor	25	15	25	15	25	15
Turning Speed (k/h)	1	1	2	1	2	1
Number of Detectors	1	1	2	1	2	1
Detector Template	Left	Right	Thru	Right	Thru	Right
Leading Detector (m)	2.0	2.0	10.0	2.0	10.0	2.0
Trailing Detector (m)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Position (m)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Size (m)	2.0	2.0	2.0	2.0	2.0	2.0
Detector 1 Type	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex
Detector 1 Channel	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 2 Position (m)	9.4	9.4	9.4	9.4	9.4	9.4
Detector 2 Size (m)	0.6	0.6	0.6	0.6	0.6	0.6
Detector 2 Type	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex
Detector 2 Channel	0.0	0.0	0.0	0.0	0.0	0.0
Detector 2 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0



Lanes, Volumes, Timings  
 1: Trafalgar Road & North Service Road/QEW WB Off Ramp

Lanes, Volumes, Timings  
 12: Site Driveway

2025 Background AM Peak Hour.syn  
 05-04-2022

2030 Total PM Peak Hour.syn  
 05-04-2022

	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Turn Type	Prot	Perm	Perm	Perm	Perm	Perm	NA	NA	Perm	Perm	NA	Perm
Protected Phases	7											
Permitted Phases		4	8	8	8	8	2	2	2	2	6	6
Detector Phase	7	4	8	8	8	8	2	2	2	2	6	6
Switch Phase												
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Minimum Split (s)	23.0	38.0	38.0	38.0	38.0	35.0	35.0	35.0	35.0	35.0	35.0	35.0
Total Split (s)	23.0	61.0	38.0	38.0	38.0	79.0	79.0	79.0	79.0	79.0	79.0	79.0
Total Split (%)	16.4%	43.6%	27.1%	27.1%	27.1%	56.4%	56.4%	56.4%	56.4%	56.4%	56.4%	56.4%
Maximum Green (s)	18.0	54.0	31.0	31.0	31.0	72.0	72.0	72.0	72.0	72.0	72.0	72.0
Yellow Time (s)	3.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
All-Red Time (s)	2.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Lost Time Adjust (s)	-1.0	-3.0	-3.0	-3.0	-3.0	-3.0	-3.0	-3.0	-3.0	-3.0	-3.0	-3.0
Total Lost Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lead/Lag	Lead	Lag	Lag	Lag	Lag	Lag	Lead	Lag	Lag	Lag	Lead	Lag
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes						
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	None	None	None	None	None	None	C-Max	C-Max	C-Max	C-Max	C-Max	C-Max
Walk Time (s)		7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0
Flash Dont Walk (s)		11.0	11.0	11.0	11.0	11.0	11.0	11.0	11.0	11.0	11.0	11.0
Pedestrian Calls (#/hr)		0	0	0	0	0	0	0	0	0	0	0
Act Effct Green (s)	6.7	32.7	30.5	30.5	30.5	99.3	99.3	99.3	99.3	99.3	99.3	99.3
Actuated g/C Ratio	0.05	0.23	0.22	0.22	0.22	0.71	0.71	0.71	0.71	0.71	0.71	0.71
v/c Ratio	0.01	0.59	0.71	0.72	0.57	0.59	0.31	1.10	0.01	1.10	0.01	0.01
Control Delay	64.0	45.7	61.0	61.9	16.1	6.1	2.2	2.2	71.4	71.4	0.0	0.0
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	64.0	45.7	61.0	61.9	16.1	6.1	2.2	2.2	71.4	71.4	0.0	0.0
LOS	E	D	E	E	B	A	A	A	A	E	E	A
Approach Delay	45.8			44.9		5.5				71.3		
Approach LOS	D			D		A				E		
Queue Length 50th (m)	0.3	53.0	71.5	71.8	16.3	62.3	6.5	6.5	~466.8	0.0		
Queue Length 95th (m)	2.3	66.9	96.5	96.9	43.0	98.6	24.3	24.3	#563.8	0.0		
Internal Link Dist (m)		96.4		141.1		121.2			185.2			
Turn Bay Length (m)	50.0					70.0						
Base Capacity (vph)	244	656	410	403	547	3504	1121	1121	3572	1107		
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0		
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0		
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0		
Reduced v/c Ratio	0.00	0.35	0.61	0.62	0.53	0.59	0.31	0.31	1.10	0.01		
Intersection Summary	Intersection LOS: D											
Area Type:	Other											
Actuated Cycle Length: 140	Cycle Length: 140											
Offset: 0 (0%); Referenced to phase 2:NBT and 6:SBT; Start of Green	Natural Cycle: 150											
Control Type: Actuated-Coordinated	Intersection LOS: D											
Maximum v/c Ratio: 1.10	Intersection Signal Delay: 46.1											

Baseline  
 Syncho 10 Report  
 Page 2

	EBT	EBR	WBL	WBT	NBL	NBR
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	10	22	45	21	4	39
Future Volume (vph)	10	22	45	21	4	39
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
FrT	0.907				0.877	
Fit Protected					0.967	0.996
Satd. Flow (prot)	1690	0	0	1801	1627	0
Fit Permitted					0.967	0.996
Satd. Flow (perm)	1690	0	0	1801	1627	0
Link Speed (k/h)	50			50	50	
Link Distance (m)	128.8			697.0	50.6	
Travel Time (s)	9.3			50.2	3.6	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	11	24	49	23	4	42
Shared Lane Traffic (%)						
Lane Group Flow (vph)	35	0	0	72	46	0
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Left	Left	Right
Median Width (m)	0.0	0.0	0.0	0.0	3.6	
Link Offset (m)	0.0				0.0	
Crosswalk Width (m)	4.8			4.8	4.8	
Two way Left Turn Lane						
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (k/h)		15	25	Free	Stop	15
Sign Control	Free	Free	Free	Free	Stop	Free
Intersection Summary	Other					
Area Type:	Other					
Control Type: Unsignalized	ICU Level of Service A					
Intersection Capacity Utilization 20.3%						
Analysis Period (min) 15						

Baseline  
 Syncho 10 Report  
 Page 33

HCM Unsignalized Intersection Capacity Analysis  
 11: Argus Rd & South Service Rd

Lanes, Volumes, Timings  
 1: Trafalgar Road & North Service Road/QEW WB Off Ramp

2030 Total PM Peak Hour.syn  
 05-04-2022

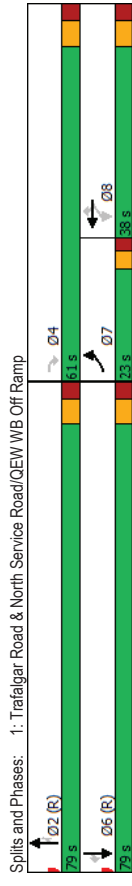
2025 Background AM Peak Hour.syn  
 05-04-2022



Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	4	4	4	4	4	4
Traffic Volume (veh/h)	12	9	327	141	26	44
Future Volume (Veh/h)	12	9	327	141	26	44
Sign Control	Free	Free	Free	Stop	Stop	Stop
Grade	0%	0%	0%	0%	0%	0%
Peak Hour Factor	0.45	0.50	0.87	0.68	0.75	0.65
Hourly flow rate (vph)	27	18	376	207	35	68
Pedestrians			5			
Lane Width (m)			3.6			
Walking Speed (m/s)			1.2			
Percent Blockage			0			
Right turn flare (veh)			None	None		
Median type			None	None		
Median storage (veh)						
Upstream signal (m)			358			
pX, platoon unblocked						
vC, conflicting volume			583		556	480
vC1, stage 1 conf/vol						
vC2, stage 2 conf/vol						
vCu, unblocked vol			583		556	480
tC, single (s)			4.1		6.4	6.2
tC, 2 stage (s)			2.2		3.5	3.3
tF (s)			97		93	88
p0 queue free %			1001		480	590
cM capacity (veh/h)						
Direction_Lane #	EB 1	WB 1	SB 1			
Volume Total	45	583	103			
Volume Left	27	0	35			
Volume Right	0	207	68			
GSH	1001	1700	547			
Volume to Capacity	0.03	0.34	0.19			
Queue Length 95th (m)	0.7	0.0	5.5			
Control Delay (s)	5.3	0.0	13.1			
Lane LOS	A	B	B			
Approach Delay (s)	5.3	0.0	13.1			
Approach LOS	B	B	B			
Intersection Summary						
Average Delay			2.2			
Intersection Capacity Utilization			39.9%		ICU Level of Service	A
Analysis Period (min)			15			

Intersection Capacity Utilization 105.7%  
 Analysis Period (min) 15  
 ICU Level of Service G

~ Volume exceeds capacity, queue is theoretically infinite.  
 Queue shown is maximum after two cycles.  
 # 95th percentile volume exceeds capacity, queue may be longer.  
 Queue shown is maximum after two cycles.



Spills and Phases: 1: Trafalgar Road & North Service Road/QEW WB Off Ramp



HCM Unsignalized Intersection Capacity Analysis  
 10: Lyons Lane & South Service Road

2030 Total PM Peak Hour.syn  
 05-04-2022



Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	4	6	1	28	19	6
Traffic Volume (veh/h)	4	6	1	28	19	6
Future Volume (Veh/h)	4	6	1	28	19	6
Sign Control	Free	Free	0%	Stop	0%	0%
Grade	0%	0%	0%	0%	0%	0%
Peak Hour Factor	0.38	0.62	0.25	0.62	0.50	0.42
Hourly flow rate (vph)	11	10	4	45	38	14
Pedestrians					7	
Lane Width (m)					3.6	
Walking Speed (m/s)					1.2	
Percent Blockage					1	
Right turn flare (veh)			None	None		
Median type						
Median storage (veh)						
Upstream signal (m)			393			
pX, platoon unblocked					66	34
vC, conflicting volume						
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
tC, single (s)		56			66	34
tC, 2 stage (s)		4.1			6.4	6.2
tF (s)		2.2			3.5	3.3
p0 queue free %		99			96	99
cM capacity (veh/h)		1553			933	1040
Direction, Lane #	EB 1	WB 1	SB 1			
Volume Total	21	49	52			
Volume Left	11	0	38			
Volume Right	0	45	14			
GSH	1553	1700	959			
Volume to Capacity	0.01	0.03	0.05			
Queue Length 95th (m)	0.2	0.0	1.4			
Control Delay (s)	3.9	0.0	9.0			
Lane LOS	A	A	A			
Approach Delay (s)	3.9	0.0	9.0			
Approach LOS	A	A	A			
Intersection Summary						
Average Delay	4.5					
Intersection Capacity Utilization	15.4%					
ICU Level of Service	A					
Analysis Period (min)	15					

Lanes, Volumes, Timings  
 2: Trafalgar Road & QEW SB On Ramps

2025 Background AM Peak Hour.syn  
 05-04-2022



Lane Group	EBL	EBR	NBL	NBT	SBL	SBR
Lane Configurations						
Traffic Volume (vph)	0	0	0	2221	2397	1856
Future Volume (vph)	0	0	0	2221	2397	1856
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	0.91	0.91	1.00
Flt Protected						0.850
Satd. Flow (prot)	0	0	0	5085	5085	1583
Flt Permitted						
Satd. Flow (perm)	0	0	0	5085	5085	1583
Link Speed (k/h)				50	50	
Link Distance (m)				186.0	145.2	
Travel Time (s)				13.4	10.5	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	0	0	0	2414	2605	2017
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	0	0	2414	2605	2017
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Left	Left	Right
Median Width(m)	0.0	0.0	0.0	0.0	0.0	0.0
Link Offset(m)	0.0	0.0	0.0	0.0	0.0	0.0
Crosswalk Width(m)	4.8	4.8	4.8	4.8	4.8	4.8
Two way Left Turn Lane						
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (k/h)	25	15	25	Free	Free	15
Sign Control	Stop	Stop	Free	Free	Free	Free
Intersection Summary						
Area Type:	Other					
Control Type:	Unsignalized					
Intersection Capacity Utilization	118.3%					
ICU Level of Service	H					
Analysis Period (min)	15					



HCM Unsignalized Intersection Capacity Analysis 2025 Background AM Peak Hour.syn  
 2: Trafalgar Road & QEW SB On Ramps 05-04-2022

Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations				↑↑↑	↑↑↑	↑
Traffic Volume (veh/h)	0	0	0	2221	2397	1856
Future Volume (Veh/h)	0	0	0	2221	2397	1856
Sign Control	Stop	Free	Free	Free	Free	Free
Grade	0%			0%	0%	0%
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	0	0	0	2414	2605	2017
Pedestrians						
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)						
Median type			None	None	None	None
Median storage (veh)						
Upstream signal (m)				186	145	
pX, platoon unblocked	0.56	0.38	0.38			
vC, conflicting volume	3410	868	2605			
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	0	0	0			
tC, single (s)	6.8	6.9	4.1			
tC, 2 stage (s)						
tF (s)	3.5	3.3	2.2			
p0 queue free %	100	100	100			
cM capacity (veh/h)	576	408	609			
Direction_Lane #	NB.1	NB.2	NB.3	SB.1	SB.2	SB.3
Volume Total	805	805	805	868	868	868
Volume Left	0	0	0	0	0	0
Volume Right	0	0	0	0	0	0
GSH	1700	1700	1700	1700	1700	1700
Volume to Capacity	0.47	0.47	0.47	0.51	0.51	0.51
Queue Length 95th (m)	0.0	0.0	0.0	0.0	0.0	0.0
Control Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0
Lane LOS						
Approach Delay (s)	0.0			0.0		
Approach LOS						
Intersection Summary						
Average Delay	0.0					
Intersection Capacity Utilization	118.3% ICU Level of Service					
Analysis Period (min)	15					

Lanes, Volumes, Timings 2030 Total PM Peak Hour.syn  
 10: Lyons Lane & South Service Road 05-04-2022

Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		←	←	←	←	←
Traffic Volume (vph)	4	6	1	28	19	6
Future Volume (vph)	4	6	1	28	19	6
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor			0.876		0.964	
Ft Protected		0.974			0.965	
Satd. Flow (prot)		1666		1246	1591	
Ft Permitted		0.974			0.965	
Satd. Flow (perm)		1666		1246	1591	
Link Speed (k/h)		50		50	50	
Link Distance (m)		60.5		89.6	37.6	
Travel Time (s)		4.4		6.5	2.7	
Conf. Peds. (#/hr)		7		7		
Peak Hour Factor	0.38	0.62	0.25	0.62	0.50	0.42
Heavy Vehicles (%)	0%	0%	0%	22%	0%	0%
Adj. Flow (vph)	11	10	4	45	38	14
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	21	49	0	52	0
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right
Median Width(m)		0.0	0.0		3.6	
Link Offset(m)		0.0	0.0		0.0	
Crosswalk Width(m)		4.8	4.8		4.8	
Two way Left Turn Lane						
Headway Factor	1.14	1.14	1.14	1.14	1.14	1.14
Turning Speed (k/h)	24			14	24	14
Sign Control		Free	Free		Stop	
Intersection Summary						
Area Type:	CBD					
Control Type:	Unsignalized					
Intersection Capacity Utilization	15.4% ICU Level of Service A					
Analysis Period (min)	15					

HCM Signalized Intersection Capacity Analysis  
8: Cross Ave & Lyons Lane

2030 Total PM Peak Hour.syn  
05-04-2022

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	38	193	7	23	542	73	158	0	226	52	0	28
Traffic Volume (vph)	38	193	7	23	542	73	158	0	226	52	0	28
Future Volume (vph)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Ideal Flow (vphpl)	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5
Total Lost time (s)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frb, ped/bikes	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt	1.00	1.00	0.85	1.00	1.00	1.00	1.00	1.00	0.85	1.00	1.00	0.85
Flt Protected	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00
Satd. Flow (prot)	1388	2954	1425	1593	3154	1422	1593	1425	1609	1398	1398	1398
Flt Permitted	0.28	1.00	1.00	0.62	1.00	1.00	0.76	1.00	1.00	0.76	1.00	1.00
Satd. Flow (perm)	405	2954	1425	1040	3154	1422	1269	1425	1283	1398	1398	1398
Peak-hour factor, PHF	0.50	0.92	0.92	0.92	0.84	0.58	0.92	0.92	0.92	0.44	0.92	0.41
Adj. Flow (vph)	76	210	8	25	645	126	172	0	246	118	0	68
RTOR Reduction (vph)	0	0	6	0	0	89	0	0	107	0	0	30
Lane Group Flow (vph)	76	210	2	25	645	37	172	0	139	118	0	38
Conf. Peds. (#/hr)	1					1				9		
Heavy Vehicles (%)	17%	10%	2%	2%	3%	0%	2%	2%	2%	0%	2%	4%
Turn Type	Perm	NA	Perm	Perm	NA	Perm	Perm	Perm	Perm	Perm	Perm	Perm
Protected Phases	4		4	8	8	2	2	2	2	6	6	6
Permitted Phases	4		4	8	8	2	2	2	2	6	6	6
Actuated Green, G (s)	18.5	18.5	18.5	18.5	18.5	18.5	35.7	35.7	35.7	35.7	35.7	35.7
Effective Green, g (s)	18.5	18.5	18.5	18.5	18.5	18.5	35.7	35.7	35.7	35.7	35.7	35.7
Actuated g/C Ratio	0.29	0.29	0.29	0.29	0.29	0.29	0.56	0.56	0.56	0.56	0.56	0.56
Clearance Time (s)	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Lane Grp. Cap (vph)	118	864	417	304	923	416	716	804	724	789	789	789
v/s Ratio Prot	0.07											
v/s Ratio Perm	0.19	0.00	0.02	0.03	0.14	0.03	0.14	0.10	0.09	0.03	0.03	0.03
vic Ratio	0.64	0.24	0.01	0.08	0.70	0.09	0.24	0.17	0.16	0.05	0.05	0.05
Uniform Delay, d1	19.5	17.0	15.8	16.2	19.9	16.2	6.9	6.6	6.6	6.2	6.2	6.2
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	11.4	0.1	0.0	0.1	2.3	0.1	0.8	0.5	0.5	0.1	0.1	0.1
Delay (s)	30.9	17.2	15.8	16.3	22.2	16.3	7.7	7.1	7.1	6.3	6.3	6.3
Level of Service	C	B	B	B	C	B	A	A	A	A	A	A
Approach Delay (s)	20.7			21.1			7.4			6.8		
Approach LOS	C			C			A			A		
Intersection Summary												
HCM 2000 Control Delay										B		
HCM 2000 Volume to Capacity ratio										9.0		
Actuated Cycle Length (s)												
Intersection Capacity Utilization										A		
Analysis Period (min)										15		
C Critical Lane Group												

Lanes, Volumes, Timings  
3: Trafalgar Road & QEW EB Off Ramp

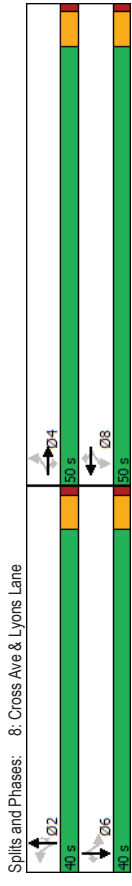
2025 Background AM Peak Hour.syn  
05-04-2022

Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	907	661	0	1314	2397	0
Traffic Volume (vph)	907	661	0	1314	2397	0
Future Volume (vph)	1900	1900	1900	1900	1900	1900
Ideal Flow (vphpl)	0.97	1.00	1.00	0.91	0.91	1.00
Lane Util. Factor	0.950					
Flt Protected	3400	1599	0	4893	4988	0
Satd. Flow (prot)	0.950					
Flt Permitted	3400	1599	0	4893	4988	0
Satd. Flow (perm)	Yes					Yes
Right Turn on Red	1					
Satd. Flow (RTOR)	50		50		50	
Link Speed (k/h)	71.3		50.7		186.0	
Link Distance (m)	5.1		3.7		13.4	
Travel Time (s)	0.92		0.92		0.92	
Peak Hour Factor	3%	1%	0%	6%	4%	0%
Heavy Vehicles (%)	986	718	0	1428	2605	0
Adj. Flow (vph)						
Shared Lane Traffic (%)						
Lane Group Flow (vph)	986	718	0	1428	2605	0
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Left	Left	Right
Median Width (m)	7.2		0.0		0.0	
Link Offset (m)	0.0		0.0		0.0	
Crosswalk Width (m)	4.8		4.8		4.8	
Two way Left Turn Lane	1.00	1.00	1.00	1.00	1.00	1.00
Headway Factor	25	15	25	2	2	15
Turning Speed (k/h)	1					
Number of Detectors	Left	Right	Thru	Thru	Thru	
Detector Template	2.0	2.0	10.0	10.0	10.0	
Leading Detector (m)	0.0	0.0	0.0	0.0	0.0	
Trailing Detector (m)	0.0	0.0	0.0	0.0	0.0	
Detector 1 Position (m)	2.0	2.0	0.6	0.6	0.6	
Detector 1 Size (m)	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	
Detector 1 Type	0.0	0.0	0.0	0.0	0.0	
Detector 1 Channel	0.0	0.0	0.0	0.0	0.0	
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	
Detector 2 Position (m)	9.4	9.4	0.6	0.6	0.6	
Detector 2 Size (m)	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	
Detector 2 Type	0.0	0.0	0.0	0.0	0.0	
Detector 2 Channel	0.0	0.0	0.0	0.0	0.0	
Detector 2 Extend (s)	0.0	0.0	0.0	0.0	0.0	
Turn Type	Perm	Perm	NA	NA	NA	
Protected Phases	4	4	2	2	6	
Permitted Phases	4	4	2	2	6	
Switch Phase	4	4	2	2	6	



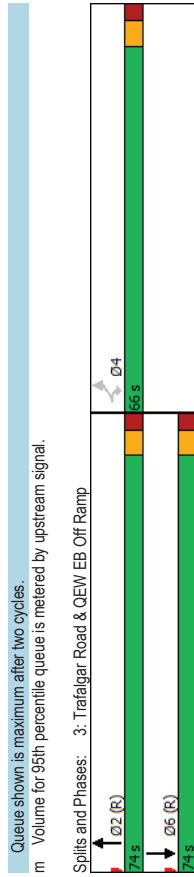
Analysis Period (min) 15

Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Minimum Initial (s)	10.0	10.0	29.0	29.0	29.0	29.0
Minimum Split (s)	38.0	38.0	36.0	36.0	36.0	36.0
Total Split (s)	66.0	66.0	74.0	74.0	74.0	74.0
Total Split (%)	47.1%	47.1%	52.9%	52.9%	52.9%	52.9%
Maximum Green (s)	59.0	59.0	67.0	67.0	67.0	67.0
Yellow Time (s)	4.0	4.0	4.0	4.0	4.0	4.0
All-Red Time (s)	3.0	3.0	3.0	3.0	3.0	3.0
Lost Time Adjust (s)	-3.0	-3.0	-3.0	-3.0	-3.0	-3.0
Total Lost Time (s)	4.0	4.0	4.0	4.0	4.0	4.0
Lead/Lag						
Lead-Lag Optimize?						
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	None	None	C-Max	C-Max	C-Max	C-Max
Walk Time (s)	7.0	7.0	7.0	7.0	7.0	7.0
Flash Dont Walk (s)	11.0	11.0	11.0	11.0	11.0	11.0
Pedestrian Calls (#/hr)	0	0	0	0	0	0
Act Effct Green (s)	62.0	62.0	70.0	70.0	70.0	70.0
Actuated g/C Ratio	0.44	0.44	0.50	0.50	0.50	0.50
v/c Ratio	0.66	1.01	0.58	1.04	0.58	1.04
Control Delay	33.2	76.0	46.4	52.4	46.4	52.4
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	33.2	76.0	46.4	52.4	46.4	52.4
LOS	C	E	D	D	D	D
Approach Delay	51.2		46.4	52.4	46.4	52.4
Approach LOS	D		D	D	D	D
Queue Length 50th (m)	114.5	~213.6	158.3	~300.0	158.3	~300.0
Queue Length 95th (m)	138.3	#299.6	174.9	~366.4	174.9	~366.4
Internal Link Dist (m)	47.3		26.7	162.0	26.7	162.0
Turn Bay Length (m)						
Base Capacity (vph)	1505	708	2446	2494	2446	2494
Starvation Cap Reductn	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	0.66	1.01	0.58	1.04	0.58	1.04
<b>Intersection Summary</b>						
Area Type:	Other					
Cycle Length:	140					
Actuated Cycle Length:	140					
Offset:	0 (0%). Referenced to phase 2:NBT and 6:SBT, Start of Green					
Natural Cycle:	90					
Control Type:	Actuated-Coordinated					
Maximum v/c Ratio:	1.04					
Intersection Signal Delay:	50.6 Intersection LOS: D					
Intersection Capacity Utilization:	118.3% ICU Level of Service H					
Analysis Period (min)	15					
~ Volume exceeds capacity, queue is theoretically infinite.						
Queue shown is maximum after two cycles.						
# 95th percentile volume exceeds capacity, queue may be longer.						



Splits and Phases: 8: Cross Ave & Lyons Lane

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Perm	NA	Perm	NA	Perm	NA	Perm	Perm	Perm	Perm	Perm	Perm	Perm
Protected Phases	4	4	4	8	8	8	2	2	2	2	6	6
Detector Phase	4	4	4	8	8	8	2	2	2	2	6	6
Switch Phase												
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Minimum Split (s)	22.5	22.5	22.5	22.5	22.5	22.5	22.5	22.5	22.5	22.5	22.5	22.5
Total Split (s)	50.0	50.0	50.0	50.0	50.0	50.0	40.0	40.0	40.0	40.0	40.0	40.0
Total Split (%)	55.6%	55.6%	55.6%	55.6%	55.6%	55.6%	44.4%	44.4%	44.4%	44.4%	44.4%	44.4%
Maximum Green (s)	45.5	45.5	45.5	45.5	45.5	45.5	35.5	35.5	35.5	35.5	35.5	35.5
Yellow Time (s)	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5
LeadLag												
Lead-Lag Optimize?												
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	None	None	None	None	None	None	Max	Max	Max	Max	Max	Max
Walk Time (s)	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0
Flash Dont Walk (s)	11.0	11.0	11.0	11.0	11.0	11.0	11.0	11.0	11.0	11.0	11.0	11.0
Pedestrian Calls (#/hr)	0	0	0	0	0	0	0	0	0	0	0	0
Act Effct Green (s)	18.5	18.5	18.5	18.5	18.5	18.5	35.7	35.7	35.7	35.7	35.7	35.7
Actuated g/C Ratio	0.29	0.29	0.29	0.29	0.29	0.29	0.56	0.56	0.56	0.56	0.56	0.56
v/c Ratio	0.64	0.24	0.02	0.08	0.70	0.25	0.24	0.23	0.16	0.23	0.16	0.08
Control Delay	46.0	17.3	4.1	16.1	24.1	4.9	9.2	0.5	8.6	0.5	8.6	0.2
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	46.0	17.3	4.1	16.1	24.1	4.9	9.2	0.5	8.6	0.5	8.6	0.2
LOS	D	B	A	B	C	A	A	A	A	A	A	A
Approach Delay	24.4			20.8			4.1				5.5	
Approach LOS	C			C			A				A	
Queue Length 50th (m)	8.0	10.2	0.0	2.2	36.5	0.0	9.6	0.0	6.2	0.0	6.2	0.0
Queue Length 95th (m)	9.9	17.5	1.7	7.0	47.8	2.1	23.6	0.0	7.4	0.0	7.4	0.0
Internal Link Dist (m)	257.3			16.1			148.6				17.7	
Turn Bay Length (m)	60.0	60.0	60.0	60.0	60.0	60.0	60.0	60.0	60.0	60.0	60.0	60.0
Base Capacity (vph)	292	2136	1035	751	2280	1062	716	1083	720	877	720	877
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.26	0.10	0.01	0.03	0.28	0.12	0.24	0.23	0.16	0.23	0.16	0.08



HCM Signalized Intersection Capacity Analysis  
 3: Trafalgar Road & QEW EB Off Ramp  
 2025 Background AM Peak Hour.syn  
 05-04-2022

Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	EBL	EBR	NBL	NBT	SBT	SBR
Traffic Volume (vph)	907	661	0	1314	2397	0
Future Volume (vph)	907	661	0	1314	2397	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0
Lane Util. Factor	0.97	1.00	0.91	1.00	0.91	1.00
Fr	1.00	0.85	1.00	1.00	1.00	1.00
Flt Protected	0.95	1.00	1.00	1.00	1.00	1.00
Satd. Flow (prot)	3400	1599	4893	4988	4988	4988
Flt Permitted	0.95	1.00	1.00	1.00	1.00	1.00
Satd. Flow (perm)	3400	1599	4893	4988	4988	4988
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92
RTOR Reduction (vph)	986	718	0	1428	2605	0
Adj. Flow (vph)	0	1	0	0	0	0
Lane Group Flow (vph)	986	717	0	1428	2605	0
Heavy Vehicles (%)	3%	1%	0%	6%	4%	0%
Perm	Perm	Perm	NA	NA	NA	NA
Protected Phases	4	4	2	2	6	6
Permitted Phases	59.0	59.0	67.0	67.0	67.0	67.0
Actuated Green, G (s)	62.0	62.0	70.0	70.0	70.0	70.0
Effective Green, g (s)	0.44	0.44	0.50	0.50	0.50	0.50
Actuated g/C Ratio	7.0	7.0	7.0	7.0	7.0	7.0
Clearance Time (s)	3.0	3.0	3.0	3.0	3.0	3.0
Vehicle Extension (s)	1505	708	2446	2494	2494	2494
Lane Grp Cap (vph)	v/s Ratio	Perm	0.29	c0.52		
v/s Ratio Perm	0.29	c0.45				
v/c Ratio	0.86	1.01	0.58	1.04	1.04	1.04
Uniform Delay, d1	30.6	39.0	24.7	35.0	35.0	35.0
Progression Factor	1.00	1.00	1.84	0.89	0.89	0.89
Incremental Delay, d2	1.0	37.2	0.7	21.5	21.5	21.5
Delay (s)	C	E	D	D	D	D
Level of Service	50.4	46.1	52.5	52.5	52.5	52.5
Approach Delay (s)	D	D	D	D	D	D
Approach LOS	D	D	D	D	D	D
Intersection Summary	HCM 2000 Level of Service D					
HCM 2000 Control Delay	11.0					
HCM 2000 Volume to Capacity ratio	11.0					
Actuated Cycle Length (s)	11.0					
Intersection Capacity Utilization	118.3%					
Analysis Period (min)	15					
c Critical Lane Group	15					

Lanes, Volumes, Timings  
 8: Cross Ave & Lyons Lane  
 2030 Total PM Peak Hour.syn  
 05-04-2022

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Traffic Volume (vph)	38	193	7	23	542	73	158	0	226	52	0	28
Future Volume (vph)	38	193	7	23	542	73	158	0	226	52	0	28
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	60.0	60.0	60.0	60.0	60.0	60.0	60.0	60.0	60.0	60.0	60.0	60.0
Storage Length (m)	1	1	1	1	1	1	1	1	1	1	1	1
Lane Util. Factor	7.5	7.5	7.5	7.5	7.5	7.5	7.5	7.5	7.5	7.5	7.5	7.5
Fr	1.00	0.95	1.00	0.95	1.00	0.98	1.00	1.00	1.00	1.00	1.00	1.00
Flt Protected	0.950	0.850	0.850	0.850	0.850	0.850	0.850	0.850	0.850	0.850	0.850	0.850
Satd. Flow (prot)	1388	2954	1425	1593	3154	1454	1593	1676	1425	1624	1676	1398
Flt Permitted	0.277	0.620	0.620	0.620	0.620	0.620	0.620	0.620	0.620	0.620	0.620	0.620
Satd. Flow (perm)	405	2954	1425	1039	3154	1421	1269	1676	1425	1277	1676	1398
Right Turn on Red	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Satd. Flow (RTOR)	18	18	18	18	18	18	18	18	18	18	18	18
Link Speed (k/h)	50	50	50	50	50	50	50	50	50	50	50	50
Link Distance (m)	281.3	281.3	281.3	40.1	40.1	172.6	172.6	172.6	172.6	172.6	172.6	172.6
Travel Time (s)	20.3	20.3	20.3	2.9	2.9	12.4	12.4	12.4	12.4	12.4	12.4	12.4
Conf. Peds. (#/hr)	1	1	1	1	1	1	1	1	1	1	1	1
Peak Hour Factor	0.50	0.92	0.92	0.84	0.58	0.92	0.92	0.92	0.92	0.44	0.92	0.41
Heavy Vehicles (%)	17%	10%	2%	2%	3%	0%	2%	2%	2%	0%	2%	4%
Adj. Flow (vph)	76	210	8	25	645	126	172	0	246	118	0	68
Shared Lane Traffic (%)	76	210	8	25	645	126	172	0	246	118	0	68
Lane Group Flow (vph)	No	No	No	No	No	No	No	No	No	No	No	No
Enter Blocked Intersec	Left	Right	Left	Right	Left	Right	Left	Right	Left	Right	Left	Right
Lane Alignment	3.6	3.6	3.6	3.6	3.6	3.6	3.6	3.6	3.6	3.6	3.6	3.6
Median Width (m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Link Offset (m)	4.8	4.8	4.8	4.8	4.8	4.8	4.8	4.8	4.8	4.8	4.8	4.8
Crosswalk Width (m)	1.14	1.14	1.14	1.14	1.14	1.14	1.14	1.14	1.14	1.14	1.14	1.14
Headway Factor	25	15	25	15	25	15	25	15	25	15	25	15
Turning Speed (k/h)	1	2	1	2	1	2	1	2	1	2	1	2
Number of Detectors	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Detector Template	2.0	10.0	2.0	2.0	10.0	2.0	2.0	10.0	2.0	2.0	10.0	2.0
Leading Detector (m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Trailing Detector (m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Position (m)	2.0	0.6	2.0	2.0	0.6	2.0	2.0	0.6	2.0	2.0	0.6	2.0
Detector 1 Size (m)	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex
Detector 1 Type	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Channel	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 2 Position (m)	9.4	9.4	9.4	9.4	9.4	9.4	9.4	9.4	9.4	9.4	9.4	9.4
Detector 2 Size (m)	0.6	0.6	0.6	0.6	0.6	0.6	0.6	0.6	0.6	0.6	0.6	0.6
Detector 2 Type	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex
Detector 2 Channel	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 2 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0

HCM Signalized Intersection Capacity Analysis  
 7: GO Bus Terminal/Argus Rd & Cross Ave

2025 Background AM Peak Hour Syn  
 4: Trafalgar Road & Argus Road/QEW EB On Ramp

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	29	1027	44	117	469	76	92	2	106	203	21	191
Traffic Volume (vph)	29	1027	44	117	469	76	92	2	106	203	21	191
Future Volume (vph)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Ideal Flow (vphpl)	3.3	3.6	3.6	3.3	3.6	3.6	3.3	3.6	3.6	3.3	3.6	3.6
Lane Width	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Total Lost time (s)	1.00	0.95	1.00	1.00	0.95	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Lane Util. Factor	1.00	1.00	0.97	1.00	1.00	0.96	1.00	1.00	0.96	1.00	0.97	1.00
Frpb, ped/bikes	0.99	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.97	1.00
Flt Protected	1.00	1.00	0.85	1.00	1.00	0.85	1.00	1.00	0.85	1.00	0.85	1.00
Satd. Flow (prot)	1554	3249	706	797	3249	1395	783	855	699	1528	1308	1308
Flt Permitted	0.47	1.00	1.00	0.09	1.00	1.00	0.30	1.00	1.00	0.75	1.00	1.00
Satd. Flow (perm)	771	3249	706	75	3249	1395	250	855	699	1210	1308	1308
Peak-hour factor, PHF	0.66	0.81	0.75	0.94	0.95	0.81	0.70	0.25	0.67	0.86	0.75	0.78
Adj. Flow (vph)	44	1288	59	124	494	94	131	8	158	236	28	245
RTOR Reduction (vph)	0	0	35	0	0	40	0	0	105	0	189	0
Lane Group Flow (vph)	44	1288	24	124	494	54	131	8	53	236	84	0
Conf. Peds. (#/hr)	9	4	4	4	4	9	12	20	20	20	20	12
Heavy Vehicles (%)	0%	0%	100%	97%	0%	0%	0%	100%	100%	0%	100%	0%
Turn Type	Perm	NA	Perm	pm-pt	NA	Perm	pm-pt	NA	Perm	Perm	NA	NA
Protected Phases	2	2	2	6	6	6	8	8	8	8	4	4
Permitted Phases	40.5	40.5	40.5	57.5	57.5	57.5	35.0	35.0	35.0	35.0	22.0	22.0
Actuated Green, G (s)	42.5	42.5	42.5	59.5	59.5	59.5	37.0	37.0	35.0	24.0	24.0	24.0
Effective Green, g (s)	0.41	0.41	0.41	0.55	0.57	0.57	0.35	0.35	0.33	0.23	0.23	0.23
Actuated g/C Ratio	6.0	6.0	6.0	4.0	4.0	4.0	4.5	4.5	6.0	6.0	6.0	6.0
Clearance Time (s)	5.0	5.0	5.0	2.5	5.0	5.0	3.0	4.0	4.0	4.0	4.0	4.0
Vehicle Extension (s)	313	1321	287	131	1849	794	142	302	234	277	300	300
Lane Grp Cap (vph)	c0.39	c0.12	0.15	c0.09	0.01	c0.23	0.08	0.20	0.08	0.20	0.06	0.06
v/s Ratio Prot	0.06	0.03	0.40	0.04	0.04	0.04	0.04	0.04	0.08	0.20	0.08	0.20
v/c Ratio	0.14	0.96	0.08	0.95	0.27	0.07	0.92	0.03	0.23	0.85	0.28	0.28
Uniform Delay, d1	19.5	30.2	19.0	30.5	11.4	10.1	30.0	22.0	25.0	38.5	33.1	33.1
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	0.4	16.3	0.3	61.7	0.2	0.1	52.5	0.0	0.7	22.2	0.7	0.7
Delay (s)	19.9	46.5	19.3	92.2	11.6	10.2	82.5	22.1	25.7	60.7	33.8	33.8
Level of Service	B	D	B	F	B	B	F	C	C	E	C	C
Approach Delay (s)	44.4	D	25.4	C	50.7	D	46.3	D	D	D	D	D
Approach LOS	D	D	C	C	D	D	D	D	D	D	D	D
Intersection Summary	HCM 2000 Control Delay											
HCM 2000 Control Delay	40.7 HCM 2000 Level of Service											
HCM 2000 Volume to Capacity ratio	0.97											
Actuated Cycle Length (s)	104.5 Sum of lost time (s)											
Intersection Capacity Utilization	78.2% ICU Level of Service											
Analysis Period (min)	15											
c Critical Lane Group	D											

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	0	0	18	0	0	0	0	1314	623	0	2301	757
Traffic Volume (vph)	0	0	18	0	0	0	0	1314	623	0	2301	757
Future Volume (vph)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Ideal Flow (vphpl)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Storage Length (m)	0	0	0	0	0	0	0	0	0	0	0	0
Storage Lanes	0	0	1	0	0	0	0	0	1	0	0	0
Taper Length (m)	7.5	7.5	7.5	7.5	7.5	7.5	7.5	7.5	7.5	7.5	7.5	7.5
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.91	1.00	1.00	0.91	0.91
Ped Bike Factor	0.865	0.865	0.865	0.865	0.865	0.865	0.865	0.865	0.865	0.865	0.865	0.865
Fit Protected	0	0	1522	0	0	0	0	5085	1568	0	4921	0
Satd. Flow (prot)	0	0	1522	0	0	0	0	5085	1568	0	4921	0
Flt Permitted	0	0	1522	0	0	0	0	5085	1568	0	4921	0
Link Speed (k/h)	50	50	50	50	50	50	50	50	50	50	50	50
Link Distance (m)	165.2	165.2	165.2	196.1	196.1	196.1	271.0	271.0	271.0	271.0	50.7	50.7
Travel Time (s)	11.9	11.9	11.9	14.1	14.1	14.1	19.5	19.5	19.5	19.5	3.7	3.7
Conf. Peds. (#/hr)	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	0%	0%	8%	0%	0%	0%	0%	2%	3%	0%	2%	0%
Adj. Flow (vph)	0	0	20	0	0	0	0	1428	677	0	2501	823
Shared Lane Traffic (%)	0	0	20	0	0	0	0	1428	677	0	3324	0
Lane Group Flow (vph)	0	0	20	0	0	0	0	1428	677	0	3324	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	3.6	3.6	0.0	3.6	3.6
Link Offset(m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Crosswalk Width(m)	4.8	4.8	4.8	4.8	4.8	4.8	4.8	4.8	4.8	4.8	4.8	4.8
Two way Left Turn Lane	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Headway Factor	25	25	25	25	25	25	25	25	25	25	25	25
Turning Speed (k/h)	15	15	15	15	15	15	15	15	15	15	15	15
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
Intersection Summary	Other											
Area Type:	Unsignalized											
Control Type:	ICU Level of Service C											
Intersection Capacity Utilization	71.9%											
Analysis Period (min)	15											

HCM Unsignalized Intersection Capacity Analysis  
 4: Trafalgar Road & Argus Road/QEW EB On Ramp

Lanes, Volumes, Timings  
 7: GO Bus Terminal/Argus Rd & Cross Ave

2025 Background AM Peak Hour.syn  
 05-04-2022

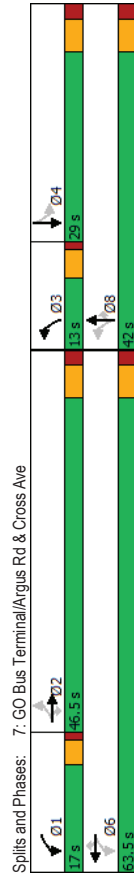
2030 Total PM Peak Hour.syn  
 05-04-2022

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	0	0	18	0	0	0	0	1314	623	0	2301	757
Traffic Volume (veh/h)	0	0	18	0	0	0	0	1314	623	0	2301	757
Future Volume (Veh/h)	0	0	18	0	0	0	0	1314	623	0	2301	757
Sign Control	Stop	Stop	Stop	0%	0%	0%	Free	0%	0%	Free	0%	0%
Grade	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Peak Hour Factor	0	0	20	0	0	0	0	1428	677	0	2501	823
Hourly flow rate (vph)	0	0	20	0	0	0	0	1428	677	0	2501	823
Pedestrians	24											
Lane Width (m)	3.6											
Walking Speed (m/s)	1.2											
Percent Blockage	2											
Right turn flare (veh)												
Median type							None					None
Median storage (veh)												
Upstream signal (m)							271					51
pX, platoon unblocked	0.58	0.58	0.50	0.58	0.58	0.86	0.50			0.86		0.86
vC, conflicting volume	3412	4364	1269	2262	4776	4776	3348			1428		1428
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	923	2575	0	0	3289	0	2215			910		910
tC, single (s)	7.5	6.5	7.1	7.5	6.5	6.9	4.1			4.1		4.1
tC, 2 stage (s)												
tF (s)	3.5	4.0	3.4	3.5	4.0	3.3	2.2			2.2		2.2
p0 queue free %	100	100	96	100	100	100	100			100		100
cM capacity (veh/h)	127	15	526	561	5	934	118			648		648
Direction_Lane #	EB1	NB1	NB2	NB3	NB4	SB1	SB2	SB3	SB.3			
Volume Total	20	476	476	476	677	1000	1000	1323				
Volume Left	0	0	0	0	0	0	0	0				
Volume Right	20	0	0	0	677	0	0	823				
GSH	526	1700	1700	1700	1700	1700	1700	1700				
Volume to Capacity	0.04	0.28	0.28	0.28	0.40	0.59	0.59	0.78				
Queue Length 95th (m)	0.9	0.0	0.0	0.0	0.0	0.0	0.0	0.0				
Control Delay (s)	12.1	0.0	0.0	0.0	0.0	0.0	0.0	0.0				
Lane LOS	B											
Approach Delay (s)	12.1	0.0				0.0						
Approach LOS	B											
Intersection Summary												
Average Delay	0.0											
Intersection Capacity Utilization	71.9%											
ICU Level of Service	C											
Analysis Period (min)	15											

Baseline

Syncho 10 Report  
 Page 12

Intersection Capacity Utilization 78.2%  
 Analysis Period (min) 15  
 ICU Level of Service D  
 # 95th percentile volume exceeds capacity, queue may be longer.  
 Queue shown is maximum after two cycles.



Splits and Phases: 7: GO Bus Terminal/Argus Rd & Cross Ave

Baseline

Syncho 10 Report  
 Page 23

Lanes, Volumes, Timings  
7: GO Bus Terminal/Argus Rd & Cross Ave

Lanes, Volumes, Timings  
5: Trafalgar Road & Cross Avenue/South Service Road

2030 Total PM Peak Hour.syn  
05-04-2022

2025 Background AM Peak Hour.syn  
05-04-2022

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Detector 2 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Turn Type	Perm	NA	Perm	pm-pt	NA	Perm	pm-pt	NA	Perm	Perm	NA	NA
Protected Phases	2	2	2	1	6	6	3	8	8	4	4	4
Permitted Phases	2	2	2	6	6	6	8	8	8	4	4	4
Detector Phase	2	2	2	1	6	6	3	8	8	4	4	4
Switch Phase												
Minimum Initial (s)	22.0	22.0	22.0	8.0	22.0	22.0	5.0	10.0	10.0	10.0	10.0	10.0
Minimum Split (s)	45.0	45.0	45.0	12.5	29.0	29.0	9.5	29.0	29.0	29.0	29.0	29.0
Total Split (s)	46.5	46.5	46.5	17.0	63.5	63.5	13.0	42.0	42.0	29.0	29.0	29.0
Total Split (%)	44.1%	44.1%	44.1%	16.1%	60.2%	60.2%	12.3%	39.8%	39.8%	27.5%	27.5%	27.5%
Maximum Green (s)	40.5	40.5	40.5	13.0	57.5	57.5	8.5	36.0	36.0	23.0	23.0	23.0
Yellow Time (s)	4.0	4.0	4.0	3.0	4.0	4.0	3.5	4.0	4.0	4.0	4.0	4.0
All-Red Time (s)	2.0	2.0	2.0	1.0	2.0	2.0	1.0	2.0	2.0	2.0	2.0	2.0
Lost Time Adjust (s)	-2.0	-2.0	-2.0	0.0	-2.0	-2.0	-2.0	0.0	0.0	-2.0	-2.0	-2.0
Total Lost Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	2.5	4.0	6.0	4.0	4.0	4.0
Lead/Lag	Lag	Lag	Lag	Lead	Lead	Lead	Lead	Lag	Lag	Lag	Lag	Lag
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Vehicle Extension (s)	5.0	5.0	5.0	2.5	5.0	5.0	3.0	4.0	4.0	4.0	4.0	4.0
Recall Mode	Min	Min	Min	Min	Min	Min	None	Min	Min	Min	Min	Min
Walk Time (s)	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0
Flash Dont Walk (s)	16.0	16.0	16.0	16.0	16.0	16.0	16.0	16.0	16.0	16.0	16.0	16.0
Pedestrian Calls (#/hr)	0	0	0	0	0	0	0	0	0	0	0	0
Act Effct Green (s)	42.5	42.5	42.5	59.5	59.5	59.5	38.5	37.0	35.0	24.0	24.0	24.0
Actuated g/C Ratio	0.41	0.41	0.41	0.57	0.57	0.57	0.37	0.35	0.33	0.23	0.23	0.23
v/c Ratio	0.14	0.96	0.17	0.94	0.27	0.11	0.90	0.03	0.47	0.85	0.56	0.56
Control Delay	21.6	47.9	1.0	92.2	12.1	2.6	83.4	22.5	9.1	66.7	11.2	11.2
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	21.6	47.9	1.0	92.2	12.1	2.6	83.4	22.5	9.1	66.7	11.2	11.2
LOS	C	D	A	F	B	A	F	C	A	E	B	B
Approach Delay	45.0											36.9
Approach LOS	D											D
Queue Length 50th (m)	5.9	138.7	0.0	18.9	27.2	0.0	21.0	1.1	0.0	48.6	4.7	4.7
Queue Length 95th (m)	9.9	145.2	0.0	#57.0	37.1	5.3	#34.8	1.3	4.3	#84.3	13.9	13.9
Internal Link Dist (m)	350.0											156.7
Turn Bay Length (m)	20.0	60.0	20.0	60.0	185.1	63.5	145	310	344	289	499	499
Base Capacity (vph)	313	1321	357	132	1851	835	145	310	344	289	499	499
Stallcap Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.14	0.96	0.17	0.94	0.27	0.11	0.90	0.03	0.46	0.82	0.55	0.55
Intersection Summary												
Area Type:	CBD											
Cycle Length:	105.5											
Actuated Cycle Length:	104.5											
Natural Cycle:	100											
Control Type:	Semi Act-Uncoord											
Maximum v/c Ratio:	0.96											
Intersection Signal Delay:	38.3											
Intersection LOS:	D											

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔
Traffic Volume (vph)	433	90	125	47	93	134	125	1302	59	287	1487	461
Future Volume (vph)	433	90	125	47	93	134	125	1302	59	287	1487	461
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (m)	25.0	0.0	0.0	25.0	0.0	0.0	50.0	0.0	0.0	25.0	0.0	0.0
Storage Lanes	2	0	0	1	1	1	1	1	0	1	1	0
Taper Length (m)	7.5	1.00	1.00	7.5	1.00	1.00	7.5	1.00	0.91	1.00	7.5	0.91
Lane Util. Factor	0.97	1.00	1.00	1.00	1.00	1.00	1.00	0.99	0.99	1.00	0.99	0.99
Ped Bike Factor	1.00	0.99	0.99	0.99	0.99	0.99	0.99	0.99	0.99	0.99	0.99	0.99
Fit	0.913					0.850		0.994			0.965	
Fit Protected	0.950					0.950		0.950			0.950	
Satd. Flow (prot)	3213	1585	0	1752	1759	1553	1641	4965	0	1752	4809	0
Flow Permitted	0.950			0.613		0.072		0.072		0.069		
Satd. Flow (perm)	3206	1565	0	1123	1759	1531	124	4965	0	127	4809	0
Right Turn on Red	Yes			Yes		Yes		Yes		Yes		Yes
Satd. Flow (RTOR)	46			183		6		6		76		76
Link Speed (k/h)	50			50		50		50		50		50
Link Distance (m)	243.7			238.9		17.2		266.5		271.0		271.0
Travel Time (s)	17.5			17.2		19.2		19.2		19.5		19.5
Confl. Peds. (#/hr)	1			4		4		10		52		10
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	9%	12%	3%	8%	4%	10%	3%	0%	3%	3%	0%	4%
Adj. Flow (vph)	471	98	136	51	101	146	136	1415	64	312	1616	501
Shared Lane Traffic (%)												
Lane Group Flow (vph)	471	234	0	51	101	146	136	1479	0	312	2117	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Left	Left	Right	Right
Median Width (m)	7.2			7.2		7.2		3.6		3.6		3.6
Link Offset (m)	0.0			0.0		0.0		0.0		0.0		0.0
Crosswalk Width (m)	4.8			4.8		4.8		4.8		4.8		4.8
Two way Left Turn Lane												
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (k/h)	25	15	25	15	25	15	25	15	25	15	25	15
Number of Detectors	1	2	1	2	1	1	1	2	1	2	1	2
Detector Template	Left	Thru	Left	Thru	Right	Thru	Left	Thru	Left	Thru	Left	Thru
Leading Detector (m)	2.0	10.0	2.0	10.0	2.0	2.0	10.0	10.0	2.0	10.0	2.0	10.0
Trailing Detector (m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Position (m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Size (m)	2.0	0.6	2.0	0.6	2.0	0.6	2.0	0.6	2.0	0.6	2.0	0.6
Detector 1 Type	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 2 Position (m)	9.4			9.4		9.4		9.4		9.4		9.4
Detector 2 Size (m)	0.6			0.6		0.6		0.6		0.6		0.6
Detector 2 Type	Ch+Ex			Ch+Ex		Ch+Ex		Ch+Ex		Ch+Ex		Ch+Ex
Detector 2 Channel												
Detector 2 Extend (s)	0.0			0.0		0.0		0.0		0.0		0.0





HCM Signalized Intersection Capacity Analysis  
6: Trafalgar Road

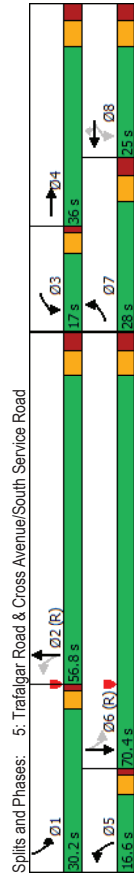
2030 Total PM Peak Hour.syn  
05-04-2022

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations	559	657	213	59	894	728	152	488	43	584	687	564	
Traffic Volume (vph)	559	657	213	59	894	728	152	488	43	584	687	564	
Future Volume (vph)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	
Ideal Flow (vphpl)	4.0	2.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	
Total Lost time (s)	0.97	0.95	1.00	0.95	1.00	1.00	0.95	1.00	0.97	1.00	1.00	1.00	
Lane Util. Factor	1.00	0.98	1.00	1.00	0.94	1.00	1.00	0.97	1.00	1.00	0.95	1.00	
Frb, ped/bikes	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Frb, ped/bikes	1.00	0.96	1.00	1.00	0.85	1.00	1.00	0.85	1.00	1.00	0.85	1.00	
Frt	0.95	1.00	0.95	1.00	1.00	0.95	1.00	0.95	1.00	0.95	1.00	1.00	
Flt Protected	3467	3331	1802	3574	1503	1770	3610	1559	3433	1900	1503	1503	
Satd. Flow (prot)	0.95	1.00	0.21	1.00	1.00	0.12	1.00	1.00	0.95	1.00	1.00	1.00	
Flt Permitted	3467	3331	391	3574	1503	219	3610	1559	3433	1900	1503	1503	
Satd. Flow (perm)	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	
Peak-hour factor, PHF	608	714	232	64	972	791	165	530	47	635	747	613	
Adj. Flow (vph)	0	22	0	0	216	0	0	35	0	190	0	190	
RTOR Reduction (vph)	608	924	0	64	972	575	165	530	12	635	747	423	
Lane Group Flow (vph)	21	14	14	14	21	17	10	10	10	10	10	17	
Conf. Peds. (#/hr)	1%	3%	1%	0%	1%	1%	2%	0%	2%	0%	2%	0%	
Heavy Vehicles (%)	Prot	NA	pm+pt	NA	pm	pm+pt	NA	pm	pt	NA	pm	NA	
Turn Type	7	4	3	8	5	2	2	1	1	6	6	6	
Protected Phases	Permitted Phases	8	8	2	2	2	2	2	2	2	2	2	
Actuated Green, G (s)	21.0	57.4	45.6	40.0	40.0	33.0	33.0	33.0	22.0	48.0	48.0	48.0	
Effective Green, g (s)	22.0	60.4	47.6	43.0	43.0	36.0	36.0	36.0	23.0	51.0	51.0	51.0	
Actuated g/C Ratio	0.16	0.43	0.34	0.31	0.31	0.30	0.26	0.26	0.16	0.36	0.36	0.36	
Clearance Time (s)	5.0	5.0	5.0	7.0	7.0	5.0	7.0	7.0	5.0	7.0	7.0	7.0	
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	
Lane Grp Cap (vph)	544	1437	199	1087	461	154	928	400	583	692	547	547	
v/s Ratio Prot	c0.18	0.28	0.02	0.27	0.06	0.15	0.15	0.15	c0.18	c0.39	c0.39	c0.39	
v/s Ratio Perm	1.12	0.64	0.32	0.89	1.25	1.07	0.57	0.03	1.13	1.08	0.77	0.77	
Uniform Delay, d1	59.0	31.3	32.3	46.2	48.5	44.4	45.3	38.9	58.5	44.5	39.4	39.4	
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Incremental Delay, d2	75.1	1.0	0.9	8.8	128.2	92.8	2.5	0.1	74.7	54.2	8.2	8.2	
Delay (s)	134.1	32.3	33.2	55.0	176.7	137.2	47.8	39.1	151.8	102.1	69.2	69.2	
Level of Service	F	C	C	D	F	F	D	D	D	F	F	E	
Approach Delay (s)	72.1	72.1	106.9	67.1	67.1	67.1	67.1	67.1	67.1	107.8	107.8	107.8	
Approach LOS	E	E	F	E	E	E	E	E	E	F	F	F	
Intersection Summary													
HCM 2000 Control Delay	93.5											HCM 2000 Level of Service	F
HCM 2000 Volume to Capacity ratio	1.17												
Actuated Cycle Length (s)	140.0											Sum of lost time (s)	16.0
Intersection Capacity Utilization	98.6%											ICU Level of Service	F
Analysis Period (min)	15												
c Critical Lane Group													

Lanes, Volumes, Timings  
5: Trafalgar Road & Cross Avenue/South Service Road

2025 Background AM Peak Hour.syn  
05-04-2022

Intersection Capacity Utilization 81.0% ICU Level of Service D  
Analysis Period (min) 15  
# 95th percentile volume exceeds capacity, queue may be longer.  
Queue shown is maximum after two cycles.  
m Volume for 95th percentile queue is metered by upstream signal.



HCM Signalized Intersection Capacity Analysis  
 5: Trafalgar Road & Cross Avenue/South Service Road

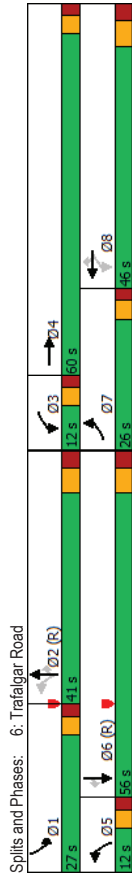
2025 Background AM Peak Hour.syn  
 05-04-2022

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations	←	←	←	←	←	←	←	←	←	←	←	←	
Traffic Volume (vph)	433	90	125	47	93	134	125	1302	59	287	1487	461	
Future Volume (vph)	433	90	125	47	93	134	125	1302	59	287	1487	461	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	
Total Lost time (s)	4.0	4.0	1.5	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	
Lane Util. Factor	0.97	1.00	1.00	1.00	1.00	1.00	1.00	0.91	1.00	0.91	1.00	0.91	
Frbp, ped/bikes	1.00	0.99	1.00	1.00	0.99	1.00	1.00	0.99	1.00	0.99	1.00	0.99	
Frbp, ped/bikes	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Frt	1.00	0.91	1.00	1.00	0.85	1.00	0.99	1.00	0.99	1.00	0.96	1.00	
Flt Protected	0.95	1.00	0.95	1.00	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.95	
Satd. Flow (prot)	3213	1565	1745	1759	1531	1641	4962	1752	4806	1752	4806	1752	
Flt Permitted	0.95	1.00	0.61	1.00	1.00	0.07	1.00	1.00	0.07	1.00	0.07	1.00	
Satd. Flow (perm)	3213	1565	1126	1759	1531	124	4962	127	4806	127	4806	127	
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	
Adj. Flow (vph)	471	98	136	51	101	146	136	1415	64	312	1616	501	
RTOR Reduction (vph)	0	36	0	0	0	128	0	3	0	0	37	0	
Lane Group Flow (vph)	471	198	0	51	101	18	136	1476	0	312	2080	0	
Conf. Peds. (#/hr)	1	1	4	4	4	1	10	52	52	52	10	10	
Heavy Vehicles (%)	9%	6%	12%	3%	8%	4%	10%	3%	0%	3%	3%	4%	
Turn Type	Prot	NA	pm+pt	NA	Perm	pm+pt	NA	pm+pt	NA	pm+pt	NA	NA	
Protected Phases	7	4		3	8		5	2		1	6		
Permitted Phases				8		8	2			6			
Actuated Green, G (s)	20.8	28.2	24.1	14.5	14.5	67.2	55.8	83.7	68.3	83.7	68.3	83.7	
Effective Green, g (s)	23.8	31.2	30.1	17.5	17.5	67.2	58.8	83.7	71.3	83.7	71.3	83.7	
Actuated g/C Ratio	0.17	0.22	0.22	0.12	0.12	0.48	0.42	0.60	0.51	0.60	0.51	0.60	
Clearance Time (s)	7.0	7.0	4.5	7.0	7.0	4.0	7.0	4.0	7.0	4.0	7.0	4.0	
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	
Lane Grp. Cap (vph)	546	348	297	219	191	183	2084	353	2447	353	2447	353	
v/s Ratio Prot	c0.15	c0.13	0.02	0.06	0.06	0.30	c0.15	c0.43	c0.15	c0.43	c0.15	c0.43	
v/s Ratio Perm	0.86	0.57	0.17	0.46	0.10	0.74	0.71	0.88	0.85	0.88	0.85	0.85	
Uniform Delay, d1	56.5	48.4	44.4	56.9	54.2	30.9	33.5	42.7	29.7	42.7	29.7	42.7	
Progression Factor	1.00	1.00	1.00	1.00	1.00	0.88	1.22	1.21	0.63	1.21	0.63	1.21	
Incremental Delay, d2	13.2	2.1	0.3	1.5	0.2	8.6	1.1	2.7	0.4	2.7	0.4	2.7	
Delay (s)	69.7	50.6	44.7	58.4	54.5	35.8	41.9	54.3	19.0	54.3	19.0	54.3	
Level of Service	E	D	D	E	D	D	D	D	D	D	D	B	
Approach Delay (s)	63.4			54.1			41.4			41.4		23.6	
Approach LOS	E			D			D			D		C	
<b>Intersection Summary</b>													
HCM 2000 Control Delay	36.6											HCM 2000 Level of Service	D
HCM 2000 Volume to Capacity ratio	0.85												
Actuated Cycle Length (s)	140.0											Sum of lost time (s)	16.0
Intersection Capacity Utilization	81.0%											ICU Level of Service	D
Analysis Period (min)	15												
c Critical Lane Group													

Lanes, Volumes, Timings  
 6: Trafalgar Road

2030 Total PM Peak Hour.syn  
 05-04-2022

Intersection Capacity Utilization 98.6% ICU Level of Service F  
 Analysis Period (min) 15  
 ~ Volume exceeds capacity, queue is theoretically infinite.  
 Queue shown is maximum after two cycles.  
 # 95th percentile volume exceeds capacity, queue may be longer.  
 Queue shown is maximum after two cycles.







HCM Signalized Intersection Capacity Analysis  
 5: Trafalgar Road & Cross Avenue/South Service Road

2030 Total PM Peak Hour.syn  
 05-04-2022

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔
Traffic Volume (vph)	1150	250	172	226	245	351	195	1461	120	154	1140	460
Future Volume (vph)	1150	250	172	226	245	351	195	1461	120	154	1140	460
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	7.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lane Util. Factor	0.97	0.95	1.00	0.97	0.95	1.00	1.00	0.91	1.00	0.91	1.00	0.91
Frpb, ped/bikes	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.87	1.00	1.00
Flpb, ped/bikes	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt	1.00	1.00	0.85	1.00	1.00	0.85	1.00	1.00	0.85	1.00	1.00	0.85
Flt Protected	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	0.95	1.00	0.95	1.00
Satd. Flow (prot)	3400	3343	1413	3433	3471	1599	1719	5136	1409	1805	5136	1485
Flt Permitted	0.95	1.00	1.00	0.95	1.00	1.00	0.10	1.00	1.00	0.10	1.00	1.00
Satd. Flow (perm)	3400	3343	1413	3433	3471	1599	173	5136	1409	199	5136	1485
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	1250	272	187	246	266	382	212	1588	130	167	1239	500
RTOR Reduction (vph)	0	0	120	0	0	94	0	70	0	70	0	239
Lane Group Flow (vph)	1250	272	67	246	266	288	212	1588	60	167	1239	261
Conf. Peds. (#/hr)	15	15	15	15	15	15	18	18	70	70	70	18
Heavy Vehicles (%)	3%	8%	9%	2%	4%	1%	5%	1%	0%	0%	1%	3%
Turn Type	Prot	NA	Perm	Prot	NA	pm+ov	pm+pt	NA	pm+ov	pm+pt	NA	Perm
Protected Phases	7	4		3	8	1	5	2	3	1	6	
Permitted Phases			4			8	2		2	6		6
Actuated Green, G (s)	46.0	50.0	50.0	14.6	15.6	27.2	57.0	41.8	56.4	49.8	38.2	38.2
Effective Green, g (s)	49.0	53.0	50.0	14.6	18.6	27.2	57.0	44.8	56.4	49.8	41.2	41.2
Actuated g/C Ratio	0.35	0.38	0.36	0.10	0.13	0.19	0.41	0.32	0.40	0.36	0.29	0.29
Clearance Time (s)	7.0	7.0	4.0	7.0	4.0	7.0	4.0	7.0	4.0	7.0	4.0	7.0
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Lane Grp Cap (vph)	1190	1285	504	358	461	310	238	1643	567	203	1511	437
v/s Ratio Prot	c0.37	0.08		0.07	0.08	c0.08	c0.10	c0.31	0.01	0.07	0.24	
v/s Ratio Perm			0.05			0.10	0.27		0.03	0.22		0.18
v/c Ratio	1.05	0.22	0.13	0.69	0.58	0.93	0.89	0.97	0.11	0.82	0.82	0.60
Uniform Delay, d1	45.5	29.4	30.4	60.5	57.0	55.4	38.4	46.9	26.1	36.6	46.0	42.3
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.39	1.04	2.85	1.03	0.76	0.60
Incremental Delay, d2	40.4	0.1	0.1	5.4	1.8	32.6	10.4	5.7	0.0	14.6	3.1	3.5
Delay (s)	85.9	29.5	30.5	65.9	58.8	88.0	63.6	54.4	74.5	52.5	38.1	29.0
Level of Service	F	C	C	E	E	F	E	D	E	D	D	C
Approach Delay (s)		70.9			73.2			56.7				37.0
Approach LOS		E			E			E				D
Intersection Summary												
HCM 2000 Control Delay												E
HCM 2000 Volume to Capacity ratio	56.9											
HCM 2000 Volume to Capacity ratio	0.98											
Actuated Cycle Length (s)	140.0											16.0
Intersection Capacity Utilization	92.8%											F
Analysis Period (min)	15											
c Critical Lane Group												

Lanes, Volumes, Timings  
 6: Trafalgar Road

2025 Background AM Peak Hour.syn  
 05-04-2022

Intersection Capacity Utilization	81.0%
Analysis Period (min)	15
ICU Level of Service D	
~ Volume exceeds capacity, queue is theoretically infinite.	
Queue shown is maximum after two cycles.	
# 95th percentile volume exceeds capacity, queue may be longer.	
Queue shown is maximum after two cycles.	
m Volume for 95th percentile queue is metered by upstream signal.	



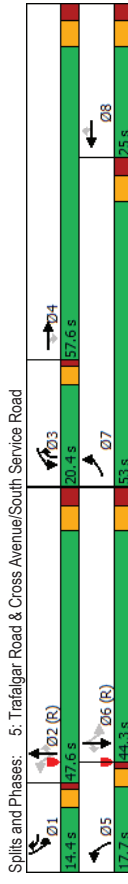
HCM Signalized Intersection Capacity Analysis  
 6: Trafalgar Road

2025 Background AM Peak Hour.syn  
 05-04-2022

Lanes, Volumes, Timings  
 5: Trafalgar Road & Cross Avenue/South Service Road  
 2030 Total PM Peak Hour.syn  
 05-04-2022

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations	←	←	←	←	←	←	←	←	←	←	←	←	
Traffic Volume (vph)	404	541	84	21	443	653	56	372	53	600	552	374	
Future Volume (vph)	404	541	84	21	443	653	56	372	53	600	552	374	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	
Total Lost time (s)	4.0	2.0	2.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	
Lane Util. Factor	0.97	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	0.97	1.00	1.00	
Frb, ped/bikes	1.00	1.00	0.95	1.00	1.00	0.93	1.00	0.99	1.00	1.00	1.00	0.97	
Frb, ped/bikes	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Frt	1.00	1.00	0.85	1.00	1.00	0.85	1.00	0.98	1.00	1.00	1.00	0.85	
Flt Protected	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	0.95	1.00	1.00	1.00	
Satd. Flow (prot)	3433	3471	1524	1703	3505	1460	1770	3488	3367	1863	1474	1474	
Flt Permitted	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	0.95	1.00	1.00	1.00	
Satd. Flow (perm)	3433	3471	1524	1703	3505	1460	1770	3488	3367	1863	1474	1474	
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	
Adj. Flow (vph)	439	588	91	23	482	710	61	404	58	662	600	407	
RTOR Reduction (vph)	0	0	53	0	0	286	0	8	0	0	0	193	
Lane Group Flow (vph)	439	588	38	23	482	424	61	454	0	652	600	214	
Conf. Peds. (#/hr)	25	25	7	7	7	25	9	9	18	18	9	9	
Heavy Vehicles (%)	2%	4%	1%	6%	3%	3%	2%	1%	0%	4%	2%	6%	
Turn Type	Prot	NA	Perm	Prot	NA	Perm	Prot	NA	Prot	NA	Perm	Prot	
Protected Phases	7	4		3	3	8	5	2	1		6		
Permitted Phases			4			8					6		
Actuated Green, G (s)	18.0	54.8	54.8	4.2	39.0	39.0	5.6	32.0	27.0	53.4	53.4	53.4	
Effective Green, g (s)	19.0	57.8	57.8	5.2	42.0	42.0	6.6	35.0	28.0	56.4	56.4	56.4	
Actuated g/C Ratio	0.14	0.41	0.41	0.04	0.30	0.30	0.05	0.25	0.20	0.40	0.40	0.40	
Clearance Time (s)	5.0	5.0	5.0	5.0	7.0	7.0	5.0	7.0	5.0	7.0	7.0	7.0	
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	
Lane Grp Cap (vph)	465	1433	629	63	1051	438	83	872	673	750	593	593	
v/s Ratio Prot	c0.13	0.17		0.01	0.14		0.03	0.13	c0.19	c0.32			
v/s Ratio Perm			0.02			c0.29					0.14		
v/c Ratio	0.94	0.41	0.06	0.37	0.46	0.97	0.73	0.52	0.97	0.80	0.36	0.36	
Uniform Delay, d1	60.0	29.1	24.7	65.8	39.8	48.3	65.8	45.3	55.6	36.8	29.2	29.2	
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.47	0.92	1.61	1.61	
Incremental Delay, d2	28.0	0.2	0.0	3.6	0.3	34.2	28.3	2.2	18.9	5.2	1.0	1.0	
Delay (s)	88.0	29.2	24.8	69.4	40.1	82.6	94.1	47.5	100.4	38.9	47.9	47.9	
Level of Service	F	C	C	E	D	F	F	D	F	D	D	D	
Approach Delay (s)		51.9		65.5		52.9		65.2		65.2		65.2	
Approach LOS		D		E		D		E		D		E	
Intersection Summary													
HCM 2000 Control Delay	60.6											HCM 2000 Level of Service	E
HCM 2000 Volume to Capacity ratio	0.93												
Actuated Cycle Length (s)	140.0											Sum of lost time (s)	16.0
Intersection Capacity Utilization	81.0%											ICU Level of Service	D
Analysis Period (min)	15												
c Critical Lane Group													

Intersection Capacity Utilization	92.8%
Analysis Period (min)	15
ICU Level of Service	F
Volume exceeds capacity, queue is theoretically infinite.	
Queue shown is maximum after two cycles.	
# 95th percentile volume exceeds capacity, queue may be longer.	
Queue shown is maximum after two cycles.	
m Volume for 95th percentile queue is metered by upstream signal.	







Lanes, Volumes, Timings  
7: GO Bus Terminal/Argus Rd & Cross Ave

Lanes, Volumes, Timings  
5: Trafalgar Road & Cross Avenue/South Service Road

2025 Background AM Peak Hour.syn  
05-04-2022

2030 Total PM Peak Hour.syn  
05-04-2022

	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Detector 2 Extend (s)	0.0			0.0			10.0			10.0		0.0
Turn Type	Perm	NA	pm-pt	NA	Perm	NA	Perm	NA	NA	Perm	NA	NA
Protected Phases	2	2	1	6	8	8	8	8	8	8	4	4
Permitted Phases	2	2	6	6	6	6	6	6	6	6	4	4
Detector Phase	2	2	1	6	6	6	6	6	6	6	4	4
Switch Phase												
Minimum Initial (s)	22.0	22.0	8.0	22.0	22.0	22.0	10.0	10.0	10.0	10.0	10.0	10.0
Minimum Split (s)	45.0	45.0	12.5	29.0	29.0	29.0	29.0	29.0	29.0	29.0	29.0	29.0
Total Split (s)	45.4	45.4	12.6	58.0	58.0	58.0	32.0	32.0	32.0	32.0	32.0	32.0
Total Split (%)	50.4%	50.4%	14.0%	64.4%	64.4%	64.4%	35.6%	35.6%	35.6%	35.6%	35.6%	35.6%
Maximum Green (s)	39.4	39.4	8.6	52.0	52.0	52.0	26.0	26.0	26.0	26.0	26.0	26.0
Yellow Time (s)	4.0	4.0	3.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
All-Red Time (s)	2.0	2.0	1.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
Lost Time Adjust (s)	-2.0	-2.0	0.0	-2.0	-2.0	-2.0	-2.0	-2.0	-2.0	-2.0	-2.0	-2.0
Total Lost Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lead/Lag	Lag	Lag	Lead									
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Vehicle Extension (s)	5.0	5.0	2.5	5.0	5.0	5.0	4.0	4.0	4.0	4.0	4.0	4.0
Recall Mode	Min	Min	Min	Min	Min	Min	Min	Min	Min	Min	Min	Min
Walk Time (s)	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0
Flash Dont Walk (s)	16.0	16.0	16.0	16.0	16.0	16.0	16.0	16.0	16.0	16.0	16.0	16.0
Pedestrian Calls (#/hr)	0	0	0	0	0	0	0	0	0	0	0	0
Act Effct Green (s)	24.6	24.6	36.8	28.0	28.0	28.0	28.0	28.0	28.0	28.0	28.0	28.0
Actuated g/C Ratio	0.34	0.34	0.51	0.51	0.38	0.38	0.38	0.38	0.38	0.38	0.38	0.38
v/c Ratio	0.30	0.47	0.24	0.46	0.87	0.15	0.18	0.18	0.18	0.18	0.30	0.30
Control Delay	22.7	20.4	12.5	12.5	130.2	0.6	16.6	16.6	16.6	16.6	33.0	33.0
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	22.7	20.4	12.5	12.5	130.2	0.6	16.6	16.6	16.6	16.6	33.0	33.0
LOS	C	C	B	B	F	A	A	A	A	B	B	C
Approach Delay	20.7			12.5			46.5				31.2	
Approach LOS	C			B			D				C	
Queue Length 50th (m)	6.6	27.7	3.5	33.1	5.1	0.0	6.9	51.6			6.9	51.6
Queue Length 95th (m)	8.7	39.5	8.3	44.8	11.7	0.0	14.4	44.0			14.4	44.0
Internal Link Dist (m)				219.7			57.9				156.7	
Turn Bay Length (m)	20.0			20.0			46	502			15.0	
Base Capacity (vph)	349	1736	207	2369			434	677			677	
Stevation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.18	0.28	0.24	0.31	0.87	0.15	0.18	0.18	0.18	0.18	0.30	0.30
Intersection Summary	CBD											
Area Type:	CBD											
Cycle Length:	90											
Actuated Cycle Length:	72.8											
Natural Cycle:	100											
Control Type:	Semi Act-Uncoordinated											
Maximum v/c Ratio:	0.90											
Intersection Signal Delay:	22.4											
Intersection LOS:	C											
Baseline	Synchro 10 Report Page 22											

HCM Unsignalized Intersection Capacity Analysis  
 4: Trafalgar Road & Argus Road/QEW EB On Ramp

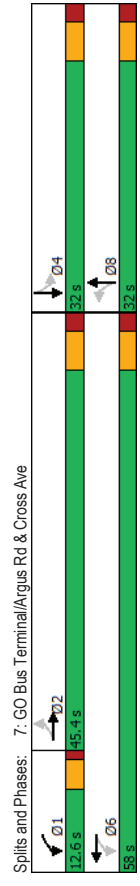
2030 Total PM Peak Hour.syn  
 05-04-2022

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	0	0	60	0	0	0	0	2037	924	0	1984	484
Future Volume (Veh/h)	0	0	60	0	0	0	0	2037	924	0	1984	484
Sign Control	Stop											
Grade	0%											
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	0	0	65	0	0	0	0	2214	1004	0	2157	526
Pedestrians	24											
Lane Width (m)	3.6											
Walking Speed (m/s)	1.2											
Percent Blockage	2											
Right turn flare (veh)	None											
Median type	None											
Median storage (veh)	None											
Upstream signal (m)	271											
pX, platoon unblocked	0.78	0.78	0.66	0.78	0.78	0.76	0.66			0.76		
vC, conflicting volume	3182	4658	1006	2933	4921	738	2707			2214		
vC1, stage 1 conf vol												
vC2, stage 2 conf vol	741	2633	0	421	2970	0	1785			1496		
vCu, unblocked vol	7.5	6.5	7.1	7.5	6.5	6.9	4.1			4.1		
tC, single (s)												
tC, 2 stage (s)	3.5	4.0	3.4	3.5	4.0	3.3	2.2			2.2		
tF (s)	100	100	91	100	100	100	100			100		
p0 queue free %	232	18	689	362	11	830	228			346		
cM capacity (veh/h)												
Direction_Lane #	EB 1	NB 1	NB 2	NB 3	NB 4	SB 1	SB 2	SB 3	SB 3			
Volume Total	65	738	738	738	1004	863	863	957				
Volume Left	0	0	0	0	0	0	0	0				
Volume Right	65	0	0	0	1004	0	0	526				
GSH	689	1700	1700	1700	1700	1700	1700	1700				
Volume to Capacity	0.09	0.43	0.43	0.43	0.59	0.51	0.51	0.56				
Queue Length 95th (m)	2.5	0.0	0.0	0.0	0.0	0.0	0.0	0.0				
Control Delay (s)	10.8	0.0	0.0	0.0	0.0	0.0	0.0	0.0				
Lane LOS	B	B	B	B	B	B	B	B				
Approach Delay (s)	10.8	0.0				0.0						
Approach LOS	B	B				B						
Intersection Summary												
Average Delay	0.1											
Intersection Capacity Utilization	60.5% ICU Level of Service											
Analysis Period (min)	15											

Lanes, Volumes, Timings  
 7: GO Bus Terminal/Argus Rd & Cross Ave

2025 Background AM Peak Hour.syn  
 05-04-2022

Intersection Capacity Utilization 81.6%  
 Analysis Period (min) 15  
 # 95th percentile volume exceeds capacity, queue may be longer.  
 Queue shown is maximum after two cycles.



HCM Signalized Intersection Capacity Analysis  
 7: GO Bus Terminal/Argus Rd & Cross Ave

Lanes, Volumes, Timings  
 4: Trafalgar Road & Argus Road/QEW EB On Ramp

2025 Background AM Peak Hour.syn  
 05-04-2022

2030 Total PM Peak Hour.syn  
 05-04-2022

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	33	400	15	41	621	24	21	0	51	59	17	521
Traffic Volume (vph)	33	400	15	41	621	24	21	0	51	59	17	521
Future Volume (vph)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Ideal Flow (vphpl)	3.3	3.6	3.6	3.3	3.6	3.6	3.3	3.6	3.6	3.3	3.6	3.6
Lane Width	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Total Lost time (s)	1.00	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00	0.99	1.00
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	1.00	1.00	0.97	1.00
Frpb, ped/bikes	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Flt	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00
Flt Protected	1569	3048	817	3189	805	722	1525	1387				
Satd. Flow (prot)	0.37	1.00	0.33	1.00	0.33	1.00	0.14	1.00	0.71	1.00		
Flt Permitted	615	3048	288	3189	121	722	1139	1387				
Satd. Flow (perm)	0.52	0.87	0.65	0.84	0.88	0.79	0.53	0.25	0.70	0.78	0.62	0.89
Peak-hour factor, PHF	63	460	23	49	706	30	40	0	73	76	27	585
Adj. Flow (vph)	0	5	0	0	4	0	0	45	0	0	144	0
RTOR Reduction (vph)	63	478	0	49	732	0	40	28	0	76	468	0
Lane Group Flow (vph)	1	3	3	1	3	1	3	20	20	20	3	0
Confl. Peds. (#/hr)	0%	1%	100%	92%	1%	5%	95%	0%	91%	0%	93%	0%
Heavy Vehicles (%)	Perm	NA	NA	pm-pt	NA	Perm	NA	NA	NA	Perm	NA	NA
Turn Type	2			1	6		8		8		4	
Protected Phases	2			6			8		8		4	
Permitted Phases	22.6	22.6	34.8	34.8	34.8	26.0	26.0	26.0	26.0	26.0	26.0	26.0
Actuated Green, G (s)	24.6	24.6	34.8	36.8	36.8	28.0	28.0	28.0	28.0	28.0	28.0	28.0
Effective Green, g (s)	0.34	0.34	0.48	0.51	0.51	0.38	0.38	0.38	0.38	0.38	0.38	0.38
Actuated g/C Ratio	6.0	6.0	4.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0
Clearance Time (s)	5.0	5.0	2.5	5.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Vehicle Extension (s)	207	1029	197	1612	46	277	438	533				
Lane Grp Cap (vph)	0.16	0.16	0.03	0.23	0.04	0.04			0.34		0.34	
v/s Ratio Prot	0.10	0.10	0.09	0.09	0.33	0.33			0.07		0.07	
v/s Ratio Perm	0.30	0.46	0.25	0.45	0.87	0.10	0.17	0.88			0.88	
Uniform Delay, d1	17.8	18.9	11.0	11.6	20.7	14.3	14.8	20.8			20.8	
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00			1.00	
Incremental Delay, d2	1.7	0.7	0.5	0.4	85.4	0.2	0.3	15.5			15.5	
Level of Service	B	B	B	B	F	B	B	D			B	
Delay (s)	19.5	19.6	11.5	12.0	106.1	14.6	15.0	36.3			36.3	
Approach Delay (s)	19.6			12.0			47.0		34.0			
Approach LOS	B			B			D		C			

Baseline  
 Synchro 10 Report  
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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	0	0	60	0	0	0	0	0	2037	924	0	1984
Traffic Volume (vph)	0	0	60	0	0	0	0	0	2037	924	0	1984
Future Volume (vph)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Ideal Flow (vphpl)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	70.0	0.0	0.0	0.0
Storage Length (m)	0	0	1	0	0	0	0	0	1	0	0	0
Storage Lanes	7.5	7.5	7.5	7.5	7.5	7.5	7.5	7.5	7.5	7.5	7.5	7.5
Taper Length (m)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.91
Lane Util. Factor	1.00	1.00	0.865	1.00	1.00	1.00	1.00	1.00	0.91	1.00	1.00	0.91
Ped Bike Factor	0	0	0	0	0	0	0	0	0	0	0	0
Flt Protected	0	0	1522	0	0	0	0	0	5085	1568	0	4957
Satd. Flow (prot)	0	0	1522	0	0	0	0	0	5085	1568	0	4957
Flt Permitted	50	50	165.2	50	50	196.1	50	50	271.0	19.5	50.7	50
Link Speed (k/h)	11.9	11.9	14.1	11.9	11.9	14.1	11.9	11.9	19.5	3.7	11.9	11.9
Link Distance (m)	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Travel Time (s)	0%	0%	8%	0%	2%	0%	0%	0%	2%	3%	0%	2%
Confl. Peds. (#/hr)	0	0	65	0	0	0	0	0	2214	1004	0	2157
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	0%	0%	8%	0%	2%	0%	0%	0%	2%	3%	0%	2%
Adj. Flow (vph)	0	0	65	0	0	0	0	0	2214	1004	0	2157
Shared Lane Traffic (%)	0	0	65	0	0	0	0	0	2214	1004	0	2683
Lane Group Flow (vph)	0	0	65	0	0	0	0	0	2214	1004	0	2683
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	3.6	0.0	3.6	0.0
Link Offset(m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Crosswalk Width(m)	4.8	4.8	4.8	4.8	4.8	4.8	4.8	4.8	4.8	4.8	4.8	4.8
Two way Left Turn Lane	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Headway Factor	25	25	15	25	25	15	25	25	15	25	15	25
Turning Speed (k/h)	Stop	Stop	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free
Sign Control	Intersection Summary	Intersection Summary	Intersection Summary	Intersection Summary	Intersection Summary	Intersection Summary	Intersection Summary	Intersection Summary	Intersection Summary	Intersection Summary	Intersection Summary	Intersection Summary
Area Type:	Other	Other	Other	Other	Other	Other	Other	Other	Other	Other	Other	Other
Control Type:	Unsignalized	Unsignalized	Unsignalized	Unsignalized	Unsignalized	Unsignalized	Unsignalized	Unsignalized	Unsignalized	Unsignalized	Unsignalized	Unsignalized
Intersection Capacity Utilization	60.5%	60.5%	60.5%	60.5%	60.5%	60.5%	60.5%	60.5%	60.5%	60.5%	60.5%	60.5%
Analysis Period (min)	15	15	15	15	15	15	15	15	15	15	15	15
ICU Level of Service	B	B	B	B	B	B	B	B	B	B	B	B

Baseline  
 Synchro 10 Report  
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HCM Signalized Intersection Capacity Analysis  
3: Trafalgar Road & QEW EB Off Ramp

2030 Total PM Peak Hour.syn  
05-04-2022



Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	↑	↑	↑↑	↑↑	↑↑	↑↑
Traffic Volume (vph)	1090	561	0	2037	1907	0
Future Volume (vph)	1090	561	0	2037	1907	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0
Lane Util. Factor	0.97	1.00	0.91	0.91	0.91	1.00
Frbp, ped/bikes	1.00	0.98	1.00	1.00	1.00	1.00
Fltb, ped/bikes	1.00	1.00	1.00	1.00	1.00	1.00
Flt	1.00	0.85	1.00	1.00	1.00	1.00
Flt Protected	0.95	1.00	1.00	1.00	1.00	1.00
Satd. Flow (prot)	3502	1573	5085	5085	5085	0
Flt Permitted	0.95	1.00	1.00	1.00	1.00	1.00
Satd. Flow (perm)	3502	1573	5085	5085	5085	0
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	1185	610	0	2214	2073	0
RTOR Reduction (vph)	0	2	0	0	0	0
Lane Group Flow (vph)	1185	608	0	2214	2073	0
Conf. Peds. (#/hr)	2	2	2	2	2	2
Heavy Vehicles (%)	0%	1%	0%	2%	2%	0%
Turn Type	Perm	Perm	NA	2%	2%	0%
Protected Phases				NA	NA	
Permitted Phases	4	4	2	2	6	
Actuated Green, G (s)	57.5	57.5	68.5	68.5	68.5	
Effective Green, g (s)	60.5	60.5	71.5	71.5	71.5	
Actuated g/C Ratio	0.43	0.43	0.51	0.51	0.51	
Clearance Time (s)	7.0	7.0	7.0	7.0	7.0	
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	
Lane Grp. Cap (vph)	1513	679	2596	2596	2596	
v/s Ratio Prot			c0.44	0.41		
v/s Ratio Perm	0.34	c0.39				
vic Ratio	0.78	0.90	0.85	0.80	0.80	
Uniform Delay, d1	34.1	36.8	29.7	28.3	28.3	
Progression Factor	1.00	1.00	1.48	0.80	0.80	
Incremental Delay, d2	2.7	14.4	1.1	1.8	1.8	
Delay (s)	36.8	51.2	44.9	24.3	24.3	
Level of Service	D	D	D	C	C	
Approach Delay (s)	41.7	D	44.9	24.3	24.3	
Approach LOS	D	D	D	C	C	
Intersection Summary						
HCM 2000 Control Delay			36.9	HCM 2000 Level of Service	D	
HCM 2000 Volume to Capacity ratio			0.89			
Actuated Cycle Length (s)			140.0	Sum of lost time (s)	11.0	
Intersection Capacity Utilization			78.5%	ICU Level of Service	D	
Analysis Period (min)			15			
c Critical Lane Group						

Baseline

Synchro 10 Report  
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Lanes, Volumes, Timings  
8: Lyons Lane/Commercial Driveway & Cross Ave

2025 Background AM Peak Hour.syn  
05-04-2022



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑	↑	↑	↑	↑	↑	↑	↑	↑	↑	↑	↑
Traffic Volume (vph)	59	179	205	228	122	5	18	3	14	17	24	42
Future Volume (vph)	59	179	205	228	122	5	18	3	14	17	24	42
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (m)	3.3	3.6	3.6	3.3	3.6	3.3	3.6	3.6	3.6	3.3	3.6	3.6
Storage Length (m)	0.0	0.0	0.0	25.0	0.0	20.0	0.0	0.0	0.0	0.0	0.0	0.0
Storage Lanes	1	0	0	1	0	0	1	0	0	0	1	0
Taper Length (m)	7.5			7.5			7.5			7.5		7.5
Lane Util. Factor	1.00	0.95	0.95	1.00	0.95	0.95	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor	0.99	0.99	0.99	1.00	1.00	1.00	0.99	0.99	0.99	1.00	0.99	0.99
Flt	0.916			0.989			0.872				0.902	
Flt Protected	0.950			0.950			0.950				0.950	
Satd. Flow (prot)	1540	2804	0	1570	2726	0	1570	1471	0	1468	1497	0
Flt Permitted	0.647			0.395			0.708				0.740	
Satd. Flow (perm)	1040	2804	0	652	2726	0	1166	1471	0	1139	1497	0
Right Turn on Red		Yes			Yes			Yes			Yes	
Satd. Flow (RTOR)	281			12			23				49	
Link Speed (k/h)	50			50			50				50	
Link Distance (m)	40.1			374.0			69.1				70.9	
Travel Time (s)	2.9			26.9			5.0				5.1	
Conf. Peds. (#/hr)	5		1	1		5	3		3		3	
Peak Hour Factor	0.78	0.81	0.73	0.66	0.79	0.42	0.80	0.75	0.60	0.62	0.92	0.86
Heavy Vehicles (%)	2%	11%	0%	0%	19%	0%	0%	0%	0%	7%	0%	3%
Adj. Flow (vph)	76	221	281	345	154	12	23	4	23	27	26	49
Shared Lane Traffic (%)												
Lane Group Flow (vph)	76	502	0	345	166	0	23	27	0	27	75	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(m)	3.3			3.3			3.3			3.3		3.3
Link Offset(m)	0.0			0.0			0.0			0.0		0.0
Crosswalk Width(m)	4.8			4.8			4.8			4.8		4.8
Two way Left Turn Lane												
Headway Factor	1.19	1.14	1.14	1.19	1.14	1.14	1.19	1.14	1.14	1.19	1.14	1.14
Turning Speed (k/h)	24		14	24		14	24		14	24		24
Number of Detectors	1	2		1	2		1	2		1	2	
Detector Template	Left	Thru		Left	Thru		Left	Thru		Left	Thru	
Leading Detector (m)	2.0	10.0		2.0	10.0		2.0	10.0		2.0	10.0	
Trailing Detector (m)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Position(m)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Size(m)	2.0	0.6		2.0	0.6		2.0	0.6		2.0	0.6	
Detector 1 Type	Ch+Ex	Ch+Ex		Ch+Ex	Ch+Ex		Ch+Ex	Ch+Ex		Ch+Ex	Ch+Ex	
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Queue (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Delay (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 2 Position(m)												
Detector 2 Size(m)												
Detector 2 Type	Ch+Ex	Ch+Ex		Ch+Ex	Ch+Ex		Ch+Ex	Ch+Ex		Ch+Ex	Ch+Ex	
Detector 2 Channel												

Baseline

Synchro 10 Report  
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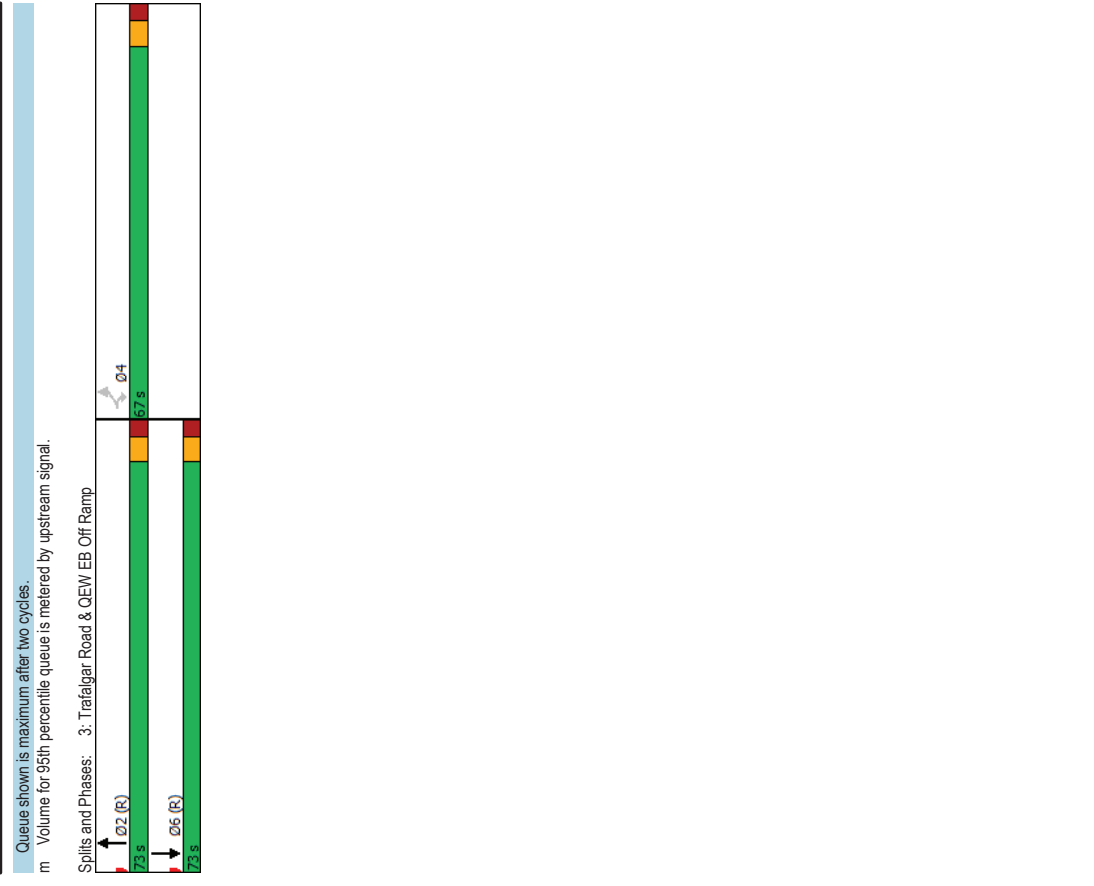
Lanes, Volumes, Timings  
8: Lyons Lane/Commercial Driveway & Cross Ave

Lanes, Volumes, Timings  
3: Trafalgar Road & QEW EB Off Ramp

2025 Background AM Peak Hour.syn  
05-04-2022

2030 Total PM Peak Hour.syn  
05-04-2022

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Detector 2 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Turn Type	Perm	NA	NA	pm-pt	NA	NA	Perm	NA	NA	Perm	NA	NA
Permitted Phases	2	2	6	1	6	8	8	8	4	4	4	4
Detector Phase	2	2	1	6	8	8	8	8	4	4	4	4
Switch Phase												
Minimum Initial (s)	35.0	35.0	8.0	35.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0
Minimum Split (s)	41.0	41.0	12.5	41.0	28.0	28.0	28.0	28.0	28.0	28.0	28.0	28.0
Total Split (s)	41.0	41.0	21.0	62.0	28.0	28.0	28.0	28.0	28.0	28.0	28.0	28.0
Total Split (%)	45.6%	45.6%	23.3%	68.9%	31.1%	31.1%	31.1%	31.1%	31.1%	31.1%	31.1%	31.1%
Maximum Green (s)	35.0	35.0	17.0	56.0	22.0	22.0	22.0	22.0	22.0	22.0	22.0	22.0
Yellow Time (s)	4.0	4.0	3.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
All-Red Time (s)	2.0	2.0	1.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
Lost Time Adjust (s)	-2.0	-2.0	0.0	-2.0	-2.0	-2.0	-2.0	-2.0	-2.0	-2.0	-2.0	-2.0
Total Lost Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lead/Lag	Lag	Lag	Lead									
Lead-Lag Optimize?	Yes	Yes	Yes									
Vehicle Extension (s)	5.0	5.0	2.5	5.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Recall Mode	Min	Min	Min	Min	Min	Min	Min	Min	Min	Min	Min	Min
Walk Time (s)	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0
Flash Dont Walk (s)	15.0	15.0	15.0	15.0	15.0	15.0	15.0	15.0	15.0	15.0	15.0	15.0
Pedestrian Calls (#/hr)	0	0	0	0	0	0	0	0	0	0	0	0
Act Effct Green (s)	37.0	37.0	51.9	12.3	12.3	12.3	12.3	12.3	12.3	12.3	12.3	12.3
Actuated g/C Ratio	0.51	0.51	0.72	0.17	0.17	0.17	0.17	0.17	0.17	0.17	0.17	0.17
v/c Ratio	0.14	0.32	0.57	0.08	0.12	0.10	0.14	0.25	0.14	0.25	0.14	0.25
Control Delay	11.1	5.2	7.6	2.9	28.1	14.1	28.5	15.6	28.5	15.6	28.5	15.6
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	11.1	5.2	7.6	2.9	28.1	14.1	28.5	15.6	28.5	15.6	28.5	15.6
LOS	B	A	A	A	C	B	C	B	C	B	C	B
Approach Delay	5.9	6.1	20.5									
Approach LOS	A	A	C									
Queue Length 50th (m)	5.2	8.0	13.8	2.6	2.8	0.5	3.3	3.2	3.3	3.2	3.3	3.2
Queue Length 95th (m)	12.0	15.0	16.4	4.6	8.4	5.4	7.2	14.7	7.2	14.7	7.2	14.7
Internal Link Dist (m)	16.1		350.0		45.1		46.9					
Turn Bay Length (m)	533	1575	685	2194	387	504	378	530	378	530	378	530
Base Capacity (vph)	0	0	0	0	0	0	0	0	0	0	0	0
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Spillover Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.14	0.32	0.50	0.08	0.06	0.05	0.07	0.14	0.06	0.05	0.07	0.14
<b>Intersection Summary</b>												
Area Type:	CBD											
Cycle Length:	90											
Actuated Cycle Length:	72.2											
Natural Cycle:	85											
Control Type:	Semi Act-Uncoord											
Maximum v/c Ratio:	0.57											
Intersection Signal Delay:	7.7											





Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Detector Phase	4	4		2	2	6
Switch Phase						
Minimum Initial (s)	10.0	10.0		29.0	29.0	29.0
Minimum Split (s)	38.0	38.0		36.0	36.0	36.0
Total Split (s)	67.0	67.0		73.0	73.0	73.0
Total Split (%)	47.9%	47.9%		52.1%	52.1%	52.1%
Maximum Green (s)	60.0	60.0		66.0	66.0	66.0
Yellow Time (s)	4.0	4.0		4.0	4.0	4.0
All-Red Time (s)	3.0	3.0		3.0	3.0	3.0
Lost Time Adjust (s)	-3.0	-3.0		-3.0	-3.0	-3.0
Total Lost Time (s)	4.0	4.0		4.0	4.0	4.0
Lead/Lag						
Lead-Lag Optimize?						
Vehicle Extension (s)	3.0	3.0		3.0	3.0	3.0
Recall Mode	None	None		C-Max	C-Max	C-Max
Walk Time (s)	7.0	7.0		7.0	7.0	7.0
Flash Dont Walk (s)	11.0	11.0		11.0	11.0	11.0
Pedestrian Calls (#/hr)	0	0		0	0	0
Act Effct Green (s)	60.5	60.5		71.5	71.5	71.5
Actuated g/C Ratio	0.43	0.43		0.51	0.51	0.51
v/c Ratio	0.78	0.90		0.85	0.80	0.80
Control Delay	38.2	53.6		46.1	24.9	24.9
Queue Delay	0.0	0.0		0.3	0.0	0.0
Total Delay	38.2	53.6		46.4	24.9	24.9
LOS	D	D		D	C	C
Approach Delay	43.5			46.4	24.9	
Approach LOS	D			D	C	
Queue Length 50th (m)	145.7	156.5		246.1	127.4	
Queue Length 95th (m)	173.4	#239.0		m242.3	179.3	
Internal Link Dist (m)	47.3			26.7	162.0	
Turn Bay Length (m)						
Base Capacity (vph)	1575	709		2597	2597	
Starvation Cap Reductn	0	0		0	0	
Spillback Cap Reductn	0	0		64	0	
Storage Cap Reductn	0	0		0	0	
Reduced v/c Ratio	0.75	0.86		0.87	0.80	
<b>Intersection Summary</b>						
Area Type:	Other					
Cycle Length:	140					
Actuated Cycle Length:	140					
Offset: Q (0%), Referenced to phase 2:NBT and 6:SBT, Start of Green						
Natural Cycle:	80					
Control Type:	Actuated-Coordinated					
Maximum v/c Ratio:	0.90					
Intersection Signal Delay:	38.2					Intersection LOS: D
Intersection Capacity Utilization	78.5%					ICU Level of Service D
Analysis Period (min)	15					
# 95th percentile volume exceeds capacity, queue may be longer.						



Lane Group	Ø1	Ø2	Ø3	Ø4	Ø5	Ø6
Detector Phase						
Minimum Initial (s)	21.5	41.5	25.5	25.5	62.5	62.5
Minimum Split (s)						
Total Split (s)						
Total Split (%)						
Maximum Green (s)						
Yellow Time (s)						
All-Red Time (s)						
Lost Time Adjust (s)						
Total Lost Time (s)						
Lead/Lag						
Lead-Lag Optimize?						
Vehicle Extension (s)						
Recall Mode						
Walk Time (s)						
Flash Dont Walk (s)						
Pedestrian Calls (#/hr)						
Act Effct Green (s)						
Actuated g/C Ratio						
v/c Ratio						
Control Delay						
Queue Delay						
Total Delay						
LOS						
Approach Delay						
Approach LOS						
Queue Length 50th (m)						
Queue Length 95th (m)						
Internal Link Dist (m)						
Turn Bay Length (m)						
Base Capacity (vph)						
Starvation Cap Reductn						
Spillback Cap Reductn						
Storage Cap Reductn						
Reduced v/c Ratio						
<b>Intersection Summary</b>						
Area Type:	Other					
Cycle Length:	140					
Actuated Cycle Length:	140					
Offset: Q (0%), Referenced to phase 2:NBT and 6:SBT, Start of Green						
Natural Cycle:	80					
Control Type:	Actuated-Coordinated					
Maximum v/c Ratio:	0.90					
Intersection Signal Delay:	38.2					Intersection LOS: D
Intersection Capacity Utilization	78.5%					ICU Level of Service D
Analysis Period (min)	15					
# 95th percentile volume exceeds capacity, queue may be longer.						

HCM Signalized Intersection Capacity Analysis  
 8: Lyons Lane/Commercial Driveway & Cross Ave

2025 Background AM Peak Hour.syn  
 05-04-2022

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	←	←	←	←	←	←	←	←	←	←	←	←
Traffic Volume (vph)	59	179	205	228	122	5	18	3	14	17	24	42
Future Volume (vph)	59	179	205	228	122	5	18	3	14	17	24	42
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width	3.3	3.6	3.6	3.3	3.6	3.6	3.3	3.6	3.6	3.3	3.6	3.6
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lane Util. Factor	1.00	0.95	1.00	1.00	0.95	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frbp, ped/bikes	1.00	0.99	1.00	1.00	1.00	1.00	0.99	1.00	1.00	1.00	0.99	1.00
Fipb, ped/bikes	0.99	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Flt	1.00	0.92	1.00	0.99	1.00	0.87	1.00	0.87	1.00	0.90	1.00	0.90
Flt Protected	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00
Satd. Flow (prot)	1530	2804	1570	2727	1566	1473	1566	1473	1463	1483	1498	1498
Flt Permitted	0.65	1.00	0.65	1.00	0.65	1.00	0.71	1.00	0.74	1.00	0.74	1.00
Satd. Flow (perm)	1042	2804	653	2727	1168	1473	1168	1473	1139	1498	1498	1498
Peak-hour factor, PHF	0.78	0.81	0.73	0.66	0.79	0.42	0.80	0.75	0.60	0.62	0.92	0.86
Adj. Flow (vph)	76	221	281	345	154	12	22	4	23	27	26	49
RTOR Reduction (vph)	0	137	0	0	3	0	0	19	0	0	41	0
Lane Group Flow (vph)	76	365	0	345	163	0	23	8	0	27	34	0
Conf. Peds. (#/hr)	5	1	1	0	1	5	3	3	3	3	3	3
Heavy Vehicles (%)	2%	11%	0%	0%	19%	0%	0%	0%	0%	7%	0%	3%
Turn Type	Perm	NA	NA	pm-pt	NA	Perm	NA	NA	Perm	NA	NA	NA
Permitted Phases	2	6	6	8	8	8	8	8	4	4	4	4
Actuated Green, G (s)	35.1	35.1	50.0	50.0	50.0	10.3	10.3	10.3	10.3	10.3	10.3	10.3
Effective Green, g (s)	37.1	37.1	50.0	52.0	12.3	12.3	12.3	12.3	12.3	12.3	12.3	12.3
Actuated g/C Ratio	0.51	0.51	0.69	0.72	0.17	0.17	0.17	0.17	0.17	0.17	0.17	0.17
Clearance Time (s)	6.0	6.0	4.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0
Vehicle Extension (s)	5.0	5.0	2.5	5.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lane Grp Cap (vph)	534	1438	589	1961	609	0.06	198	250	193	254	254	254
v/s Ratio Prot	0.13	0.13	0.09	0.06	0.02	0.01	0.01	0.01	0.02	0.02	0.02	0.02
v/s Ratio Perm	0.07	0.14	0.25	0.59	0.08	0.12	0.03	0.12	0.14	0.14	0.14	0.14
Uniform Delay, d1	9.2	9.9	4.9	3.0	25.4	25.0	25.4	25.0	25.5	25.5	25.5	25.5
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	0.3	0.2	1.2	0.0	0.4	0.1	0.1	0.1	0.5	0.3	0.3	0.3
Delay (s)	9.5	10.0	6.1	3.1	25.8	25.1	25.8	25.1	26.0	25.8	25.8	25.8
Level of Service	A	B	A	A	A	C	C	C	C	C	C	C
Approach Delay (s)	10.0	10.0	5.1	5.1	25.4	25.4	25.4	25.4	25.9	25.9	25.9	25.9
Approach LOS	A	A	A	A	C	C	C	C	C	C	C	C
Intersection Summary												
HCM 2000 Control Delay	9.9 HCM 2000 Level of Service A											
HCM 2000 Volume to Capacity ratio	0.50											
Actuated Cycle Length (s)	72.3 Sum of lost time (s) 12.0											
Intersection Capacity Utilization	77.6% ICU Level of Service D											
Analysis Period (min)	15											
c Critical Lane Group												

Lanes, Volumes, Timings  
 3: Trafalgar Road & QEW EB Off Ramp

2030 Total PM Peak Hour.syn  
 05-04-2022

Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	←	←	←	←	←	←
Traffic Volume (vph)	1090	561	0	2037	1907	0
Future Volume (vph)	1090	561	0	2037	1907	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	0.97	1.00	1.00	0.91	0.91	1.00
Ped Bike Factor	0.98					
Flt	0.850					
Flt Protected	0.950					
Satd. Flow (prot)	3502	1599	0	5085	5085	0
Flt Permitted	0.950					
Satd. Flow (perm)	3502	1573	0	5085	5085	0
Right Turn on Red	Yes					
Satd. Flow (RTOR)	3					
Link Speed (k/h)	50	50	50	50	50	50
Link Distance (m)	71.3	71.3	50.7	186.0	186.0	186.0
Travel Time (s)	5.1	5.1	3.7	13.4	13.4	13.4
Conf. Peds. (#/hr)	2					
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	0%	1%	0%	2%	2%	0%
Adj. Flow (vph)	1185	610	0	2214	2073	0
Shared Lane Traffic (%)						
Lane Group Flow (vph)	1185	610	0	2214	2073	0
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Left	Left	Right
Median Width (m)	7.2	7.2	0.0	0.0	0.0	0.0
Link Offset (m)	0.0	0.0	0.0	0.0	0.0	0.0
Crosswalk Width (m)	4.8	4.8	4.8	4.8	4.8	4.8
Two way Left Turn Lane						
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (k/h)	25	15	25	2	2	15
Number of Detectors	1	1	2	2	2	2
Detector Template	Left	Right	Thru	Thru	Thru	Thru
Leading Detector (m)	2.0	2.0	10.0	10.0	10.0	10.0
Trailing Detector (m)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Position (m)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Size (m)	2.0	2.0	0.6	0.6	0.6	0.6
Detector 1 Type	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex
Detector 1 Channel	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 2 Position (m)	0.0	0.0	9.4	9.4	9.4	9.4
Detector 2 Size (m)	0.6	0.6	0.6	0.6	0.6	0.6
Detector 2 Type	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex
Detector 2 Channel	0.0	0.0	0.0	0.0	0.0	0.0
Detector 2 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0
Turn Type	Perm	Perm	NA	NA	NA	NA
Permitted Phases	4	4	2	6	6	6

HCM Unsignalized Intersection Capacity Analysis  
 2: Trafalgar Road & QEW SB On Ramps

Lanes, Volumes, Timings  
 9: Cross Ave & Lyons Lane

2030 Total PM Peak Hour.syn  
 05-04-2022

2025 Background AM Peak Hour.syn  
 05-04-2022



Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Volume (veh/h)	0	0	0	3127	1907	768
Future Volume (Veh/h)	0	0	0	3127	1907	768
Sign Control	Stop	Free	Free	Free	Free	Free
Grade	0%	0%	0%	0%	0%	0%
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	0	0	0	3399	2073	835
Pedestrians						
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)						
Median type			None	None	None	None
Median storage (veh)						
Upstream signal (m)			186	145		
pX, platoon unblocked	0.46	0.81	0.81			
vC, conflicting volume	3206	691	2073			
vC1, stage 1 conf/vol						
vC2, stage 2 conf/vol						
vCu, unblocked vol	0	0	1515			
tC, single (s)	6.8	6.9	4.1			
tC, 2 stage (s)						
tF (s)	3.5	3.3	2.2			
p0 queue free %	100	100	100			
cM capacity (veh/h)	473	882	355			
Direction_Lane #	NB.1	NB.2	NB.3	SB.1	SB.2	SB.3
Volume Total	1133	1133	1133	691	691	691
Volume Left	0	0	0	0	0	0
Volume Right	0	0	0	0	0	0
gSH	1700	1700	1700	1700	1700	1700
Volume to Capacity	0.67	0.67	0.67	0.41	0.41	0.41
Queue Length 95th (m)	0.0	0.0	0.0	0.0	0.0	0.0
Control Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0
Lane LOS						
Approach Delay (s)	0.0			0.0		
Approach LOS						
Intersection Summary						
Average Delay	0.0					
Intersection Capacity Utilization	78.5%					
Analysis Period (min)	15					
	ICU Level of Service					
	D					

Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations						
Traffic Volume (vph)	24	429	195	6	5	4
Future Volume (vph)	24	429	195	6	5	4
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (m)	5.0					
Storage Lanes	1					
Taper Length (m)	7.5					
Lane Util. Factor	1.00	0.95	0.95	0.95	1.00	1.00
Ped Bike Factor						
Flt		0.992			0.932	
Flt Protected		0.950			0.976	
Satd. Flow (prot)	1624	3094	2798	0	1383	0
Flt Permitted		0.950			0.976	
Satd. Flow (perm)	1624	3094	2798	0	1383	0
Link Speed (k/h)		50	50		50	
Link Distance (m)		281.3	40.1		41.7	
Travel Time (s)		20.3	2.9		3.0	
Confl. Peds. (#/hr)	4			4	7	
Peak Hour Factor	0.69	0.58	0.88	0.50	0.62	0.50
Heavy Vehicles (%)	0%	5%	16%	0%	0%	25%
Adj. Flow (vph)	35	740	222	12	8	8
Shared Lane Traffic (%)						
Lane Group Flow (vph)	35	740	234	0	16	0
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Left	Left	Right	Left	Right
Median Width(m)		3.6	3.6		3.6	
Link Offset(m)		0.0	0.0		0.0	
Crosswalk Width(m)		4.8	4.8		4.8	
Two way Left Turn Lane						
Headway Factor	1.14	1.14	1.14	1.14	1.14	1.14
Turning Speed (k/h)	25			15	25	15
Sign Control		Free	Free		Stop	
Intersection Summary						
Area Type:	CBD					
Control Type:	Unsignalized					
Intersection Capacity Utilization	23.8%					
Analysis Period (min)	15					
	ICU Level of Service A					



HCM Unsignalized Intersection Capacity Analysis 2025 Background AM Peak Hour.syn  
 9: Cross Ave & Lyons Lane 05-04-2022

Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	↔	↔	↔	↔	↔	↔
Traffic Volume (veh/h)	24	429	195	6	5	4
Future Volume (Veh/h)	24	429	195	6	5	4
Sign Control	Free	Free	Free	Stop	0%	0%
Grade	0%	0%	0%	0%	0%	0%
Peak Hour Factor	0.69	0.88	0.88	0.50	0.62	0.50
Hourly flow rate (vph)	35	740	222	12	8	8
Pedestrians			7		4	
Lane Width (m)		3.6	3.6		3.6	
Walking Speed (m/s)		1.2	1.2		1.2	
Percent Blockage		1			0	
Right turn flare (veh)		None	None			
Median type		None	None			
Median storage (veh)		40				
Upstream signal (m)						
pX, platoon unblocked		238			679	121
vC, conflicting volume						
vC1, stage 1 conf vol						
vC2, stage 2 conf vol		238			679	121
tC, single (s)		4.1			6.8	7.4
tC, 2 stage (s)		2.2			3.5	3.5
p0 queue free %		97			98	99
cM capacity (veh/h)		1336			376	836
Direction_Lane #	EB1	EB2	EB3	WB1	WB2	SB1
Volume Total	35	370	370	148	86	16
Volume Left	35	0	0	0	0	8
Volume Right	0	0	0	0	12	8
GSH	1336	1700	1700	1700	1700	519
Volume to Capacity	0.03	0.22	0.22	0.09	0.05	0.03
Queue Length 95th (m)	0.6	0.0	0.0	0.0	0.0	0.8
Control Delay (s)	7.8	0.0	0.0	0.0	0.0	12.2
Lane LOS	A					B
Approach Delay (s)	0.4			0.0		12.2
Approach LOS						B
Intersection Summary						
Average Delay	0.5					
Intersection Capacity Utilization	23.8% ICU Level of Service A					
Analysis Period (min)	15					

Lanes, Volumes, Timings 2030 Total PM Peak Hour.syn  
 2: Trafalgar Road & QEW SB On Ramps 05-04-2022

Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	↔	↔	↔	↔	↔	↔
Traffic Volume (vph)	0	0	0	3127	1907	768
Future Volume (vph)	0	0	0	3127	1907	768
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	0.91	0.91	1.00
FrT						0.850
FrT Protected						
Satd. Flow (prot)	0	0	0	5085	5085	1583
FrT Permitted						
Satd. Flow (perm)	0	0	0	5085	5085	1583
Link Speed (k/h)	50			50	50	
Link Distance (m)	159.6			186.0	145.2	
Travel Time (s)	11.5			13.4	10.5	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	0	0	0	3399	2073	835
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	0	0	3399	2073	835
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Left	Left	Right
Median Width (m)	0.0			0.0	0.0	
Link Offset (m)	0.0			0.0	0.0	
Crosswalk Width (m)	4.8			4.8	4.8	
Two way Left Turn Lane						
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (k/h)	25	15	25			15
Sign Control	Stop			Free	Free	
Intersection Summary						
Area Type:	Other					
Control Type:	Unsignalized					
Intersection Capacity Utilization	78.5% ICU Level of Service D					
Analysis Period (min)	15					

HCM Signalized Intersection Capacity Analysis  
1: Trafalgar Road & North Service Road/QEW WB Off Ramp



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	21	0	286	575	142	286	0	2629	498	0	1814	10
Traffic Volume (vph)	21	0	286	575	142	286	0	2629	498	0	1814	10
Future Volume (vph)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	3.5	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lane Util. Factor	1.00	1.00	0.95	0.95	1.00	0.91	1.00	0.91	1.00	0.91	1.00	1.00
Frb, ped/bikes	1.00	1.00	1.00	1.00	1.00	0.98	1.00	0.93	1.00	1.00	1.00	1.00
Frb, ped/bikes	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt	1.00	0.85	1.00	1.00	0.85	1.00	0.85	1.00	0.85	1.00	0.85	1.00
Flt Protected	0.95	1.00	0.95	0.97	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Satd. Flow (prot)	1805	1615	1681	1724	1557	1557	5085	1482	4217	4217	1497	1497
Flt Permitted	0.95	1.00	0.95	0.97	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Satd. Flow (perm)	1805	1615	1681	1724	1557	1557	5085	1482	4217	4217	1497	1497
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	23	0	311	625	154	311	0	2858	541	0	1972	11
RTOR Reduction (vph)	0	0	21	0	0	49	0	122	0	0	0	4
Lane Group Flow (vph)	23	0	290	387	392	262	0	2858	419	0	1972	7
Conf. Peds. (#/hr)	2	0	0	0	0	2	14	14	14	14	14	14
Heavy Vehicles (%)	0%	0%	0%	2%	1%	2%	0%	2%	1%	0%	23%	0%
Turn Type	Prot	Perm	Perm	NA	Perm	NA	Perm	NA	Perm	NA	Perm	NA
Protected Phases	7			8			2			6		
Permitted Phases			4	8		8		2		2		6
Actuated Green, G (s)	3.0	41.4	33.9	33.9	33.9	33.9	84.6	84.6	84.6	84.6	84.6	84.6
Effective Green, g (s)	4.0	44.4	36.9	36.9	36.9	36.9	87.6	87.6	87.6	87.6	87.6	87.6
Actuated g/C Ratio	0.03	0.32	0.26	0.26	0.26	0.26	0.63	0.63	0.63	0.63	0.63	0.63
Clearance Time (s)	4.5	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Lane Grp. Cap (vph)	51	512	443	454	410	410	3181	927	2638	936	2638	936
v/s Ratio Prot	0.01						c0.56			0.47		
v/s Ratio Perm	0.45	0.57	0.87	0.86	0.64	0.64	0.90	0.45	0.75	0.75	0.75	0.01
Uniform Delay, d1	66.9	39.8	49.3	49.1	45.7	45.7	22.4	13.7	18.4	9.9	18.4	9.9
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	0.60	0.65	1.00	1.00	1.00	1.00
Incremental Delay, d2	6.2	1.4	17.1	15.5	3.3	3.3	2.6	0.9	2.0	2.0	2.0	0.0
Delay (s)	73.1	41.2	66.4	64.7	48.9	48.9	16.0	9.8	20.4	9.9	20.4	9.9
Level of Service	E	D	E	E	D	D	B	A	C	A	C	A
Approach Delay (s)		43.4		60.8			15.0		20.4		20.4	
Approach LOS		D		E			B		C		C	

Intersection Summary	CBD											
Area Type:	CBD											
Control Type:	Unsignalized											
Intersection Capacity Utilization	15.1%											
ICU Level of Service A												
Analysis Period (min)	15											

HCM Unsignalized Intersection Capacity Analysis 2025 Background AM Peak Hour.syn  
 10: Lyons Lane & South Service Road

Lanes, Volumes, Timings 2030 Total PM Peak Hour.syn  
 1: Trafalgar Road & North Service Road/QEW WB Off Ramp



Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	0	4	3	3	3	1
Traffic Volume (veh/h)	0	4	3	3	3	1
Future Volume (Veh/h)	0	4	3	3	3	1
Sign Control	Free	Free	Stop	Stop	0%	0%
Grade	0%	0%	0%	0%	0%	0%
Peak Hour Factor	0.25	0.50	0.75	0.75	0.38	0.25
Hourly flow rate (vph)	0	8	4	4	8	4
Pedestrians						
Lane Width (m)		3.6			3.6	
Walking Speed (m/s)		1.2			1.2	
Percent Blockage		0			1	
Right turn flare (veh)						
Median type		None				
Median storage (veh)						
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume	14				21	12
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	14				21	12
tC, single (s)	4.1				6.7	6.2
tC, 2 stage (s)						
tF (s)	2.2				3.8	3.3
p0 queue free %	100				99	100
cM capacity (veh/h)	1609				916	1069
Direction_Lane #	EB 1	WB 1	SB 1			
Volume Total	8	8	12			
Volume Left	0	0	8			
Volume Right	0	4	4			
GSH	1609	1700	962			
Volume to Capacity	0.00	0.00	0.01			
Queue Length 95th (m)	0.0	0.0	0.3			
Control Delay (s)	0.0	0.0	8.8			
Lane LOS			A			
Approach Delay (s)	0.0	0.0	8.8			
Approach LOS			A			
Intersection Summary						
Average Delay			3.8			
Intersection Capacity Utilization			15.1%		ICU Level of Service	A
Analysis Period (min)			15			



Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	0	4	3	3	3	1
Traffic Volume (veh/h)	0	4	3	3	3	1
Future Volume (Veh/h)	0	4	3	3	3	1
Sign Control	Free	Free	Stop	Stop	0%	0%
Grade	0%	0%	0%	0%	0%	0%
Peak Hour Factor	0.25	0.50	0.75	0.75	0.38	0.25
Hourly flow rate (vph)	0	8	4	4	8	4
Pedestrians						
Lane Width (m)		3.6			3.6	
Walking Speed (m/s)		1.2			1.2	
Percent Blockage		0			1	
Right turn flare (veh)						
Median type		None				
Median storage (veh)						
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume	14				21	12
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	14				21	12
tC, single (s)	4.1				6.7	6.2
tC, 2 stage (s)						
tF (s)	2.2				3.8	3.3
p0 queue free %	100				99	100
cM capacity (veh/h)	1609				916	1069
Direction_Lane #	EB 1	WB 1	SB 1			
Volume Total	8	8	12			
Volume Left	0	0	8			
Volume Right	0	4	4			
GSH	1609	1700	962			
Volume to Capacity	0.00	0.00	0.01			
Queue Length 95th (m)	0.0	0.0	0.3			
Control Delay (s)	0.0	0.0	8.8			
Lane LOS			A			
Approach Delay (s)	0.0	0.0	8.8			
Approach LOS			A			
Intersection Summary						
Average Delay			3.8			
Intersection Capacity Utilization			15.1%		ICU Level of Service	A
Analysis Period (min)			15			

Lanes, Volumes, Timings  
1: Trafalgar Road & North Service Road/QEW WB Off Ramp

Lanes, Volumes, Timings  
11: Argus Rd & South Service Rd

2030 Total PM Peak Hour.syn  
05-04-2022

2025 Background AM Peak Hour.syn  
05-04-2022

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Turn Type	7											
Protected Phases	7											
Permitted Phases	4	8	8	8	8	8	2	2	2	2	6	6
Detector Phase	4	8	8	8	8	8	2	2	2	2	6	6
Switch Phase												
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Minimum Split (s)	9.5	38.0	38.0	38.0	38.0	38.0	35.0	35.0	35.0	35.0	35.0	35.0
Total Split (s)	9.5	51.5	42.0	42.0	42.0	42.0	88.5	88.5	88.5	88.5	88.5	88.5
Total Split (%)	6.8%	36.8%	30.0%	30.0%	30.0%	30.0%	63.2%	63.2%	63.2%	63.2%	63.2%	63.2%
Maximum Green (s)	5.0	44.5	35.0	35.0	35.0	35.0	81.5	81.5	81.5	81.5	81.5	81.5
Yellow Time (s)	3.5	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
All-Red Time (s)	1.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Lost Time Adjust (s)	-1.0	-3.0	-3.0	-3.0	-3.0	-3.0	-3.0	-3.0	-3.0	-3.0	-3.0	-3.0
Total Lost Time (s)	3.5	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lead/Lag	Lead	Lag	Lag	Lag	Lag	Lag						
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes						
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	None	None	None	None	None	None	C-Max	C-Max	C-Max	C-Max	C-Max	C-Max
Walk Time (s)	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0
Flash Dont Walk (s)	11.0	11.0	11.0	11.0	11.0	11.0	11.0	11.0	11.0	11.0	11.0	11.0
Pedestrian Calls (#/hr)	0	0	0	0	0	0	0	0	0	0	0	0
Act Effct Green (s)	6.0	42.6	36.9	36.9	36.9	36.9	89.4	89.4	89.4	89.4	89.4	89.4
Actuated g/C Ratio	0.04	0.30	0.26	0.26	0.26	0.26	0.64	0.64	0.64	0.64	0.64	0.64
v/c Ratio	0.30	0.61	0.88	0.86	0.68	0.51	0.88	0.51	0.73	0.73	0.73	0.73
Control Delay	75.2	41.8	70.2	68.4	44.1	15.8	4.4	4.4	20.2	20.2	20.2	20.2
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	75.2	41.8	70.2	68.4	44.1	15.8	4.4	4.4	20.2	20.2	20.2	20.2
LOS	E	D	E	E	D	D	B	A	A	C	C	A
Approach Delay	44.1						14.0				20.1	
Approach LOS	D						B				C	
Queue Length 50th (m)	6.6	66.2	112.9	114.0	64.2	115.0	20.2	20.2	149.3	149.3	149.3	0.0
Queue Length 95th (m)	16.7	97.8	#170.5	#169.8	99.2	218.4	m34.8	m34.8	169.9	169.9	169.9	0.0
Internal Link Dist (m)		96.4				141.1					185.2	
Turn Bay Length (m)	50.0						121.2		70.0			
Base Capacity (vph)	77	568	457	469	471	3248	1064	1064	2693	2693	979	979
Starvation Cap Reductn	0	0	0	0	0	22	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.30	0.55	0.85	0.84	0.66	0.89	0.51	0.51	0.73	0.73	0.73	0.01
Intersection Summary												
Area Type:	Other											
Actuated Cycle Length:	140											
Offset: 0 (0%):	Referenced to phase 2:NBT and 6:SBT, Start of Green											
Natural Cycle:	115											
Control Type:	Actuated-Coordinated											
Maximum v/c Ratio:	0.88											
Intersection Signal Delay:	25.0											
Intersection LOS:	C											

Baseline  
Syncho 10 Report  
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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	1	4	4	1	1	1						
Traffic Volume (vph)	5	607	127	4	4	4						
Future Volume (vph)	1	5	607	127	4	4						
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900						
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00						
Ped Bike Factor												
Ft Protected		0.973								0.942		
Satd. Flow (prot)	0	1352	1625	0	1566	0				0.972		
Fit Permitted		0.988								0.972		
Satd. Flow (perm)	0	1352	1625	0	1566	0				0.972		
Link Speed (k/h)		50	50		50					50		
Link Distance (m)		177.7	165.2		103.5					103.5		
Travel Time (s)		12.8	11.9		7.5					7.5		
Conf. Peds. (#/hr)	1									5		1
Peak Hour Factor	0.25	0.42	0.86	0.72	0.25	0.25				0.25		0.25
Heavy Vehicles (%)	100%	0%	3%	0%	0%	0%				0%		0%
Adj. Flow (vph)	4	12	706	176	16	12				16		12
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	16	882	0	28	0				28		0
Enter Blocked Intersection	No	No	No	No	No	No				No		No
Lane Alignment	Left	Left	Left	Right	Left	Right				Left		Right
Median Width(m)		0.0	0.0		3.6					3.6		
Link Offset(m)		0.0	0.0		0.0					0.0		
Crosswalk Width(m)		4.8	4.8		4.8					4.8		
Two way Left Turn Lane												
Headway Factor	1.14	1.14	1.14	1.14	1.14	1.14				1.14		1.14
Turning Speed (k/h)	24									24		14
Sign Control		Free	Free		Free	Stop				Stop		
Intersection Summary												
Area Type:	CBD											
Control Type:	Unsignalized											
Intersection Capacity Utilization	54.4%											
Analysis Period (min)	15											
ICU Level of Service A												

Baseline  
Syncho 10 Report  
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HCM Unsignalized Intersection Capacity Analysis  
 11: Argus Rd & South Service Rd

2025 Background AM Peak Hour.syn  
 05-04-2022



Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	1	4	1	1	4	3
Traffic Volume (veh/h)	1	5	607	127	4	3
Future Volume (Veh/h)	1	5	607	127	4	3
Sign Control	Free	Free	Free	Free	Stop	0%
Grade	0%	0%	0%	0%	0%	0%
Peak Hour Factor	0.25	0.42	0.86	0.72	0.25	0.25
Hourly flow rate (vph)	4	12	706	176	16	12
Pedestrians	1	5	3.6	3.6	3.6	3.6
Lane Width (m)	1.2	1.2	1.2	1.2	1.2	1.2
Percent Blockage	0	0	0	0	0	0
Right turn flare (veh)	None	None	None	None	None	None
Median type	358					
Upstream signal (m)	883					820
pX, platoon unblocked						
vC, conflicting volume						
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
tC, single (s)	5.1	0.42	0.86	0.72	0.25	0.25
tC, 2 stage (s)	3.1	3.1	3.5	3.3	3.3	3.3
p0 queue free %	99	99	95	97	97	97
cM capacity (veh/h)	474					390
Direction, Lane #	EB1	WB1	SB1			
Volume Total	16	882	28			
Volume Left	4	0	16			
Volume Right	0	176	12			
GSH	474	1700	361			
Volume to Capacity	0.01	0.52	0.08			
Queue Length 95th (m)	0.2	0.0	2.0			
Control Delay (s)	3.2	0.0	15.8			
Lane LOS	A	C	C			
Approach Delay (s)	3.2	0.0	15.8			
Approach LOS	C					
Intersection Summary						
Average Delay	0.5					
Intersection Capacity Utilization	54.4%					
Analysis Period (min)	15					
ICU Level of Service	A					

Lanes, Volumes, Timings  
 1: Trafalgar Road & North Service Road/QEW WB Off Ramp

2030 Total PM Peak Hour.syn  
 05-04-2022



Lane Group	EBL	EBT	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	1	1	1	1	1	1	1	1	1	1	1
Traffic Volume (vph)	21	0	286	575	142	286	0	2629	498	0	1814
Future Volume (vph)	21	0	286	575	142	286	0	2629	498	0	1814
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (m)	50.0	0.0	0.0	0.0	0.0	0.0	0.0	70.0	0.0	0.0	0.0
Storage Lanes	1	1	1	1	1	1	1	1	1	1	1
Taper Length (m)	7.5	1.0	7.5	0.95	1.0	1.0	0.91	1.0	1.0	7.5	1.0
Lane Util. Factor	1.00	1.00	0.850	0.950	0.98	0.850	0.93	0.850	0.850	0.91	0.93
Ped Bike Factor	1.00	1.00	0.850	0.950	0.98	0.850	0.93	0.850	0.850	0.91	0.93
Fit	0.950	0.950	0.950	0.971	0.971	0.950	0.971	0.950	0.950	0.971	0.950
Satd. Flow (prot)	1805	0	1615	1681	1725	1583	0	5085	1599	0	4217
Fit Permitted	0.950	0.950	0.950	0.971	0.971	0.950	0.971	0.950	0.950	0.971	0.950
Satd. Flow (perm)	1800	0	1615	1681	1725	1557	0	5085	1482	0	4217
Right Turn on Red	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Satd. Flow (RTOR)	31	31	50	50	66	66	50	327	50	50	66
Link Speed (k/h)	120.4	8.7	165.1	111.9	10.5	10.5	145.2	209.2	15.1	15.1	15.1
Travel Time (s)	2	2	2	2	2	2	2	2	2	2	2
Conf. Peds. (#/hr)	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Peak Hour Factor	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%
Heavy Vehicles (%)	23	0	311	625	154	311	0	2858	541	0	1972
Adj. Flow (vph)	23	0	311	625	154	311	0	2858	541	0	1972
Shared Lane Traffic (%)	38%										
Lane Group Flow (vph)	23	0	311	387	392	311	0	2858	541	0	1972
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Left	Left	Left	Left	Left	Left	Left	Left	Left
Median Width (m)	3.6	3.6	3.6	3.6	3.6	3.6	3.6	3.6	3.6	3.6	3.6
Link Offset (m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Crosswalk Width (m)	4.8	4.8	4.8	4.8	4.8	4.8	4.8	4.8	4.8	4.8	4.8
Two way Left Turn Lane											
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (k/h)	25	15	25	15	25	15	25	15	25	15	25
Number of Detectors	1	1	1	2	1	2	1	2	1	2	1
Detector Template	Left	Right	Left	Thru	Right	Thru	Right	Thru	Right	Thru	Right
Leading Detector (m)	2.0	2.0	2.0	10.0	2.0	10.0	2.0	10.0	2.0	10.0	2.0
Trailing Detector (m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Position (m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Size (m)	2.0	2.0	2.0	0.6	2.0	0.6	2.0	0.6	2.0	0.6	2.0
Detector 1 Type	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex
Detector 1 Channel											
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 2 Position (m)				9.4		9.4		9.4		9.4	
Detector 2 Size (m)				0.6		0.6		0.6		0.6	
Detector 2 Type				Ch+Ex		Ch+Ex		Ch+Ex		Ch+Ex	
Detector 2 Channel											
Detector 2 Extend (s)				0.0		0.0		0.0		0.0	

HCM Unsignalized Intersection Capacity Analysis  
 12: Site Driveway  
 2030 Total AM Peak Hour.syn  
 05-04-2022

Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	8	8	18	10	8	78
Traffic Volume (veh/h)	8	8	18	10	8	78
Future Volume (Veh/h)	8	8	18	10	8	78
Sign Control	Free	Free	Free	Stop	Stop	Stop
Grade	0%	0%	0%	0%	0%	0%
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	9	9	20	11	9	85
Pedestrians						
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)						
Median type	None					
Median storage (veh)						
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume						
vC1, stage 1 conf/vol						
vC2, stage 2 conf/vol						
tC, single (s)						
tC, 2 stage (s)						
tF (s)						
p0 queue free %						
cM capacity (veh/h)						
Direction, Lane #	EB 1	WB 1	NB 1			
Volume Total	18	31	94			
Volume Left	0	20	9			
Volume Right	9	0	85			
GSH	1700	1599	1052			
Volume to Capacity	0.01	0.01	0.09			
Queue Length 95th (m)	0.0	0.3	2.3			
Control Delay (s)	0.0	4.7	8.8			
Lane LOS	A	A	A			
Approach Delay (s)	0.0	4.7	8.8			
Approach LOS	A	A	A			
Intersection Summary						
Average Delay	6.8					
Intersection Capacity Utilization	20.1%					
Analysis Period (min)	15					
ICU Level of Service	A					

Lanes, Volumes, Timings  
 1: Trafalgar Road & North Service Road/QEW WB Off Ramp  
 2025 Background PM Peak Hour.syn  
 05-04-2022

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	4	4	4	4	4	4	4	4	4	4	4	4
Traffic Volume (vph)	20	0	262	485	137	382	0	2813	532	0	2659	12
Future Volume (vph)	20	0	262	485	137	382	0	2813	532	0	2659	12
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (m)	50.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	70.0	0.0	0.0	0.0
Storage Lanes	1	1	1	1	1	1	0	0	1	0	0	1
Taper Length (m)	7.5	1.0	1.0	7.5	0.95	1.00	1.00	0.91	1.00	1.00	7.5	1.00
Lane Util. Factor	1.00	1.00	1.00	0.95	0.99	0.850	0.850	0.97	0.850	0.850	0.91	1.00
Ped Bike Factor	1.00	1.00	0.850	0.950	0.972	0.850	0.850	0.97	0.850	0.850	0.91	0.850
Flt Protected	0.950			0.950	0.972							
Satd. Flow (prot)	1805	0	1615	1681	1727	1583	0	5085	1599	0	4217	1615
Flt Permitted	0.950			0.950	0.972							
Satd. Flow (perm)	1800	0	1615	1681	1727	1563	0	5085	1556	0	4217	1497
Right Turn on Red	Yes			Yes	Yes			Yes	Yes		Yes	Yes
Satd. Flow (RTOR)	31			31	341			50	129		50	70
Link Speed (k/h)	50			50	50			145.2	209.2		15.1	
Link Distance (m)	120.4			165.1	11.9			10.5				
Travel Time (s)	8.7			11.9								
Conf. Peds. (#/hr)	2			2	14			14	14		14	14
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	0%	0%	0%	2%	1%	2%	0%	2%	1%	0%	23%	0%
Adj. Flow (vph)	22	0	285	527	149	415	0	3058	578	0	2890	13
Shared Lane Traffic (%)				37%								
Lane Group Flow (vph)	22	0	285	332	344	415	0	3058	578	0	2890	13
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Right	Left	Left	Left	Right	Left	Left	Right
Median Width(m)	3.6			3.6				0.0	0.0		0.0	
Link Offset(m)	0.0			0.0				0.0	0.0		0.0	
Crosswalk Width(m)	4.8			4.8				4.8	4.8		4.8	
Two way Left Turn Lane												
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (k/h)	25	15	25	25	15	25	15	25	15	25	15	25
Number of Detectors	1	1	1	2	1	2	2	1	2	1	2	1
Detector Template	Left	Right	Left	Thru	Right	Thru	Right	Thru	Right	Thru	Right	Right
Leading Detector (m)	2.0	2.0	2.0	10.0	2.0	10.0	2.0	10.0	2.0	10.0	2.0	2.0
Trailing Detector (m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Position(m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Size(m)	2.0	2.0	2.0	0.6	2.0	0.6	2.0	0.6	2.0	0.6	2.0	0.6
Detector 1 Type	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 2 Position(m)				9.4		9.4		9.4			9.4	
Detector 2 Size(m)				0.6		0.6		0.6			0.6	
Detector 2 Type				Ch+Ex		Ch+Ex		Ch+Ex			Ch+Ex	
Detector 2 Channel												
Detector 2 Extend (s)				0.0		0.0		0.0			0.0	

Lanes, Volumes, Timings  
1: Trafalgar Road & North Service Road/QEW WB Off Ramp

Lanes, Volumes, Timings  
12: Site Driveway

2025 Background PM Peak Hour.syn  
05-04-2022

2030 Total AM Peak Hour.syn  
05-04-2022

	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Group	Prot	7									NA	Perm
Protected Phases											6	
Permitted Phases		4	8	8	8	8	2	2	2	2	6	6
Detector Phase		4	8	8	8	8	2	2	2	2	6	6
Switch Phase												
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Minimum Split (s)	23.0	38.0	38.0	38.0	38.0	35.0	35.0	35.0	35.0	35.0	35.0	35.0
Total Split (s)	23.0	61.0	38.0	38.0	38.0	79.0	79.0	79.0	79.0	79.0	79.0	79.0
Total Split (%)	16.4%	43.6%	27.1%	27.1%	27.1%	56.4%	56.4%	56.4%	56.4%	56.4%	56.4%	56.4%
Maximum Green (s)	18.0	54.0	31.0	31.0	31.0	72.0	72.0	72.0	72.0	72.0	72.0	72.0
Yellow Time (s)	3.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
All-Red Time (s)	2.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Lost Time Adjust (s)	-1.0	-3.0	-3.0	-3.0	-3.0	-3.0	-3.0	-3.0	-3.0	-3.0	-3.0	-3.0
Total Lost Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lead/Lag	Lead	Lag	Lag	Lag	Lag	Lag	Lag	Lag	Lag	Lag	Lag	Lag
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	None	None	None	None	None	C-Max	C-Max	C-Max	C-Max	C-Max	C-Max	C-Max
Walk Time (s)		7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0
Flash Dont Walk (s)		11.0	11.0	11.0	11.0	11.0	11.0	11.0	11.0	11.0	11.0	11.0
Pedestrian Calls (#/hr)		0	0	0	0	0	0	0	0	0	0	0
Act Effct Green (s)	8.2	44.0	36.1	36.1	36.1	88.0	88.0	88.0	88.0	88.0	88.0	88.0
Actuated g/C Ratio	0.06	0.31	0.26	0.26	0.26	0.63	1.00	0.63	1.00	0.63	0.63	0.63
v/c Ratio	0.21	0.54	0.77	0.77	0.77	0.96	0.37	0.96	0.37	1.09	0.01	0.01
Control Delay	66.7	37.1	59.7	59.8	0.4	25.2	0.3	74.3	0.0	74.3	0.0	0.0
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	66.7	37.1	59.7	59.8	0.4	25.2	0.3	74.3	0.0	74.3	0.0	0.0
LOS	E	D	E	E	A	C	C	A	A	E	E	A
Approach Delay		39.2		37.2		21.3		73.9				
Approach LOS		D		D		C		E				
Queue Length 50th (m)	6.2	57.5	93.5	97.1	0.0	-285.8	0.0	-364.5	0.0	-364.5	0.0	0.0
Queue Length 95th (m)	15.6	78.4	125.8	129.7	0.0	#398.9	m0.0	#415.1	0.0	#415.1	0.0	0.0
Internal Link Dist (m)		96.4		141.1		121.2		185.2				
Turn Bay Length (m)	50.0					70.0						
Base Capacity (vph)	244	675	452	465	1563	3194	1556	2649	966	2649	966	966
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.09	0.42	0.73	0.74	0.27	0.96	0.37	1.09	0.01	1.09	0.01	0.01
Intersection Summary	Other											
Area Type:	Other											
Control Type:	Unsignalized											
Intersection Capacity Utilization	20.1%											
Analysis Period (min)	15											
ICU Level of Service A												

Baseline

Syncho 10 Report  
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Lanes, Volumes, Timings  
1: Trafalgar Road & North Service Road/QEW WB Off Ramp

Lanes, Volumes, Timings  
12: Site Driveway

2025 Background PM Peak Hour.syn  
05-04-2022

2030 Total AM Peak Hour.syn  
05-04-2022

	EBT	EBR	WBT	WBR	NBT	NBR	SBT	SBR
Lane Group	EBT	EBR	WBT	WBR	NBT	NBR	SBT	SBR
Lane Configurations	8	8	18	10	8	8	8	78
Future Volume (vph)	8	8	18	10	8	8	8	78
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FrT	0.932						0.878	
Flt Protected							0.969	0.995
Satd. Flow (prot)	1736	0	0	1805	1627	0		
Flt Permitted							0.969	0.995
Satd. Flow (perm)	1736	0	0	1805	1627	0		
Link Speed (k/h)	50						50	50
Link Distance (m)	129.7						696.1	54.3
Travel Time (s)	9.3						50.1	3.9
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	9	9	20	11	9	85		
Shared Lane Traffic (%)								
Lane Group Flow (vph)	18	0	0	31	94	0		
Enter Blocked Intersection	No	No	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Left	Left	Right	Left	Right
Median Width (m)	0.0	0.0	0.0	0.0	0.0	3.6		
Link Offset (m)	0.0	0.0	0.0	0.0	0.0	0.0		
Crosswalk Width (m)	4.8					4.8	4.8	4.8
Two way Left Turn Lane								
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (k/h)	Free	15	25	25	25	15	25	15
Sign Control	Free	Free	Free	Free	Free	Stop	Stop	Stop
Intersection Summary	Other							
Area Type:	Other							
Control Type:	Unsignalized							
Intersection Capacity Utilization	20.1%							
Analysis Period (min)	15							
ICU Level of Service A								

Baseline

Syncho 10 Report  
Page 33

HCM Unsignalized Intersection Capacity Analysis  
 11: Argus Rd & South Service Rd

Lanes, Volumes, Timings  
 1: Trafalgar Road & North Service Road/QEW WB Off Ramp

2030 Total AM Peak Hour.syn  
 05-04-2022

2025 Background PM Peak Hour.syn  
 05-04-2022



Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	1	6	6	159	19	59
Traffic Volume (veh/h)	1	6	6	670	159	59
Future Volume (Veh/h)						
Sign Control	Free	Free	Free	Stop	Stop	Stop
Grade	0%	0%	0%	0%	0%	0%
Peak Hour Factor	0.25	0.42	0.86	0.72	0.25	0.25
Hourly flow rate (vph)	4	14	779	221	76	236
Pedestrians	1	5			1	
Lane Width (m)	3.6	3.6			3.6	
Walking Speed (m/s)	1.2	1.2			1.2	
Percent Blockage	0	0			0	
Right turn flare (veh)						
Median type	None	None				
Median storage (veh)						
Upstream signal (m)		358				
pX, platoon unblocked						
vC, conflicting volume	1001				918	892
vC1, stage 1 conf vol						
vC2, stage 2 conf vol	1001				918	892
vCu, unblocked vol	5.1				6.4	6.2
tC, single (s)						
tC, 2 stage (s)	3.1				3.5	3.3
tF (s)	99				75	31
p0 queue free %	419				300	343
cM capacity (veh/h)						
Direction_Lane #	EB 1	WB 1	SB 1			
Volume Total	18	1000	312			
Volume Left	4	0	76			
Volume Right	0	221	236			
GSH	419	1700	332			
Volume to Capacity	0.01	0.59	0.94			
Queue Length 95th (m)	0.2	0.0	77.3			
Control Delay (s)	3.1	0.0	71.6			
Lane LOS	A	F	F			
Approach Delay (s)	3.1	0.0	71.6			
Approach LOS	F	F	F			
Intersection Summary						
Average Delay			16.8			
Intersection Capacity Utilization			62.2%		ICU Level of Service	B
Analysis Period (min)			15			

Intersection Capacity Utilization	94.6%	ICU Level of Service	F
Analysis Period (min)	15		
~ Volume exceeds capacity, queue is theoretically infinite.			
Queue shown is maximum after two cycles.			
#	95th percentile volume exceeds capacity, queue may be longer.		
Queue shown is maximum after two cycles.			
m	Volume for 95th percentile queue is metered by upstream signal.		



Splits and Phases: 1: Trafalgar Road & North Service Road/QEW WB Off Ramp



HCM Signalized Intersection Capacity Analysis 2025 Background PM Peak Hour.syn  
 1: Trafalgar Road & North Service Road/QEW WB Off Ramp

05-04-2022

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations	20	0	262	485	137	382	0	2813	532	0	2659	12	
Traffic Volume (vph)	20	0	262	485	137	382	0	2813	532	0	2659	12	
Future Volume (vph)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	
Ideal Flow (veh/pl)	4.0	4.0	4.0	4.0	4.0	4.0	1.0	4.0	1.0	4.0	4.0	4.0	
Total Lost time (s)	1.00	1.00	0.95	0.95	1.00	0.91	1.00	0.91	1.00	1.00	0.91	1.00	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	0.99	1.00	0.97	1.00	1.00	0.93	1.00	
Frbp, ped/bikes	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Frt	1.00	0.85	1.00	1.00	1.00	0.85	1.00	0.85	1.00	0.85	1.00	0.85	
Flt Protected	0.95	1.00	0.95	0.97	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Satd. Flow (prot)	1805	1615	1681	1728	1563	5085	1556	5085	1556	4217	1497	5	
Flt Permitted	0.95	1.00	0.95	0.97	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Satd. Flow (perm)	1805	1615	1681	1728	1563	5085	1556	5085	1556	4217	1497	5	
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	
Adj. Flow (vph)	22	0	285	527	149	415	0	3058	578	0	2890	13	
RTOR Reduction (vph)	0	0	21	0	0	0	0	0	0	0	0	8	
Lane Group Flow (vph)	22	0	264	332	344	415	0	3058	578	0	2890	8	
Conf. Peds. (#/hr)	2	0	264	332	344	415	2	14	14	14	14	14	
Heavy Vehicles (%)	0%	0%	0%	2%	1%	2%	0%	2%	1%	0%	23%	0%	
Turn Type	Prot	Perm	Perm	NA	Free	NA	Free	NA	Free	NA	Perm	NA	
Protected Phases	7	0	8	0	2	0	2	0	0	6	0	6	
Permitted Phases	4	0	8	0	Free	Free	Free	0	Free	Free	0	6	
Actuated Green, G (s)	4.9	43.0	33.1	33.1	140.0	83.0	140.0	83.0	140.0	83.0	83.0	83.0	
Effective Green, g (s)	5.9	46.0	36.1	36.1	140.0	86.0	140.0	86.0	140.0	86.0	86.0	86.0	
Actuated g/C Ratio	0.04	0.33	0.26	0.26	1.00	0.61	1.00	0.61	1.00	0.61	0.61	0.61	
Clearance Time (s)	5.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	
Lane Grp Cap (vph)	76	530	433	445	1563	3123	1556	2590	919	2590	919	919	
v/s Ratio Prot	0.01					0.60					c0.69		
v/s Ratio Perm	0.29	0.50	0.77	0.77	0.27	0.98	0.37	0.37	0.37	0.37	1.12	0.01	
Uniform Delay, d1	65.0	37.7	48.1	48.2	0.0	26.1	0.0	27.0	10.5	27.0	10.5	10.5	
Progression Factor	1.00	1.00	1.00	1.00	1.00	0.74	1.00	1.00	1.00	1.00	1.00	1.00	
Incremental Delay, d2	2.1	0.7	7.9	8.1	0.4	7.4	0.3	58.1	0.0	58.1	0.0	0.0	
Delay (s)	67.1	38.5	56.0	56.3	0.4	26.8	0.3	85.1	10.5	85.1	10.5	10.5	
Level of Service	E	D	E	E	A	C	A	F	A	F	B	B	
Approach Delay (s)								22.6			84.8		
Approach LOS								C			F		
Intersection Summary													
HCM 2000 Control Delay	47.7											HCM 2000 Level of Service	D
HCM 2000 Volume to Capacity ratio	1.00												
Actuated Cycle Length (s)	140.0											Sum of lost time (s)	12.0
Intersection Capacity Utilization	94.6%											ICU Level of Service	F
Analysis Period (min)	15												

Lanes, Volumes, Timings 2030 Total AM Peak Hour.syn  
 11: Argus Rd & South Service Rd

05-04-2022

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	SBL	SBT	SBR	
Lane Configurations	1	4	4	1	1	1	1	1	1	
Traffic Volume (vph)	1	6	670	159	19	59	19	59	59	
Future Volume (vph)	1	6	670	159	19	59	19	59	59	
Ideal Flow (veh/pl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Ped Bike Factor				0.970		0.988			0.988	
Flt Protected	0	0.989				0.988			0.988	
Satd. Flow (prot)	0	1384	1621	0	1517	0			1517	
Flt Permitted	0	0.989				0.988			0.988	
Satd. Flow (perm)	0	1384	1621	0	1517	0			1517	
Link Speed (k/h)		50	50		50				50	
Link Distance (m)		177.7	165.2		103.5				103.5	
Travel Time (s)		12.8	11.9		7.5				7.5	
Conf. Peds. (#/hr)	1			1		1		1	5	
Peak Hour Factor	0.25	0.42	0.86	0.72	0.25	0.25			0.25	
Heavy Vehicles (%)	100%	0%	3%	0%	0%	0%			0%	
Adj. Flow (vph)	4	14	779	221	76	236			236	
Shared Lane Traffic (%)										
Lane Group Flow (vph)	0	18	1000	0	312	0			312	
Enter Blocked Intersection	No	No	No	No	No	No			No	
Lane Alignment	Left	Left	Left	Right	Left	Right			Right	
Median Width(m)	0.0	0.0	0.0	3.6	0.0	0.0			3.6	
Link Offset(m)	0.0	0.0	0.0	0.0	0.0	0.0			0.0	
Crosswalk Width(m)	4.8	4.8	4.8	4.8	4.8	4.8			4.8	
Two way Left Turn Lane										
Headway Factor	1.14	1.14	1.14	1.14	1.14	1.14			1.14	
Turning Speed (k/h)	24		Free	Free	Free	Stop			24	
Sign Control										
Intersection Summary										
Area Type:	CBD									
Control Type:	Unsignalized									
Intersection Capacity Utilization	62.2%									
Analysis Period (min)	15									

HCM Unsignalized Intersection Capacity Analysis  
 10: Lyons Lane & South Service Road

Lanes, Volumes, Timings  
 2: Trafalgar Road & QEW SB On Ramps

2030 Total AM Peak Hour.syn  
 05-04-2022

2025 Background PM Peak Hour.syn  
 05-04-2022



Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	0	4	1	1	1	1
Traffic Volume (veh/h)	0	10	8	19	16	2
Future Volume (Veh/h)	0	10	8	19	16	2
Sign Control	Free	Free	Free	Stop	Stop	Stop
Grade	0%	0%	0%	0%	0%	0%
Peak Hour Factor	0.25	0.50	0.75	0.75	0.38	0.25
Hourly flow rate (vph)	0	20	11	25	42	8
Pedestrians						
Lane Width (m)			3.6	3.6		
Walking Speed (m/s)			1.2	1.2		
Percent Blockage			0		1	
Right turn flare (veh)			None	None		
Median type			None	None		
Median storage (veh)			393			
Upstream signal (m)						
pX, platoon unblocked			42		50	30
vC, conflicting volume						
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol			42		50	30
tC, single (s)			4.1		6.7	6.2
tC, 2 stage (s)			2.2		3.8	3.3
tF (s)			100		95	99
p0 queue free %			1572		881	1046
cM capacity (veh/h)						
Direction, Lane #	EB 1	WB 1	SB 1			
Volume Total	20	36	50			
Volume Left	0	0	42			
Volume Right	0	25	8			
GSH	1572	1700	904			
Volume to Capacity	0.00	0.02	0.06			
Queue Length 95th (m)	0.0	0.0	1.4			
Control Delay (s)	0.0	0.0	9.2			
Lane LOS	A	A	A			
Approach Delay (s)	0.0	0.0	9.2			
Approach LOS	A	A	A			
Intersection Summary						
Average Delay	4.3					
Intersection Capacity Utilization	15.1%					
ICU Level of Service	A					
Analysis Period (min)	15					

Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Volume (vph)	0	0	0	3345	1739	1667
Future Volume (vph)	0	0	0	3345	1739	1667
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	0.91	0.91	1.00
Flt Protected						0.850
Satd. Flow (prot)	0	0	0	5085	5085	1583
Flt Permitted						
Satd. Flow (perm)	0	0	0	5085	5085	1583
Link Speed (k/h)	50			50	50	
Link Distance (m)	159.6			186.0	145.2	
Travel Time (s)	11.5			13.4	10.5	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	0	0	0	3636	1890	1812
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	0	0	3636	1890	1812
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Left	Left	Right
Median Width (m)	0.0			0.0	0.0	
Link Offset (m)	0.0			0.0	0.0	
Crosswalk Width (m)	4.8			4.8	4.8	
Two way Left Turn Lane						
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (k/h)	25	15	25			15
Sign Control	Stop			Free	Free	Free
Intersection Summary						
Area Type:	Other					
Control Type:	Unsignalized					
Intersection Capacity Utilization	106.6%					
ICU Level of Service	G					
Analysis Period (min)	15					

HCM Unsignalized Intersection Capacity Analysis  
 2: Trafalgar Road & QEW SB On Ramps

Lanes, Volumes, Timings  
 10: Lyons Lane & South Service Road

2025 Background PM Peak Hour.syn  
 05-04-2022

2030 Total AM Peak Hour.syn  
 05-04-2022

Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations			↑↑↑	↑↑↑	↑↑↑	↑
Traffic Volume (veh/h)	0	0	3345	1739	1667	1667
Future Volume (Veh/h)	0	0	3345	1739	1667	1667
Sign Control	Stop	Free	Free	Free	Free	Free
Grade	0%	0%	0%	0%	0%	0%
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	0	0	3636	1890	1812	1812
Pedestrians						
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)			None	None	None	None
Median type						
Median storage (veh)						
Upstream signal (m)			186	145		
pX, platoon unblocked	0.31					
vC, conflicting volume	3102	630	1890			
vC1, stage 1 conf/vol						
vC2, stage 2 conf/vol						
vCu, unblocked vol	50	630	1890			
tC, single (s)	6.8	6.9	4.1			
tC, 2 stage (s)						
tF (s)	3.5	3.3	2.2			
p0 queue free %	100	100	100			
cM capacity (veh/h)	299	424	312			
Direction_Lane #	NB.1	NB.2	NB.3	SB.1	SB.2	SB.3
Volume Total	1212	1212	1212	630	630	1812
Volume Left	0	0	0	0	0	0
Volume Right	0	0	0	0	0	1812
GSH	1700	1700	1700	1700	1700	1700
Volume to Capacity	0.71	0.71	0.71	0.37	0.37	1.07
Queue Length 95th (m)	0.0	0.0	0.0	0.0	0.0	0.0
Control Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0
Lane LOS						
Approach Delay (s)	0.0			0.0		
Approach LOS						
Intersection Summary						
Average Delay	0.0					
Intersection Capacity Utilization	106.6% ICU Level of Service G					
Analysis Period (min)	15					

Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		←	←	←	←	←
Traffic Volume (vph)	0	10	8	19	16	2
Future Volume (vph)	0	10	8	19	16	2
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor						
Flt Protected		0.906			0.978	
Satd. Flow (prot)		1710	1549		1257	
Flt Permitted					0.960	
Satd. Flow (perm)		1710	1549		1257	
Link Speed (k/h)		50	50		50	
Link Distance (m)		60.5	89.6		37.6	
Travel Time (s)		4.4	6.5		2.7	
Conf. Peds. (#/hr)		6			6	
Peak Hour Factor		0.25	0.50		0.75	
Heavy Vehicles (%)		0%	0%		0%	
Adj. Flow (vph)		0	20		25	
Shared Lane Traffic (%)						
Lane Group Flow (vph)		0	20		36	
Enter Blocked Intersection		No	No		No	
Lane Alignment		Left	Left		Right	
Median Width(m)		0.0	0.0		3.6	
Link Offset(m)		0.0	0.0		0.0	
Crosswalk Width(m)		4.8	4.8		4.8	
Two way Left Turn Lane						
Headway Factor		1.14	1.14		1.14	
Turning Speed (k/h)		24			24	
Sign Control		Free	Free		Stop	
Intersection Summary						
Area Type:	CBD					
Control Type:	Unsignalized					
Intersection Capacity Utilization	15.1% ICU Level of Service A					
Analysis Period (min)	15					

HCM Signalized Intersection Capacity Analysis

2030 Total AM Peak Hour.syn  
05-04-2022

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	85	307	109	207	123	29	36	1	60	65	25	66
Traffic Volume (vph)	85	307	109	207	123	29	36	1	60	65	25	66
Future Volume (vph)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Ideal Flow (vphpl)	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5
Total Lost time (s)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Lane Util. Factor	0.99	1.00	0.85	1.00	1.00	0.97	1.00	1.00	1.00	1.00	1.00	1.00
Frb. ped/bikes	0.99	1.00	0.85	1.00	1.00	1.00	1.00	1.00	1.00	0.99	1.00	1.00
Frb. ped/bikes	1.00	1.00	0.85	1.00	1.00	0.85	1.00	1.00	0.85	1.00	1.00	0.85
Flt Protected	0.95	1.00	1.00	0.95	1.00	1.00	1.00	1.00	0.95	1.00	1.00	1.00
Satd. Flow (prot)	1614	3094	1425	1593	2801	1414	1593	1676	1425	1612	1676	1163
Flt Permitted	0.66	1.00	1.00	0.26	1.00	1.00	0.74	1.00	1.00	0.76	1.00	1.00
Satd. Flow (perm)	1127	3094	1425	439	2801	1414	1240	1676	1425	1285	1676	1163
Peak-hour factor, PHF	0.69	0.68	0.92	0.92	0.88	0.50	0.92	0.92	0.92	0.82	0.92	0.50
Adj. Flow (vph)	123	529	118	225	140	58	39	1	65	105	27	132
RTOR Reduction (vph)	0	0	88	0	0	30	0	0	40	0	0	82
Lane Group Flow (vph)	123	529	30	225	140	28	39	1	25	105	27	50
Conf. Peds. (#/hr)	4					4					7	
Heavy Vehicles (%)	0%	5%	2%	2%	16%	0%	2%	2%	2%	0%	2%	25%
Turn Type	Perm	NA	Perm	pm+pt	NA	Perm	Perm	NA	Perm	Perm	NA	Perm
Protected Phases	4		4	3	8		2		2		6	
Permitted Phases	4		4	8		8	2		2		6	6
Actuated Green, G (s)	17.2	17.2	17.2	32.8	32.8	32.8	25.8	25.8	25.8	25.8	25.8	25.8
Effective Green, g (s)	17.2	17.2	17.2	32.8	32.8	32.8	25.8	25.8	25.8	25.8	25.8	25.8
Actuated g/C Ratio	0.25	0.25	0.25	0.49	0.49	0.49	0.38	0.38	0.38	0.38	0.38	0.38
Clearance Time (s)	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Lane Grp. Cap (vph)	286	787	362	402	1359	686	473	639	543	490	639	443
v/s Ratio Prot	c0.17			c0.09	0.05		0.00				0.02	
v/s Ratio Perm	0.11		0.02	0.18	0.02	0.03	0.02	0.02	0.02	c0.08	0.04	
Uniform Delay, d1	0.43	0.67	0.08	0.56	0.10	0.04	0.08	0.00	0.05	0.21	0.04	0.11
Progression Factor	21.1	22.7	19.2	11.3	9.4	9.1	13.3	12.9	13.2	14.1	13.1	13.5
Incremental Delay, d2	1.0	2.3	0.1	1.7	0.0	0.0	0.3	0.0	0.2	1.0	0.1	0.5
Delay (s)	22.1	24.9	19.3	13.0	9.5	9.2	13.7	12.9	13.3	15.1	13.3	14.0
Level of Service	C	C	B	B	A	A	B	B	B	B	B	B
Approach LOS	23.6			11.3			13.4				14.4	
Approach LOS	C			B			B				B	
Intersection Summary												
HCM 2000 Control Delay	18.0 HCM 2000 Level of Service B											
HCM 2000 Volume to Capacity ratio	0.44											
Actuated Cycle Length (s)	67.6 Sum of lost time (s) 13.5											
Intersection Capacity Utilization	44.1% ICU Level of Service A											
Analysis Period (min)	15											
C Critical Lane Group												

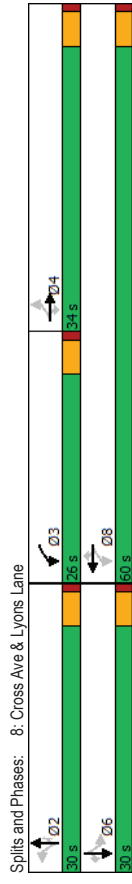
2025 Background PM Peak Hour.syn  
05-04-2022

Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	1044	491	0	2301	1739	0
Traffic Volume (vph)	1044	491	0	2301	1739	0
Future Volume (vph)	1900	1900	1900	1900	1900	1900
Ideal Flow (vphpl)	0.97	1.00	1.00	0.91	0.91	1.00
Lane Util. Factor	0.98					
Ped Bike Factor	0.850					
Flt Protected	0.950					
Satd. Flow (prot)	3502	1599	0	5085	5085	0
Flt Permitted	0.950					
Satd. Flow (perm)	3502	1573	0	5085	5085	0
Right Turn on Red	Yes					Yes
Satd. Flow (RTOR)	12					
Link Speed (k/h)	50		50		50	
Link Distance (m)	71.3		50.7		186.0	
Travel Time (s)	5.1		3.7		13.4	
Conf. Peds. (#/hr)	2					
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	0%	1%	0%	2%	2%	0%
Adj. Flow (vph)	1135	534	0	2501	1890	0
Shared Lane Traffic (%)						
Lane Group Flow (vph)	1135	534	0	2501	1890	0
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Left	Right	Right
Median Width(m)	7.2		0.0	0.0	0.0	
Link Offset(m)	0.0		0.0	0.0	0.0	
Crosswalk Width(m)	4.8		4.8		4.8	
Two way Left Turn Lane						
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (k/h)	25	15	25		15	
Number of Detectors	1	1	2	2	2	
Detector Template	Left	Right	Thru	Thru	Thru	
Leading Detector (m)	2.0	2.0	10.0	10.0	10.0	
Trailing Detector (m)	0.0	0.0	0.0	0.0	0.0	
Detector 1 Position(m)	0.0	0.0	0.0	0.0	0.0	
Detector 1 Size(m)	2.0	2.0	0.6	0.6	0.6	
Detector 1 Type	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	
Detector 1 Channel						
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	
Detector 2 Position(m)			9.4	9.4	9.4	
Detector 2 Size(m)			0.6	0.6	0.6	
Detector 2 Type			Ch+Ex	Ch+Ex	Ch+Ex	
Detector 2 Channel						
Detector 2 Extend (s)			0.0	0.0	0.0	
Turn Type	Perm	Perm	NA	NA	NA	
Protected Phases	4	4	2	6		
Permitted Phases						



Analysis Period (min) 15

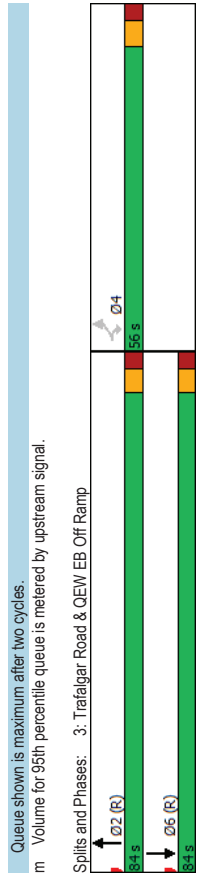
Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Detector Phase	4	4		2	6	
Switch Phase						
Minimum Initial (s)	10.0	10.0	29.0	29.0	29.0	
Minimum Split (s)	38.0	38.0	36.0	36.0	36.0	
Total Split (s)	56.0	56.0	84.0	84.0	84.0	
Total Split (%)	40.0%	40.0%	60.0%	60.0%	60.0%	
Maximum Green (s)	49.0	49.0	77.0	77.0	77.0	
Yellow Time (s)	4.0	4.0	4.0	4.0	4.0	
All-Red Time (s)	3.0	3.0	3.0	3.0	3.0	
Lost Time Adjust (s)	-3.0	-3.0	-3.0	-3.0	-3.0	
Total Lost Time (s)	4.0	4.0	4.0	4.0	4.0	
Lead/Lag						
Lead-Lag Optimize?						
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	
Recall Mode	None	None	C-Max	C-Max	C-Max	
Walk Time (s)	7.0	7.0	7.0	7.0	7.0	
Flash Dont Walk (s)	11.0	11.0	11.0	11.0	11.0	
Pedestrian Calls (#/hr)	0	0	0	0	0	
Act Effct Green (s)	51.0	51.0	81.0	81.0	81.0	
Actuated g/C Ratio	0.36	0.36	0.58	0.58	0.58	
v/c Ratio	0.89	0.92	0.85	0.64	0.64	
Control Delay	51.6	64.0	43.7	16.2	16.2	
Queue Delay	0.0	0.0	0.0	0.0	0.0	
Total Delay	51.6	64.0	43.8	16.2	16.2	
LOS	D	E	D	D	B	
Approach Delay	55.6		43.8	16.2		
Approach LOS	E		D	D	B	
Queue Length 50th (m)	157.6	143.8	275.7	84.9		
Queue Length 95th (m)	188.1	#214.7	m278.2	m82.7		
Internal Link Dist (m)	47.3		26.7	162.0		
Turn Bay Length (m)						
Base Capacity (vph)	1300	591	2941	2941		
Starvation Cap Reductn	0	0	9	0		
Spillback Cap Reductn	0	0	0	0		
Storage Cap Reductn	0	0	0	0		
Reduced v/c Ratio	0.87	0.90	0.85	0.64		
<b>Intersection Summary</b>						
Area Type:	Other					
Cycle Length:	140					
Actuated Cycle Length:	140					
Offset: Q (0%), Referenced to phase 2:NBT and 6:SBT, Start of Green						
Natural Cycle:	90					
Control Type:	Actuated-Coordinated					
Maximum v/c Ratio:	0.92					
Intersection Signal Delay:	38.4			Intersection LOS: D		
Intersection Capacity Utilization	106.6%			ICU Level of Service G		
Analysis Period (min)	15					
# 95th percentile volume exceeds capacity, queue may be longer.						



Splits and Phases: 8: Cross Ave & Lyons Lane

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Perm	NA	NA	Perm	pm+pt	NA	Perm	NA	Perm	NA	Perm	NA	Perm
Protected Phases	4	4	4	8	8	8	2	2	2	6	6	6
Detector Phase	4	4	4	3	8	8	2	2	2	6	6	6
Switch Phase												
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Minimum Split (s)	22.5	22.5	22.5	9.5	22.5	22.5	22.5	22.5	22.5	22.5	22.5	22.5
Total Split (s)	34.0	34.0	34.0	26.0	60.0	60.0	30.0	30.0	30.0	30.0	30.0	30.0
Total Split (%)	37.8%	37.8%	37.8%	28.9%	66.7%	66.7%	33.3%	33.3%	33.3%	33.3%	33.3%	33.3%
Maximum Green (s)	29.5	29.5	29.5	21.5	55.5	55.5	25.5	25.5	25.5	25.5	25.5	25.5
Yellow Time (s)	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5
LeadLag	Lag	Lag	Lag	Lag	Lag	Lag	Lag	Lag	Lag	Lag	Lag	Lag
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	None	None	None	None	None	None	Max	Max	Max	Max	Max	Max
Walk Time (s)	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0
Flash Dont Walk (s)	11.0	11.0	11.0	11.0	11.0	11.0	11.0	11.0	11.0	11.0	11.0	11.0
Pedestrian Calls (#/hr)	0	0	0	0	0	0	0	0	0	0	0	0
Act Effct Green (s)	17.2	17.2	17.2	32.8	32.8	32.8	25.8	25.8	25.8	25.8	25.8	25.8
Actuated g/C Ratio	0.25	0.25	0.25	0.48	0.48	0.48	0.38	0.38	0.38	0.38	0.38	0.38
v/c Ratio	0.43	0.67	0.26	0.56	0.10	0.08	0.08	0.00	0.11	0.22	0.04	0.25
Control Delay	26.6	27.5	6.1	15.6	9.1	2.8	16.9	17.0	4.9	18.2	16.4	5.3
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	26.6	27.5	6.1	15.6	9.1	2.8	16.9	17.0	4.9	18.2	16.4	5.3
LOS	C	C	A	B	A	A	B	B	A	B	B	A
Approach Delay	24.1			11.7			9.5					11.5
Approach LOS	C			B			A					B
Queue Length 50th (m)	13.6	32.9	0.0	16.6	4.9	0.0	3.2	0.1	0.0	9.1	2.2	0.0
Queue Length 95th (m)	21.4	30.0	11.1	28.7	8.7	0.9	11.1	1.2	7.4	15.9	8.4	0.0
Internal Link Dist (m)	257.3			16.1			125.2					17.7
Turn Bay Length (m)	60.0	60.0	60.0	60.0	60.0	60.0	60.0	60.0	60.0	60.0	60.0	60.0
Base Capacity (vph)	494	1362	693	583	2320	1177	472	637	587	487	637	524
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.25	0.39	0.17	0.39	0.06	0.05	0.08	0.00	0.11	0.22	0.04	0.25

Intersection Summary	
Area Type:	CBD
Cycle Length:	90
Actuated Cycle Length:	67.7
Natural Cycle:	55
Control Type:	Semi Act-Uncoord
Maximum v/c Ratio:	0.67
Intersection Signal Delay:	17.6
Intersection Capacity Utilization:	44.1%
ICU Level of Service:	A





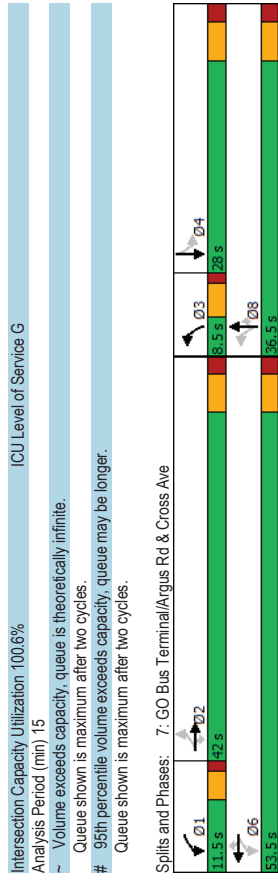




HCM Unsignalized Intersection Capacity Analysis 2025 Background PM Peak Hour.syn  
 4: Trafalgar Road & Argus Road/QEW EB On Ramp

Lanes, Volumes, Timings 2030 Total AM Peak Hour.syn  
 7: GO Bus Terminal/Argus Rd & Cross Ave

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	0	0	45	0	0	0	0	2301	471	0	1832	398
Traffic Volume (veh/h)	0	0	45	0	0	0	0	2301	471	0	1832	398
Future Volume (Veh/h)	0	0	45	0	0	0	0	2301	471	0	1832	398
Sign Control	Stop	Stop	Stop	0%	0%	0%	Free	Free	Free	Free	Free	Free
Grade	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Peak Hour Factor	0	0	49	0	0	0	0	2501	512	0	1991	433
Hourly flow rate (vph)	0	0	49	0	0	0	0	2501	512	0	1991	433
Pedestrians	24											
Lane Width (m)	3.6											
Walking Speed (m/s)	1.2											
Percent Blockage	2											
Right turn flare (veh)												
Median type							None	None	None	None	None	None
Median storage (veh)												
Upstream signal (m)							271					51
pX, platoon unblocked	0.86	0.86	0.75	0.86	0.86	0.73	0.75					0.73
vC, conflicting volume	3065	4732	904	3165	4949	834	2448					2501
vC1, stage 1 conf vol												
vC2, stage 2 conf vol	847	2794	0	963	3046	0	1761					1765
vCu, unblocked vol	7.5	6.5	7.1	7.5	6.5	6.9	4.1					4.1
tC, 2 stage (s)												
tF (s)	3.5	4.0	3.4	3.5	4.0	3.3	2.2					2.2
p0 queue free %	100	100	94	100	100	100	100					100
cM capacity (veh/h)	214	16	782	168	10	797	264					262
Direction_Lane #	EB 1	NB 1	NB 2	NB 3	NB 4	SB 1	SB 2	SB 3				
Volume Total	49	834	834	834	512	796	796	831				
Volume Left	0	0	0	0	0	0	0	0				
Volume Right	49	0	0	0	512	0	0	433				
GSH	782	1700	1700	1700	1700	1700	1700	1700				
Volume to Capacity	0.06	0.49	0.49	0.49	0.30	0.47	0.47	0.49				
Queue Length 95th (m)	1.6	0.0	0.0	0.0	0.0	0.0	0.0	0.0				
Control Delay (s)	9.9	0.0	0.0	0.0	0.0	0.0	0.0	0.0				
Lane LOS	A											
Approach Delay (s)	9.9	0.0										
Approach LOS	A											
Intersection Summary												
Average Delay	0.1											
Intersection Capacity Utilization	54.7%											
ICU Level of Service	A											
Analysis Period (min)	15											



Lanes, Volumes, Timings  
7: GO Bus Terminal/Argus Rd & Cross Ave

Lanes, Volumes, Timings  
5: Trafalgar Road & Cross Avenue/South Service Road

Lanes, Volumes, Timings  
2030 Total AM Peak Hour.syn

Lanes, Volumes, Timings  
2025 Background PM Peak Hour.syn

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Detector 2 Extend (s)	0.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
Turn Type	Perm	NA	Perm	pm-pt	NA	Perm	pm-pt	NA	Perm	Perm	NA	NA
Protected Phases		2	2	6	6	6	6	8	8	4	4	
Permitted Phases	2	2	2	6	6	6	6	8	8	4	4	
Detector Phase	2	2	2	2	2	2	2	2	2	2	2	2
Switch Phase												
Minimum Initial (s)	22.0	22.0	22.0	8.0	22.0	22.0	5.0	10.0	10.0	10.0	10.0	10.0
Minimum Split (s)	45.0	45.0	45.0	12.5	29.0	29.0	9.5	29.0	29.0	29.0	29.0	29.0
Total Split (s)	42.0	42.0	42.0	11.5	53.5	53.5	8.5	36.5	36.5	28.0	28.0	28.0
Total Split (%)	46.7%	46.7%	46.7%	12.8%	59.4%	59.4%	9.4%	40.6%	40.6%	31.1%	31.1%	31.1%
Maximum Green (s)	36.0	36.0	36.0	7.5	47.5	47.5	4.0	30.5	30.5	22.0	22.0	22.0
Yellow Time (s)	4.0	4.0	4.0	3.0	4.0	4.0	3.5	4.0	4.0	4.0	4.0	4.0
All-Red Time (s)	2.0	2.0	2.0	1.0	2.0	2.0	1.0	2.0	2.0	2.0	2.0	2.0
Lost Time Adjust (s)	-2.0	-2.0	-2.0	0.0	-2.0	-2.0	-2.0	0.0	0.0	-2.0	-2.0	-2.0
Total Lost Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	2.5	4.0	4.0	4.0	4.0	4.0
Lead/Lag	Lag	Lag	Lag	Lead	Lead	Lead	Lead	Lag	Lag	Lag	Lag	Lag
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Vehicle Extension (s)	5.0	5.0	5.0	2.5	5.0	5.0	3.0	4.0	4.0	4.0	4.0	4.0
Recall Mode	Min	Min	Min	Min	Min	Min	None	Min	Min	Min	Min	Min
Walk Time (s)	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0
Flash Dont Walk (s)	16.0	16.0	16.0	16.0	16.0	16.0	16.0	16.0	16.0	16.0	16.0	16.0
Pedestrian Calls (#/hr)	0	0	0	0	0	0	0	0	0	0	0	0
Act Effct Green (s)	27.9	27.9	27.9	39.5	39.5	39.5	34.1	30.6	24.1	24.1	24.1	24.1
Actuated G/C Ratio	0.35	0.35	0.35	0.49	0.49	0.49	0.43	0.38	0.30	0.30	0.30	0.30
v/c Ratio	0.39	0.66	0.21	0.93	0.55	0.07	1.03	0.47	0.63	1.31	1.31	1.31
Control Delay	26.3	23.1	1.5	74.3	15.5	0.6	124.2	5.2	30.8	175.7	175.7	175.7
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	26.3	23.1	1.5	74.3	15.5	0.6	124.2	5.2	30.8	175.7	175.7	175.7
LOS	C	C	A	E	B	A	F	A	C	C	F	F
Approach Delay	21.4				23.1		46.1			143.7		
Approach LOS	C				C		D			F		
Queue Length 50th (m)	8.6	42.3	0.0	14.1	47.8	0.0	-10.8	0.0	24.8	-121.5		
Queue Length 95th (m)	10.3	55.4	0.0	#40.9	61.6	0.3	#17.5	0.0	44.1	#111.2		
Internal Link Dist (m)	350.0				219.7		57.9			156.7		
Turn Bay Length (m)	20.0	60.0	20.0	60.0	60.0	873	105	437	367	522		
Base Capacity (vph)	257	1530	409	162	1994	873	105	437	367	522		
Stallcap Cap Reductn	0	0	0	0	0	0	0	0	0	0		
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0		
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0		
Reduced v/c Ratio	0.28	0.41	0.18	0.93	0.43	0.05	1.03	0.47	0.53	1.31		
Intersection Summary	Intersection LOS: E											
Area Type:	CBD											
Cycle Length: 90												
Actuated Cycle Length: 80.1												
Natural Cycle: 130												
Control Type: Semi Act-Uncoord												
Maximum v/c Ratio: 1.31												
Intersection Signal Delay: 59.9												
Baseline												
Synchro 10 Report Page 22												



HCM Signalized Intersection Capacity Analysis  
6: Trafalgar Road

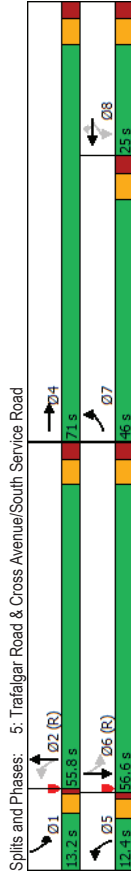
2030 Total AM Peak Hour.syn  
05-04-2022

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations	507	608	96	11	511	626	68	486	57	456	444	379	
Traffic Volume (vph)	507	608	96	11	511	626	68	486	57	456	444	379	
Future Volume (vph)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	
Ideal Flow (vphpl)	4.0	2.0	4.0	4.0	4.0	4.0	4.0	4.0	7.0	4.0	4.0	4.0	
Total Lost time (s)	0.97	0.95	1.00	0.95	1.00	1.00	1.00	0.95	1.00	0.97	1.00	1.00	
Lane Util. Factor	1.00	0.99	1.00	1.00	0.93	1.00	1.00	0.95	1.00	1.00	0.97	1.00	
Frb, ped/bikes	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Frb, ped/bikes	1.00	0.98	1.00	1.00	0.85	1.00	1.00	0.85	1.00	0.85	1.00	0.85	
Frt	3433	3392	1703	3505	1460	1770	3574	1529	3367	1863	1474	1474	
Satd. Flow (prot)	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	
Flt Permitted	3433	3392	1703	3505	1460	1770	3574	1529	3367	1863	1474	1474	
Satd. Flow (perm)	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	
Peak-hour factor, PHF	551	661	104	12	555	680	74	528	62	496	483	412	
Adj. Flow (vph)	0	8	0	0	0	187	0	49	0	243	0	243	
RTOR Reduction (vph)	551	757	0	12	555	493	74	528	13	496	483	169	
Lane Group Flow (vph)	25	757	7	7	25	9	9	18	18	18	18	9	
Conf. Peds. (#/hr)	2%	4%	1%	6%	3%	3%	2%	1%	0%	4%	2%	6%	
Heavy Vehicles (%)	Prot	NA	Prot	NA	Prot	NA	Prot	NA	Prot	NA	Prot	NA	
Turn Type	7	4		3	8		5	2		1		6	
Protected Phases													
Permitted Phases	22.0	66.2	2.8	45.0	45.0	7.0	29.0	29.0	20.0	42.0	42.0	6	
Actuated Green, G (s)	23.0	69.2	3.8	48.0	48.0	8.0	32.0	32.0	21.0	45.0	45.0	45.0	
Effective Green, g (s)	0.16	0.49	0.03	0.34	0.34	0.06	0.23	0.21	0.15	0.32	0.32	0.32	
Actuated g/C Ratio	5.0	5.0	5.0	7.0	7.0	5.0	7.0	7.0	5.0	7.0	7.0	7.0	
Clearance Time (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	
Vehicle Extension (s)	563	1676	46	1201	500	101	816	316	505	598	473	473	
Lane Grp. Cap (vph)	c0.16	0.22	0.01	0.16	0.16	0.04	0.15	0.15	c0.15	c0.26	c0.26	0.11	
v/s Ratio Prot	0.98	0.45	0.26	0.46	0.99	0.73	0.65	0.04	0.98	0.81	0.36	0.36	
v/s Ratio Perm	58.3	23.0	66.7	35.9	45.7	64.9	48.9	44.4	59.3	43.5	36.4	36.4	
Uniform Delay, d1	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.47	0.47	0.24	
Progression Factor	32.1	0.2	3.0	0.3	36.2	23.7	3.9	0.2	34.2	10.6	2.0	2.0	
Incremental Delay, d2	90.4	23.2	69.7	36.2	81.8	88.7	52.8	44.6	121.5	31.2	10.8	10.8	
Delay (s)	F	C	E	D	F	F	D	D	F	C	B	B	
Level of Service	51.4		61.4		56.1		57.4		57.4		E	E	
Approach Delay (s)													
Approach LOS													
Intersection Summary													
HCM 2000 Control Delay	56.6											HCM 2000 Level of Service	E
HCM 2000 Volume to Capacity ratio	0.95												
Actuated Cycle Length (s)	140.0											Sum of lost time (s)	16.0
Intersection Capacity Utilization	82.2%											ICU Level of Service	E
Analysis Period (min)	15												
c Critical Lane Group													

Lanes, Volumes, Timings  
5: Trafalgar Road & Cross Avenue/South Service Road

2025 Background PM Peak Hour.syn  
05-04-2022

Analysis Period (min)	15	ICU Level of Service F
Intersection Capacity Utilization	94.6%	
Volume exceeds capacity, queue is theoretically infinite.		
Queue shown is maximum after two cycles.		
# 95th percentile volume exceeds capacity, queue may be longer.		
Queue shown is maximum after two cycles.		
m Volume for 95th percentile queue is metered by upstream signal.		



HCM Signalized Intersection Capacity Analysis  
 5: Trafalgar Road & Cross Avenue/South Service Road

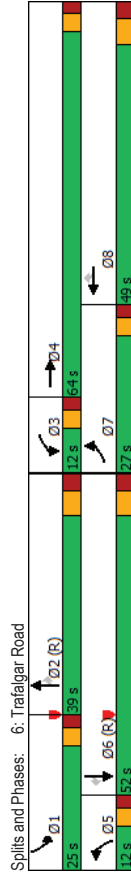
Lanes, Volumes, Timings  
 6: Trafalgar Road

2025 Background PM Peak Hour.syn  
 05-04-2022

2030 Total AM Peak Hour.syn  
 05-04-2022

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	1015	91	140	160	153	291	136	1467	44	139	1475	340
Traffic Volume (vph)	1015	91	140	160	153	291	136	1467	44	139	1475	340
Future Volume (vph)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Ideal Flow (vphpl)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Total Lost time (s)	0.97	1.00	1.00	1.00	1.00	1.00	0.91	1.00	0.91	1.00	0.91	1.00
Lane Util. Factor	1.00	0.97	1.00	1.00	1.00	1.00	1.00	0.99	1.00	0.99	1.00	0.99
Frb, ped/bikes	1.00	1.00	0.98	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Fipb, ped/bikes	1.00	0.91	1.00	1.00	0.85	1.00	1.00	1.00	1.00	1.00	0.97	1.00
Frt	0.95	1.00	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	0.95	1.00
Flt Protected	3400	1546	1726	1827	1599	1719	5072	1805	4923	1805	4923	1805
Satd. Flow (prot)	0.95	1.00	0.60	1.00	1.00	0.08	1.00	1.00	0.08	1.00	0.08	1.00
Flt Permitted	3400	1546	1096	1827	1599	148	5072	153	4923	153	4923	153
Satd. Flow (perm)	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Peak-hour factor, PHF	1103	99	152	174	166	316	148	1595	48	151	1603	370
Adj. Flow (vph)	0	40	0	0	0	99	0	3	0	0	27	0
RTOR Reduction (vph)	1103	211	0	174	166	217	148	1640	0	151	1946	0
Lane Group Flow (vph)	3%	8%	9%	2%	4%	1%	5%	1%	0%	0%	1%	3%
Conf. Peds. (#/hr)	Prot	NA	Perm	NA	Perm	pm+pt	NA	pm+pt	NA	pm+pt	NA	NA
Heavy Vehicles (%)	7	4				8	5	2		1	6	
Turn Type	Permitted Phases											
	Actuated Green, G (s)	39.0	64.0	18.0	18.0	18.0	57.2	48.8	58.8	58.8	49.6	52.6
	Effective Green, g (s)	42.0	67.0	21.0	21.0	21.0	57.2	51.8	58.8	58.8	52.6	52.6
	Actuated g/C Ratio	0.30	0.48	0.15	0.15	0.15	0.41	0.37	0.42	0.42	0.38	0.38
	Clearance Time (s)	7.0	7.0	7.0	7.0	7.0	4.0	7.0	4.0	7.0	4.0	7.0
	Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
	Lane Grp. Cap (vph)	1020	739	164	274	239	154	1876	172	1849	172	1849
	v/s Ratio Prot	c0.32	0.14		0.09		0.06	0.32	c0.06	c0.40		
	v/s Ratio Perm	1.08	0.29	1.06	0.61	0.14	0.33	0.31	0.88	0.88	1.05	1.05
	Uniform Delay, d1	49.0	22.1	59.5	55.6	58.5	34.6	41.1	33.0	43.7	33.0	43.7
	Progression Factor	1.00	1.00	1.00	1.00	1.00	1.15	1.15	0.83	0.83	0.89	0.89
	Incremental Delay, d2	52.9	0.2	87.4	3.8	33.9	34.2	2.4	29.2	33.9	29.2	33.9
	Delay (s)	101.9	22.3	146.9	59.4	92.4	85.9	49.8	56.4	72.9	56.4	72.9
	Level of Service	F	C	F	E	F	F	D	E	D	E	E
	Approach Delay (s)			87.1			98.5		52.8		71.7	
	Approach LOS			F			F		D		E	
<b>Intersection Summary</b>												
HCM 2000 Control Delay	72.5 HCM 2000 Level of Service E											
HCM 2000 Volume to Capacity ratio	1.06											
Actuated Cycle Length (s)	140.0 Sum of lost time (s) 16.0											
Intersection Capacity Utilization	94.6% ICU Level of Service F											
Analysis Period (min)	15											
c Critical Lane Group												

Intersection Capacity Utilization 82.2% ICU Level of Service E  
 Analysis Period (min) 15  
 ~ Volume exceeds capacity, queue is theoretically infinite.  
 Queue shown is maximum after two cycles.  
 # 95th percentile volume exceeds capacity, queue may be longer.  
 Queue shown is maximum after two cycles.



Lanes, Volumes, Timings  
6: Trafalgar Road

Lanes, Volumes, Timings  
6: Trafalgar Road

2030 Total AM Peak Hour.syn  
05-04-2022

2025 Background PM Peak Hour.syn  
05-04-2022

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	503	538	190	67	750	661	134	476	46	571	617	551
Traffic Volume (vph)	503	538	190	67	750	661	134	476	46	571	617	551
Future Volume (vph)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Ideal Flow (vphpl)	80.0	30.0	80.0	30.0	80.0	30.0	25.0	0.0	80.0	30.0	80.0	0.0
Storage Length (m)	2	1	1	1	1	1	1	1	1	1	1	1
Storage Lanes	7.5	7.5	7.5	7.5	7.5	7.5	7.5	7.5	7.5	7.5	7.5	7.5
Taper Length (m)	0.97	0.95	1.00	1.00	0.95	1.00	0.95	1.00	0.95	0.97	1.00	1.00
Lane Util. Factor	0.98	0.93	0.93	0.98	0.94	0.98	1.00	0.98	0.98	0.98	0.95	0.95
Ped Bike Factor	0.98	0.95	0.95	0.95	0.95	0.850	0.987	0.987	0.987	0.987	0.850	0.850
Fit	0.950	0.950	0.950	0.950	0.950	0.950	0.950	0.950	0.950	0.950	0.950	0.950
Satd. Flow (prot)	3467	3505	1599	1805	3574	1599	1770	3552	0	3433	1900	1583
Flow Permitted	0.950	0.950	0.950	0.950	0.950	0.950	0.950	0.950	0.950	0.950	0.950	0.950
Satd. Flow (perm)	3412	3505	1482	1761	3574	1503	1742	3552	0	3372	1900	1503
Right Turn on Red	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Satd. Flow (RTOR)	148	148	148	148	148	148	148	148	7	7	7	7
Link Speed (k/h)	98.1	98.1	98.1	98.1	98.1	98.1	98.1	98.1	98.1	98.1	98.1	98.1
Link Distance (m)	7.1	7.1	7.1	7.1	7.1	7.1	7.1	7.1	7.1	7.1	7.1	7.1
Travel Time (s)	21	14	14	14	21	17	17	17	10	10	17	17
Confl. Peds. (#/hr)	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Peak Hour Factor	1%	3%	1%	0%	1%	1%	2%	0%	0%	2%	0%	2%
Heavy Vehicles (%)	547	585	207	73	815	718	146	517	50	621	671	599
Adj. Flow (vph)	547	585	207	73	815	718	146	517	50	621	671	599
Shared Lane Traffic (%)	547	585	207	73	815	718	146	517	50	621	671	599
Lane Group Flow (vph)	No	No	No	No	No	No	No	No	No	No	No	No
Enter Blocked Intersection	Left	Right	Left	Right	Left	Right	Left	Right	Left	Right	Left	Right
Lane Alignment	7.2	7.2	7.2	7.2	7.2	7.2	7.2	7.2	7.2	7.2	7.2	7.2
Median Width (m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Link Offset (m)	4.8	4.8	4.8	4.8	4.8	4.8	4.8	4.8	4.8	4.8	4.8	4.8
Crosswalk Width (m)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Two way Left Turn Lane	25	15	25	15	25	15	25	15	25	25	15	15
Headway Factor	1	2	1	1	2	1	1	2	1	2	1	1
Turning Speed (k/h)	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Number of Detectors	2.0	10.0	2.0	2.0	10.0	2.0	2.0	10.0	2.0	10.0	2.0	2.0
Detector Template	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Leading Detector (m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Trailing Detector (m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Position (m)	2.0	0.6	2.0	2.0	0.6	2.0	0.6	2.0	0.6	2.0	0.6	2.0
Detector 1 Size (m)	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex
Detector 1 Type	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Channel	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Delay (s)	9.4	9.4	9.4	9.4	9.4	9.4	9.4	9.4	9.4	9.4	9.4	9.4
Detector 2 Position (m)	0.6	0.6	0.6	0.6	0.6	0.6	0.6	0.6	0.6	0.6	0.6	0.6
Detector 2 Size (m)	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex
Detector 2 Type	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 2 Channel	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 2 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0

Baseline

Baseline

Synchro 10 Report  
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Synchro 10 Report  
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HCM Signalized Intersection Capacity Analysis  
 5: Trafalgar Road & Cross Avenue/South Service Road

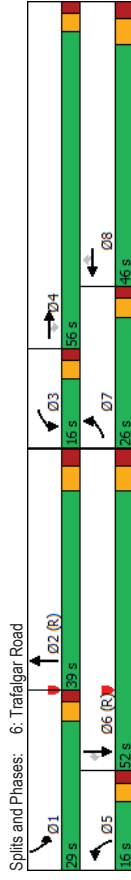
2030 Total AM Peak Hour.syn  
 05-04-2022

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔
Traffic Volume (vph)	615	247	148	67	229	156	144	941	541	336	912	560
Future Volume (vph)	615	247	148	67	229	156	144	941	541	336	912	560
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lane Util. Factor	0.97	0.95	1.00	0.97	0.95	1.00	1.00	0.91	1.00	0.91	1.00	0.91
Frb, ped/bikes	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Fipb, ped/bikes	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt	1.00	1.00	0.85	1.00	1.00	0.85	1.00	1.00	0.85	1.00	1.00	0.85
Flt Protected	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00
Satd. Flow (prot)	3213	3406	1411	3400	3343	1545	1639	5036	1471	1752	5036	1499
Flt Permitted	0.95	1.00	1.00	0.95	1.00	1.00	0.27	1.00	1.00	0.12	1.00	1.00
Satd. Flow (perm)	3213	3406	1411	3400	3343	1545	470	5036	1471	220	5036	1499
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	668	288	161	73	249	170	157	1023	588	365	991	609
RTOR Reduction (vph)	0	0	123	0	0	43	0	0	87	0	0	342
Lane Group Flow (vph)	668	288	38	73	249	127	157	1023	501	365	991	267
Conf. Peds. (#/hr)	1	1	4	4	4	4	1	10	10	52	52	10
Heavy Vehicles (%)	9%	6%	12%	3%	8%	4%	10%	3%	0%	3%	3%	4%
Turn Type	Prot	NA	Perm	Prot	NA	pm+ov	pm+pt	NA	pm+ov	pm+pt	NA	Perm
Protected Phases	7	4		3	8	1	5	2	3	1	6	
Permitted Phases			4			8	2		2	6		6
Actuated Green, G (s)	30.9	29.7	29.7	19.5	15.3	42.8	53.2	41.3	60.8	72.8	56.9	56.9
Effective Green, g (s)	33.9	32.7	32.7	19.5	18.3	42.8	53.2	44.3	60.8	72.8	59.9	59.9
Actuated g/C Ratio	0.24	0.23	0.23	0.14	0.13	0.31	0.38	0.32	0.43	0.52	0.43	0.43
Clearance Time (s)	7.0	7.0	4.0	7.0	4.0	7.0	4.0	7.0	4.0	7.0	4.0	7.0
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Lane Grp Cap (vph)	778	795	329	473	436	472	277	1593	638	415	2154	641
v/s Ratio Prot	c0.21	0.08		0.02	c0.07	0.05	0.05	0.20	0.11	c0.17	0.20	
v/s Ratio Perm			0.03		0.03	0.17		0.23	0.28		0.28	0.18
v/c Ratio	0.86	0.34	0.11	0.15	0.57	0.27	0.57	0.64	0.79	0.88	0.46	0.42
Uniform Delay, d1	50.8	44.6	42.2	53.0	57.2	36.8	29.8	41.1	34.0	37.0	28.5	27.9
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.11	1.07	1.17	1.02	0.79	0.50
Incremental Delay, d2	9.3	0.3	0.2	0.2	1.8	0.3	1.2	0.9	3.0	6.4	0.2	0.6
Delay (s)	60.1	44.9	42.4	53.2	59.0	37.1	34.3	45.0	42.8	44.0	22.7	14.6
Level of Service	E	D	D	D	E	D	C	D	D	D	C	B
Approach Delay (s)	53.8			50.5			43.3				24.1	
Approach LOS	D			D			D				C	
<b>Intersection Summary</b>												
HCM 2000 Control Delay	39.1 HCM 2000 Level of Service D											
HCM 2000 Volume to Capacity ratio	0.83											
Actuated Cycle Length (s)	140.0 Sum of lost time (s) 16.0											
Intersection Capacity Utilization	80.5% ICU Level of Service D											
Analysis Period (min)	15											
c Critical Lane Group												

Lanes, Volumes, Timings  
 6: Trafalgar Road

2025 Background PM Peak Hour.syn  
 05-04-2022

Intersection Capacity Utilization	88.3%
Analysis Period (min)	15
ICU Level of Service	E
Volume exceeds capacity, queue is theoretically infinite.	
Queue shown is maximum after two cycles.	
# 95th percentile volume exceeds capacity, queue may be longer.	
Queue shown is maximum after two cycles.	
m Volume for 95th percentile queue is metered by upstream signal.	

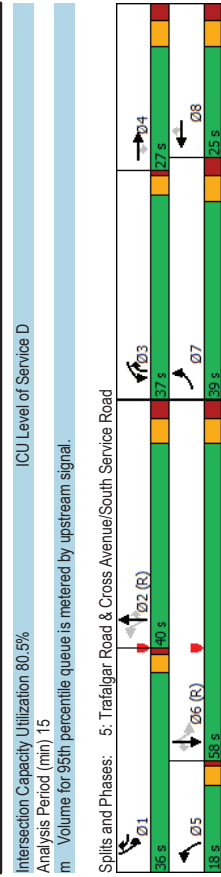




HCM Signalized Intersection Capacity Analysis 2025 Background PM Peak Hour.syn 05-04-2022

Lanes, Volumes, Timings 2030 Total AM Peak Hour.syn 05-04-2022

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	←	←	←	←	←	←	←	←	←	←	←	←
Traffic Volume (vph)	503	538	190	67	750	661	134	476	46	571	617	551
Future Volume (vph)	503	538	190	67	750	661	134	476	46	571	617	551
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	2.0	5.0	4.0	4.0	7.0	4.0	4.0	4.0	4.0	4.0	4.0
Lane Util. Factor	0.97	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	0.97	1.00	1.00
Frb, ped/bikes	1.00	1.00	0.93	1.00	1.00	0.94	1.00	1.00	1.00	1.00	1.00	0.95
Frb, ped/bikes	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt	1.00	1.00	0.85	1.00	1.00	0.85	1.00	0.99	1.00	1.00	1.00	0.85
Flt Protected	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	0.95	1.00	1.00	0.95
Satd. Flow (prot)	3467	3505	1482	1805	3574	1503	1770	3551	3433	1900	1503	1503
Flt Permitted	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	0.95	1.00	1.00	0.95
Satd. Flow (perm)	3467	3505	1482	1805	3574	1503	1770	3551	3433	1900	1503	1503
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	547	585	207	73	815	718	146	517	50	621	671	599
RTOR Reduction (vph)	0	0	90	0	0	244	0	5	0	0	0	197
Lane Group Flow (vph)	547	585	117	73	815	474	146	562	0	621	671	402
Conf. Peds. (#/hr)	21	3%	1%	0%	1%	1%	2%	0%	2%	0%	2%	0%
Heavy Vehicles (%)	1%	3%	1%	0%	1%	1%	2%	0%	2%	0%	2%	0%
Turn Type	Prot	NA	Perm	Prot	NA	Perm	Prot	NA	Prot	NA	Prot	NA
Protected Phases	7	4		3	8		5	2		1		6
Permitted Phases			4			8						6
Actuated Green, G (s)	21.0	54.6	54.6	8.4	40.0	40.0	11.0	31.0	24.0	44.0	44.0	44.0
Effective Green, g (s)	22.0	57.6	54.6	9.4	43.0	40.0	12.0	34.0	25.0	47.0	47.0	47.0
Actuated g/C Ratio	0.16	0.41	0.39	0.07	0.31	0.29	0.09	0.24	0.18	0.34	0.34	0.34
Clearance Time (s)	5.0	5.0	5.0	5.0	7.0	7.0	5.0	7.0	5.0	7.0	7.0	7.0
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Lane Grp. Cap (vph)	544	1442	577	121	1097	429	151	862	613	637	504	504
v/s Ratio Prot	c0.16	0.17		0.04	0.23		0.08	0.16	c0.18	c0.35		
v/s Ratio Perm			0.08			c0.32						0.27
v/c Ratio	1.01	0.41	0.20	0.60	0.74	1.11	0.97	0.65	1.01	1.05	0.80	0.80
Uniform Delay, d1	59.0	29.1	28.3	63.5	43.5	50.0	63.8	47.7	57.5	46.5	42.2	42.2
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.24	1.26	1.62	1.62
Incremental Delay, d2	40.0	0.2	0.2	8.2	2.8	75.4	62.6	3.8	20.9	33.4	3.2	3.2
Delay (s)	99.0	29.3	28.5	71.7	46.3	125.4	126.4	51.5	92.1	91.8	71.7	71.7
Level of Service	F	C	C	E	D	F	F	D	F	F	F	E
Approach Delay (s)	57.6			82.8			66.8				85.5	
Approach LOS	E			F			E				F	
Intersection Summary												
HCM 2000 Control Delay	75.6 HCM 2000 Level of Service E											
HCM 2000 Volume to Capacity ratio	1.05											
Actuated Cycle Length (s)	140.0 Sum of lost time (s) 16.0											
Intersection Capacity Utilization	88.3% ICU Level of Service E											
Analysis Period (min)	15											
c Critical Lane Group												



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Turn Type	7	4	4	3	8	1	5	2	3	2	1	6
Protected Phases	NA	Perm	4	3	8	8	2	2	3	2	6	6
Permitted Phases	7	4	4	3	8	1	5	2	3	1	6	6
Detector Phase	7	4	4	3	8	1	5	2	3	1	6	6
Switch Phase												
Minimum Initial (s)	10.0	5.0	5.0	12.0	10.0	7.0	7.0	27.0	12.0	7.0	27.0	27.0
Minimum Split (s)	17.0	25.0	25.0	17.0	25.0	11.5	11.5	34.0	17.0	11.5	34.0	34.0
Total Split (s)	39.0	27.0	27.0	37.0	25.0	36.0	18.0	40.0	37.0	36.0	58.0	58.0
Total Split (%)	27.9%	19.3%	19.3%	26.4%	17.9%	25.7%	12.9%	28.6%	26.4%	25.7%	41.4%	41.4%
Maximum Green (s)	32.0	20.0	20.0	33.0	18.0	32.0	14.0	33.0	33.0	32.0	51.0	51.0
Yellow Time (s)	4.0	4.0	4.0	3.0	4.0	3.0	4.0	3.0	4.0	3.0	4.0	4.0
All-Red Time (s)	3.0	3.0	3.0	1.0	3.0	1.0	3.0	1.0	3.0	1.0	3.0	3.0
Lost Time Adjust (s)	-3.0	-3.0	-3.0	0.0	-3.0	0.0	-3.0	0.0	0.0	-3.0	-3.0	-3.0
Total Lost Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lead/Lag	Lead	Lag	Lag	Lead	Lag	Lead	Lag	Lead	Lag	Lead	Lag	Lag
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	None	None	None	None	None	None	None	None	None	None	C-Max	C-Max
Walk Time (s)	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0
Flash Dont Walk (s)	11.0	11.0	11.0	11.0	11.0	11.0	11.0	11.0	11.0	11.0	11.0	11.0
Pedestrian Calls (#/hr)	0	0	0	0	0	0	0	0	0	0	0	0
Act Effct Green (s)	33.9	32.7	32.7	19.5	18.3	45.9	56.1	44.2	63.7	75.7	59.8	59.8
Actuated g/C Ratio	0.24	0.23	0.23	0.14	0.13	0.33	0.40	0.32	0.46	0.54	0.43	0.43
v/c Ratio	0.86	0.34	0.36	0.15	0.57	0.31	0.55	0.64	0.78	0.87	0.46	0.62
Control Delay	62.6	47.1	9.3	50.8	62.2	20.2	25.5	47.1	30.4	41.0	23.5	2.2
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	62.6	47.1	9.3	50.8	62.2	20.2	25.5	47.1	30.4	41.0	23.5	2.2
LOS	E	D	A	D	E	C	C	D	C	D	C	A
Approach Delay	51.0			46.0			39.6				20.1	
Approach LOS	D			D			D				C	
Queue Length 50th (m)	95.6	33.1	0.0	10.0	36.2	21.2	26.2	84.6	93.1	65.9	73.0	5.1
Queue Length 95th (m)	120.0	52.5	20.7	15.8	50.5	37.6	34.7	85.4	108.6	88.6	71.2	7.0
Internal Link Dist (m)	219.7			214.9			242.5				247.0	
Turn Bay Length (m)	25.0	60.0	25.0	60.0	25.0	60.0	50.0	60.0	60.0	25.0	60.0	60.0
Base Capacity (vph)	803	796	453	801	501	596	312	1589	892	470	2152	982
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.83	0.34	0.36	0.09	0.50	0.29	0.50	0.64	0.66	0.78	0.46	0.62
Intersection Summary												
Area Type:	Other											
Actuated Cycle Length:	140											
Offset: S4.5 (93%), Referenced to phase 2:N2BL and 6:SBTL - Start of Green												
Natural Cycle:	110											
Control Type:	Actuated-Coordinated											
Maximum v/c Ratio:	0.87											
Intersection Signal Delay:	35.4											
Intersection LOS:	D											

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	15	886	17	38	368	44	16	2	49	151	20	136
Traffic Volume (vph)	15	886	17	38	368	44	16	2	49	151	20	136
Future Volume (vph)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Ideal Flow (vphpl)	3.3	3.6	3.6	3.3	3.6	3.6	3.3	3.6	3.6	3.3	3.6	3.6
Lane Width (m)	20.0	0.0	0.0	20.0	0.0	0.0	0.0	0.0	0.0	15.0	0.0	0.0
Storage Length (m)	20.0	0.0	0.0	20.0	0.0	0.0	0.0	0.0	0.0	15.0	0.0	0.0
Storage Lanes	1	0	0	1	0	0	1	0	0	1	0	0
Taper Length (m)	7.5	0.0	0.0	7.5	0.0	0.0	7.5	0.0	0.0	7.5	0.0	0.0
Lane Util. Factor	1.00	0.95	0.95	1.00	0.95	0.95	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor	0.99	1.00	1.00	0.99	1.00	0.99	0.99	0.95	0.95	0.97	0.98	0.98
Fit	0.997			0.982			0.865			0.870		
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1570	3172	0	797	3176	0	785	701	0	1570	1282	0
Flt Permitted	0.496			0.129			0.526			0.704		
Satd. Flow (perm)	812	3172	0	108	3176	0	431	701	0	1123	1282	0
Right Turn on Red		Yes		Yes			Yes		Yes	Yes		Yes
Satd. Flow (RTOR)		3		33			73		73	174		174
Link Speed (k/h)	374.0			50			50		50	180.7		180.7
Travel Time (s)	26.9			17.5			5.9		5.9	13.0		13.0
Confl. Peds. (#/hr)	9	4	4	4	9	12	20	20	20	20	20	12
Peak Hour Factor	0.66	0.81	0.75	0.94	0.95	0.81	0.70	0.25	0.67	0.86	0.75	0.78
Heavy Vehicles (%)	0%	0%	100%	97%	0%	0%	100%	100%	100%	0%	100%	0%
Adj. Flow (vph)	23	1084	23	40	387	54	23	8	73	176	27	174
Shared Lane Traffic (%)												
Lane Group Flow (vph)	23	1117	0	40	441	0	23	81	0	176	201	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(m)	7.2			7.2			3.3		3.3	3.3		3.3
Link Offset(m)	0.0			0.0			0.0		0.0	0.0		0.0
Crosswalk Width(m)	4.8			4.8			4.8		4.8	4.8		4.8
Two way Left Turn Lane												
Headway Factor	1.19	1.14	1.14	1.19	1.14	1.14	1.19	1.14	1.14	1.19	1.14	1.14
Turning Speed (k/h)	24	14	24	24	14	24	24	14	24	14	24	14
Number of Detectors	1	2	1	1	2	1	1	2	1	2	1	2
Detector Template	Left	Thru	Left	Left	Thru	Left	Thru	Left	Thru	Left	Thru	Thru
Leading Detector (m)	2.0	10.0	2.0	2.0	10.0	2.0	10.0	2.0	10.0	2.0	10.0	10.0
Trailing Detector (m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Position(m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Size(m)	2.0	0.6	2.0	0.6	2.0	0.6	2.0	0.6	2.0	0.6	2.0	0.6
Detector 1 Type	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex
Detector 1 Channel	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 2 Position(m)	9.4			9.4			9.4		9.4		9.4	
Detector 2 Size(m)	0.6			0.6			0.6		0.6		0.6	
Detector 2 Type	Ch+Ex			Ch+Ex			Ch+Ex		Ch+Ex		Ch+Ex	
Detector 2 Channel	Ch+Ex			Ch+Ex			Ch+Ex		Ch+Ex		Ch+Ex	

Lanes, Volumes, Timings  
 7: GO Bus Terminal/Argus Rd & Cross Ave

Lanes, Volumes, Timings  
 5: Trafalgar Road & Cross Avenue/South Service Road

2025 Background PM Peak Hour.syn  
 05-04-2022

2030 Total AM Peak Hour.syn  
 05-04-2022

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Detector 2 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0	10.0	10.0	10.0	10.0	0.0	0.0
Turn Type	Perm	NA	pm-pt	NA	Perm	NA	Perm	NA	NA	Perm	NA	NA
Protected Phases	2	2	1	6	8	8	8	8	8	4	4	4
Permitted Phases	2	2	6	6	6	6	8	8	8	4	4	4
Detector Phase	2	2	1	6	6	6	8	8	8	4	4	4
Switch Phase												
Minimum Initial (s)	22.0	22.0	8.0	22.0	22.0	22.0	10.0	10.0	10.0	10.0	10.0	10.0
Minimum Split (s)	45.0	45.0	12.5	29.0	29.0	29.0	29.0	29.0	29.0	29.0	29.0	29.0
Total Split (s)	46.0	46.0	15.0	61.0	61.0	61.0	29.0	29.0	29.0	29.0	29.0	29.0
Total Split (%)	51.1%	51.1%	16.7%	67.8%	67.8%	67.8%	32.2%	32.2%	32.2%	32.2%	32.2%	32.2%
Maximum Green (s)	40.0	40.0	11.0	65.0	65.0	65.0	23.0	23.0	23.0	23.0	23.0	23.0
Yellow Time (s)	4.0	4.0	3.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
All-Red Time (s)	2.0	2.0	1.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
Lost Time Adjust (s)	-2.0	-2.0	0.0	-2.0	-2.0	-2.0	-2.0	-2.0	-2.0	-2.0	-2.0	-2.0
Total Lost Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lead/Lag	Lag	Lag	Lead									
Lead-Lag Optimize?	Yes	Yes	Yes									
Vehicle Extension (s)	5.0	5.0	2.5	5.0	5.0	5.0	4.0	4.0	4.0	4.0	4.0	4.0
Recall Mode	Min	Min	Min	Min	Min	Min	Min	Min	Min	Min	Min	Min
Walk Time (s)	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0
Flash Dont Walk (s)	16.0	16.0	16.0	16.0	16.0	16.0	16.0	16.0	16.0	16.0	16.0	16.0
Pedestrian Calls (#/hr)	0	0	0	0	0	0	0	0	0	0	0	0
Act Effct Green (s)	37.4	37.4	49.8	49.8	49.8	49.8	19.6	19.6	19.6	19.6	19.6	19.6
Actuated g/C Ratio	0.48	0.48	0.64	0.64	0.64	0.64	0.25	0.25	0.25	0.25	0.25	0.25
v/c Ratio	0.06	0.73	0.28	0.22	0.22	0.21	0.35	0.35	0.62	0.44	0.44	0.44
Control Delay	12.5	20.0	11.2	6.0	6.0	29.6	12.3	37.3	37.3	9.4	9.4	9.4
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	12.5	20.0	11.2	6.0	6.0	29.6	12.3	37.3	37.3	9.4	9.4	9.4
LOS	B	B	B	A	C	B	C	B	D	D	A	A
Approach Delay			19.8				16.1				22.4	
Approach LOS			B				B				C	
Queue Length 50th (m)	1.8	70.0	2.1	12.4	3.0	1.0	3.0	1.0	25.6	3.4	3.4	3.4
Queue Length 95th (m)	4.6	89.3	6.5	21.6	7.4	0.0	7.4	0.0	45.5	12.2	12.2	12.2
Internal Link Dist (m)			350.0				57.9				156.7	
Turn Bay Length (m)	20.0		20.0				142		280		15.0	
Base Capacity (vph)	449	1756	169	2393			369		538		369	
Stevation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.05	0.64	0.24	0.18	0.16	0.29	0.48	0.37				
Intersection Summary	CBD											
Area Type:	CBD											
Cycle Length: 90	77.6											
Actuated Cycle Length: 77.6												
Natural Cycle: 90												
Control Type: Semi Act-Uncoord												
Maximum v/c Ratio: 0.73												
Intersection Signal Delay: 17.1	Intersection LOS: B											

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔
Traffic Volume (vph)	615	247	148	67	229	156	144	941	541	336	912	560
Future Volume (vph)	615	247	148	67	229	156	144	941	541	336	912	560
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (m)	25.0	60.0	25.0	60.0	25.0	60.0	50.0	60.0	25.0	60.0	25.0	60.0
Storage Lanes	2	1	2	1	2	1	1	1	1	1	1	1
Taper Length (m)	7.5	7.5	7.5	7.5	7.5	7.5	7.5	7.5	7.5	7.5	7.5	7.5
Lane Util. Factor	0.97	0.95	1.00	0.97	0.95	1.00	1.00	0.91	1.00	1.00	0.91	1.00
Ped Bike Factor	1.00	0.98	0.98	0.99	0.99	0.99	0.99	0.87	0.87	0.87	0.97	0.97
Fit	0.850	0.850	0.850	0.850	0.850	0.850	0.850	0.850	0.850	0.850	0.850	0.850
Fit Protected	0.950	0.950	0.950	0.950	0.950	0.950	0.950	0.950	0.950	0.950	0.950	0.950
Satd. Flow (prot)	3213	3406	1442	3400	3343	1553	1641	5036	1615	1752	5036	1553
Fit Permitted	0.950	0.950	0.950	0.950	0.950	0.950	0.273	0.273	0.273	0.119	0.119	0.119
Satd. Flow (perm)	3207	3406	1411	3378	3343	1531	469	5036	1404	220	5036	1499
Right Turn on Red	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Satd. Flow (RTOR)	161	161	161	62	62	62	50	50	154	50	50	597
Link Speed (k/h)	243.7	17.5	17.5	238.9	17.2	17.2	19.2	19.2	19.5	19.5	19.5	19.5
Travel Time (s)	1	4	4	1	10	10	52	52	10	10	10	10
Confl. Peds. (#/hr)	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Peak Hour Factor	9%	12%	3%	8%	4%	10%	3%	0%	3%	3%	3%	4%
Heavy Vehicles (%)	668	268	161	73	249	170	157	1023	588	365	991	609
Adj. Flow (vph)	668	268	161	73	249	170	157	1023	588	365	991	609
Shared Lane Traffic (%)												
Lane Group Flow (vph)	No	No	No	No	No	No	No	No	No	No	No	No
Enter Blocked Intersecion	Left	Right	Left	Right	Left	Right	Left	Left	Right	Left	Left	Right
Lane Alignment	Left	Right	Left	Right	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(m)	7.2	7.2	7.2	7.2	7.2	7.2	3.6	3.6	3.6	3.6	3.6	3.6
Link Offset(m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Crosswalk Width(m)	4.8	4.8	4.8	4.8	4.8	4.8	4.8	4.8	4.8	4.8	4.8	4.8
Two way Left Turn Lane												
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (k/h)	25	15	25	15	25	15	25	15	25	15	25	15
Number of Detectors	1	2	1	1	2	1	1	2	1	1	2	1
Detector Template	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Leading Detector (m)	2.0	10.0	2.0	2.0	10.0	2.0	2.0	10.0	2.0	2.0	10.0	2.0
Trailing Detector (m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Position(m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Size(m)	2.0	0.6	2.0	2.0	0.6	2.0	0.6	2.0	0.6	2.0	0.6	2.0
Detector 1 Type	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 2 Position(m)	9.4	9.4	9.4	9.4	9.4	9.4	9.4	9.4	9.4	9.4	9.4	9.4
Detector 2 Size(m)	0.6	0.6	0.6	0.6	0.6	0.6	0.6	0.6	0.6	0.6	0.6	0.6
Detector 2 Type	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex
Detector 2 Channel												
Detector 2 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0

HCM Unsignalized Intersection Capacity Analysis  
 4: Trafalgar Road & Argus Road/QEW EB On Ramp

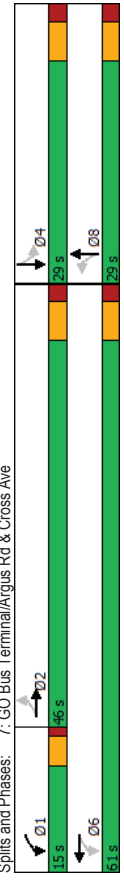
2030 Total AM Peak Hour.syn  
 05-04-2022

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	0	0	39	0	0	0	0	1201	513	0	1783	853
Traffic Volume (veh/h)	0	0	39	0	0	0	0	1201	513	0	1783	853
Future Volume (Veh/h)	0	0	39	0	0	0	0	1201	513	0	1783	853
Sign Control	Stop	Stop	Stop	0%	0%	0%	Free	Free	Free	Free	Free	Free
Grade	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	0	0	42	0	0	0	0	1305	558	0	1938	927
Pedestrians	24											
Lane Width (m)	3.6											
Walking Speed (m/s)	1.2											
Percent Blockage	2											
Right turn flare (veh)												
Median type							None	None	None	None	None	None
Median storage (veh)												
Upstream signal (m)							271					51
pX, platoon unblocked	0.79	0.79	0.73	0.79	0.79	0.89	0.73			0.89		
vC, conflicting volume	2860	3730	1134	1951	4194	435	2889			1305		
vC1, stage 1 conf vol												
vC2, stage 2 conf vol	1505	2609	0	350	3197	0	2306			923		
vCu, unblocked vol	7.5	6.5	7.1	7.5	6.5	6.9	4.1			4.1		
tC, single (s)												
tC, 2 stage (s)	3.5	4.0	3.4	3.5	4.0	3.3	2.2			2.2		
tF (s)	100	100	95	100	100	100	100			100		
p0 queue free %	65	19	767	429	8	975	159			688		
cM capacity (veh/h)												
Direction_Lane #	EB.1	NB.1	NB.2	NB.3	NB.4	SB.1	SB.2	SB.3				
Volume Total	42	435	435	435	558	775	775	1315				
Volume Left	0	0	0	0	0	0	0	0				
Volume Right	42	0	0	0	558	0	0	927				
GSH	767	1700	1700	1700	1700	1700	1700	1700				
Volume to Capacity	0.05	0.26	0.26	0.26	0.33	0.46	0.46	0.77				
Queue Length 95th (m)	1.4	0.0	0.0	0.0	0.0	0.0	0.0	0.0				
Control Delay (s)	10.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0				
Lane LOS	A											
Approach Delay (s)	10.0	0.0										
Approach LOS	A											
Intersection Summary												
Average Delay	0.1											
Intersection Capacity Utilization	64.3%											
ICU Level of Service	C											
Analysis Period (min)	15											

Lanes, Volumes, Timings  
 7: GO Bus Terminal/Argus Rd & Cross Ave

2025 Background PM Peak Hour.syn  
 05-04-2022

Intersection Capacity Utilization 58.8%  
 Analysis Period (min) 15  
 ICU Level of Service B



HCM Signalized Intersection Capacity Analysis  
7: GO Bus Terminal/Argus Rd & Cross Ave

2025 Background PM Peak Hour.syn  
05-04-2022

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	15	886	17	38	368	44	16	2	49	151	20	136
Traffic Volume (vph)	15	886	17	38	368	44	16	2	49	151	20	136
Future Volume (vph)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Ideal Flow (vphpl)	3.3	3.6	3.6	3.3	3.6	3.6	3.3	3.6	3.6	3.3	3.6	3.6
Lane Width	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Total Lost time (s)	1.00	0.95	1.00	1.00	0.95	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Lane Util. Factor	0.99	1.00	1.00	1.00	1.00	1.00	0.99	1.00	1.00	0.97	1.00	0.98
Frbp. ped/bikes	0.95	1.00	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.95
Flt Protected	1557	3172	797	3176	778	704	778	704	778	1523	1285	1285
Satd. Flow (prot)	0.50	1.00	0.13	1.00	0.53	1.00	0.53	1.00	0.70	1.00	0.70	1.00
Flt Permitted	814	3172	109	3176	431	704	431	704	1129	1285	1285	1285
Peak-hour factor, PHF	0.66	0.81	0.75	0.94	0.95	0.81	0.70	0.25	0.67	0.86	0.75	0.78
Adj. Flow (vph)	23	1084	23	40	387	54	23	8	73	176	27	174
RTOR Reduction (vph)	0	2	0	0	12	0	0	0	55	0	0	130
Lane Group Flow (vph)	23	1115	0	40	429	0	23	26	0	176	71	0
Confl. Peds. (#/hr)	9	4	4	4	4	9	12	12	20	20	20	12
Heavy Vehicles (%)	0%	0%	100%	97%	0%	0%	100%	100%	100%	100%	0%	100%
Turn Type	Perm	NA	NA	pmp-pt	NA	Perm	NA	NA	NA	Perm	NA	NA
Protected Phases	2			1	6		8		8		4	
Permitted Phases	2			6		8		8		4		
Actuated Green, G (s)	35.4	35.4	47.8	47.8	47.8	17.6	17.6	17.6	17.6	17.6	17.6	17.6
Effective Green, g (s)	37.4	37.4	47.8	49.8	47.8	19.6	19.6	19.6	19.6	19.6	19.6	19.6
Actuated g/C Ratio	0.48	0.48	0.62	0.64	0.62	0.25	0.25	0.25	0.25	0.25	0.25	0.25
Clearance Time (s)	6.0	6.0	4.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0
Vehicle Extension (s)	5.0	5.0	2.5	5.0	5.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lane Grp Cap (vph)	393	1532	141	2043	141	109	178	285	325	285	325	325
v/s Ratio Prot	0.035	0.035	0.03	0.14	0.03	0.14	0.04	0.04	0.04	0.16	0.06	0.06
v/s Ratio Perm	0.06	0.73	0.28	0.21	0.28	0.21	0.15	0.15	0.15	0.62	0.22	0.22
Uniform Delay, d1	10.6	15.9	8.9	5.7	22.8	22.4	22.4	22.4	22.4	25.6	22.8	22.8
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	0.1	2.2	0.8	0.1	0.8	0.1	1.3	0.5	0.5	4.5	0.5	0.5
Delay (s)	10.8	18.1	9.8	5.8	24.1	23.0	23.0	23.0	23.0	30.1	23.3	23.3
Level of Service	B	B	A	A	A	C	C	C	C	C	C	C
Approach Delay (s)	18.0	18.0	6.1	6.1	6.1	23.2	23.2	23.2	23.2	26.5	26.5	26.5
Approach LOS	B	B	A	A	A	C	C	C	C	C	C	C
Intersection Summary	Intersection Summary											
HCM 2000 Control Delay	17.0 HCM 2000 Level of Service B											
HCM 2000 Volume to Capacity ratio	0.64											
Actuated Cycle Length (s)	77.4 Sum of lost time (s) 12.0											
Intersection Capacity Utilization	58.8% ICU Level of Service B											
Analysis Period (min)	15											
C Critical Lane Group	C											

Lanes, Volumes, Timings  
4: Trafalgar Road & Argus Road/QEW EB On Ramp

2030 Total AM Peak Hour.syn  
05-04-2022

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	0	0	39	0	0	0	0	1201	513	0	1783	853
Traffic Volume (vph)	0	0	39	0	0	0	0	1201	513	0	1783	853
Future Volume (vph)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Ideal Flow (vphpl)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	7.5	7.5	0.0	7.5	7.5
Storage Length (m)	0	0	1	0	0	0	0	0	0	1	0	0
Storage Lanes	7.5	7.5	7.5	7.5	7.5	7.5	7.5	7.5	7.5	7.5	7.5	7.5
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.91	0.91
Ped Bike Factor	0.865	0.865	0.865	0.865	0.865	0.865	0.865	0.865	0.865	0.865	0.951	0.951
Flt Protected	0	0	1522	0	0	0	0	5085	1568	0	4867	0
Satd. Flow (prot)	0	0	1522	0	0	0	0	5085	1568	0	4867	0
Flt Permitted	50	50	50	50	50	50	50	50	50	50	50	50
Link Speed (k/h)	165.2	165.2	165.2	165.2	165.2	165.2	165.2	165.2	165.2	165.2	165.2	165.2
Link Distance (m)	11.9	11.9	11.9	11.9	11.9	11.9	11.9	11.9	11.9	11.9	11.9	11.9
Travel Time (s)	14.1	14.1	14.1	14.1	14.1	14.1	14.1	14.1	14.1	14.1	14.1	14.1
Confl. Peds. (#/hr)	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	0%	0%	8%	0%	2%	0%	0%	2%	3%	0%	2%	0%
Adj. Flow (vph)	0	0	42	0	0	0	0	1305	558	0	1938	927
Shared Lane Traffic (%)	0	0	42	0	0	0	0	1305	558	0	2865	0
Lane Group Flow (vph)	No	No	No	No	No	No	No	No	No	No	No	No
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(m)	0.0	0.0	0.0	0.0	0.0	0.0	3.6	3.6	0.0	0.0	3.6	3.6
Link Offset(m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Crosswalk Width(m)	4.8	4.8	4.8	4.8	4.8	4.8	4.8	4.8	4.8	4.8	4.8	4.8
Two way Left Turn Lane	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Headway Factor	25	25	15	25	25	15	25	25	15	25	25	15
Turning Speed (k/h)	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
Intersection Summary	Intersection Summary											
Area Type:	Other											
Control Type:	Unsignalized											
Intersection Capacity Utilization	64.3% ICU Level of Service C											
Analysis Period (min)	15											

HCM Signalized Intersection Capacity Analysis  
 3: Trafalgar Road & QEW EB Off Ramp

2030 Total AM Peak Hour.syn  
 05-04-2022



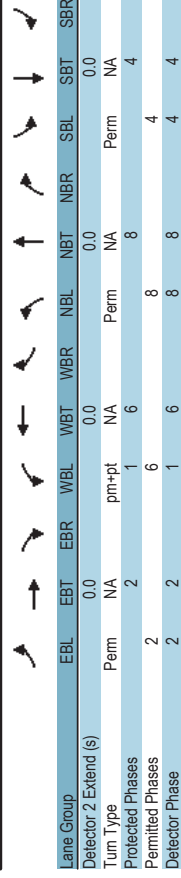
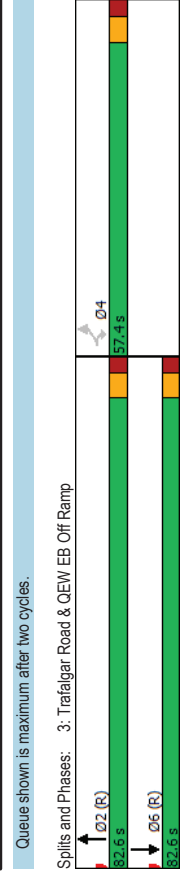
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	↑↑	↑	↑↑↑↑	↑↑↑↑	↑↑↑↑	↑↑
Traffic Volume (vph)	1013	909	0	1201	1727	0
Future Volume (vph)	1013	909	0	1201	1727	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0
Lane Util. Factor	0.97	1.00	0.91	1.00	0.91	1.00
Fit	1.00	0.85	1.00	1.00	1.00	1.00
Flt Protected	0.95	1.00	1.00	1.00	1.00	1.00
Satd. Flow (prot)	3400	1599	4893	4988	4988	4988
Flt Permitted	0.95	1.00	1.00	1.00	1.00	1.00
Satd. Flow (perm)	3400	1599	4893	4988	4988	4988
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	1101	988	0	1305	1877	0
RTOR Reduction (vph)	0	7	0	0	0	0
Lane Group Flow (vph)	1101	981	0	1305	1877	0
Heavy Vehicles (%)	3%	1%	0%	6%	4%	0%
Turn Type	Perm	Perm	NA	NA	NA	NA
Protected Phases	4	4	2	2	6	
Permitted Phases						
Actuated Green, G (s)	50.4	50.4	75.6	75.6	75.6	
Effective Green, g (s)	53.4	53.4	78.6	78.6	78.6	
Actuated g/C Ratio	0.38	0.38	0.56	0.56	0.56	
Clearance Time (s)	7.0	7.0	7.0	7.0	7.0	
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	
Lane Grp Cap (vph)	1296	609	2747	2800	2800	
v/s Ratio Prot			0.27	c0.38		
v/s Ratio Perm	0.32	c0.61				
v/c Ratio	0.85	1.61	0.48	0.48	0.67	
Uniform Delay, d1	39.6	43.3	18.4	21.6		
Progression Factor	1.00	1.00	1.88	0.68		
Incremental Delay, d2	5.4	282.6	0.4	0.9		
Delay (s)	45.0	325.9	34.9	15.5		
Level of Service	D	F	C	B		
Approach Delay (s)	177.9		34.9	15.5		
Approach LOS	F		C	B		
Intersection Summary						
HCM 2000 Control Delay	84.7				HCM 2000 Level of Service	F
HCM 2000 Volume to Capacity ratio	1.07					
Actuated Cycle Length (s)	140.0				Sum of lost time (s)	11.0
Intersection Capacity Utilization	96.3%				ICU Level of Service	F
Analysis Period (min)	15					
c Critical Lane Group						

Lanes, Volumes, Timings  
 8: Lyons Lane/Commercial Driveway & Cross Ave

2025 Background PM Peak Hour.syn  
 05-04-2022



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑↑	↑↑	↑↑	↑↑	↑↑	↑↑	↑↑	↑↑	↑↑	↑↑	↑↑	↑↑
Traffic Volume (vph)	39	159	14	15	326	13	243	4	164	16	2	66
Future Volume (vph)	39	159	14	15	326	13	243	4	164	16	2	66
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (m)	3.3	3.6	3.6	3.3	3.6	3.6	3.3	3.6	3.6	3.3	3.6	3.6
Storage Length (m)	0.0	0.0	0.0	25.0	0.0	20.0	0.0	0.0	0.0	0.0	0.0	0.0
Storage Lanes	1	0	0	1	0	0	1	0	0	0	1	0
Taper Length (m)	7.5	7.5	7.5	7.5	7.5	7.5	7.5	7.5	7.5	7.5	7.5	7.5
Lane Util. Factor	1.00	0.95	0.95	1.00	0.95	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor	1.00	1.00	1.00	1.00	1.00	0.98	1.00	0.98	1.00	0.99	1.00	0.99
Fit	0.986			0.990		0.858		0.858		0.855		0.855
Flt Protected	0.950			0.950		0.950		0.950		0.950		0.950
Satd. Flow (prot)	1525	2907	0	1570	3068	0	1570	1444	0	1570	1416	0
Flt Permitted	0.533			0.558		0.684		0.537		0.537		0.537
Satd. Flow (perm)	852	2907	0	921	3068	0	1129	1444	0	885	1416	0
Right Turn on Red			Yes		Yes		Yes		Yes		Yes	
Satd. Flow (RTOR)			14		13		208		110		110	
Link Speed (k/h)			50		50		50		50		50	
Link Distance (m)			40.1		374.0		69.1		70.9		70.9	
Travel Time (s)			2.9		26.9		5.0		5.1		5.1	
Confl. Peds. (#/hr)	3		1	1	3	1	4	4	4	4	4	1
Peak Hour Factor	0.58	0.84	0.75	0.54	0.95	0.55	0.74	0.33	0.79	0.70	0.50	0.60
Heavy Vehicles (%)	3%	11%	0%	0%	5%	0%	0%	0%	0%	0%	0%	2%
Adj. Flow (vph)	67	189	19	28	343	24	328	12	208	23	4	110
Shared Lane Traffic (%)												
Lane Group Flow (vph)	67	208	0	28	367	0	328	220	0	23	114	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(m)		3.3		3.3		3.3		3.3		3.3		3.3
Link Offset(m)		0.0		0.0		0.0		0.0		0.0		0.0
Crosswalk Width(m)		4.8		4.8		4.8		4.8		4.8		4.8
Two way Left Turn Lane												
Headway Factor	1.19	1.14	1.14	1.19	1.14	1.14	1.19	1.14	1.14	1.19	1.14	1.14
Turning Speed (k/h)	24	24	14	24	14	24	24	14	24	24	14	24
Number of Detectors	1	2	1	2	1	2	1	2	1	2	1	2
Detector Template	Left	Thru	Left	Thru	Left	Thru	Left	Thru	Left	Thru	Left	Thru
Leading Detector (m)	2.0	10.0	2.0	10.0	2.0	10.0	2.0	10.0	2.0	10.0	2.0	10.0
Trailing Detector (m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Position(m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Size(m)	2.0	0.6	2.0	0.6	2.0	0.6	2.0	0.6	2.0	0.6	2.0	0.6
Detector 1 Type	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 2 Position(m)		9.4		9.4		9.4		9.4		9.4		9.4
Detector 2 Size(m)		0.6		0.6		0.6		0.6		0.6		0.6
Detector 2 Type		Ch+Ex		Ch+Ex		Ch+Ex		Ch+Ex		Ch+Ex		Ch+Ex
Detector 2 Channel												

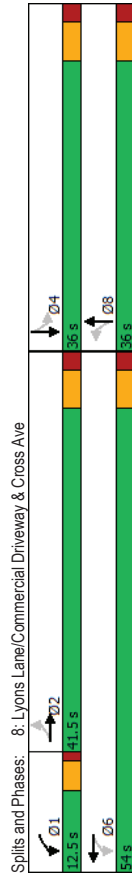


Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Detector 2 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Turn Type	Perm	NA	NA	pim-pt	NA	NA	Perm	NA	NA	Perm	NA	NA
Protected Phases				1	6		8					4
Permitted Phases	2	2	6	6	6	8	8	8	8	4	4	4
Detector Phase	2	2	2	1	6	8	8	8	8	4	4	4
Switch Phase												
Minimum Initial (s)	35.0	35.0	8.0	35.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0
Minimum Split (s)	41.0	41.0	12.5	41.0	28.0	28.0	28.0	28.0	28.0	28.0	28.0	28.0
Total Split (s)	41.5	41.5	12.5	54.0	36.0	36.0	36.0	36.0	36.0	36.0	36.0	36.0
Total Split (%)	46.1%	46.1%	13.9%	60.0%	40.0%	40.0%	40.0%	40.0%	40.0%	40.0%	40.0%	40.0%
Maximum Green (s)	35.5	35.5	8.5	48.0	30.0	30.0	30.0	30.0	30.0	30.0	30.0	30.0
Yellow Time (s)	4.0	4.0	3.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
All-Red Time (s)	2.0	2.0	1.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
Lost Time Adjust (s)	-2.0	-2.0	0.0	-2.0	-2.0	-2.0	-2.0	-2.0	-2.0	-2.0	-2.0	-2.0
Total Lost Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lead/Lag	Lag	Lag	Lead	Lead								
Lead-Lag Optimize?	Yes	Yes	Yes	Yes								
Vehicle Extension (s)	5.0	5.0	2.5	5.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Recall Mode	Min	Min	Min	Min	Min	Min	Min	Min	Min	Min	Min	Min
Walk Time (s)	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0
Flash Dont Walk (s)	15.0	15.0	15.0	15.0	15.0	15.0	15.0	15.0	15.0	15.0	15.0	15.0
Pedestrian Calls (#/hr)	0	0	0	0	0	0	0	0	0	0	0	0
Act Effct Green (s)	37.1	37.1	49.1	29.5	29.5	29.5	29.5	29.5	29.5	29.5	29.5	29.5
Actuated g/C Ratio	0.43	0.43	0.57	0.57	0.34	0.34	0.34	0.34	0.34	0.34	0.34	0.34
v/c Ratio	0.18	0.17	0.05	0.21	0.86	0.35	0.08	0.21	0.08	0.21	0.08	0.21
Control Delay	18.2	15.2	9.4	9.7	49.1	5.3	19.8	5.5	19.8	5.5	19.8	5.5
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	18.2	15.2	9.4	9.7	49.1	5.3	19.8	5.5	19.8	5.5	19.8	5.5
LOS	B	B	A	A	D	A	A	B	A	B	A	A
Approach Delay	16.0			9.7			31.5					7.9
Approach LOS	B			A			C					A
Queue Length 50th (m)	7.4	11.2	2.2	15.9	52.5	1.4	2.7	0.5	2.7	0.5	2.7	0.5
Queue Length 95th (m)	10.2	17.2	3.5	23.5	65.7	0.0	6.1	0.4	6.1	0.4	6.1	0.4
Internal Link Dist (m)				350.0			45.1					46.9
Turn Bay Length (m)			25.0				20.0					
Base Capacity (vph)	369	1269	586	1781	417	666	328	593	328	593	328	593
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.18	0.16	0.05	0.21	0.79	0.33	0.07	0.19	0.07	0.19	0.07	0.19
Intersection Summary												
Area Type: CBD												
Cycle Length: 90												
Actuated Cycle Length: 86.6												
Natural Cycle: 85												
Control Type: Semi Act-Uncoord												
Maximum v/c Ratio: 0.86												
Intersection Signal Delay: 19.6												



Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Minimum Initial (s)	10.0	10.0	29.0	29.0	29.0	29.0
Minimum Split (s)	38.0	38.0	36.0	36.0	36.0	36.0
Total Split (s)	57.4	57.4	82.6	82.6	82.6	82.6
Total Split (%)	41.0%	41.0%	59.0%	59.0%	59.0%	59.0%
Maximum Green (s)	50.4	50.4	75.6	75.6	75.6	75.6
Yellow Time (s)	4.0	4.0	4.0	4.0	4.0	4.0
All-Red Time (s)	3.0	3.0	3.0	3.0	3.0	3.0
Lost Time Adjust (s)	-3.0	-3.0	-3.0	-3.0	-3.0	-3.0
Total Lost Time (s)	4.0	4.0	4.0	4.0	4.0	4.0
Lead/Lag						
Lead-Lag Optimize?						
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	None	None	C-Max	C-Max	C-Max	C-Max
Walk Time (s)	7.0	7.0	7.0	7.0	7.0	7.0
Flash Dont Walk (s)	11.0	11.0	11.0	11.0	11.0	11.0
Pedestrian Calls (#/hr)	0	0	0	0	0	0
Act Effct Green (s)	53.4	53.4	78.6	78.6	78.6	78.6
Actuated g/C Ratio	0.38	0.38	0.56	0.56	0.56	0.56
v/c Ratio	0.85	1.60	0.48	0.48	0.67	0.67
Control Delay	47.2	309.9	35.1	15.7	15.7	15.7
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	47.2	309.9	35.1	15.7	15.7	15.7
LOS	D	F	D	B	B	B
Approach Delay	171.4		35.1	15.7	15.7	
Approach LOS	F		D	B	B	
Queue Length 50th (m)	150.4	~412.1	127.3	108.3	108.3	
Queue Length 95th (m)	180.3	#496.2	157.9	69.9	69.9	
Internal Link Dist (m)	47.3		26.7	162.0	162.0	
Turn Bay Length (m)						
Base Capacity (vph)	1296	616	2747	2800	2800	
Starvation Cap Reductn	0	0	0	0	0	
Spillback Cap Reductn	0	0	0	0	0	
Storage Cap Reductn	0	0	0	0	0	
Reduced v/c Ratio	0.85	1.60	0.48	0.48	0.67	
Intersection Summary						
Area Type:	Other					
Cycle Length:	140					
Actuated Cycle Length:	140					
Offset:	0 (0%). Referenced to phase 2:NBT and 6:SBT, Start of Green					
Natural Cycle:	80					
Control Type:	Actuated-Coordinated					
Maximum v/c Ratio:	1.60					
Intersection Signal Delay:	82.2					
Intersection Capacity Utilization:	96.3%					
Analysis Period (min):	15					
~ Volume exceeds capacity, queue is theoretically infinite.						
~ Queue shown is maximum after two cycles.						
# 95th percentile volume exceeds capacity, queue may be longer.						

Intersection Capacity Utilization 64.3%  
 Analysis Period (min) 15





HCM Signalized Intersection Capacity Analysis  
 8: Lyons Lane/Commercial Driveway & Cross Ave  
 2025 Background PM Peak Hour.syn  
 05-04-2022

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	39	15	15	326	13	243	4	164	16	2	66	66
Traffic Volume (vph)	39	15	15	326	13	243	4	164	16	2	66	66
Future Volume (vph)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Ideal Flow (vphpl)	3.3	3.6	3.6	3.3	3.6	3.3	3.6	3.6	3.3	3.6	3.6	3.6
Lane Width	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Total Lost time (s)	1.00	0.95	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Flpb. ped/bikes	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Flpb. ped/bikes	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Flt Protected	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00
Satd. Flow (prot)	1519	2907	1569	3068	1569	1444	1569	1444	1569	1444	1569	1417
Flt Permitted	0.53	1.00	0.56	1.00	0.68	1.00	0.68	1.00	0.54	1.00	0.54	1.00
Satd. Flow (perm)	853	2907	921	3068	1129	1444	1129	1444	885	1417	885	1417
Peak-hour factor, PHF	0.58	0.84	0.75	0.54	0.95	0.55	0.74	0.33	0.79	0.70	0.50	0.60
Adj. Flow (vph)	67	189	19	28	343	24	328	12	208	23	4	110
RTOR Reduction (vph)	0	8	0	0	6	0	0	137	0	0	73	0
Lane Group Flow (vph)	67	200	0	28	361	0	328	83	0	23	41	0
Conf. Peds. (#/hr)	3	1	1	0	3	1	0	4	4	4	4	1
Heavy Vehicles (%)	3%	11%	0%	5%	0%	0%	0%	0%	0%	0%	0%	2%
Turn Type	Perm	NA	NA	pmp-pt	NA	NA	Perm	NA	NA	Perm	NA	NA
Protected Phases	2			6			8				4	
Permitted Phases	2			6			8			4		
Actuated Green, G (s)	35.1	35.1	47.1	47.1	47.1	27.4	27.4	27.4	27.4	27.4	27.4	27.4
Effective Green, g (s)	37.1	37.1	47.1	49.1	49.1	29.4	29.4	29.4	29.4	29.4	29.4	29.4
Actuated g/C Ratio	0.43	0.43	0.54	0.57	0.34	0.34	0.34	0.34	0.34	0.34	0.34	0.34
Clearance Time (s)	6.0	6.0	4.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0
Vehicle Extension (s)	5.0	5.0	2.5	5.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lane Grp Cap (vph)	365	1246	561	1741	383	490	300	481				
v/s Ratio Prot	0.07		0.00	c0.12		0.06		0.03				
v/s Ratio Perm	0.08		0.02		c0.29		0.03					
v/c Ratio	0.18	0.16	0.05	0.21	0.86	0.17	0.08	0.09				
Uniform Delay, d1	15.3	15.1	9.2	9.2	26.6	20.0	19.4	19.4				
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	0.5	0.1	0.0	0.1	17.4	0.2	0.1	0.1				
Delay (s)	15.8	15.3	9.3	9.3	44.0	20.2	19.5	19.5				
Level of Service	B	B	A	A	D	C	B	B				
Approach Delay (s)	15.4		9.3		34.5		19.5					
Approach LOS	B		A		C		B					
Intersection Summary												
HCM 2000 Control Delay	21.7 HCM 2000 Level of Service C											
HCM 2000 Volume to Capacity ratio	0.47											
Actuated Cycle Length (s)	86.5 Sum of lost time (s) 12.0											
Intersection Capacity Utilization	64.3% ICU Level of Service C											
Analysis Period (min)	15											
c Critical Lane Group												

Lanes, Volumes, Timings  
 3: Trafalgar Road & QEW EB Off Ramp  
 2030 Total AM Peak Hour.syn  
 05-04-2022

Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	1013	909	0	1201	1727	0
Traffic Volume (vph)	1013	909	0	1201	1727	0
Future Volume (vph)	1900	1900	1900	1900	1900	1900
Ideal Flow (vphpl)	0.97	1.00	1.00	0.91	0.91	1.00
Lane Util. Factor	0.950					
Flt Protected	3400	1599	0	4893	4988	0
Satd. Flow (prot)	0.950					
Flt Permitted	3400	1599	0	4893	4988	0
Satd. Flow (perm)	Yes					Yes
Right Turn on Red	11					
Satd. Flow (RTOR)	50		50		50	
Link Speed (k/h)	71.3		50.7		186.0	
Link Distance (m)	5.1		3.7		13.4	
Travel Time (s)	0.92		0.92		0.92	
Peak Hour Factor	3%	1%	0%	6%	4%	0%
Heavy Vehicles (%)	1101	988	0	1305	1877	0
Adj. Flow (vph)						
Shared Lane Traffic (%)	1101	988	0	1305	1877	0
Lane Group Flow (vph)	No	No	No	No	No	No
Enter Blocked Intersection	7.2	0.0	0.0	0.0	0.0	0.0
Lane Alignment	Left	Right	Left	Left	Left	Right
Median Width (m)	0.0	0.0	0.0	4.8	4.8	4.8
Link Offset (m)	4.8					
Crosswalk Width (m)	1.00	1.00	1.00	1.00	1.00	1.00
Two way Left Turn Lane	25	15	25		2	2
Headway Factor	1					
Turning Speed (k/h)	1					
Number of Detectors	Left	Right	Thru	Thru	Thru	
Detector Template	2.0	2.0	10.0	10.0	10.0	
Leading Detector (m)	0.0	0.0	0.0	0.0	0.0	
Trailing Detector (m)	0.0	0.0	0.0	0.0	0.0	
Detector 1 Position (m)	2.0	2.0	0.6	0.6	0.6	
Detector 1 Size (m)	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	
Detector 1 Type	0.0	0.0	0.0	0.0	0.0	
Detector 1 Channel	0.0	0.0	0.0	0.0	0.0	
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	
Detector 2 Position (m)	9.4	9.4	9.4	9.4	9.4	
Detector 2 Size (m)	0.6	0.6	0.6	0.6	0.6	
Detector 2 Type	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	
Detector 2 Channel	0.0	0.0	0.0	0.0	0.0	
Detector 2 Extend (s)	Perm	Perm	Perm	NA	NA	NA
Turn Type	Perm	Perm	Perm	2	6	6
Protected Phases	4	4	4			
Permitted Phases	4	4	4	2	6	6
Switch Phase	4	4	4	2	6	6

HCM Unsignalized Intersection Capacity Analysis  
 2: Trafalgar Road & QEW SB On Ramps

Lanes, Volumes, Timings  
 9: Cross Ave & Lyons Lane

2030 Total AM Peak Hour.syn  
 05-04-2022

2025 Background PM Peak Hour.syn  
 05-04-2022

Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations				↑↑↑	↑↑↑	↑
Traffic Volume (veh/h)	0	0	0	2214	1727	744
Future Volume (Veh/h)	0	0	0	2214	1727	744
Sign Control	Stop	Free	Free	Free	Free	Free
Grade	0%	0%	0%	0%	0%	0%
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	0	0	0	2407	1877	809
Pedestrians						
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)						
Median type			None	None	None	
Median storage (veh)						
Upstream signal (m)			186	145		
pX, platoon unblocked	0.74	0.73	0.73			
vC, conflicting volume	2679	626	1877			
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	0	0	921			
tC, single (s)	6.8	6.9	4.1			
tC, 2 stage (s)						
tF (s)	3.5	3.3	2.2			
p0 queue free %	100	100	100			
cM capacity (veh/h)	755	795	540			
Direction_Lane #	NB.1	NB.2	NB.3	SB.1	SB.2	SB.3
Volume Total	802	802	802	626	626	809
Volume Left	0	0	0	0	0	0
Volume Right	0	0	0	0	0	0
GSH	1700	1700	1700	1700	1700	1700
Volume to Capacity	0.47	0.47	0.47	0.37	0.37	0.48
Queue Length 95th (m)	0.0	0.0	0.0	0.0	0.0	0.0
Control Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0
Lane LOS						
Approach Delay (s)	0.0			0.0		
Approach LOS						
Intersection Summary	CBD					
Average Delay	0.0					
Intersection Capacity Utilization	96.3% ICU Level of Service					
Analysis Period (min)	15					

Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	↑	↑↑	↑↑	↑↑	↑	↑
Traffic Volume (vph)	6	191	623	7	16	30
Future Volume (vph)	6	191	623	7	16	30
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (m)	5.0	0.0	0.0	0.0	0.0	0.0
Storage Lanes	1	0	0	0	1	0
Taper Length (m)	7.5	0	0	0	7.5	0
Lane Util. Factor	1.00	0.95	0.95	0.95	1.00	1.00
Ped Bike Factor						
Flt		0.998	0.998	0.998	0.910	0.984
Flt Protected		0.950	0.950	0.950	0.984	0.984
Satd. Flow (prot)		1388	2954	3150	0	1491
Flt Permitted		0.950	0.950	0.950	0.984	0.984
Satd. Flow (perm)		1388	2954	3150	0	1491
Link Speed (k/h)		50	50	50	50	50
Link Distance (m)		281.3	40.1	41.7	3.0	3.0
Travel Time (s)		20.3	2.9	3.0	3.0	3.0
Confl. Peds. (#/hr)	1			1	9	
Peak Hour Factor	0.50	0.92	0.84	0.58	0.44	0.41
Heavy Vehicles (%)	17%	10%	3%	0%	0%	4%
Adj. Flow (vph)	12	208	742	12	36	73
Shared Lane Traffic (%)						
Lane Group Flow (vph)	12	208	754	0	109	0
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Left	Left	Right	Left	Right
Median Width(m)	3.6	3.6	3.6	3.6	3.6	3.6
Link Offset(m)	0.0	0.0	0.0	0.0	0.0	0.0
Crosswalk Width(m)	4.8	4.8	4.8	4.8	4.8	4.8
Two way Left Turn Lane						
Headway Factor	1.14	1.14	1.14	1.14	1.14	1.14
Turning Speed (k/h)	25	Free	Free	Free	Stop	Stop
Sign Control						
Intersection Summary	CBD					
Area Type:	Unsignalized					
Control Type:	Unsignalized					
Intersection Capacity Utilization	29.4%					
Analysis Period (min)	15					

HCM Unsignalized Intersection Capacity Analysis 2025 Background PM Peak Hour.syn  
 9: Cross Ave & Lyons Lane 05-04-2022

Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	↔	↔	↔	↔	↔	↔
Traffic Volume (veh/h)	6	191	623	7	16	30
Future Volume (Veh/h)	6	191	623	7	16	30
Sign Control	Free	Free	Free	Stop	Stop	0%
Grade	0%	0%	0%	0%	0%	0%
Peak Hour Factor	0.50	0.92	0.84	0.58	0.44	0.41
Hourly flow rate (vph)	12	208	742	12	36	73
Pedestrians						
Lane Width (m)			3.6		3.6	
Walking Speed (m/s)			1.2		1.2	
Percent Blockage			1		0	
Right turn flare (veh)			None		None	
Median type			None		None	
Median storage (veh)			40		0.95	0.95
Upstream signal (m)			755		886	378
pX, platoon unblocked						
vC, conflicting volume						
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol					784	251
tC, single (s)					6.8	7.0
tC, 2 stage (s)						
tF (s)					3.5	3.3
p0 queue free %					88	90
cM capacity (veh/h)					312	708
Direction_Lane #	EB1	EB2	EB3	WB1	WB2	SB1
Volume Total	12	104	104	495	259	109
Volume Left	12	0	0	0	0	36
Volume Right	0	0	0	0	12	73
GSH	801	1700	1700	1700	1700	498
Volume to Capacity	0.01	0.06	0.06	0.29	0.15	0.22
Queue Length 95th (m)	0.4	0.0	0.0	0.0	0.0	6.6
Control Delay (s)	9.6	0.0	0.0	0.0	0.0	14.2
Lane LOS	A	A	A	B	B	B
Approach Delay (s)	0.5			0.0		14.2
Approach LOS				B		B
Intersection Summary						
Average Delay	1.5					
Intersection Capacity Utilization	29.4%					
ICU Level of Service	A					
Analysis Period (min)	15					

Lanes, Volumes, Timings 2030 Total AM Peak Hour.syn  
 2: Trafalgar Road & QEW SB On Ramps 05-04-2022

Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	↔	↔	↔	↔	↔	↔
Traffic Volume (vph)	0	0	0	2214	1727	744
Future Volume (vph)	0	0	0	2214	1727	744
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	0.91	0.91	1.00
Frt						0.850
Flt Protected						
Satd. Flow (prot)	0	0	0	5085	5085	1583
Flt Permitted						
Satd. Flow (perm)	0	0	0	5085	5085	1583
Link Speed (k/h)	50			50	50	
Link Distance (m)	159.6			186.0	145.2	
Travel Time (s)	11.5			13.4	10.5	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	0	0	0	2407	1877	809
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	0	0	2407	1877	809
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Left	Left	Left	Left	Right
Median Width (m)	0.0			0.0	0.0	
Link Offset (m)	0.0			0.0	0.0	
Crosswalk Width (m)	4.8			4.8	4.8	
Two way Left Turn Lane						
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (k/h)	25	15	25			15
Sign Control	Stop	Stop	Free	Free	Free	Free
Intersection Summary						
Area Type:	Other					
Control Type:	Unsignalized					
Intersection Capacity Utilization	96.3%					
ICU Level of Service	F					
Analysis Period (min)	15					

HCM Signalized Intersection Capacity Analysis  
 1: Trafalgar Road & North Service Road/QEW WB Off Ramp

Lanes, Volumes, Timings  
 10: Lyons Lane & South Service Road

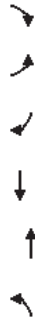
2030 Total AM Peak Hour.syn  
 05-04-2022

2025 Background PM Peak Hour.syn  
 05-04-2022

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	1	0	171	501	38	219	0	1777	437	0	1799	7
Traffic Volume (vph)	1	0	171	501	38	219	0	1777	437	0	1799	7
Future Volume (vph)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Ideal Flow (vphpl)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Total Lost time (s)	1.00	1.00	1.00	0.95	0.95	0.95	0.91	1.00	0.91	1.00	0.91	1.00
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.96	1.00	0.96	1.00	0.95
Frb, ped/bikes	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frb, ped/bikes	1.00	0.85	1.00	0.89	0.85	1.00	0.85	1.00	0.85	1.00	0.85	1.00
Frt	0.95	1.00	0.95	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Satd. Flow (prot)	1805	1568	1719	1454	1447	4940	1478	5036	1533	5036	1533	1533
Satd. Flow (perm)	0.95	1.00	0.95	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	1	0	186	545	41	238	0	1932	475	0	1955	8
RTOR Reduction (vph)	0	0	18	0	62	85	0	140	0	0	0	4
Lane Group Flow (vph)	1	0	168	545	81	51	0	1932	335	0	1955	4
Conf. Peds. (#/hr)	0%	0%	3%	5%	23%	6%	0%	5%	5%	0%	3%	0%
Heavy Vehicles (%)	Prot	Perm	Perm	NA	Perm	NA	Perm	NA	Perm	NA	Perm	NA
Turn Type	7	8	8	8	8	8	2	2	2	6	6	6
Protected Phases	1.2	56.2	50.0	50.0	50.0	69.8	69.8	69.8	69.8	69.8	69.8	69.8
Actuated Green, G (s)	2.2	59.2	53.0	53.0	53.0	72.8	72.8	72.8	72.8	72.8	72.8	72.8
Effective Green, g (s)	0.02	0.42	0.38	0.38	0.38	0.52	0.52	0.52	0.52	0.52	0.52	0.52
Actuated g/C Ratio	5.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0
Clearance Time (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Vehicle Extension (s)	28	663	650	550	547	2568	768	2618	797	2618	797	797
Lane Grp Cap (vph)	0.00	0.11	0.32	0.04	0.04	0.23	0.39	0.39	0.39	0.39	0.39	0.39
v/s Ratio Prot	0.04	0.25	0.84	0.15	0.09	0.75	0.44	0.75	0.44	0.75	0.44	0.75
v/s Ratio Perm	67.9	26.1	39.6	28.6	28.0	26.5	20.9	26.4	20.9	26.4	20.9	26.2
Uniform Delay, d1	1.00	1.00	1.00	1.00	1.00	0.45	0.33	1.00	0.45	1.00	1.00	1.00
Progression Factor	0.5	0.2	9.3	0.1	0.1	1.6	1.4	2.0	1.4	2.0	2.0	2.0
Incremental Delay, d2	68.4	26.3	48.9	28.7	28.1	13.5	8.2	28.4	8.2	28.4	16.2	16.2
Delay (s)	E	C	D	C	C	B	A	C	B	A	C	B
Level of Service	26.5	C	42.0	D	D	12.4	B	28.3	B	28.3	C	B
Approach Delay (s)	C											
Approach LOS	C											
Intersection Summary												
HCM 2000 Control Delay	23.2 HCM 2000 Level of Service C											
HCM 2000 Volume to Capacity ratio	0.78											
Actuated Cycle Length (s)	140.0 Sum of lost time (s) 12.0											
Intersection Capacity Utilization	83.1% ICU Level of Service E											
Analysis Period (min)	15											
c Critical Lane Group												

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	3	4	1	5	1	5	14	5	14	5	14	5
Traffic Volume (vph)	3	5	1	5	1	5	14	5	14	5	14	5
Future Volume (vph)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Ideal Flow (vphpl)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Lane Util. Factor	0.976	0.976	0.910	0.976	0.976	0.976	0.976	0.976	0.976	0.976	0.976	0.976
Frt Protected	0	1669	1357	0	1584	0	0	1584	0	1584	0	1584
Satd. Flow (prot)	0	1669	1357	0	1584	0	0	1584	0	1584	0	1584
Satd. Flow (perm)	0	1669	1357	0	1584	0	0	1584	0	1584	0	1584
Link Speed (k/h)	50	50	50	50	50	50	50	50	50	50	50	50
Link Distance (m)	60.5	89.6	37.6	60.5	89.6	37.6	60.5	89.6	37.6	60.5	89.6	37.6
Travel Time (s)	4.4	6.5	2.7	4.4	6.5	2.7	4.4	6.5	2.7	4.4	6.5	2.7
Conf. Peds. (#/hr)	7	7	7	7	7	7	7	7	7	7	7	7
Peak Hour Factor	0.38	0.62	0.25	0.62	0.50	0.42	0.38	0.62	0.25	0.62	0.50	0.42
Heavy Vehicles (%)	0%	0%	0%	22%	0%	0%	0%	22%	0%	0%	0%	0%
Adj. Flow (vph)	8	8	4	8	4	8	28	12	4	8	28	12
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	16	12	0	40	0	0	40	0	40	0	40
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Left	Right	Left	Right	Left	Right	Left	Right	Left	Right
Median Width(m)	0.0	0.0	0.0	3.6	0.0	3.6	0.0	3.6	0.0	3.6	0.0	3.6
Link Offset(m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Crosswalk Width(m)	4.8	4.8	4.8	4.8	4.8	4.8	4.8	4.8	4.8	4.8	4.8	4.8
Two way Left Turn Lane	1.14	1.14	1.14	1.14	1.14	1.14	1.14	1.14	1.14	1.14	1.14	1.14
Headway Factor	24	24	24	24	24	24	24	24	24	24	24	24
Turning Speed (k/h)	Free	Free	Free	Free	Free	Free	Free	Free	Free	Free	Free	Free
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Stop	Stop	Stop	Stop	Stop	Stop
Intersection Summary												
Area Type:	CBD											
Control Type:	Unsignalized											
Intersection Capacity Utilization	15.4%											
Analysis Period (min)	15											
ICU Level of Service A												

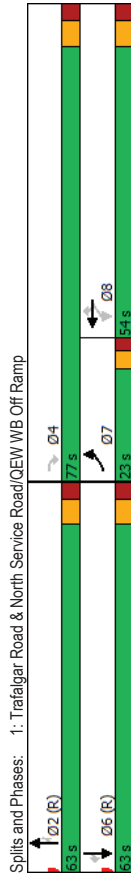
HCM Unsignalized Intersection Capacity Analysis 2025 Background PM Peak Hour.syn  
 10: Lyons Lane & South Service Road 05-04-2022



Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	3	5	1	5	14	5
Traffic Volume (veh/h)	3	5	1	5	14	5
Future Volume (Veh/h)	3	5	1	5	14	5
Sign Control	Free	Free	Stop	Stop	0%	0%
Grade	0%	0%	0%	0%	0%	0%
Peak Hour Factor	0.38	0.62	0.25	0.62	0.50	0.42
Hourly flow rate (vph)	8	8	4	8	28	12
Pedestrians					7	
Lane Width (m)					3.6	
Walking Speed (m/s)					1.2	
Percent Blockage					1	
Right turn flare (veh)					None	
Median type					None	
Median storage (veh)					None	
Upstream signal (m)					None	
pX, platoon unblocked					19	39
vC, conflicting volume					39	15
vC1, stage 1 conf vol					39	15
vC2, stage 2 conf vol					6.4	6.2
tC, single (s)					3.5	3.3
tC, 2 stage (s)					97	99
tF (s)					967	1064
p0 queue free %						
cM capacity (veh/h)						
Direction_Lane #	EB 1	WB 1	SB 1	EB 1	WB 1	SB 1
Volume Total	16	12	40	16	12	40
Volume Left	8	0	28	8	0	28
Volume Right	0	8	12	0	8	12
GSH	1601	1700	995	1601	1700	995
Volume to Capacity	0.00	0.01	0.04	0.00	0.01	0.04
Queue Length 95th (m)	0.1	0.0	1.0	0.1	0.0	1.0
Control Delay (s)	3.6	0.0	8.8	3.6	0.0	8.8
Lane LOS	A	A	A	A	A	A
Approach Delay (s)	3.6	0.0	8.8	3.6	0.0	8.8
Approach LOS	A	A	A	A	A	A
Intersection Summary						
Average Delay	6.0					
Intersection Capacity Utilization	15.4% ICU Level of Service A					
Analysis Period (min)	15					

Lanes, Volumes, Timings 2030 Total AM Peak Hour.syn  
 1: Trafalgar Road & North Service Road/QEW WB Off Ramp 05-04-2022

Intersection Capacity Utilization 83.1% ICU Level of Service E  
 Analysis Period (min) 15  
 m Volume for 95th percentile queue is metered by upstream signal.



Lanes, Volumes, Timings  
1: Trafalgar Road & North Service Road/QEW WB Off Ramp

2030 Total AM Peak Hour.syn  
05-04-2022

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Turn Type	7											
Protected Phases	Prot											
Permitted Phases	4	8	8	8	8	8	2	2	2	2	6	6
Detector Phase	7	4	8	8	8	8	2	2	2	2	6	6
Switch Phase												
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Minimum Split (s)	23.0	38.0	38.0	38.0	38.0	38.0	35.0	35.0	35.0	35.0	35.0	35.0
Total Split (s)	23.0	77.0	54.0	54.0	54.0	54.0	63.0	63.0	63.0	63.0	63.0	63.0
Total Split (%)	16.4%	55.0%	38.6%	38.6%	38.6%	38.6%	45.0%	45.0%	45.0%	45.0%	45.0%	45.0%
Maximum Green (s)	18.0	70.0	47.0	47.0	47.0	47.0	56.0	56.0	56.0	56.0	56.0	56.0
Yellow Time (s)	3.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
All-Red Time (s)	2.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Lost Time Adjust (s)	-1.0	-3.0	-3.0	-3.0	-3.0	-3.0	-3.0	-3.0	-3.0	-3.0	-3.0	-3.0
Total Lost Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lead/Lag	Lead	Lag	Lag	Lag	Lag	Lag	Lag	Lag	Lag	Lag	Lag	Lag
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	None	None	None	None	None	None	C-Max	C-Max	C-Max	C-Max	C-Max	C-Max
Walk Time (s)	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0
Flash Dont Walk (s)	11.0	11.0	11.0	11.0	11.0	11.0	11.0	11.0	11.0	11.0	11.0	11.0
Pedestrian Calls (#/hr)	0	0	0	0	0	0	0	0	0	0	0	0
Act Effct Green (s)	6.7	55.2	53.0	53.0	53.0	53.0	76.8	76.8	76.8	76.8	76.8	76.8
Actuated g/C Ratio	0.05	0.39	0.38	0.38	0.38	0.38	0.55	0.55	0.55	0.55	0.55	0.55
v/c Ratio	0.01	0.29	0.84	0.23	0.22	0.22	0.71	0.50	0.71	0.50	0.71	0.01
Control Delay	64.0	23.4	51.4	9.8	4.5	4.5	13.1	4.3	4.3	26.8	26.8	0.0
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	64.0	23.4	51.4	9.8	4.5	4.5	13.1	4.3	4.3	26.8	26.8	0.0
LOS	E	C	D	A	A	A	B	A	A	C	C	A
Approach Delay	23.6			36.4			11.3			26.7		
Approach LOS	C			D			B			C		
Queue Length 50th (m)	0.3	30.9	142.4	8.3	0.0	0.0	78.5	12.7	144.9	144.9	0.0	0.0
Queue Length 95th (m)	2.3	38.7	174.3	22.0	13.1	13.1	170.0	m38.8	218.5	218.5	0.0	0.0
Internal Link Dist (m)		96.4		141.1			121.2			185.2		
Turn Bay Length (m)	50.0						70.0					
Base Capacity (vph)	244	832	669	627	646	646	2709	941	2762	872	872	872
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.00	0.22	0.81	0.23	0.21	0.21	0.71	0.50	0.71	0.50	0.71	0.01
Intersection Summary												
Area Type:	Other											
Actuated Cycle Length:	140											
Offset: 0 (0%):	Referenced to phase 2:NBT and 6:SBT, Start of Green											
Natural Cycle:	100											
Control Type:	Actuated-Coordinated											
Maximum v/c Ratio:	0.84											
Intersection Signal Delay:	21.2											
Intersection LOS:	C											

Lanes, Volumes, Timings  
11: Argus Rd & South Service Rd

2025 Background PM Peak Hour.syn  
05-04-2022

Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	4	4	4	4	4	4
Traffic Volume (vph)	11	8	296	87	14	15
Future Volume (vph)	11	8	296	87	14	15
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor			0.963		0.926	
Flt Protected			0.971		0.978	
Satd. Flow (prot)	0	1578	1535	0	1549	0
Flt Permitted			0.971		0.978	
Satd. Flow (perm)	0	1578	1535	0	1549	0
Link Speed (k/h)			50		50	
Link Distance (m)			177.7		103.5	
Travel Time (s)			12.8		7.5	
Conf. Peds. (#/hr)					5	
Peak Hour Factor	0.45	0.50	0.87	0.68	0.75	0.65
Heavy Vehicles (%)	0%	13%	10%	0%	0%	0%
Adj. Flow (vph)	24	16	340	128	19	23
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	40	468	0	42	0
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Left	Left	Right	Left	Right
Median Width(m)			0.0		3.6	
Link Offset(m)			0.0		0.0	
Crosswalk Width(m)			4.8		4.8	
Two way Left Turn Lane						
Headway Factor	1.14	1.14	1.14	1.14	1.14	1.14
Turning Speed (k/h)	24		Free		Stop	
Sign Control			Free		Stop	
Intersection Summary						
Area Type:	CBD					
Control Type:	Unsignalized					
Intersection Capacity Utilization	33.2%					
Analysis Period (min)	15					
ICU Level of Service A						

HCM Unsignalized Intersection Capacity Analysis  
 11: Argus Rd & South Service Rd

2025 Background PM Peak Hour.syn  
 05-04-2022



Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	11	8	296	87	14	15
Traffic Volume (veh/h)	11	8	296	87	14	15
Future Volume (Veh/h)	11	8	296	87	14	15
Sign Control	Free	Free	0%	0%	Stop	0%
Grade	0%	0%	0%	0%	0%	0%
Peak Hour Factor	0.45	0.50	0.87	0.68	0.75	0.65
Hourly flow rate (vph)	24	16	340	128	19	23
Pedestrians						
Lane Width (m)			5			
Walking Speed (m/s)			3.6			
Percent Blockage			1.2			
Right turn flare (veh)			0			
Median type			None			
Median storage (veh)			None			
Upstream signal (m)			368			
pX, platoon unblocked			468			
vC, conflicting volume			468			
vC1, stage 1 conf/vol			473			
vC2, stage 2 conf/vol			473			
tC, single (s)			4.1			
tC, 2 stage (s)			6.4			
tF (s)			2.2			
p0 queue free %			98			
cM capacity (veh/h)			1104			
Direction_Lane#	EB 1	WB 1	SB 1			
Volume Total	40	468	42			
Volume Left	24	0	19			
Volume Right	0	128	23			
GSH	1104	1700	595			
Volume to Capacity	0.02	0.28	0.07			
Queue Length 95th (m)	0.5	0.0	1.8			
Control Delay (s)	5.1	0.0	11.5			
Lane LOS	A	B	B			
Approach Delay (s)	5.1	0.0	11.5			
Approach LOS	B					
Intersection Summary						
Average Delay			1.2			
Intersection Capacity Utilization			33.2%			
Analysis Period (min)			15			
				ICU Level of Service		A

Lanes, Volumes, Timings  
 1: Trafalgar Road & North Service Road/QEW WB Off Ramp

2030 Total AM Peak Hour.syn  
 05-04-2022



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	1	0	171	501	38	219	0	1777	437	0	1799	7
Traffic Volume (vph)	1	0	171	501	38	219	0	1777	437	0	1799	7
Future Volume (vph)	1	0	171	501	38	219	0	1777	437	0	1799	7
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (m)	50.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	70.0	0.0	0.0	0.0
Storage Lanes	1	1	1	1	1	1	0	0	1	0	0	1
Taper Length (m)	7.5			7.5			7.5			7.5		
Lane Util. Factor	1.00	1.00	1.00	1.00	0.95	0.95	1.00	0.91	1.00	1.00	0.91	1.00
Ped Bike Factor								0.96				0.95
Fit			0.850		0.893		0.850		0.850			0.850
Fit Protected			0.950		0.950							
Satd. Flow (prot)	1805	0	1568	1719	1454	1447	0	4940	1538	0	5036	1615
Fit Permitted	0.950			0.950								
Satd. Flow (perm)	1805	0	1568	1719	1454	1447	0	4940	1478	0	5036	1533
Right Turn on Red	Yes		Yes		Yes			Yes	Yes		Yes	Yes
Satd. Flow (RTOR)	31		100		136			291			50	70
Link Speed (k/h)	50		50		50			50			50	50
Link Distance (m)	120.4		165.1		111.9			145.2			209.2	15.1
Travel Time (s)	8.7		11.9					10.5			15.1	
Conf. Peds. (#/hr)					8						5	8
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	0%	0%	3%	5%	23%	6%	0%	5%	5%	0%	3%	0%
Adj. Flow (vph)	1	0	186	545	41	238	0	1932	475	0	1955	8
Shared Lane Traffic (%)					43%							
Lane Group Flow (vph)	1	0	186	545	143	136	0	1932	475	0	1955	8
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Right	Right	Left	Left	Right	Left	Right	Right
Median Width (m)	3.6		3.6		3.6			3.6			3.6	3.6
Link Offset (m)	0.0		0.0		0.0			0.0			0.0	0.0
Crosswalk Width (m)	4.8		4.8		4.8			4.8			4.8	4.8
Two way Left Turn Lane												
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (k/h)	25	15	25	15	25	15	25	15	25	15	25	15
Number of Detectors	1	1	1	2	1	2	2	1	2	1	2	1
Detector Template	Left	Right	Left	Thru	Right	Thru	Right	Thru	Right	Left	Thru	Right
Leading Detector (m)	2.0	2.0	2.0	10.0	2.0	10.0	2.0	10.0	2.0	10.0	2.0	2.0
Trailing Detector (m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Position (m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Size (m)	2.0	2.0	2.0	0.6	2.0	0.6	2.0	0.6	2.0	0.6	2.0	0.6
Detector 1 Type	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 2 Position (m)				9.4		9.4		9.4			9.4	
Detector 2 Size (m)				0.6		0.6		0.6			0.6	
Detector 2 Type				Ch+Ex		Ch+Ex		Ch+Ex			Ch+Ex	
Detector 2 Channel												
Detector 2 Extend (s)				0.0		0.0		0.0			0.0	

HCM Unsignalized Intersection Capacity Analysis  
 12: Site Driveway  
 2025 Total PM Peak Hour.syn  
 05-04-2022

Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	8	22	45	19	4	39
Traffic Volume (veh/h)	8	22	45	19	4	39
Future Volume (Veh/h)	Free	Free	Stop	Stop	0%	0%
Sign Control	0%	0%	0%	0%	0%	0%
Grade	0.92	0.92	0.92	0.92	0.92	0.92
Peak Hour Factor	9	24	49	21	4	42
Hourly flow rate (vph)						
Pedestrians						
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)						
Median type	None					
Median storage (veh)						
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume						
vC1, stage 1 conf/vol	33				140	21
vC2, stage 2 conf/vol						
vCu, unblocked vol	33				140	21
tC, single (s)	4.1				6.4	6.2
tC, 2 stage (s)						
tF (s)	2.2				3.5	3.3
cM capacity (veh/h)	1579				100	96
cM capacity (veh/h)	1579				827	1056
Direction, Lane #	EB 1	WB 1	NB 1			
Volume Total	33	70	46			
Volume Left	0	49	4			
Volume Right	24	0	42			
GSH	1700	1579	1032			
Volume to Capacity	0.02	0.03	0.04			
Queue Length 95th (m)	0.0	0.8	1.1			
Control Delay (s)	0.0	5.2	8.7			
Lane LOS	A	A	A			
Approach Delay (s)	0.0	5.2	8.7			
Approach LOS	A	A	A			
Intersection Summary						
Average Delay	5.1					
Intersection Capacity Utilization	20.2%				ICU Level of Service	A
Analysis Period (min)	15					

Lanes, Volumes, Timings  
 1: Trafalgar Road & North Service Road/QEW WB Off Ramp  
 2030 Background AM Peak Hour.syn  
 05-04-2022

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	1	0	171	499	38	219	0	1744	417	0	1789	7
Traffic Volume (vph)	1	0	171	499	38	219	0	1744	417	0	1789	7
Future Volume (vph)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Ideal Flow (vphpl)	50.0	0.0	0.0	0.0	0.0	0.0	0.0	70.0	0.0	0.0	0.0	0.0
Storage Length (m)	1	1	1	1	1	1	0	1	1	0	1	1
Storage Lanes	7.5			7.5			7.5			7.5		
Taper Length (m)	1.00	1.00	1.00	1.00	0.95	0.95	1.00	0.91	1.00	1.00	0.91	1.00
Lane Util. Factor												
Ped Bike Factor			0.850		0.893		0.850		0.850		0.96	0.850
Fit Protected	0.950			0.950								0.850
Satd. Flow (prot)	1805	0	1568	1719	1454	1447	0	4940	1538	0	5036	1615
Fit Permitted	0.950			0.950								
Satd. Flow (perm)	1805	0	1568	1719	1454	1447	0	4940	1478	0	5036	1533
Right Turn on Red	Yes		Yes		Yes			Yes		Yes		Yes
Satd. Flow (RTOR)	31		31		100		136		282		50	70
Link Speed (k/h)	50		50		50		50		50		50	50
Link Distance (m)	120.4		120.4		165.1		165.1		145.2		209.2	209.2
Travel Time (s)	8.7		8.7		11.9		10.5		10.5		15.1	15.1
Confl. Peds. (#/hr)							8		5		5	8
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	0%	0%	3%	5%	23%	6%	0%	5%	5%	0%	3%	0%
Adj. Flow (vph)	1	0	186	542	41	238	0	1896	453	0	1945	8
Shared Lane Traffic (%)							43%					
Lane Group Flow (vph)	1	0	186	542	143	136	0	1896	453	0	1945	8
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Right	Right	Left	Left	Right	Left	Left	Right
Median Width (m)	3.6		3.6		3.6		3.6		3.6		3.6	3.6
Link Offset (m)	0.0		0.0		0.0		0.0		0.0		0.0	0.0
Crosswalk Width (m)	4.8		4.8		4.8		4.8		4.8		4.8	4.8
Two way Left Turn Lane												
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (k/h)	25	15	25	15	25	15	25	15	25	15	25	15
Number of Detectors	1	1	1	2	1	2	2	1	2	1	2	1
Detector Template	Left	Right	Left	Thru	Right	Thru	Right	Thru	Right	Thru	Right	Right
Leading Detector (m)	2.0	2.0	2.0	10.0	2.0	10.0	2.0	10.0	2.0	10.0	2.0	2.0
Trailing Detector (m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Position (m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Size (m)	2.0	2.0	2.0	0.6	2.0	0.6	2.0	0.6	2.0	0.6	2.0	2.0
Detector 1 Type	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 2 Position (m)				9.4		9.4		9.4		9.4		9.4
Detector 2 Size (m)				0.6		0.6		0.6		0.6		0.6
Detector 2 Type				Ch+Ex		Ch+Ex		Ch+Ex		Ch+Ex		Ch+Ex
Detector 2 Channel												
Detector 2 Extend (s)				0.0		0.0		0.0		0.0		0.0



Lanes, Volumes, Timings  
1: Trafalgar Road & North Service Road/QEW WB Off Ramp

Lanes, Volumes, Timings  
12: Site Driveway

2030 Background AM Peak Hour.syn  
05-04-2022

2025 Total PM Peak Hour.syn  
05-04-2022

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Turn Type	Prot	Perm	Perm	Perm	Perm	Perm	NA	NA	NA	Perm	NA	Perm
Protected Phases	7											
Permitted Phases		4	8	8	8	8	2	2	2	2	6	6
Detector Phase	7	4	8	8	8	8	2	2	2	2	6	6
Switch Phase												
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Minimum Split (s)	23.0	38.0	38.0	38.0	38.0	38.0	35.0	35.0	35.0	35.0	35.0	35.0
Total Split (s)	23.0	77.0	54.0	54.0	54.0	54.0	63.0	63.0	63.0	63.0	63.0	63.0
Total Split (%)	16.4%	55.0%	38.6%	38.6%	38.6%	38.6%	45.0%	45.0%	45.0%	45.0%	45.0%	45.0%
Maximum Green (s)	18.0	70.0	47.0	47.0	47.0	47.0	56.0	56.0	56.0	56.0	56.0	56.0
Yellow Time (s)	3.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
All-Red Time (s)	2.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Lost Time Adjust (s)	-1.0	-3.0	-3.0	-3.0	-3.0	-3.0	-3.0	-3.0	-3.0	-3.0	-3.0	-3.0
Total Lost Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lead/Lag	Lead	Lag	Lag	Lag	Lag	Lag	Lag	Lag	Lag	Lag	Lag	Lag
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	None	None	None	None	None	None	C-Max	C-Max	C-Max	C-Max	C-Max	C-Max
Walk Time (s)		7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0
Flash Dont Walk (s)		11.0	11.0	11.0	11.0	11.0	11.0	11.0	11.0	11.0	11.0	11.0
Pedestrian Calls (#/hr)		0	0	0	0	0	0	0	0	0	0	0
Act Effct Green (s)	6.7	55.0	52.8	52.8	52.8	52.8	77.0	77.0	77.0	77.0	77.0	77.0
Actuated g/C Ratio	0.05	0.39	0.38	0.38	0.38	0.38	0.55	0.55	0.55	0.55	0.55	0.55
v/c Ratio	0.01	0.29	0.84	0.23	0.22	0.22	0.70	0.48	0.70	0.01	0.70	0.01
Control Delay	64.0	23.5	51.5	9.8	4.5	4.5	12.9	4.4	4.4	26.6	0.0	0.0
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	64.0	23.5	51.5	9.8	4.5	4.5	12.9	4.4	4.4	26.6	0.0	0.0
LOS	E	C	D	A	A	A	B	A	A	C	C	A
Approach Delay		23.7		36.4			11.2			26.4		
Approach LOS		C		D			B			C		
Queue Length 50th (m)	0.3	31.1	141.8	8.3	0.0	0.0	97.9	16.9	16.9	143.0	0.0	0.0
Queue Length 95th (m)	2.3	38.7	172.7	22.0	13.1	13.1	154.7	m38.1	m38.1	216.6	0.0	0.0
Internal Link Dist (m)		96.4		141.1			121.2			185.2		
Turn Bay Length (m)	50.0						70.0					
Base Capacity (vph)	244	832	667	626	645	645	2717	940	940	2770	874	874
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.00	0.22	0.81	0.23	0.21	0.21	0.70	0.48	0.48	0.70	0.01	0.01
Intersection Summary	Other											
Area Type:	Other											
Control Type:	Unsignalized											
Intersection Capacity Utilization	20.2%											
Analysis Period (min)	15											
ICU Level of Service A												

Baseline

Baseline

HCM Unsignalized Intersection Capacity Analysis  
 11: Argus Rd & South Service Rd

Lanes, Volumes, Timings  
 1: Trafalgar Road & North Service Road/QEW WB Off Ramp

2025 Total PM Peak Hour.syn  
 05-04-2022

2030 Background AM Peak Hour.syn  
 05-04-2022



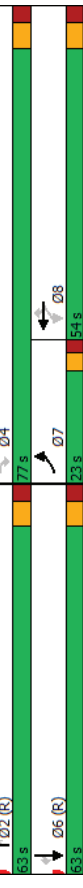
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	11	8	296	132	25	43
Traffic Volume (veh/h)	11	8	296	132	25	43
Future Volume (Veh/h)	Free	Free	0%	Stop	0%	0%
Sign Control	0%	0%	0%	0%	0%	0%
Grade	0.45	0.50	0.87	0.68	0.75	0.65
Peak Hour Factor	24	16	340	194	33	66
Hourly flow rate (vph)	5	3.6	1.2	0	None	None
Pedestrians	358	534	506	437		
Lane Width (m)	534	534	506	437		
Walking Speed (m/s)	4.1	2.2	3.5	3.3		
Percent Blockage	98	1044	94	89		
Right turn flare (veh)	1044	40	534	99		
Median type	None	None	None	None		
Median storage (veh)	358	534	506	437		
Upstream signal (m)	534	534	506	437		
pX, platoon unblocked	4.1	2.2	3.5	3.3		
vC, conflicting volume	98	1044	94	89		
vC1, stage 1 conf/vol	1044	40	534	99		
vC2, stage 2 conf/vol	24	0	33	0		
tC, single (s)	194	66	66	66		
tC, 2 stage (s)	1044	1700	583	583		
tF (s)	0.02	0.31	0.17	0.17		
p0 queue free %	0.6	0.0	4.9	0.6		
cM capacity (veh/h)	5.2	0.0	12.4	12.4		
Direction_Lane #	EB1	WB1	SB1			
Volume Total	40	534	99			
Volume Left	24	0	33			
Volume Right	0	194	66			
GSH	1044	1700	583			
Volume to Capacity	0.02	0.31	0.17			
Queue Length 95th (m)	0.6	0.0	4.9	0.6		
Control Delay (s)	5.2	0.0	12.4	12.4		
Lane LOS	A	B	B			
Approach Delay (s)	5.2	0.0	12.4			
Approach LOS	B	B	B			
<b>Intersection Summary</b>						
Average Delay			2.1			
Intersection Capacity Utilization			37.4%	ICU Level of Service A		
Analysis Period (min)			15			

Intersection Capacity Utilization 82.8%

Analysis Period (min) 15

m Volume for 95th percentile queue is metered by upstream signal.

Spills and Phases: 1: Trafalgar Road & North Service Road/QEW WB Off Ramp



HCM Signalized Intersection Capacity Analysis  
 1: Trafalgar Road & North Service Road/QEW WB Off Ramp

2030 Background AM Peak Hour.syn  
 05-04-2022



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	1	0	171	499	38	219	0	1744	417	0	1789	7
Traffic Volume (vph)	1	0	171	499	38	219	0	1744	417	0	1789	7
Future Volume (vph)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Ideal Flow (vphpl)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Total Lost time (s)	1.00	1.00	1.00	0.95	0.95	0.95	1.00	0.91	1.00	0.91	1.00	1.00
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.96	1.00	0.96	1.00	0.95
Frb, ped/bikes	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frb, ped/bikes	1.00	0.85	1.00	0.89	0.85	1.00	0.85	1.00	0.85	1.00	0.85	1.00
Frt	0.95	1.00	0.95	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Flt Protected	1805	1568	1719	1454	1447	4940	1478	5036	1533	5036	1533	1533
Satd. Flow (prot)	0.95	1.00	0.95	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Flt Permitted	1805	1568	1719	1454	1447	4940	1478	5036	1533	5036	1533	1533
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	1	0	186	542	41	238	0	1896	453	0	1945	8
RTOR Reduction (vph)	0	0	18	0	62	85	0	135	0	0	0	4
Lane Group Flow (vph)	1	0	168	542	81	51	0	1896	318	0	1945	4
Conf. Peds. (#/hr)	0%	0%	3%	5%	23%	6%	0%	5%	5%	0%	3%	0%
Heavy Vehicles (%)	Prot	Per	Per	Per	Per	Per	Per	Per	Per	Per	Per	Per
Turn Type	7	8	8	8	8	8	2	2	2	2	6	6
Protected Phases	1.2	56.0	49.8	49.8	49.8	70.0	70.0	70.0	70.0	70.0	70.0	70.0
Actuated Green, G (s)	2.2	59.0	52.8	52.8	52.8	73.0	73.0	73.0	73.0	73.0	73.0	73.0
Effective Green, g (s)	0.02	0.42	0.38	0.38	0.38	0.52	0.52	0.52	0.52	0.52	0.52	0.52
Actuated g/C Ratio	5.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0
Clearance Time (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Vehicle Extension (s)	28	660	648	548	545	2575	770	2625	799	2625	799	799
Lane Grp. Cap (vph)	0.00	v/s Ratio	Prot	Per	Per	Per	Per	Per	Per	Per	Per	Per
v/s Ratio Prot	0.11	c0.32	0.06	0.04	0.04	0.22	0.22	0.22	0.22	0.22	0.22	0.22
v/s Ratio Perm	0.04	0.25	0.84	0.15	0.09	0.74	0.41	0.41	0.41	0.41	0.74	0.01
Uniform Delay, d1	67.9	26.2	39.7	28.8	28.2	26.0	20.4	26.1	16.1	26.1	16.1	16.1
Progression Factor	1.00	1.00	1.00	1.00	1.00	0.46	0.37	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	0.5	0.2	0.2	0.1	0.1	1.5	1.3	1.9	0.0	1.9	0.0	0.0
Delay (s)	68.4	26.5	48.9	28.9	28.2	13.3	8.9	28.1	16.1	28.1	16.1	16.1
Level of Service	E	C	D	C	C	B	A	C	B	C	B	B
Approach Delay (s)	26.7	42.0	42.0	42.0	42.0	12.5	12.5	28.0	28.0	28.0	28.0	28.0
Approach LOS	C	D	D	D	D	B	B	C	C	C	C	C
Intersection Summary												
HCM 2000 Control Delay	23.2 HCM 2000 Level of Service C											
HCM 2000 Volume to Capacity ratio	0.77											
Actuated Cycle Length (s)	140.0 Sum of lost time (s)											
Intersection Capacity Utilization	82.8% ICU Level of Service E											
Analysis Period (min)	15											
c Critical Lane Group												

Lanes, Volumes, Timings  
 11: Argus Rd & South Service Rd

2025 Total PM Peak Hour.syn  
 05-04-2022



Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	11	8	296	132	25	43
Traffic Volume (vph)	11	8	296	132	25	43
Future Volume (vph)	1900	1900	1900	1900	1900	1900
Ideal Flow (vphpl)	1.00	1.00	1.00	1.00	1.00	1.00
Total Lost time (s)	1.00	1.00	0.951	0.984	0.910	0.984
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frb, ped/bikes	1.00	1.00	1.00	1.00	1.00	1.00
Frb, ped/bikes	1.00	0.971	0.971	0.971	0.971	0.971
Frt	0	1578	1529	0	1531	0
Flt Protected	0	1578	1529	0	1531	0
Satd. Flow (prot)	0	1578	1529	0	1531	0
Flt Permitted	0	1578	1529	0	1531	0
Satd. Flow (perm)	0	1578	1529	0	1531	0
Link Speed (k/h)	50	50	50	50	50	50
Link Distance (m)	177.7	165.2	103.5	103.5	103.5	103.5
Travel Time (s)	12.8	11.9	7.5	7.5	7.5	7.5
Conf. Peds. (#/hr)	0.45	0.50	0.87	0.68	0.75	0.65
Peak Hour Factor	0%	13%	10%	0%	0%	0%
Heavy Vehicles (%)	24	16	340	194	33	66
Adj. Flow (vph)	24	16	340	194	33	66
Shared Lane Traffic (%)	0	40	534	0	99	0
Lane Group Flow (vph)	No	No	No	No	No	No
Enter Blocked Intersection	Left	Left	Right	Left	Right	Right
Lane Alignment	0.0	0.0	0.0	0.0	3.6	3.6
Median Width(m)	0.0	0.0	0.0	0.0	4.8	4.8
Link Offset(m)	4.8	4.8	4.8	4.8	4.8	4.8
Crosswalk Width(m)	1.14	1.14	1.14	1.14	1.14	1.14
Two way Left Turn Lane	24	Free	Free	Free	Stop	Stop
Headway Factor	1.14	1.14	1.14	1.14	1.14	1.14
Turning Speed (k/h)	24	Free	Free	Free	Stop	Stop
Sign Control						
Intersection Summary						
Area Type:	CBD					
Control Type:	Unsignalized					
Intersection Capacity Utilization	37.4%					
Analysis Period (min)	15					
ICU Level of Service A						

HCM Unsignalized Intersection Capacity Analysis  
 10: Lyons Lane & South Service Road

2025 Total PM Peak Hour.syn  
 05-04-2022



Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	3	5	1	27	18	5
Traffic Volume (veh/h)	3	5	1	27	18	5
Future Volume (Veh/h)	3	5	1	27	18	5
Sign Control	Free	Free	Free	Stop	Stop	0%
Grade	0%	0%	0%	0%	0%	0%
Peak Hour Factor	0.38	0.62	0.25	0.62	0.50	0.42
Hourly flow rate (vph)	8	8	4	44	36	12
Pedestrians						
Lane Width (m)					7	3.6
Walking Speed (m/s)					1.2	
Percent Blockage					1	
Right turn flare (veh)					None	None
Median type					None	None
Median storage (veh)						
Upstream signal (m)						
pX, platoon unblocked					57	33
vC, conflicting volume						
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
tC, single (s)					57	33
tC, 2 stage (s)					6.4	6.2
tF (s)					3.5	3.3
p0 queue free %					96	99
cM capacity (veh/h)					945	1040
Direction, Lane #	EB1	WB1	SB1			
Volume Total	16	48	48			
Volume Left	8	0	36			
Volume Right	0	44	12			
GSH	1554	1700	967			
Volume to Capacity	0.01	0.03	0.05			
Queue Length 95th (m)	0.1	0.0	1.3			
Control Delay (s)	3.7	0.0	8.9			
Lane LOS	A	A	A			
Approach Delay (s)	3.7	0.0	8.9			
Approach LOS	A	A	A			
Intersection Summary						
Average Delay				4.3		
Intersection Capacity Utilization				15.4%		ICU Level of Service A
Analysis Period (min)				15		

Lanes, Volumes, Timings  
 2: Trafalgar Road & QEW SB On Ramps

2030 Background AM Peak Hour.syn  
 05-04-2022



Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Volume (vph)	0	0	0	2161	1756	744
Future Volume (vph)	0	0	0	2161	1756	744
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	0.91	0.91	1.00
Flt Protected						0.850
Satd. Flow (prot)	0	0	0	5085	5085	1583
Flt Permitted						
Satd. Flow (perm)	0	0	0	5085	5085	1583
Link Speed (k/h)	50	50	50	50	50	50
Link Distance (m)	159.6	159.6	186.0	145.2	145.2	145.2
Travel Time (s)	11.5	11.5	13.4	10.5	10.5	10.5
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	0	0	0	2349	1909	809
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	0	0	2349	1909	809
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Left	Left	Right
Median Width(m)	0.0	0.0	0.0	0.0	0.0	0.0
Link Offset(m)	0.0	0.0	0.0	0.0	0.0	0.0
Crosswalk Width(m)	4.8	4.8	4.8	4.8	4.8	4.8
Two way Left Turn Lane						
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (k/h)	25	15	25	Free	Free	15
Sign Control	Stop	Stop	Free	Free	Free	Free
Intersection Summary						
Area Type:						Other
Control Type:						Unsignalized
Intersection Capacity Utilization						96.5%
Analysis Period (min)						15
ICU Level of Service F						

HCM Unsignalized Intersection Capacity Analysis 2030 Background AM Peak Hour.syn  
 2: Trafalgar Road & QEW SB On Ramps 05-04-2022

Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations			↑↑↑↑	↑↑↑↑	↑↑↑↑	↑
Traffic Volume (veh/h)	0	0	0	2161	1756	744
Future Volume (Veh/h)	0	0	0	2161	1756	744
Sign Control	Stop	Free	Free	Free	Free	Free
Grade	0%	0%	0%	0%	0%	0%
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	0	0	0	2349	1903	809
Pedestrians						
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)						
Median type			None	None	None	None
Median storage (veh)						
Upstream signal (m)				186	145	
pX, platoon unblocked	0.75	0.74	0.74			
vC, conflicting volume	2692	636	1909			
vC1, stage 1 conf/vol						
vC2, stage 2 conf/vol						
vCu, unblocked vol	0	0	977			
tC, single (s)	6.8	6.9	4.1			
tC, 2 stage (s)						
tF (s)	3.5	3.3	2.2			
p0 queue free %	100	100	100			
cM capacity (veh/h)	764	797	516			
Direction_Lane #	NB.1	NB.2	NB.3	SB.1	SB.2	SB.3
Volume Total	783	783	783	636	636	636
Volume Left	0	0	0	0	0	0
Volume Right	0	0	0	0	0	0
GSV	1700	1700	1700	1700	1700	1700
Volume to Capacity	0.46	0.46	0.46	0.37	0.37	0.48
Queue Length 95th (m)	0.0	0.0	0.0	0.0	0.0	0.0
Control Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0
Lane LOS						
Approach Delay (s)	0.0	0.0	0.0			
Approach LOS						
Intersection Summary						
Average Delay	0.0					
Intersection Capacity Utilization	96.5% ICU Level of Service					
Analysis Period (min)	15					

Lanes, Volumes, Timings 2025 Total PM Peak Hour.syn  
 10: Lyons Lane & South Service Road 05-04-2022

Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		←	←	←	←	←
Traffic Volume (vph)	3	5	1	27	18	5
Future Volume (vph)	3	5	1	27	18	5
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor						
Ft		0.876			0.966	
Ft Protected		0.976			0.964	
Satd. Flow (prot)		0	1669	1247	0	1592
Ft Permitted		0.976			0.964	
Satd. Flow (perm)		0	1669	1247	0	1592
Link Speed (k/h)		50	50	50	50	50
Link Distance (m)		60.5	89.6	37.6		
Travel Time (s)		4.4	6.5	2.7		
Conf. Peds. (#/hr)		7		7		
Peak Hour Factor		0.38	0.62	0.25	0.62	0.42
Heavy Vehicles (%)		0%	0%	0%	22%	0%
Adj. Flow (vph)		8	8	4	44	36
Shared Lane Traffic (%)						
Lane Group Flow (vph)		0	16	48	0	48
Enter Blocked Intersection		No	No	No	No	No
Lane Alignment		Left	Left	Right	Left	Right
Median Width(m)		0.0	0.0	0.0	3.6	
Link Offset(m)		0.0	0.0	0.0	0.0	
Crosswalk Width(m)		4.8	4.8	4.8	4.8	
Two way Left Turn Lane						
Headway Factor		1.14	1.14	1.14	1.14	1.14
Turning Speed (k/h)		24	Free	Free	24	14
Sign Control					Stop	
Intersection Summary						
Area Type:	CBD					
Control Type:	Unsignalized					
Intersection Capacity Utilization	15.4% ICU Level of Service A					
Analysis Period (min)	15					

HCM Unsignalized Intersection Capacity Analysis  
 9: Cross Ave & Lyons Lane  
 2025 Total PM Peak Hour.syn  
 05-04-2022



Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	9	191	623	26	18	32
Traffic Volume (veh/h)	9	191	623	26	18	32
Future Volume (Veh/h)	9	191	623	26	18	32
Sign Control	Free	Free	Free	Stop	Stop	Stop
Grade	0%	0%	0%	0%	0%	0%
Peak Hour Factor	0.50	0.92	0.84	0.58	0.44	0.41
Hourly flow rate (vph)	18	208	742	45	41	78
Pedestrians			9		1	
Lane Width (m)			3.6		3.6	
Walking Speed (m/s)			1.2		1.2	
Percent Blockage			1		0	
Right turn flare (veh)			None		None	
Median type			None		None	
Median storage (veh)			40			
Upstream signal (m)			0.95		0.95	0.95
pX, platoon unblocked			788		914	394
vC, conflicting volume						
vC1, stage 1 conf/vol						
vC2, stage 2 conf/vol						
vCU, unblocked vol			669		802	254
tC, single (s)			4.4		6.8	7.0
tC, 2 stage (s)						
tF (s)			2.4		3.5	3.3
p0 queue free %			98		86	89
cM capacity (veh/h)			780		299	701
Direction_Lane #	EB1	EB2	EB3	WB1	WB2	SB1
Volume Total	18	104	104	495	292	119
Volume Left	18	0	0	0	0	41
Volume Right	0	0	0	0	45	78
GSH	780	1700	1700	1700	1700	479
Volume to Capacity	0.02	0.06	0.06	0.29	0.17	0.25
Queue Length 95th (m)	0.6	0.0	0.0	0.0	0.0	7.8
Control Delay (s)	9.7	0.0	0.0	0.0	0.0	15.0
Lane LOS	A	A	A	B	B	B
Approach Delay (s)	0.8		0.0	0.0	15.0	
Approach LOS					B	
Intersection Summary						
Average Delay	1.7					
Intersection Capacity Utilization	30.1%					
ICU Level of Service	A					
Analysis Period (min)	15					

Lanes, Volumes, Timings  
 3: Trafalgar Road & QEW EB Off Ramp  
 2030 Background AM Peak Hour.syn  
 05-04-2022



Lane Group	EBL	EBR	NBL	NBT	SBL	SBR
Lane Configurations	1	1	1	1	1	1
Traffic Volume (vph)	1013	903	0	1148	1756	0
Future Volume (vph)	1013	903	0	1148	1756	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	0.97	1.00	1.00	0.91	0.91	1.00
Frt	0.850					
Flt Protected	0.950					
Satd. Flow (prot)	3400	1599	0	4893	4988	0
Flt Permitted	0.950					
Satd. Flow (perm)	3400	1599	0	4893	4988	0
Right Turn on Red	Yes					Yes
Satd. Flow (RTOR)	10					
Link Speed (k/h)	50		50			50
Link Distance (m)	71.3		50.7			186.0
Travel Time (s)	5.1		3.7			13.4
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	3%	1%	0%	6%	4%	0%
Adj. Flow (vph)	1101	982	0	1248	1909	0
Shared Lane Traffic (%)						
Lane Group Flow (vph)	1101	982	0	1248	1909	0
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Left	Left	Right
Median Width(m)	7.2		0.0		0.0	0.0
Link Offset(m)	0.0		0.0		0.0	0.0
Crosswalk Width(m)	4.8		4.8		4.8	4.8
Two way Left Turn Lane						
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (k/h)	25	15	25		15	15
Number of Detectors	1		1		2	2
Detector Template	Left	Right	Thru	Thru	Thru	Thru
Leading Detector (m)	2.0	2.0	10.0	10.0	10.0	10.0
Trailing Detector (m)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Position(m)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Size(m)	2.0	2.0	0.6	0.6	0.6	0.6
Detector 1 Type	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex
Detector 1 Channel						
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 2 Position(m)			9.4	9.4	9.4	9.4
Detector 2 Size(m)			0.6	0.6	0.6	0.6
Detector 2 Type			Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex
Detector 2 Channel						
Detector 2 Extend (s)			0.0	0.0	0.0	0.0
Turn Type	Perm	Perm	NA	NA	NA	NA
Protected Phases			4	4	2	6
Permitted Phases	4	4				
Detector Phase	4	4				
Switch Phase			2	2	6	6

Lanes, Volumes, Timings  
3: Trafalgar Road & QEW EB Off Ramp

Lanes, Volumes, Timings  
9: Cross Ave & Lyons Lane

2030 Background AM Peak Hour.syn  
05-04-2022

2025 Total PM Peak Hour.syn  
05-04-2022



Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Minimum Initial (s)	10.0	10.0	29.0	29.0	29.0	29.0
Minimum Split (s)	38.0	38.0	36.0	36.0	36.0	36.0
Total Split (s)	57.4	57.4	82.6	82.6	82.6	82.6
Total Split (%)	41.0%	41.0%	59.0%	59.0%	59.0%	59.0%
Maximum Green (s)	50.4	50.4	75.6	75.6	75.6	75.6
Yellow Time (s)	4.0	4.0	4.0	4.0	4.0	4.0
All-Red Time (s)	3.0	3.0	3.0	3.0	3.0	3.0
Lost Time Adjust (s)	-3.0	-3.0	-3.0	-3.0	-3.0	-3.0
Total Lost Time (s)	4.0	4.0	4.0	4.0	4.0	4.0
Lead/Lag						
Lead/Lag Optimize?						
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	None	None	C-Max	C-Max	C-Max	C-Max
Walk Time (s)	7.0	7.0	7.0	7.0	7.0	7.0
Flash Dont Walk (s)	11.0	11.0	11.0	11.0	11.0	11.0
Pedestrian Calls (#/hr)	0	0	0	0	0	0
Act Effct Green (s)	53.4	53.4	78.6	78.6	78.6	78.6
Actuated g/C Ratio	0.38	0.38	0.56	0.56	0.56	0.56
v/c Ratio	0.85	1.59	0.45	0.88	0.88	0.88
Control Delay	47.2	305.8	32.7	16.2	16.2	16.2
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	47.2	305.8	32.7	16.2	16.2	16.2
LOS	D	F	C	B	B	B
Approach Delay	169.1		32.7	16.2	16.2	16.2
Approach LOS	F		C	B	B	B
Queue Length 50th (m)	150.4	~408.8	109.2	119.8	119.8	119.8
Queue Length 95th (m)	180.3	#492.8	151.3	72.1	72.1	72.1
Internal Link Dist (m)	47.3		26.7	162.0	162.0	162.0
Turn Bay Length (m)						
Base Capacity (vph)	1296	616	2747	2800	2800	2800
Starvation Cap Reductn	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	0.85	1.59	0.45	0.68	0.68	0.68
Intersection Summary	Other					
Area Type:	Other					
Cycle Length:	140					
Actuated Cycle Length:	140					
Offset: 0 (0%). Referenced to phase 2:NBT and 6:SBT, Start of Green						
Natural Cycle:	80					
Control Type:	Actuated-Coordinated					
Maximum v/c Ratio:	1.59					
Intersection Signal Delay:	80.9					
Intersection Capacity Utilization:	96.5%					
Analysis Period (min):	15					
~ Volume exceeds capacity, queue is theoretically infinite.						
# 95th percentile volume exceeds capacity, queue may be longer.						

Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	9	191	623	26	18	32
Traffic Volume (vph)	9	191	623	26	18	32
Future Volume (vph)	1900	1900	1900	1900	1900	1900
Ideal Flow (vphpl)	5.0	0.0	0.0	0.0	0.0	0.0
Storage Length (m)	1	0	0	0	1	0
Taper Length (m)	7.5				7.5	
Lane Util. Factor	1.00	0.95	0.95	0.95	1.00	1.00
Ped Bike Factor						
Fit		0.950	0.991		0.912	
Fit Protected		0.950			0.983	
Satd. Flow (prot)		1388	2954	3131	0	1494
Fit Permitted		0.950			0.983	
Satd. Flow (perm)		1388	2954	3131	0	1494
Link Speed (k/h)		50	50	50	50	50
Link Distance (m)		281.3	40.1	41.7		
Travel Time (s)		20.3	2.9	3.0		
Confl. Peds. (#/hr)		1		1		9
Peak Hour Factor		0.50	0.92	0.84	0.58	0.44
Heavy Vehicles (%)		17%	10%	3%	0%	4%
Adj. Flow (vph)		18	208	742	45	41
Shared Lane Traffic (%)						
Lane Group Flow (vph)		18	208	787	0	119
Enter Blocked Intersection		No	No	No	No	No
Lane Alignment		Left	Left	Right	Left	Right
Median Width(m)		3.6	3.6	3.6	3.6	3.6
Link Offset(m)		0.0	0.0	0.0	0.0	0.0
Crosswalk Width(m)		4.8	4.8	4.8	4.8	4.8
Two way Left Turn Lane		1.14	1.14	1.14	1.14	1.14
Headway Factor		25	15	25	15	15
Turning Speed (k/h)		Free	Free	Free	Stop	Stop
Sign Control		Free	Free	Free	Stop	Stop
Intersection Summary	CBD					
Area Type:	CBD					
Control Type:	Unsignalized					
Intersection Capacity Utilization:	30.1%					
Analysis Period (min):	15					
ICU Level of Service A						

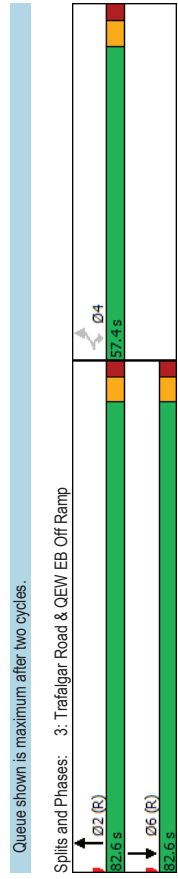
HCM Signalized Intersection Capacity Analysis  
 8: Lyons Lane/Commercial Driveway & Cross Ave

2025 Total PM Peak Hour.syn  
 05-04-2022

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations	39	161	14	15	345	13	243	4	164	16	2	66	
Traffic Volume (vph)	39	161	14	15	345	13	243	4	164	16	2	66	
Future Volume (vph)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	
Ideal Flow (vphpl)	3.3	3.6	3.6	3.3	3.6	3.6	3.3	3.6	3.6	3.3	3.6	3.6	
Lane Width	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	
Total Lost time (s)	1.00	0.95	1.00	1.00	0.95	1.00	1.00	0.98	1.00	0.99	1.00	0.99	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Frbp, ped/bikes	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Fipb, ped/bikes	1.00	0.99	1.00	0.99	1.00	0.95	1.00	0.86	1.00	0.86	1.00	0.86	
Flt Protected	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	
Satd. Flow (prot)	1519	2908	1569	3070	1569	3070	1569	1444	1569	1444	1569	1417	
Flt Permitted	0.52	1.00	0.56	1.00	0.56	1.00	0.68	1.00	0.54	1.00	0.54	1.00	
Satd. Flow (perm)	836	2908	836	919	3070	836	1129	1444	885	1417	885	1417	
Peak-hour factor, PHF	0.58	0.84	0.75	0.54	0.95	0.55	0.74	0.33	0.79	0.70	0.50	0.60	
Adj. Flow (vph)	67	192	19	28	363	24	328	12	208	23	4	110	
RTOR Reduction (vph)	0	8	0	0	5	0	0	137	0	0	0	73	
Lane Group Flow (vph)	67	203	0	28	382	0	328	83	0	23	41	0	
Conf. Peds. (#/hr)	3	3	1	1	3	1	3	1	4	4	4	1	
Heavy Vehicles (%)	3%	11%	0%	0%	5%	0%	0%	0%	0%	0%	0%	2%	
Turn Type	Perm	NA	pm-pt	NA	Perm	NA	Perm	NA	Perm	NA	Perm	NA	
Protected Phases	2	1	6	8	8	8	8	8	8	8	8	4	
Permitted Phases	2	1	6	8	8	8	8	8	8	8	8	4	
Actuated Green, G (s)	35.1	35.1	47.1	47.1	47.1	27.4	27.4	27.4	27.4	27.4	27.4	27.4	
Effective Green, g (s)	37.1	37.1	47.1	49.1	49.1	29.4	29.4	29.4	29.4	29.4	29.4	29.4	
Actuated g/C Ratio	0.43	0.43	0.54	0.57	0.57	0.34	0.34	0.34	0.34	0.34	0.34	0.34	
Clearance Time (s)	6.0	6.0	4.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0	
Vehicle Extension (s)	5.0	5.0	2.5	5.0	5.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	
Lane Grp Cap (vph)	358	1247	560	1742	383	383	490	300	481	300	481	481	
v/s Ratio Prot	0.07	0.07	0.00	c0.12	0.06	c0.29	0.06	0.03	0.03	0.03	0.03	0.03	
v/s Ratio Perm	0.08	0.19	0.16	0.05	0.22	0.86	0.17	0.08	0.08	0.08	0.09	0.09	
Uniform Delay, d1	15.3	15.2	9.2	9.2	26.6	20.0	19.4	19.4	19.4	19.4	19.4	19.4	
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Incremental Delay, d2	0.5	0.1	0.0	0.1	17.4	0.2	0.1	0.1	0.1	0.1	0.1	0.1	
Delay (s)	15.9	15.3	9.3	9.4	44.0	20.2	19.5	19.5	19.5	19.5	19.5	19.5	
Level of Service	B	B	A	A	D	C	B	B	B	B	B	B	
Approach Delay (s)	15.4	15.4	9.4	9.4	34.5	19.5	19.5	19.5	19.5	19.5	19.5	19.5	
Approach LOS	B	B	A	A	C	B	B	B	B	B	B	B	
Intersection Summary													
HCM 2000 Control Delay	21.6											HCM 2000 Level of Service	C
HCM 2000 Volume to Capacity ratio	0.48												
Actuated Cycle Length (s)	86.5											Sum of lost time (s)	12.0
Intersection Capacity Utilization	64.3%											ICU Level of Service	C
Analysis Period (min)	15												
c Critical Lane Group													

Lanes, Volumes, Timings  
 3: Trafalgar Road & QEW EB Off Ramp

2030 Background AM Peak Hour.syn  
 05-04-2022





HCM Signalized Intersection Capacity Analysis  
 3: Trafalgar Road & QEW EB Off Ramp

Lanes, Volumes, Timings  
 8: Lyons Lane/Commercial Driveway & Cross Ave

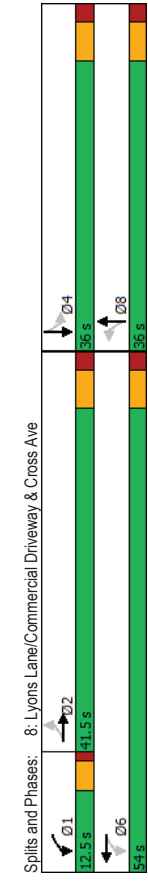
2030 Background AM Peak Hour.syn  
 05-04-2022

2025 Total PM Peak Hour.syn  
 05-04-2022



Intersection Capacity Utilization 64.3%  
 Analysis Period (min) 15  
 ICU Level of Service C

Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	EBL	EBR	NBL	NBT	SBT	SBR
Traffic Volume (vph)	1013	903	0	1148	1756	0
Future Volume (vph)	1013	903	0	1148	1756	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0
Lane Util. Factor	0.97	1.00	0.91	0.91	0.91	0.91
Frt	1.00	0.85	1.00	1.00	1.00	1.00
Flt Protected	0.95	1.00	1.00	1.00	1.00	1.00
Satd. Flow (prot)	3400	1599	4893	4988	4988	4988
Flt Permitted	0.95	1.00	1.00	1.00	1.00	1.00
Satd. Flow (perm)	3400	1599	4893	4988	4988	4988
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	1101	982	0	1248	1909	0
RTOR Reduction (vph)	0	6	0	0	0	0
Lane Group Flow (vph)	1101	976	0	1248	1909	0
Heavy Vehicles (%)	3%	1%	0%	6%	4%	0%
Turn Type	Perm	Perm	NA	NA	NA	NA
Protected Phases			2	6		
Permitted Phases	4	4				
Actuated Green, G (s)	50.4	50.4	75.6	75.6	75.6	
Effective Green, g (s)	53.4	53.4	78.6	78.6	78.6	
Actuated g/C Ratio	0.38	0.38	0.56	0.56	0.56	
Clearance Time (s)	7.0	7.0	7.0	7.0	7.0	
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	
Lane Grp Cap (vph)	1296	609	2747	2800	2800	
v/s Ratio Prot			0.26	0.38		
v/s Ratio Perm	0.32	0.61				
v/c Ratio	0.85	1.60	0.45	0.68	0.68	
Uniform Delay, d1	39.6	43.3	18.1	21.8	21.8	
Progression Factor	1.00	1.00	1.77	0.69	0.69	
Incremental Delay, d2	5.4	278.7	0.4	1.0	1.0	
Delay (s)	45.0	322.0	32.5	16.1	16.1	
Level of Service	D	F	C	B	B	
Approach Delay (s)	175.6		32.5	16.1		
Approach LOS	F		C	B		
Intersection Summary						
HCM 2000 Control Delay			83.4		HCM 2000 Level of Service	F
HCM 2000 Volume to Capacity ratio			1.08			
Actuated Cycle Length (s)			140.0		Sum of lost time (s)	11.0
Intersection Capacity Utilization			96.5%		ICU Level of Service	F
Analysis Period (min)			15			
C Critical Lane Group						



Splits and Phases: 8: Lyons Lane/Commercial Driveway & Cross Ave  
 Analysis Period (min) 15  
 ICU Level of Service C

Lanes, Volumes, Timings  
8: Lyons Lane/Commercial Driveway & Cross Ave

Lanes, Volumes, Timings  
4: Trafalgar Road & Argus Road/QEW EB On Ramp

05-04-2022

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Detector 2 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Turn Type	Perm	NA	pm-pt	NA	Perm	NA	Perm	NA	Perm	NA	Perm	NA
Protected Phases	2	2	6	1	6	8	8	8	8	4	4	4
Permitted Phases	2	2	6	1	6	8	8	8	8	4	4	4
Detector Phase	2	2	6	1	6	8	8	8	8	4	4	4
Switch Phase												
Minimum Initial (s)	35.0	35.0	8.0	35.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0
Minimum Split (s)	41.0	41.0	12.5	41.0	28.0	28.0	28.0	28.0	28.0	28.0	28.0	28.0
Total Split (s)	41.5	41.5	12.5	54.0	36.0	36.0	36.0	36.0	36.0	36.0	36.0	36.0
Total Split (%)	46.1%	46.1%	13.9%	60.0%	40.0%	40.0%	40.0%	40.0%	40.0%	40.0%	40.0%	40.0%
Maximum Green (s)	35.5	35.5	8.5	48.0	30.0	30.0	30.0	30.0	30.0	30.0	30.0	30.0
Yellow Time (s)	4.0	4.0	3.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
All-Red Time (s)	2.0	2.0	1.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
Lost Time Adjust (s)	-2.0	-2.0	0.0	-2.0	-2.0	-2.0	-2.0	-2.0	-2.0	-2.0	-2.0	-2.0
Total Lost Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lead/Lag	Lag	Lag	Lead	Lead	Lead	Lead	Lead	Lead	Lead	Lead	Lead	Lead
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Vehicle Extension (s)	5.0	5.0	2.5	5.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Recall Mode	Min	Min	Min	Min	Min	Min	Min	Min	Min	Min	Min	Min
Walk Time (s)	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0
Flash Dont Walk (s)	15.0	15.0	15.0	15.0	15.0	15.0	15.0	15.0	15.0	15.0	15.0	15.0
Pedestrian Calls (#/hr)	0	0	0	0	0	0	0	0	0	0	0	0
Act Effct Green (s)	37.1	37.1	49.1	49.1	29.5	29.5	29.5	29.5	29.5	29.5	29.5	29.5
Actuated G/C Ratio	0.43	0.43	0.57	0.57	0.34	0.34	0.34	0.34	0.34	0.34	0.34	0.34
v/c Ratio	0.19	0.17	0.05	0.22	0.86	0.35	0.86	0.35	0.86	0.21	0.86	0.21
Control Delay	18.3	15.3	9.4	9.8	49.1	5.3	49.1	5.3	19.8	5.5	49.1	5.5
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	18.3	15.3	9.4	9.8	49.1	5.3	49.1	5.3	19.8	5.5	49.1	5.5
LOS	B	B	A	A	D	A	D	A	B	A	D	A
Approach Delay	16.0	16.0	9.8	9.8	31.5	31.5	31.5	31.5	7.9	31.5	31.5	7.9
Approach LOS	B	B	A	A	C	C	C	C	A	C	C	A
Queue Length 50th (m)	7.5	11.3	2.2	17.0	52.5	1.4	52.5	1.4	2.7	5.5	52.5	1.4
Queue Length 95th (m)	10.2	17.4	3.5	25.1	65.7	0.0	65.7	0.0	6.1	0.4	65.7	0.0
Internal Link Dist (m)	16.1	16.1	350.0	350.0	45.1	45.1	45.1	45.1	45.1	45.1	45.1	45.1
Turn Bay Length (m)	25.0	25.0	25.0	25.0	20.0	20.0	20.0	20.0	20.0	20.0	20.0	20.0
Base Capacity (vph)	363	1269	584	1782	417	666	417	666	328	593	417	593
Stevation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Spillover Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.18	0.17	0.05	0.22	0.79	0.33	0.79	0.33	0.07	0.19	0.79	0.33
<b>Intersection Summary</b>												
Area Type:	CBD											
Cycle Length:	90											
Actuated Cycle Length:	86.6											
Natural Cycle:	85											
Control Type:	Semi Act-Uncoord											
Maximum v/c Ratio:	0.86											
Intersection Signal Delay:	19.5											
<b>Intersection LOS: B</b>												

05-04-2022

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	0	0	20	0	0	0	0	1148	507	0	1824	835
Future Volume (vph)	0	0	20	0	0	0	0	1148	507	0	1824	835
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Storage Lanes	0	0	1	0	0	0	0	0	1	0	0	0
Taper Length (m)	7.5	7.5	7.5	7.5	7.5	7.5	7.5	7.5	7.5	7.5	7.5	7.5
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.91	1.00	1.00	0.91	0.91
Ped Bike Factor												
Fit	0.865	0.865	0.865	0.865	0.865	0.865	0.865	0.865	0.865	0.865	0.865	0.865
Fit Protected												
Satd. Flow (prot)	0	0	1522	0	0	0	0	5085	1568	0	4876	0
Fit Permitted												
Satd. Flow (perm)	0	0	1522	0	0	0	0	5085	1568	0	4876	0
Link Speed (k/h)	50	50	50	50	50	50	50	50	50	50	50	50
Link Distance (m)	165.2	165.2	165.2	196.1	196.1	196.1	271.0	271.0	19.5	19.5	50.7	50.7
Travel Time (s)	11.9	11.9	11.9	14.1	14.1	14.1	19.5	19.5	3.7	3.7	3.7	3.7
Confl. Peds. (#/hr)												24
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	0%	0%	8%	0%	2%	0%	0%	2%	3%	0%	2%	0%
Adj. Flow (vph)	0	0	22	0	0	0	0	1248	551	0	1983	908
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	0	22	0	0	0	0	1248	551	0	2891	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width (m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	3.6	3.6	0.0	3.6	0.0
Link Offset (m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Crosswalk Width (m)	4.8	4.8	4.8	4.8	4.8	4.8	4.8	4.8	4.8	4.8	4.8	4.8
Two way Left Turn Lane												
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (k/h)	25	25	15	25	25	15	25	25	15	25	25	15
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
<b>Intersection Summary</b>												
Area Type:	Other											
Control Type:	Unsignalized											
Intersection Capacity Utilization	64.6%											
Analysis Period (min)	15											

HCM Unsignalized Intersection Capacity Analysis  
 4: Trafalgar Road & Argus Road/QEW EB On Ramp

2030 Background AM Peak Hour.syn  
 05-04-2022

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	0	0	20	0	0	0	0	148	507	0	1824	835
Traffic Volume (veh/h)	0	0	20	0	0	0	0	1148	507	0	1824	835
Future Volume (Veh/h)	0	0	20	0	0	0	0	1148	507	0	1824	835
Sign Control	Stop	Stop	0%	Stop	0%	0%	Free	Free	Free	Free	Free	0%
Grade	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	0	0	22	0	0	0	0	1248	551	0	1983	908
Pedestrians	24											
Lane Width (m)	3.6											
Walking Speed (m/s)	1.2											
Percent Blockage	2											
Right turn flare (veh)								None	None			
Median type												
Median storage (veh)												
Upstream signal (m)												
pX, platoon unblocked	0.78	0.78	0.73	0.78	0.78	0.90	0.73	271		0.90		51
vC, conflicting volume	2877	3709	1139	1909	4163	416	2915			1248		
vC1, stage 1 conf/vol												
vC2, stage 2 conf/vol												
vC, unblocked vol	1547	2618	0	300	3203	0	2320			889		
tC, single (s)	7.5	6.5	7.1	7.5	6.5	6.9	4.1			4.1		
tC, 2 stage (s)												
tF (s)	3.5	4.0	3.4	3.5	4.0	3.3	2.2			2.2		
p0 queue free %	100	100	97	100	100	100	100			100		
cM capacity (veh/h)	59	19	759	471	7	983	155			694		
Direction, Lane #	EB 1	NB 1	NB 2	NB 3	NB 4	SB 1	SB 2	SB 3				
Volume Total	22	416	416	416	551	793	793	1305				
Volume Left	0	0	0	0	0	0	0	0				
Volume Right	22	0	0	0	551	0	0	908				
GSH	759	1700	1700	1700	1700	1700	1700	1700				
Volume to Capacity	0.03	0.24	0.24	0.24	0.32	0.47	0.47	0.77				
Queue Length 95th (m)	0.7	0.0	0.0	0.0	0.0	0.0	0.0	0.0				
Control Delay (s)	9.9	0.0	0.0	0.0	0.0	0.0	0.0	0.0				
Lane LOS	A	A	A	A	A	A	A	A				
Approach Delay (s)	9.9	0.0				0.0						
Approach LOS	A	A				A						
Intersection Summary												
Average Delay	0.0											
Intersection Capacity Utilization	64.6%											
Analysis Period (min)	15											

Lanes, Volumes, Timings  
 8: Lyons Lane/Commercial Driveway & Cross Ave

2025 Total PM Peak Hour.syn  
 05-04-2022

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	39	161	14	15	345	13	243	4	164	16	2	66
Traffic Volume (vph)	39	161	14	15	345	13	243	4	164	16	2	66
Future Volume (vph)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Ideal Flow (vphpl)	3.3	3.6	3.6	3.3	3.6	3.6	3.3	3.6	3.6	3.3	3.6	3.6
Lane Width (m)	0.0	0.0	0.0	25.0	0.0	20.0	0.0	0.0	0.0	0.0	0.0	0.0
Storage Lanes	1	0	0	1	0	0	1	0	0	0	1	0
Taper Length (m)	7.5			7.5			7.5			7.5		
Lane Util. Factor	1.00	0.95	0.95	1.00	0.95	0.95	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor	1.00	1.00		1.00	1.00		1.00	0.98		1.00	0.99	
Frt	0.986			0.991			0.858			0.855		
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1525	2906	0	1570	3071	0	1570	1444	0	1570	1416	0
Flt Permitted	0.523			0.556			0.684			0.537		
Satd. Flow (perm)	836	2906	0	918	3071	0	1129	1444	0	885	1416	0
Right Turn on Red			Yes			Yes			Yes		Yes	
Satd. Flow (RTOR)	14			12			208			110		
Link Speed (k/h)	50			50			50			50		
Link Distance (m)	40.1			374.0			69.1			70.9		
Travel Time (s)	2.9			26.9			5.0			5.1		
Conf. Peds. (#/hr)	3		1	1		3	1		4	4		1
Peak Hour Factor	0.58	0.84	0.75	0.54	0.95	0.55	0.74	0.33	0.79	0.70	0.50	0.60
Heavy Vehicles (%)	3%	11%	0%	0%	5%	0%	0%	0%	0%	0%	0%	2%
Adj. Flow (vph)	67	192	19	28	363	24	328	12	208	23	4	110
Shared Lane Traffic (%)												
Lane Group Flow (vph)	67	211	0	28	387	0	328	220	0	23	114	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(m)	3.3			3.3			3.3			3.3		
Link Offset(m)	0.0			0.0			0.0			0.0		
Crosswalk Width(m)	4.8			4.8			4.8			4.8		
Two way Left Turn Lane												
Headway Factor	1.19	1.14	1.14	1.19	1.14	1.14	1.19	1.14	1.14	1.19	1.14	1.14
Turning Speed (k/h)	24	14	24	24	14	24	24	14	24	24	14	24
Number of Detectors	1	2		1	2		1	2		1	2	
Detector Template	Left	Thru		Left	Thru		Left	Thru		Left	Thru	
Leading Detector (m)	2.0	10.0		2.0	10.0		2.0	10.0		2.0	10.0	
Trailing Detector (m)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Position(m)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Size(m)	2.0	0.6		2.0	0.6		2.0	0.6		2.0	0.6	
Detector 1 Type	Ch+Ex	Ch+Ex		Ch+Ex	Ch+Ex		Ch+Ex	Ch+Ex		Ch+Ex	Ch+Ex	
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Queue (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Delay (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 2 Position(m)	9.4			9.4			9.4			9.4		
Detector 2 Size(m)	0.6			0.6			0.6			0.6		
Detector 2 Type	Ch+Ex	Ch+Ex		Ch+Ex	Ch+Ex		Ch+Ex	Ch+Ex		Ch+Ex	Ch+Ex	
Detector 2 Channel												

HCM Signalized Intersection Capacity Analysis  
 7: GO Bus Terminal/Argus Rd & Cross Ave  
 2025 Total PM Peak Hour.syn  
 05-04-2022

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	15	888	17	38	387	44	16	2	49	179	20	136
Traffic Volume (vph)	15	888	17	38	387	44	16	2	49	179	20	136
Future Volume (vph)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Ideal Flow (vphpl)	3.3	3.6	3.6	3.3	3.6	3.6	3.3	3.6	3.6	3.3	3.6	3.6
Lane Width	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Total Lost time (s)	1.00	0.95	1.00	1.00	0.95	1.00	1.00	1.00	0.95	1.00	1.00	1.00
Lane Util. Factor	0.99	1.00	1.00	1.00	1.00	0.99	1.00	1.00	0.99	1.00	0.97	1.00
Frbp, ped/bikes	0.95	1.00	1.00	1.00	0.98	1.00	0.95	1.00	0.95	1.00	0.95	1.00
Flt Protected	1557	3172	797	3179	778	704	778	704	1522	1284	1522	1284
Satd. Flow (prot)	0.49	1.00	0.13	1.00	0.53	1.00	0.53	1.00	0.70	1.00	0.70	1.00
Flt Permitted	798	3172	105	3179	436	704	436	704	1128	1284	1128	1284
Peak-hour factor, PHF	0.66	0.81	0.75	0.94	0.95	0.81	0.70	0.25	0.67	0.86	0.75	0.78
Adj. Flow (vph)	23	1096	23	40	407	54	23	8	73	208	27	174
RTOR Reduction (vph)	0	2	0	0	11	0	0	54	0	0	128	0
Lane Group Flow (vph)	23	1117	0	40	450	0	23	27	0	208	73	0
Conf. Peds. (#/hr)	9	4	4	4	4	9	12	20	20	20	20	12
Heavy Vehicles (%)	0%	0%	100%	97%	0%	0%	100%	100%	100%	0%	100%	0%
Turn Type	Perm	NA	NA	pm-pt	NA	Perm	NA	NA	NA	Perm	NA	NA
Protected Phases	2	2	1	6	6	8	8	8	8	4	4	4
Permitted Phases	2	2	6	6	8	8	8	8	8	4	4	4
Actuated Green, G (s)	35.9	35.9	48.3	48.3	48.3	19.2	19.2	19.2	19.2	19.2	19.2	19.2
Effective Green, g (s)	37.9	37.9	48.3	50.3	48.3	21.2	21.2	21.2	21.2	21.2	21.2	21.2
Actuated g/C Ratio	0.48	0.48	0.61	0.63	0.27	0.27	0.27	0.27	0.27	0.27	0.27	0.27
Clearance Time (s)	6.0	6.0	4.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0
Vehicle Extension (s)	5.0	5.0	2.5	5.0	5.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lane Grp Cap (vph)	380	1512	136	2011	60.03	0.14	116	187	300	342	300	342
v/s Ratio Prot	0.035	0.035	0.15	0.15	0.05	0.05	0.04	0.04	0.18	0.18	0.06	0.06
v/s Ratio Perm	0.06	0.74	0.29	0.22	0.20	0.15	0.15	0.15	0.69	0.69	0.21	0.21
Uniform Delay, d1	11.2	16.8	9.6	6.2	22.2	22.2	22.2	22.2	26.2	22.7	22.7	22.7
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	0.1	2.3	0.9	0.1	1.1	0.5	0.5	0.5	7.3	0.4	0.4	0.4
Delay (s)	11.3	19.2	10.5	6.4	23.7	22.7	22.7	22.7	33.5	23.1	23.1	23.1
Level of Service	B	B	B	A	A	C	C	C	C	C	C	C
Approach Delay (s)	B	B	B	A	A	C	C	C	C	C	C	C
Approach LOS	B	B	B	A	A	C	C	C	C	C	C	C
Intersection Summary	18.1 HCM 2000 Level of Service B											
HCM 2000 Control Delay	0.67											
HCM 2000 Volume to Capacity ratio	79.5 Sum of lost time (s) 12.0											
Actuated Cycle Length (s)	59.9% ICU Level of Service B											
Intersection Capacity Utilization	15											
Analysis Period (min)	c Critical Lane Group											

Lanes, Volumes, Timings  
 5: Trafalgar Road & Cross Avenue/South Service Road  
 2030 Background AM Peak Hour.syn  
 05-04-2022

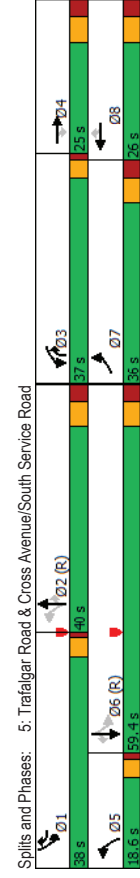
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	556	243	148	67	228	156	138	941	541	336	893	560
Traffic Volume (vph)	556	243	148	67	228	156	138	941	541	336	893	560
Future Volume (vph)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Ideal Flow (vphpl)	25.0	60.0	25.0	60.0	25.0	60.0	50.0	60.0	25.0	60.0	25.0	60.0
Storage Length (m)	2	1	2	1	2	1	1	1	1	1	1	1
Storage Lanes	7.5	7.5	7.5	7.5	7.5	7.5	7.5	7.5	7.5	7.5	7.5	7.5
Taper Length (m)	0.97	0.95	1.00	0.97	0.95	1.00	0.91	1.00	0.91	1.00	0.91	1.00
Lane Util. Factor	1.00	0.98	0.99	0.99	0.99	0.99	0.87	0.87	0.87	0.87	0.87	0.97
Ped Bike Factor	0.850	0.850	0.850	0.850	0.850	0.850	0.850	0.850	0.850	0.850	0.850	0.850
Flt Protected	0.950	0.950	0.950	0.950	0.950	0.950	0.950	0.950	0.950	0.950	0.950	0.950
Satd. Flow (prot)	3213	3406	1442	3400	3343	1553	1641	5036	1615	1752	5036	1553
Flt Permitted	0.950	0.950	0.950	0.950	0.950	0.950	0.950	0.950	0.950	0.950	0.950	0.950
Satd. Flow (perm)	3207	3406	1411	3378	3343	1531	486	5036	1404	242	5036	1499
Right Turn on Red	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Satd. Flow (RTOR)	161	161	161	86	86	162	162	162	162	162	162	162
Satd. Flow (k/h)	50	50	50	50	50	50	50	50	50	50	50	50
Link Distance (m)	243.7	243.7	243.7	243.7	243.7	243.7	243.7	243.7	243.7	243.7	243.7	243.7
Link Speed (k/h)	17.5	17.5	17.5	17.2	17.2	19.2	19.2	19.2	19.2	19.2	19.2	19.2
Travel Time (s)	1	4	4	4	4	1	10	52	52	52	52	10
Conf. Peds. (#/hr)	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Peak Hour Factor	9%	12%	3%	8%	4%	10%	3%	0%	3%	3%	3%	4%
Heavy Vehicles (%)	604	264	161	73	248	170	150	1023	588	365	971	609
Adj. Flow (vph)	604	264	161	73	248	170	150	1023	588	365	971	609
Shared Lane Traffic (%)	No	No	No	No	No	No	No	No	No	No	No	No
Lane Group Flow (vph)	No	No	No	No	No	No	No	No	No	No	No	No
Enter Blocked Intersecion	Left	Right	Left	Right	Left	Right	Left	Right	Left	Right	Left	Right
Lane Alignment	Left	Right	Left	Right	Left	Right	Left	Right	Left	Right	Left	Right
Median Width(m)	7.2	7.2	7.2	7.2	7.2	3.6	3.6	3.6	3.6	3.6	3.6	3.6
Link Offset(m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Link Offset(m)	4.8	4.8	4.8	4.8	4.8	4.8	4.8	4.8	4.8	4.8	4.8	4.8
Crosswalk Width(m)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Two way Left Turn Lane	25	15	25	15	25	15	25	15	25	15	25	15
Headway Factor	1	2	1	1	2	1	1	2	1	1	2	1
Turning Speed (k/h)	2.0	10.0	2.0	2.0	10.0	2.0	2.0	10.0	2.0	2.0	10.0	2.0
Number of Detectors	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector Template	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Leading Detector (m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Trailing Detector (m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Position(m)	2.0	0.6	2.0	2.0	0.6	2.0	0.6	2.0	0.6	2.0	0.6	2.0
Detector 1 Size(m)	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex
Detector 1 Type	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Channel	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 2 Position(m)	9.4	9.4	9.4	9.4	9.4	9.4	9.4	9.4	9.4	9.4	9.4	9.4
Detector 2 Size(m)	0.6	0.6	0.6	0.6	0.6	0.6	0.6	0.6	0.6	0.6	0.6	0.6
Detector 2 Type	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex
Detector 2 Channel	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 2 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Detector 2 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Turn Type	Perm	NA	NA	perm-pt	NA	NA	Perm	NA	NA	Perm	NA	NA
Protected Phases	2	2	6	1	6	8	8	8	8	4	4	4
Permitted Phases	2	2	6	1	6	8	8	8	8	4	4	4
Detector Phase	2	2	6	1	6	8	8	8	8	4	4	4
Switch Phase												
Minimum Initial (s)	22.0	22.0	8.0	22.0	22.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0
Minimum Split (s)	45.0	45.0	12.5	29.0	29.0	29.0	29.0	29.0	29.0	29.0	29.0	29.0
Total Split (s)	46.0	46.0	15.0	61.0	61.0	29.0	29.0	29.0	29.0	29.0	29.0	29.0
Total Split (%)	51.1%	51.1%	16.7%	67.8%	67.8%	32.2%	32.2%	32.2%	32.2%	32.2%	32.2%	32.2%
Maximum Green (s)	40.0	40.0	11.0	65.0	65.0	23.0	23.0	23.0	23.0	23.0	23.0	23.0
Yellow Time (s)	4.0	4.0	3.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
All-Red Time (s)	2.0	2.0	1.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
Lost Time Adjust (s)	-2.0	-2.0	0.0	-2.0	-2.0	-2.0	-2.0	-2.0	-2.0	-2.0	-2.0	-2.0
Total Lost Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lead/Lag	Lag	Lag	Lead	Lead	Lead							
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes							
Vehicle Extension (s)	5.0	5.0	2.5	5.0	5.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Recall Mode	Min	Min	Min	Min	Min	Min	Min	Min	Min	Min	Min	Min
Walk Time (s)	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0
Flash Dont Walk (s)	16.0	16.0	16.0	16.0	16.0	16.0	16.0	16.0	16.0	16.0	16.0	16.0
Pedestrian Calls (#/hr)	0	0	0	0	0	0	0	0	0	0	0	0
Act Effct Green (s)	37.9	37.9	50.3	50.3	50.3	21.2	21.2	21.2	21.2	21.2	21.2	21.2
Actuated g/C Ratio	0.48	0.48	0.63	0.63	0.63	0.27	0.27	0.27	0.27	0.27	0.27	0.27
v/c Ratio	0.06	0.74	0.29	0.23	0.23	0.20	0.34	0.70	0.43	0.70	0.43	0.43
Control Delay	12.7	20.9	11.8	6.5	28.9	11.9	41.0	9.1	41.0	9.1	9.1	9.1
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	12.7	20.9	11.8	6.5	28.9	11.9	41.0	9.1	41.0	9.1	9.1	9.1
LOS	B	C	B	A	C	B	C	B	D	A	D	A
Approach Delay	20.7			7.0		15.7		25.3				
Approach LOS	C			A		B		C				
Queue Length 50th (m)	2.0	76.4	2.4	15.0	3.0	1.0	3.0	1.0	3.0	31.8	3.5	3.5
Queue Length 95th (m)	4.6	89.5	6.5	22.7	7.3	0.0	54.4	12.2	54.4	12.2	12.2	12.2
Internal Link Dist (m)	350.0			219.7		57.9		156.7				
Turn Bay Length (m)	20.0			20.0		15.0		15.0				
Base Capacity (vph)	428	1703	163	2322	139	273	358	528	358	528	528	528
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.05	0.66	0.25	0.20	0.17	0.30	0.58	0.38	0.58	0.38	0.38	0.38

Intersection Summary	
Area Type:	CBD
Cycle Length: 90	
Actuated Cycle Length: 79.7	
Natural Cycle: 90	
Control Type: Semi Act-Uncoord	
Maximum v/c Ratio: 0.74	
Intersection Signal Delay: 18.2	
Intersection LOS: B	

Intersection Capacity Utilization 78.9%  
 Analysis Period (min) 15  
 m Volume for 95th percentile queue is metered by upstream signal.



Spills and Phases: 5: Trafalgar Road & Cross Avenue/South Service Road

HCM Signalized Intersection Capacity Analysis  
 5: Trafalgar Road & Cross Avenue/South Service Road  
 2030 Background AM Peak Hour.syn  
 05-04-2022

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations	556	243	148	67	228	156	138	941	541	336	893	560	
Traffic Volume (vph)	556	243	148	67	228	156	138	941	541	336	893	560	
Future Volume (vph)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	
Ideal Flow (vphpl)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	
Total Lost time (s)	0.97	0.95	1.00	0.97	1.00	0.95	1.00	0.91	1.00	0.91	1.00	1.00	
Lane Util. Factor	1.00	1.00	0.98	1.00	1.00	0.99	1.00	0.99	1.00	0.91	1.00	0.97	
Frb, ped/bikes	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Frb, ped/bikes	1.00	1.00	0.85	1.00	1.00	0.85	1.00	0.85	1.00	0.85	1.00	0.85	
Frt	0.95	1.00	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	1.00	
Flt Protected	3213	3406	1411	3400	3343	1545	1638	5036	1468	1752	5036	1499	
Satd. Flow (prot)	0.95	1.00	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	1.00	
Flt Permitted	3213	3406	1411	3400	3343	1545	1638	5036	1468	1752	5036	1499	
Satd. Flow (perm)	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	
Peak-hour factor, PHF	604	264	161	73	248	170	150	1023	588	365	971	609	
Adj. Flow (vph)	0	0	126	0	0	60	0	0	89	0	0	336	
RTOR Reduction (vph)	604	264	35	73	248	110	150	1023	499	365	971	273	
Lane Group Flow (vph)	1	4	4	4	4	1	10	10	52	52	52	10	
Conf. Peds. (#/hr)	9%	6%	12%	3%	8%	4%	10%	3%	0%	3%	3%	4%	
Heavy Vehicles (%)	Prot	NA	Perm	Prot	NA	pm+ov	pm+pt	NA	pm+ov	pm+pt	NA	Perm	
Turn Type	7	4	3	3	8	1	5	2	3	3	1	6	
Protected Phases													
Permitted Phases			4		8	2		2	2	6		6	
Actuated Green, G (s)	28.1	27.2	27.2	19.4	15.5	43.1	55.4	43.8	63.2	75.4	59.8	59.8	
Effective Green, g (s)	31.1	30.2	30.2	19.4	18.5	43.1	55.4	46.8	63.2	75.4	62.8	62.8	
Actuated g/C Ratio	0.22	0.22	0.22	0.14	0.13	0.31	0.40	0.33	0.45	0.54	0.45	0.45	
Clearance Time (s)	7.0	7.0	7.0	4.0	7.0	4.0	7.0	4.0	7.0	4.0	7.0	7.0	
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	
Lane Grp Cap (vph)	713	734	304	471	441	475	288	1683	662	427	2559	672	
v/s Ratio Prot	c0.19	0.08		0.02	c0.07	0.05	0.04	0.20	0.10	c0.17	0.19		
v/s Ratio Perm			0.02	0.03	0.16		0.24	0.29		0.29	0.18		
vic Ratio	0.85	0.36	0.11	0.15	0.56	0.23	0.52	0.61	0.75	0.85	0.43	0.41	
Uniform Delay, d1	52.2	46.7	44.1	53.1	57.0	36.1	28.1	38.9	31.9	34.1	26.4	26.0	
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.11	1.08	1.19	1.09	0.76	0.39	
Incremental Delay, d2	9.2	0.3	0.2	0.2	1.6	0.3	0.8	0.8	2.3	4.6	0.2	0.5	
Delay (s)	61.4	47.0	44.3	53.2	58.6	36.4	32.1	42.9	40.2	41.8	20.2	10.7	
Level of Service	E	D	D	D	E	D	C	D	D	D	C	B	
Approach Delay (s)	55.0			50.1			41.1				21.3		
Approach LOS	E			D			D				C		
Intersection Summary													
HCM 2000 Control Delay	37.3											HCM 2000 Level of Service	D
HCM 2000 Volume to Capacity ratio	0.81												
Actuated Cycle Length (s)	140.0											Sum of lost time (s)	16.0
Intersection Capacity Utilization	78.9%											ICU Level of Service	D
Analysis Period (min)	15												
c Critical Lane Group													

Lanes, Volumes, Timings  
 7: GO Bus Terminal/Argus Rd & Cross Ave  
 2025 Total PM Peak Hour.syn  
 05-04-2022

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	15	888	17	38	387	44	16	2	49	179	20	136
Traffic Volume (vph)	15	888	17	38	387	44	16	2	49	179	20	136
Future Volume (vph)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Ideal Flow (vphpl)	3.3	3.6	3.6	3.3	3.6	3.6	3.3	3.6	3.6	3.3	3.6	3.6
Lane Width (m)	20.0	0.0	20.0	0.0	20.0	0.0	0.0	0.0	0.0	15.0	0.0	0.0
Storage Lanes	1	0	1	0	1	0	1	0	1	0	1	0
Taper Length (m)	7.5		7.5		7.5		7.5		7.5		7.5	
Lane Util. Factor	1.00	0.95	0.95	1.00	0.95	0.95	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor	0.99	1.00		1.00	0.99	0.99	1.00	0.95	0.95	0.97	0.98	0.98
Frt	0.997			0.982			0.865		0.865		0.870	0.870
Flt Protected	0.950			0.950			0.950		0.950		0.950	
Satd. Flow (prot)	1570	3172	0	797	3177	0	785	701	0	1570	1282	0
Flt Permitted	0.487			0.125			0.532		0.532		0.704	
Satd. Flow (perm)	798	3172	0	105	3177	0	435	701	0	1123	1282	0
Right Turn on Red		Yes		Yes			Yes		Yes		Yes	
Flt Permitted	3			31			73		73		174	
Satd. Flow (RTOR)				50			50		50		50	
Link Speed (k/h)				374.0			243.7		81.9		180.7	
Link Distance (m)				26.9			17.5		5.9		13.0	
Travel Time (s)	9			4			9		12		20	
Conf. Peds. (#/hr)	0.66	0.81	0.75	0.94	0.95	0.81	0.70	0.25	0.67	0.86	0.75	0.78
Peak Hour Factor	0%	0%	100%	97%	0%	0%	100%	100%	100%	0%	100%	0%
Heavy Vehicles (%)	23	1086	23	40	407	54	23	8	73	208	27	174
Adj. Flow (vph)	23	1119	0	40	461	0	23	81	0	208	201	0
Lane Group Flow (vph)	No	No	No	No	No	No	No	No	No	No	No	No
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Left	Right	Left	Right
Median Width(m)	7.2			7.2			3.3		3.3		3.3	
Link Offset(m)	0.0			0.0			0.0		0.0		0.0	
Crosswalk Width(m)	4.8			4.8			4.8		4.8		4.8	
Two way Left Turn Lane												
Headway Factor	1.19	1.14	1.14	1.19	1.14	1.14	1.19	1.14	1.14	1.19	1.14	1.14
Turning Speed (k/h)	24	14	24	14	24	14	24	14	24	14	24	14
Number of Detectors	1	2		1	2		1	2		1	2	
Detector Template	Left	Thru	Left	Thru	Left	Thru	Left	Thru	Left	Thru	Left	Thru
Leading Detector (m)	2.0	10.0	2.0	10.0	2.0	10.0	2.0	10.0	2.0	10.0	2.0	10.0
Trailing Detector (m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Position(m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Size(m)	2.0	0.6	2.0	0.6	2.0	0.6	2.0	0.6	2.0	0.6	2.0	0.6
Detector 1 Type	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex
Detector 1 Channel	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 2 Position(m)	9.4			9.4			9.4		9.4		9.4	
Detector 2 Size(m)	0.6			0.6			0.6		0.6		0.6	
Detector 2 Type	Ch+Ex			Ch+Ex			Ch+Ex		Ch+Ex		Ch+Ex	
Detector 2 Channel	0.0			0.0			0.0		0.0		0.0	

HCM Signalized Intersection Capacity Analysis  
6: Trafalgar Road

2025 Total PM Peak Hour.syn  
05-04-2022

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	503	538	190	67	750	671	134	482	46	578	621	551
Traffic Volume (vph)	503	538	190	67	750	671	134	482	46	578	621	551
Future Volume (vph)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Ideal Flow (vphpl)	4.0	2.0	5.0	4.0	4.0	7.0	4.0	4.0	4.0	4.0	4.0	4.0
Total Lost time (s)	0.97	0.95	1.00	1.00	0.95	1.00	0.95	1.00	0.97	1.00	1.00	1.00
Lane Util. Factor	1.00	1.00	0.93	1.00	1.00	0.94	1.00	1.00	1.00	1.00	1.00	0.95
Frpb, ped/bikes	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frb, ped/bikes	1.00	1.00	0.85	1.00	1.00	0.85	1.00	0.99	1.00	1.00	1.00	0.85
Flt	0.95	1.00	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	1.00
Flt Protected	3467	3505	1482	1805	3574	1503	1770	3552	3433	1900	1503	1503
Satd. Flow (prot)	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	0.95	1.00	1.00	1.00
Flt Permitted	3467	3505	1482	1805	3574	1503	1770	3552	3433	1900	1503	1503
Satd. Flow (perm)	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Peak-hour factor, PHF	547	585	207	73	815	729	146	524	50	628	675	599
Adj. Flow (vph)	0	0	90	0	0	243	0	5	0	0	0	197
RTOR Reduction (vph)	547	585	117	73	815	486	146	569	0	628	675	402
Lane Group Flow (vph)	21	3%	1%	0%	1%	1%	2%	0%	0%	2%	0%	2%
Conf. Peds. (#/hr)	7	4	3	3	8	8	5	2	1	1	6	6
Heavy Vehicles (%)	Prot	NA	Perm	Prot	NA	Perm	Prot	NA	Prot	NA	Perm	Prot
Turn Type	7	4	3	3	8	8	5	2	1	1	6	6
Protected Phases												
Permitted Phases	21.0	54.6	54.6	8.4	40.0	40.0	11.0	31.0	24.0	44.0	44.0	44.0
Actuated Green, G (s)	22.0	57.6	54.6	9.4	43.0	40.0	12.0	34.0	25.0	47.0	47.0	47.0
Effective Green, g (s)	0.16	0.41	0.39	0.07	0.31	0.29	0.09	0.24	0.18	0.34	0.34	0.34
Actuated g/C Ratio	5.0	5.0	5.0	5.0	7.0	7.0	5.0	7.0	5.0	7.0	7.0	7.0
Clearance Time (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Vehicle Extension (s)	544	1442	577	121	1097	429	151	862	613	637	504	504
Lane Grp Cap (vph)	c0.16	0.17	0.04	0.23	0.08	0.16	0.16	0.16	c0.18	c0.36	0.27	0.80
v/s Ratio Prot												
v/s Ratio Perm	1.01	0.41	0.20	0.60	0.74	1.13	0.97	0.66	1.02	1.06	0.80	0.80
v/c Ratio	59.0	29.1	28.3	63.5	43.5	50.0	63.8	47.8	57.5	46.5	42.2	42.2
Uniform Delay, d1	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.24	1.27	1.68	1.68
Progression Factor	40.0	0.2	0.2	8.2	2.8	85.1	62.6	3.9	17.8	30.8	1.3	1.3
Incremental Delay, d2	99.0	29.3	28.5	71.7	46.3	135.1	126.4	51.7	89.0	89.9	72.2	72.2
Delay (s)	F	C	C	E	D	F	F	D	F	F	E	E
Level of Service	E	F	C	E	D	F	F	D	F	F	E	E
Approach Delay (s)	57.6			87.5			66.9				84.0	
Approach LOS	E			F			E				F	
Intersection Summary												
HCM 2000 Control Delay	76.5			HCM 2000 Level of Service					E			
HCM 2000 Volume to Capacity ratio	1.06											
Actuated Cycle Length (s)	140.0			Sum of lost time (s)					16.0			
Intersection Capacity Utilization	88.5%			ICU Level of Service					E			
Analysis Period (min)	15											
c Critical Lane Group												

Lanes, Volumes, Timings  
6: Trafalgar Road

2030 Background AM Peak Hour.syn  
05-04-2022

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	507	608	96	11	511	622	68	484	57	443	438	379
Traffic Volume (vph)	507	608	96	11	511	622	68	484	57	443	438	379
Future Volume (vph)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Ideal Flow (vphpl)	80.0	0.0	80.0	0.0	80.0	0.0	25.0	0.0	60.0	80.0	0.0	0.0
Storage Length (m)	2	0	1	1	1	1	1	1	1	1	1	1
Storage Lanes	7.5		7.5		7.5		7.5		7.5		7.5	
Taper Length (m)	0.97	0.95	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.97	1.00
Lane Util. Factor	0.97	0.99	0.99	0.99	0.99	0.93	0.99	0.95	0.95	0.97	0.97	0.97
Ped Bike Factor	0.980					0.850		0.850		0.850		0.850
Flt Protected	0.950					0.950		0.950		0.950		0.950
Satd. Flow (prot)	3433	3393	0	1703	3505	1568	1770	3574	1615	3387	1863	1524
Flt Permitted	0.950					0.950		0.950		0.950		0.950
Satd. Flow (perm)	3340	3393	0	1687	3505	1460	1750	3574	1529	3254	1863	1474
Right Turn on Red	Yes					Yes		Yes		Yes		Yes
Satd. Flow (RTOR)	16					285		50		187		358
Link Speed (k/h)	50					50		171.0		266.5		19.2
Link Distance (m)	94.0					316.9		12.3				
Travel Time (s)	6.8					22.8						
Conf. Peds. (#/hr)	25					7		9		18		9
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	2%	4%	1%	6%	3%	3%	2%	1%	0%	4%	2%	6%
Adj. Flow (vph)	551	661	104	12	555	676	74	526	62	482	476	412
Shared Lane Traffic (%)												
Lane Group Flow (vph)	551	765	0	12	555	676	74	526	62	482	476	412
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Right	Left	Right	Left	Right	Left	Right	Left	Right
Median Width (m)	7.2					7.2		7.2		7.2		7.2
Link Offset (m)	0.0					0.0		0.0		0.0		0.0
Crosswalk Width (m)	4.8					4.8		4.8		4.8		4.8
Two way Left Turn Lane												
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (k/h)	25	15	25	25	25	15	25	25	15	25	15	15
Number of Detectors	1	2	1	2	1	1	2	1	1	2	1	2
Detector Template	Left	Thru	Left	Right	Left	Right	Left	Thru	Right	Left	Thru	Right
Leading Detector (m)	2.0	10.0	2.0	10.0	2.0	10.0	2.0	10.0	2.0	10.0	2.0	10.0
Trailing Detector (m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Position (m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Size (m)	2.0	0.6	2.0	0.6	2.0	0.6	2.0	0.6	2.0	0.6	2.0	0.6
Detector 1 Type	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 2 Position (m)	9.4					9.4		9.4		9.4		9.4
Detector 2 Size (m)	0.6					0.6		0.6		0.6		0.6
Detector 2 Type	Ch+Ex					Ch+Ex		Ch+Ex		Ch+Ex		Ch+Ex
Detector 2 Channel												
Detector 2 Extend (s)	0.0					0.0		0.0		0.0		0.0

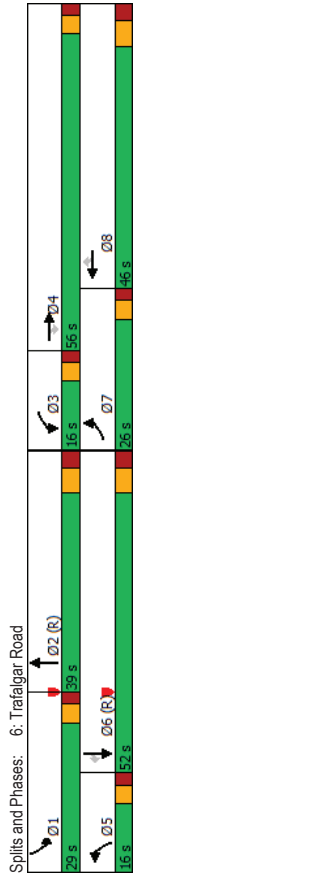


6: Trafalgar Road

EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Prot	NA	NA	Prot	NA	Prot	NA	Prot	NA	Prot	NA	Prot
7	4	4	3	8	8	5	2	2	1	6	6
7	4	4	3	8	8	5	2	2	1	6	6

6: Trafalgar Road

EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Prot	NA	NA	Prot	NA	Prot	NA	Prot	NA	Prot	NA	Prot
7	4	4	3	8	8	5	2	2	1	6	6
7	4	4	3	8	8	5	2	2	1	6	6



6: Trafalgar Road

EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Prot	NA	NA	Prot	NA	Prot	NA	Prot	NA	Prot	NA	Prot
7	4	4	3	8	8	5	2	2	1	6	6
7	4	4	3	8	8	5	2	2	1	6	6

Lane Group	Minimum Initial (s)	Minimum Split (s)	Total Split (%)	Maximum Green (s)	Yellow Time (s)	All-Red Time (s)	Lost Time Adjust (s)	Total Lost Time (s)	Lead/Lag	Lead	Lag	Lead	Lag	Lead	Lag	Lead	Lag
EBL	7.0	10.0	7.0	10.0	10.0	7.0	20.0	20.0	20.0	7.0	20.0	20.0	20.0	20.0	20.0	20.0	20.0
EBT	12.0	37.0	12.0	37.0	47.0	12.0	39.0	39.0	12.0	39.0	39.0	12.0	39.0	12.0	39.0	12.0	39.0
EBR	27.0	64.0	27.0	49.0	49.0	12.0	39.0	39.0	39.0	12.0	39.0	12.0	39.0	12.0	39.0	12.0	39.0
WBL	8.6%	35.0%	8.6%	35.0%	35.0%	8.6%	27.9%	27.9%	17.9%	37.1%	37.1%	17.9%	37.1%	17.9%	37.1%	17.9%	37.1%
WBT	22.0	59.0	22.0	42.0	42.0	7.0	32.0	32.0	20.0	45.0	45.0	20.0	45.0	20.0	45.0	20.0	45.0
WBR	3.0	3.0	3.0	4.0	4.0	3.0	4.0	4.0	4.0	3.0	4.0	3.0	4.0	3.0	4.0	3.0	4.0
NBL	2.0	2.0	2.0	3.0	3.0	2.0	3.0	3.0	3.0	2.0	3.0	2.0	3.0	2.0	3.0	2.0	3.0
NBT	-1.0	-3.0	-1.0	-3.0	-3.0	-1.0	-3.0	-3.0	-1.0	-3.0	-3.0	-1.0	-3.0	-1.0	-3.0	-1.0	-3.0
NBR	4.0	2.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
SBL	Lead	Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	Lag
SBT	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
SBR	Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	None	None	None	None	None	None	C-Max	C-Max	None	C-Max	C-Max	None	C-Max	None	C-Max	C-Max	C-Max
Walk Time (s)	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0
Flash Dont Walk (s)	11.0	11.0	11.0	11.0	11.0	11.0	11.0	11.0	11.0	11.0	11.0	11.0	11.0	11.0	11.0	11.0	11.0
Pedestrian Calls (#/hr)	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Act Effct Green (s)	23.0	69.2	8.0	45.0	45.0	8.0	35.0	32.0	21.0	48.0	48.0	21.0	48.0	21.0	48.0	21.0	48.0
Actuated g/C Ratio	0.16	0.49	0.06	0.32	0.32	0.06	0.25	0.23	0.15	0.34	0.34	0.15	0.34	0.15	0.34	0.15	0.34
v/c Ratio	0.98	0.45	0.12	0.49	1.02	0.73	0.59	0.13	0.95	0.75	0.56	0.95	0.75	0.56	0.95	0.75	0.56
Control Delay	90.8	24.5	65.7	40.1	67.5	102.1	49.4	0.5	111.2	28.2	4.4	111.2	28.2	4.4	111.2	28.2	4.4
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	90.8	24.5	65.7	40.1	67.5	102.1	49.4	0.5	111.2	28.2	4.4	111.2	28.2	4.4	111.2	28.2	4.4
LOS	F	C	E	D	E	F	D	A	F	C	A	F	C	A	F	C	A
Approach Delay	52.3	55.3	55.3	55.3	55.3	55.3	50.7	50.7	50.2	50.2	50.2	50.7	50.7	50.2	50.2	50.7	50.7
Approach LOS	D	E	E	E	E	E	D	D	D	D	D	D	D	D	D	D	D
Queue Length 50th (m)	83.4	67.7	3.4	68.6	-148.4	21.6	71.4	0.0	71.1	39.7	4.2	71.1	39.7	4.2	71.1	39.7	4.2
Queue Length 95th (m)	#121.5	100.5	10.6	87.3	#226.9	#48.6	91.1	0.0	#110.6	65.2	10.7	#110.6	65.2	10.7	#110.6	65.2	10.7
Internal Link Dist (m)	70.0	70.0	70.0	292.9	292.9	292.9	147.0	147.0	242.5	242.5	242.5	242.5	242.5	242.5	242.5	242.5	242.5
Turn Bay Length (m)	80.0	80.0	80.0	25.0	25.0	25.0	60.0	60.0	80.0	80.0	80.0	80.0	80.0	80.0	80.0	80.0	80.0
Base Capacity (vph)	563	1684	97	1126	662	101	893	493	505	638	740	505	638	740	505	638	740
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.98	0.45	0.12	0.49	1.02	0.73	0.59	0.13	0.95	0.75	0.56	0.95	0.75	0.56	0.95	0.75	0.56

Area Type	Other
Cycle Length	140
Actuated Cycle Length	140
Offset: 54.5 (39%), Referenced to phase 2:NBT and 6:SBT, Start of Green	
Natural Cycle: 130	
Control Type: Actuated-Coordinated	
Maximum v/c Ratio: 1.02	
Intersection Signal Delay: 52.2	
Intersection LOS: D	

EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
7	4	4	3	8	8	5	2	1	6	6	6
Prot	NA	Perm	Prot	NA	Perm	Prot	NA	Prot	NA	Prot	NA

EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
7.0	10.0	10.0	7.0	10.0	10.0	7.0	20.0	7.0	20.0	20.0	20.0
Minimum Initial (s)	12.0	37.0	12.0	37.0	37.0	12.0	39.0	12.0	39.0	39.0	39.0
Minimum Split (s)	26.0	56.0	16.0	46.0	46.0	16.0	39.0	29.0	52.0	52.0	52.0
Total Split (s)	18.6%	40.0%	40.0%	11.4%	32.9%	11.4%	27.9%	20.7%	37.1%	37.1%	37.1%
Total Split (%)	21.0	51.0	51.0	11.0	39.0	11.0	32.0	24.0	45.0	45.0	45.0
Maximum Green (s)	3.0	3.0	3.0	4.0	4.0	4.0	3.0	4.0	3.0	4.0	4.0
Yellow Time (s)	2.0	2.0	2.0	3.0	3.0	3.0	2.0	3.0	2.0	3.0	3.0
All-Red Time (s)	-1.0	-3.0	-3.0	-1.0	-3.0	-1.0	-3.0	-1.0	-3.0	-3.0	-3.0
Lost Time Adjust (s)	4.0	2.0	5.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Total Lost Time (s)	Lead	Lag	Lead	Lag	Lead	Lag	Lead	Lag	Lead	Lag	Lag
Lead/Lag	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Lead-Lag Optimize?	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Vehicle Extension (s)	None	None	None	None	None	None	C-Max	None	C-Max	C-Max	C-Max
Recall Mode	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0
Walk Time (s)	11.0	11.0	11.0	11.0	11.0	11.0	11.0	11.0	11.0	11.0	11.0
Flash Dont Walk (s)	0	0	0	0	0	0	0	0	0	0	0
Pedestrian Calls (#/hr)	22.0	57.6	54.6	10.8	42.0	39.0	12.0	35.0	25.0	48.0	48.0
Act Effct Green (s)	0.16	0.41	0.39	0.08	0.30	0.28	0.09	0.25	0.18	0.34	0.34
Actuated g/C Ratio	1.01	0.41	0.31	0.53	0.76	1.10	0.97	0.64	1.02	1.04	0.84
v/c Ratio	98.1	31.1	11.0	75.5	49.9	90.5	127.8	50.2	85.6	77.2	36.5
Control Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Queue Delay	98.1	31.1	11.0	75.5	49.9	90.5	127.8	50.2	85.6	77.2	36.5
Total Delay	F	C	B	E	D	F	F	D	F	E	D
LOS	Approach Delay	55.3	69.4	65.9	67.1	67.1	67.1	67.1	67.1	67.1	67.1
Approach Delay	Approach LOS	E	E	E	E	E	E	E	E	E	E
Queue Length 50th (m)	-83.7	65.7	11.4	20.6	113.6	-164.3	43.2	78.2	-101.2	-204.1	91.0
Queue Length 95th (m)	#123.2	83.0	31.2	38.0	138.8	#244.1	#88.4	99.3	m90.4	m145.5	m83.1
Internal Link Dist (m)	74.1	292.9	147.0	242.5	242.5	242.5	242.5	242.5	242.5	242.5	242.5
Turn Bay Length (m)	80.0	30.0	80.0	25.0	80.0	25.0	80.0	25.0	80.0	25.0	80.0
Base Capacity (vph)	544	1440	667	154	1072	663	151	893	613	651	709
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	1.01	0.41	0.31	0.47	0.76	1.10	0.97	0.64	1.02	1.04	0.84

Intersection Capacity Utilization 82.0%  
Analysis Period (min) 15  
~ Volume exceeds capacity, queue is theoretically infinite.  
Queue shown is maximum after two cycles.  
# 95th percentile volume exceeds capacity, queue may be longer.  
Queue shown is maximum after two cycles.

Spills and Phases: 6: Trafalgar Road

HCM Signalized Intersection Capacity Analysis  
6: Trafalgar Road

2030 Background AM Peak Hour.syn  
05-04-2022

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↔	↕	↔	↔	↕	↔	↔	↕	↔	↔	↕	↔
Traffic Volume (vph)	507	608	96	11	511	622	68	484	57	443	438	379
Future Volume (vph)	507	608	96	11	511	622	68	484	57	443	438	379
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	2.0		4.0	4.0	4.0	4.0	4.0	7.0	4.0	4.0	4.0
Lane Util. Factor	0.97	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.97	1.00	1.00
Frb. ped/bikes	1.00	0.99	1.00	1.00	0.93	1.00	1.00	0.95	1.00	1.00	1.00	0.97
Frb. ped/bikes	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt	1.00	0.98	1.00	1.00	0.85	1.00	1.00	0.85	1.00	1.00	1.00	0.85
Flt Protected	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	1.00	0.85
Satd. Flow (prot)	3433	3392	1703	3505	1460	1770	3574	1529	3367	1863	1474	1474
Flt Permitted	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	1.00	0.85
Satd. Flow (perm)	3433	3392	1703	3505	1460	1770	3574	1529	3367	1863	1474	1474
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	551	661	104	12	555	676	74	526	62	482	476	412
RTOR Reduction (vph)	0	8	0	0	0	187	0	0	49	0	0	243
Lane Group Flow (vph)	551	757	0	12	555	489	74	526	13	482	476	169
Conf. Peds. (#/hr)	25	57	7	6%	3%	3%	2%	1%	0%	4%	2%	6%
Heavy Vehicles (%)	2%	4%	1%	6%	3%	3%	2%	1%	0%	4%	2%	6%
Turn Type	Prot	NA	Prot	NA	Prot	NA	Prot	NA	Perm	Prot	NA	Perm
Protected Phases	7	4		3	8		5	2		1		6
Permitted Phases												
Actuated Green, G (s)	22.0	66.2	2.8	45.0	45.0	7.0	29.0	29.0	20.0	42.0	42.0	42.0
Effective Green, g (s)	23.0	69.2	3.8	48.0	48.0	8.0	32.0	29.0	21.0	45.0	45.0	45.0
Actuated g/C Ratio	0.16	0.49	0.03	0.34	0.34	0.06	0.23	0.21	0.15	0.32	0.32	0.32
Clearance Time (s)	5.0	5.0	5.0	7.0	7.0	5.0	7.0	7.0	5.0	7.0	7.0	7.0
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Lane Grp Cap (vph)	563	1676	46	1201	500	101	816	316	505	598	473	473
v/s Ratio Prot	c0.16	0.22	0.01	0.16	0.16	0.04	0.15	0.01	c0.14	0.26	0.11	0.11
v/s Ratio Perm												
v/c Ratio	0.98	0.45	0.26	0.46	0.98	0.73	0.64	0.64	0.04	0.95	0.80	0.36
Uniform Delay, d1	58.3	23.0	66.7	35.9	45.5	64.9	48.9	44.4	59.0	43.3	36.4	36.4
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.42	0.50	0.25	0.25
Incremental Delay, d2	32.1	0.2	3.0	0.3	34.1	23.7	3.9	0.2	27.8	10.1	2.0	2.0
Delay (s)	90.4	23.2	69.7	36.2	79.5	88.7	52.8	44.6	111.8	31.9	11.3	11.3
Level of Service	F	C	E	D	E	F	D	D	F	C	B	B
Approach Delay (s)	51.4		60.1		56.0		56.0		53.8			
Approach LOS	D		E		E		E		D			
Intersection Summary												
HCM 2000 Control Delay	55.1	HCM 2000 Level of Service										
HCM 2000 Volume to Capacity ratio	0.94	E										
Actuated Cycle Length (s)	140.0	Sum of lost time (s)										
Intersection Capacity Utilization	82.0%	D										
Analysis Period (min)	15											
c Critical Lane Group												

Baseline

Lanes, Volumes, Timings  
6: Trafalgar Road

2025 Total PM Peak Hour.syn  
05-04-2022

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↔	↕	↔	↔	↕	↔	↔	↕	↔	↔	↕	↔
Traffic Volume (vph)	503	538	190	67	750	671	134	482	46	578	621	551
Future Volume (vph)	503	538	190	67	750	671	134	482	46	578	621	551
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (m)	80.0	30.0	80.0	80.0	25.0	0.0	25.0	0.0	80.0	80.0	0.0	0.0
Storage Lanes	2	1	1	1	1	1	1	1	0	1	1	1
Taper Length (m)	7.5			7.5			7.5			7.5		
Lane Util. Factor	0.97	0.95	1.00	1.00	0.95	1.00	0.95	1.00	0.95	0.97	1.00	1.00
Ped Bike Factor	0.98	0.93	0.98	0.98	0.94	0.98	1.00	0.98	1.00	0.98	0.95	0.95
Flt	0.850			0.850			0.987			0.850		0.850
Flt Protected	0.950			0.950			0.950			0.950		0.950
Satd. Flow (prot)	3467	3505	1599	1805	3574	1599	1770	3552	0	3433	1900	1583
Flt Permitted	0.950			0.950			0.950			0.950		0.950
Satd. Flow (perm)	3412	3505	1482	1761	3574	1503	1742	3552	0	3373	1900	1503
Right Turn on Red			Yes			Yes			Yes		Yes	Yes
Satd. Flow (RTOR)			148			340			7			296
Link Speed (k/h)		50			50			50				50
Link Distance (m)	98.1		316.9				171.0		171.0			286.5
Travel Time (s)			22.8				12.3		12.3			19.2
Conf. Peds. (#/hr)	21	14	14	14	21	21	17	10	10	10	17	17
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	1%	3%	1%	0%	1%	1%	2%	0%	0%	2%	0%	2%
Adj. Flow (vph)	547	585	207	73	815	729	146	524	50	628	675	599
Shared Lane Traffic (%)												
Lane Group Flow (vph)	547	585	207	73	815	729	146	574	0	628	675	599
Enter Blocked In/intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Right	Left	Right	Left	Left	Right	Left	Left	Right
Median Width (m)	7.2		7.2		7.2		7.2		7.2			7.2
Link Offset (m)	0.0		0.0		0.0		0.0		0.0			0.0
Crosswalk Width (m)	4.8		4.8		4.8		4.8		4.8			4.8
Two way Left Turn Lane												
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (k/h)	25	15	25	25	15	25	25	15	25	25	15	15
Number of Detectors	1	2	1	1	2	1	1	1	2	1	2	1
Detector Template	Left	Thru	Right	Left	Thru	Right	Left	Thru	Left	Thru	Right	Right
Leading Detector (m)	2.0	10.0	2.0	2.0	10.0	2.0	2.0	10.0	2.0	10.0	2.0	2.0
Trailing Detector (m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Position (m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Size (m)	2.0	0.6	2.0	2.0	0.6	2.0	2.0	0.6	2.0	0.6	2.0	2.0
Detector 1 Type	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 2 Position (m)	9.4		9.4		9.4		9.4		9.4			9.4
Detector 2 Size (m)	0.6		0.6		0.6		0.6		0.6			0.6
Detector 2 Type	Ch+Ex		Ch+Ex		Ch+Ex		Ch+Ex		Ch+Ex			Ch+Ex
Detector 2 Channel												
Detector 2 Extend (s)	0.0		0.0		0.0		0.0		0.0			0.0

Baseline

Table with 17 columns: Movement, EBL, EBT, EBR, WBL, WBT, WBR, NBL, NBT, NBR, SBL, SBT, SBR. Rows include Lane Configurations, Traffic Volume (vph), Future Volume (vph), Ideal Flow (vphlp), Total Lost time (s), Lane Util. Factor, Frpb, ped/bikes, Frt, Satd. Flow (prot), Filt Permitted, Satd. Flow (perm), Peak-hour factor, PHF, Adj. Flow (vph), RTOR Reduction (vph), Lane Group Flow (vph), Conf. Peds. (#/hr), Heavy Vehicles (%), Turn Type, Protected Phases, Permitted Phases, Actuated Green, G (s), Effective Green, g (s), Actuated g/C Ratio, Clearance Time (s), Vehicle Extension (s), Lane Gp. Cap (vph), v/s Ratio Prot, v/s Ratio Perm, v/c Ratio, Uniform Delay, d1, Progression Factor, Incremental Delay, d2, Delay (s), Level of Service, Approach Delay (s), Approach LOS, Intersection Summary, HCM 2000 Control Delay, HCM 2000 Volume to Capacity ratio, Actuated Cycle Length (s), Intersection Capacity Utilization, Analysis Period (min), Critical Lane Group.

Table with 17 columns: Lane Group, EBL, EBT, EBR, WBL, WBT, WBR, NBL, NBT, NBR, SBL, SBT, SBR. Rows include Lane Configurations, Traffic Volume (vph), Future Volume (vph), Ideal Flow (vphlp), Lane Width (m), Storage Length (m), Storage Lanes, Taper Length (m), Lane Util. Factor, Ped Bike Factor, Filt Protected, Satd. Flow (prot), Filt Permitted, Satd. Flow (perm), Right Turn on Red, Satd. Flow (RTOR), Link Speed (k/h), Link Distance (m), Travel Time (s), Conf. Peds. (#/hr), Peak Hour Factor, Heavy Vehicles (%), Adj. Flow (vph), Shared Lane Traffic (%), Lane Group Flow (vph), Enter Blocked Intersection, Lane Alignment, Median Width (m), Link Offset (m), Crosswalk Width (m), Two way Left Turn Lane, Headway Factor, Turning Speed (k/h), Number of Detectors, Detector Template, Leading Detector (m), Trailing Detector (m), Detector 1 Position (m), Detector 1 Size (m), Detector 1 Type, Detector 1 Channel, Detector 1 Extend (s), Detector 1 Queue (s), Detector 1 Delay (s), Detector 2 Position (m), Detector 2 Size (m), Detector 2 Type, Detector 2 Channel.

Lanes, Volumes, Timings  
7: GO Bus Terminal/Argus Rd & Cross Ave

2030 Background AM Peak Hour.syn  
05-04-2022

	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Detector 2 Extend (s)	0.0	2.0	2.0	2.0	1.0	6.0	6.0	3.0	8.0	4.0	4.0	4.0
Turn Type	Perm	NA	Perm	pm-pt	NA	Perm	pm-pt	3	8	Perm	Perm	NA
Protected Phases				1	6	6	8	8	8	4		4
Permitted Phases	2	2	2	6	6	6	8	8	8	4		4
Detector Phase	2	2	2	1	6	6	3	8	8	4		4
Switch Phase												
Minimum initial (s)	22.0	22.0	22.0	8.0	22.0	22.0	5.0	10.0	10.0	10.0	10.0	10.0
Minimum Split (s)	45.0	45.0	45.0	12.5	29.0	29.0	9.5	29.0	29.0	29.0	29.0	29.0
Total Split (s)	42.0	42.0	42.0	11.5	53.5	53.5	8.5	36.5	36.5	28.0	28.0	28.0
Total Split (%)	46.7%	46.7%	46.7%	12.8%	59.4%	59.4%	9.4%	40.6%	40.6%	31.1%	31.1%	31.1%
Maximum Green (s)	36.0	36.0	36.0	7.5	47.5	47.5	4.0	30.5	30.5	22.0	22.0	22.0
Yellow Time (s)	4.0	4.0	4.0	3.0	4.0	4.0	3.5	4.0	4.0	4.0	4.0	4.0
All-Red Time (s)	2.0	2.0	2.0	1.0	2.0	2.0	1.0	2.0	2.0	2.0	2.0	2.0
Lost Time Adjust (s)	-2.0	-2.0	-2.0	0.0	-2.0	-2.0	-2.0	-2.0	-2.0	-2.0	-2.0	-2.0
Total Lost Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	2.5	4.0	6.0	4.0	4.0	4.0
Lead/Lag	Lag	Lag	Lag	Lead	Lead	Lead				Lag	Lag	Lag
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes				Yes	Yes	Yes
Vehicle Extension (s)	5.0	5.0	5.0	2.5	5.0	5.0	3.0	4.0	4.0	4.0	4.0	4.0
Recall Mode	Min	Min	Min	Min	Min	Min	None	Min	Min	Min	Min	Min
Walk Time (s)	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0
Flash Dont Walk (s)	16.0	16.0	16.0	16.0	16.0	16.0	16.0	16.0	16.0	16.0	16.0	16.0
Pedestrian Calls (#/hr)	0	0	0	0	0	0	0	0	0	0	0	0
Act Effct Green (s)	27.8	27.8	27.8	39.4	39.4	39.4	34.1	30.6	24.1	24.1	24.1	24.1
Actuated g/C Ratio	0.35	0.35	0.35	0.49	0.49	0.49	0.43	0.38	0.30	0.30	0.30	0.30
v/c Ratio	0.39	0.56	0.21	0.92	0.54	0.07	1.03	0.47	0.32	1.31	1.31	1.31
Control Delay	26.2	23.1	1.5	73.1	15.4	0.6	124.1	5.0	26.1	173.7	173.7	173.7
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	26.2	23.1	1.5	73.1	15.4	0.6	124.1	5.0	26.1	173.7	173.7	173.7
LOS	C	C	A	E	B	A	F	A	C	A	C	F
Approach Delay	21.4	22.9	22.9	46.0	46.0	46.0						151.9
Approach LOS	C	C	C	D	D	D						F
Queue Length 50th (m)	8.6	42.1	0.0	14.1	47.2	0.0	-10.8	0.0	14.0	-120.8	-120.8	-120.8
Queue Length 95th (m)	10.3	55.0	0.0	#40.6	60.8	0.3	#17.4	0.0	27.6	#110.6	#110.6	#110.6
Internal Link Dist (m)	350.0			219.7	219.7	219.7		57.9	57.9	57.9	57.9	57.9
Turn Bay Length (m)	20.0	60.0	20.0	60.0	60.0	60.0	60.0	60.0	60.0	60.0	60.0	60.0
Base Capacity (vph)	259	1532	410	163	1996	874	105	438	367	524	524	524
Stallion Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Spillover Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.28	0.41	0.18	0.92	0.43	0.05	1.03	0.47	0.32	1.31	1.31	1.31

Intersection Summary	
Area Type:	CBD
Cycle Length: 90	
Actuated Cycle Length: 80	
Natural Cycle: 140	
Control Type: Semi Act-Uncoord	
Maximum v/c Ratio: 1.31	
Intersection Signal Delay: 60.1	
Intersection LOS: E	

Lanes, Volumes, Timings  
5: Trafalgar Road & Cross Avenue/South Service Road

2025 Total PM Peak Hour.syn  
05-04-2022

Intersection Summary	
Area Type:	CBD
Cycle Length: 90	
Actuated Cycle Length: 80	
Natural Cycle: 140	
Control Type: Semi Act-Uncoord	
Maximum v/c Ratio: 1.31	
Intersection Signal Delay: 60.1	
Intersection LOS: E	

Intersection Capacity Utilization 96.5%

Analysis Period (min) 15

~ Volume exceeds capacity, queue is theoretically infinite.

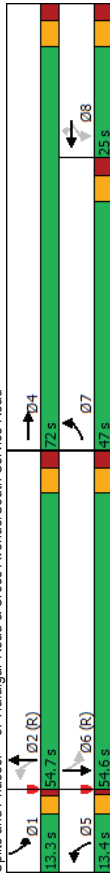
Queue shown is maximum after two cycles.

# 95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

m Volume for 95th percentile queue is metered by upstream signal.

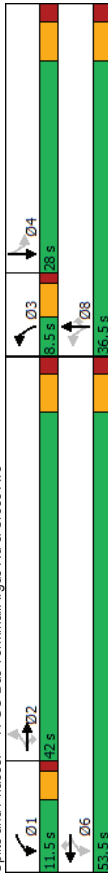
Splits and Phases: 5: Trafalgar Road & Cross Avenue/South Service Road



	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBR
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBR
Turn Type	Prot	NA	4	Perm	NA	8	Perm	pm-pt	NA	pm-pt	NA
Protected Phases	7	4		8	8	2	5	2	1	6	
Permitted Phases				8	8	2	5	2	1	6	
Detector Phase	7	4		8	8	2	5	2	1	6	
Switch Phase											
Minimum Initial (s)	10.0	5.0		10.0	10.0	10.0	7.0	27.0		7.0	27.0
Minimum Split (s)	17.0	25.0		25.0	25.0	25.0	11.5	34.0		11.5	34.0
Total Split (s)	47.0	72.0		25.0	25.0	25.0	13.4	54.7		13.3	54.6
Total Split (%)	33.6%	51.4%		17.9%	17.9%	17.9%	9.6%	39.1%		9.5%	39.0%
Maximum Green (s)	40.0	65.0		18.0	18.0	18.0	9.4	47.7		9.3	47.6
Yellow Time (s)	4.0	4.0		4.0	4.0	4.0	3.0	4.0		3.0	4.0
All-Red Time (s)	3.0	3.0		3.0	3.0	3.0	1.0	3.0		1.0	3.0
Lost Time Adjust (s)	-3.0	-3.0		-3.0	-3.0	-3.0	0.0	-3.0		0.0	-3.0
Total Lost Time (s)	4.0	4.0		4.0	4.0	4.0	4.0	4.0		4.0	4.0
Lead/Lag	Lead	Lag	Lag	Lag	Lag	Lag	Lead	Lag	Lag	Lead	Lag
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	None	None	None	None	None	None	C-Max	C-Max	None	C-Max	C-Max
Walk Time (s)	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0
Flash Dont Walk (s)	11.0	11.0	11.0	11.0	11.0	11.0	11.0	11.0	11.0	11.0	11.0
Pedestrian Calls (#/hr)	0	0	0	0	0	0	0	0	0	0	0
Act Effct Green (s)	43.0	68.0		21.0	21.0	21.0	60.1	50.7		59.9	50.6
Actuated g/C Ratio	0.31	0.49		0.15	0.15	0.15	0.43	0.36		0.43	0.36
v/c Ratio	1.09	0.32		1.06	0.62	0.93	0.97	0.89		0.85	1.10
Control Delay	99.6	16.2		142.9	66.6	71.1	78.5	50.6		55.8	89.0
Queue Delay	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0
Total Delay	99.6	16.2		142.9	66.6	71.1	78.5	50.6		55.8	89.0
LOS	F	B		F	E	E	E	D		E	F
Approach Delay	84.4			88.9			53.1			86.7	
Approach LOS	F			F			D			F	
Queue Length 50th (m)	~190.7	30.0		~55.6	47.1	60.3	38.7	165.9		22.4	~241.5
Queue Length 95th (m)	#234.3	50.2		#105.2	73.1	#119.9	m#145.9	m#57.1		#272.4	
Internal Link Dist (m)	219.7			214.9			242.5			247.0	
Turn Bay Length (m)	25.0			25.0			50.0			25.0	
Base Capacity (vph)	1044	790		164	274	339	170	1839		177	1806
Starvation Cap Reductn	0	0		0	0	0	0	0		0	0
Spillback Cap Reductn	0	0		0	0	0	0	0		0	0
Storage Cap Reductn	0	0		0	0	0	0	0		0	0
Reduced v/c Ratio	1.09	0.32		1.06	0.62	0.93	0.97	0.89		0.85	1.10
Intersection Summary											
Area Type:	Other										
Cycle Length:	140										
Actuated Cycle Length:	140										
Offset:	54.5 (39%), Referenced to phase 2:NBTL and 6:SBTL, Start of Green										
Natural Cycle:	140										
Control Type:	Actuated-Coordinated										
Maximum v/c Ratio:	1.10										
Intersection Signal Delay:	76.3										

Intersection Capacity Utilization	100.4%	ICU Level of Service	G
Analysis Period (min)	15		
~ Volume exceeds capacity, queue is theoretically infinite.			
Queue shown is maximum after two cycles.			
# 95th percentile volume exceeds capacity, queue may be longer.			
Queue shown is maximum after two cycles.			

Splits and Phases: 7: GO Bus Terminal/Argus Rd & Cross Ave



	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
<b>Lane Group</b>	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	→	→	→	←	←	←	←	←	←	←	←	←
Traffic Volume (vph)	1043	93	140	160	156	291	152	1467	44	139	1486	340
Future Volume (vph)	1043	93	140	160	156	291	152	1467	44	139	1486	340
Ideal Flow (vehpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (m)	25.0	0.0	0.0	25.0	0.0	25.0	0.0	50.0	0.0	25.0	0.0	0.0
Storage Lanes	2	0	0	1	1	1	1	1	0	1	0	0
Taper Length (m)	7.5			7.5			7.5			7.5		
Lane Util. Factor	0.97	1.00	1.00	1.00	1.00	1.00	1.00	0.91	0.91	1.00	0.91	0.91
Ped Bike Factor	0.97			0.98			0.99			0.99		0.99
Fit	0.910			0.850			0.996			0.972		
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	3400	1548	0	1770	1827	1599	1719	5074	0	1805	4924	0
Flt Permitted	0.950			0.602			0.084			0.084		
Satd. Flow (perm)	3400	1548	0	1094	1827	1599	152	5074	0	160	4924	0
Right Turn on Red	Yes			Yes			Yes			Yes		Yes
Satd. Flow (RTOR)	75			117			3			42		42
Link Speed (k/h)	50			50			50			50		50
Link Distance (m)	243.7			238.9			266.5			271.0		271.0
Travel Time (s)	17.5			17.2			19.2			19.5		19.5
Confl. Peds. (#/hr)	15			15			18			70		70
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	3%	8%	9%	2%	4%	1%	5%	1%	0%	0%	0%	3%
Adj. Flow (vph)	1134	101	152	174	170	316	165	1595	48	151	1615	370
Shared Lane Traffic (%)												
Lane Group Flow (vph)	1134	263	0	174	170	316	165	1643	0	151	1985	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(m)	7.2			7.2			3.6			3.6		3.6
Link Offset(m)	0.0			0.0			0.0			0.0		0.0
Crosswalk Width(m)	4.8			4.8			4.8			4.8		4.8
Two way Left Turn Lane												
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (k/h)	25			15			25			15		25
Number of Detectors	1	2		1	2		1	2		1	2	
Detector Template	Left	Thru		Left	Thru		Right	Left	Thru	Left	Thru	
Leading Detector (m)	2.0	10.0		2.0	10.0		2.0	10.0		2.0	10.0	
Trailing Detector (m)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Position(m)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Size(m)	2.0	0.6		2.0	0.6		2.0	0.6		2.0	0.6	
Detector 1 Type	Ch+Ex	Ch+Ex		Ch+Ex	Ch+Ex		Ch+Ex	Ch+Ex		Ch+Ex	Ch+Ex	
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Queue (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Delay (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 2 Position(m)	9.4			9.4			9.4			9.4		9.4
Detector 2 Size(m)	0.6			0.6			0.6			0.6		0.6
Detector 2 Type	Ch+Ex	Ch+Ex		Ch+Ex	Ch+Ex		Ch+Ex	Ch+Ex		Ch+Ex	Ch+Ex	
Detector 2 Channel												
Detector 2 Extend (s)	0.0			0.0			0.0			0.0		0.0

	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
<b>Movement</b>	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	→	→	→	←	←	←	←	←	←	←	←	←
Traffic Volume (vph)	38	547	47	126	756	37	57	0	144	92	19	581
Future Volume (vph)	38	547	47	126	756	37	57	0	144	92	19	581
Ideal Flow (vehpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width	3.3	3.6	3.6	3.3	3.6	3.3	3.6	3.3	3.6	3.3	3.6	3.6
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0	2.5		6.0	4.0	4.0	
Lane Util. Factor	1.00	0.95	1.00	1.00	0.95	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frbp. ped/bikes	1.00	1.00	1.00	1.00	1.00	1.00	0.98	1.00	0.97	1.00	0.99	1.00
Fipb. ped/bikes	1.00	1.00	1.00	1.00	1.00	1.00	0.95	1.00	0.85	1.00	0.86	1.00
Flt Protected	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	0.95	1.00	0.95	1.00
Satd. Flow (prot)	1570	3217	708	818	3217	1354	805		736	1538	1386	
Flt Permitted	0.33	1.00	1.00	0.25	1.00	1.00	0.15		1.00	0.76	1.00	
Satd. Flow (perm)	545	3217	708	216	3217	1354	127		736	1226	1386	
Peak-hour factor, PHF	0.52	0.87	0.65	0.84	0.88	0.79	0.53		0.25	0.70	0.78	0.62
Adj. Flow (vph)	73	629	72	150	859	47	108		0	206	118	31
RTOR Reduction (vph)	0	0	47	0	0	24	0		0	127	0	108
Lane Group Flow (vph)	73	629	25	150	859	23	108		0	79	118	576
Confl. Peds. (#/hr)	1			3			3		1	3	20	20
Heavy Vehicles (%)	0%	1%	100%	92%	1%	5%	95%		0%	91%	0%	93%
Turn Type	Perm	NA	Perm	pm-pt	NA	Perm	pm-pt		Perm	Perm	Perm	NA
Protected Phases	2		2	6		6	8		8	8	4	4
Permitted Phases	2		2	6		6	8		8	4	4	4
Actuated Green, G (s)	25.9	25.9	25.9	37.4	37.4	37.4	30.6		30.6	22.1	22.1	22.1
Effective Green, g (s)	27.9	27.9	27.9	37.4	39.4	39.4	32.6		30.6	24.1	24.1	24.1
Actuated g/C Ratio	0.35	0.35	0.35	0.47	0.49	0.49	0.41		0.38	0.30	0.30	0.30
Clearance Time (s)	6.0	6.0	6.0	6.0	6.0	6.0	4.5		6.0	6.0	6.0	6.0
Vehicle Extension (s)	5.0	5.0	5.0	2.5	5.0	5.0	3.0		4.0	4.0	4.0	4.0
Lane Grp Cap (vph)	190	1121	246	157	1584	666	102		281	369	417	417
v/s Ratio Prot	0.20			c0.09	0.27		c0.08				c0.42	
v/s Ratio Perm	0.13			0.04	c0.36		0.02		0.35	0.11	0.10	
v/c Ratio	0.38	0.56	0.10	0.96	0.54	0.03	1.06		0.28	0.32	1.38	
Uniform Delay, d1	19.6	21.1	17.6	18.0	14.1	10.5	21.6		17.1	21.6	27.9	
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00		1.00	1.00	1.00	
Incremental Delay, d2	2.7	1.1	0.4	58.1	0.7	0.0	105.9		0.7	0.7	186.4	
Delay (s)	C	C	B	E	B	B	F		B	B	C	F
Level of Service	22.3	22.2	18.0	76.0	14.7	10.5	127.5		17.8	22.3	214.4	
Approach Delay (s)	C	21.8		23.2			55.6		E		186.1	
Approach LOS	C	C	B	C			E		E		F	
<b>Intersection Summary</b>												
HCM 2000 Control Delay	70.6 HCM 2000 Level of Service											
HCM 2000 Volume to Capacity ratio	1.13											
Actuated Cycle Length (s)	80.0											
Intersection Capacity Utilization	100.4% ICU Level of Service											
Analysis Period (min)	15											
c Critical Lane Group	C											

HCM Unsignalized Intersection Capacity Analysis  
 4: Trafalgar Road & Argus Road/QEW EB On Ramp

2030 Background AM Peak Hour.syn  
 8: Cross Ave & Lyons Lane

2025 Total PM Peak Hour.syn

05-04-2022

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	0	0	56	0	0	0	0	2324	475	0	1832	443
Future Volume (Veh/h)	0	0	56	0	0	0	0	2324	475	0	1832	443
Sign Control	Stop			Stop			Free			Free		
Grade	0%											
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	0	0	61	0	0	0	0	2526	516	0	1991	482
Pedestrians	24											
Lane Width (m)	3.6											
Walking Speed (m/s)	1.2											
Percent Blockage	2											
Right turn flare (veh)	None											
Median type	None											
Median storage (veh)	None											
Upstream signal (m)	271											
pX, platoon unblocked	0.86	0.86	0.73	0.86	0.86	0.73	0.73	0.73	0.73	0.73	0.73	0.73
vC, conflicting volume	3098	4782	929	3190	5023	842	2497	2497	2526	2526	2526	2526
vC1, stage 1 conf/vol												
vC2, stage 2 conf/vol												
vC, unblocked vol	754	2705	0	860	2984	0	1752	1752	1786	1786	1786	1786
tC, single (s)	7.5	6.5	7.1	7.5	6.5	6.9	4.1	4.1	4.1	4.1	4.1	4.1
tC, 2 stage (s)												
tF (s)	3.5	4.0	3.4	3.5	4.0	3.3	2.2	2.2	2.2	2.2	2.2	2.2
p0 queue free %	100	100	92	100	100	100	100	100	100	100	100	100
cM capacity (veh/h)	251	18	761	198	12	793	259	259	256	256	256	256
Direction_Lane #	EB.1	NB.1	NB.2	NB.3	NB.4	SB.1	SB.2	SB.3				
Volume Total	61	842	842	842	516	796	796	880				
Volume Left	0	0	0	0	0	0	0	0				
Volume Right	61	0	0	0	516	0	0	482				
GSH	761	1700	1700	1700	1700	1700	1700	1700				
Volume to Capacity	0.08	0.50	0.50	0.50	0.30	0.47	0.47	0.52				
Queue Length 95th (m)	2.1	0.0	0.0	0.0	0.0	0.0	0.0	0.0				
Control Delay (s)	10.1	0.0	0.0	0.0	0.0	0.0	0.0	0.0				
Lane LOS	B											
Approach Delay (s)	10.1	0.0										
Approach LOS	B											
Intersection Summary												
Average Delay	0.1											
Intersection Capacity Utilization	55.9%											
ICU Level of Service	B											
Analysis Period (min)	15											

Baseline  
 Synchro 10 Report  
 Page 12

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	84	307	109	221	124	22	35	3	59	61	25	62
Future Volume (vph)	84	307	109	221	124	22	35	3	59	61	25	62
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (m)	60.0	60.0	60.0	60.0	60.0	60.0	60.0	60.0	60.0	60.0	60.0	60.0
Storage Lanes	1	1	1	1	1	1	1	1	1	1	1	1
Taper Length (m)	7.5	7.5	7.5	7.5	7.5	7.5	7.5	7.5	7.5	7.5	7.5	7.5
Lane Util. Factor	1.00	0.95	1.00	1.00	0.95	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor	0.99	0.99										
Fit	0.850											
Fit Protected	0.950											
Satd. Flow (prot)	1624	3094	1425	1593	2801	1454	1593	1676	1425	1624	1676	1163
Fit Permitted	0.663											
Satd. Flow (perm)	1124	3094	1425	438	2801	1410	1241	1676	1425	1279	1676	1163
Right Turn on Red	Yes											
Satd. Flow (RTOR)	118											
Link Speed (k/h)	50											
Link Distance (m)	281.3											
Travel Time (s)	20.3											
Conf. Peds. (#/hr)	4											
Peak Hour Factor	0.69	0.68	0.92	0.92	0.88	0.50	0.92	0.92	0.92	0.62	0.92	0.50
Heavy Vehicles (%)	0%											
Adj. Flow (vph)	122	529	118	240	141	44	38	3	64	98	27	124
Shared Lane Traffic (%)												
Lane Group Flow (vph)	122	529	118	240	141	44	38	3	64	98	27	124
Enter Blocked Intersection	No											
Lane Alignment	Left											
Median Width (m)	3.6											
Link Offset (m)	0.0											
Crosswalk Width (m)	4.8											
Two way Left Turn Lane												
Headway Factor	1.14	1.14	1.14	1.14	1.14	1.14	1.14	1.14	1.14	1.14	1.14	1.14
Turning Speed (k/h)	25	15	25	15	25	15	25	15	25	15	25	15
Number of Detectors	1	2	1	1	2	1	1	2	1	1	2	1
Detector Template	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Leading Detector (m)	2.0	10.0	2.0	2.0	10.0	2.0	2.0	10.0	2.0	2.0	10.0	2.0
Trailing Detector (m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Position (m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Size (m)	2.0	0.6	2.0	2.0	0.6	2.0	0.6	2.0	0.6	2.0	0.6	2.0
Detector 1 Type	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex
Detector 1 Channel												
Detector 1 Extend (s)	0.0											
Detector 1 Queue (s)	0.0											
Detector 1 Delay (s)	0.0											
Detector 2 Position (m)	9.4											
Detector 2 Size (m)	0.6											
Detector 2 Type	Ch+Ex											
Detector 2 Channel	Ch+Ex											
Detector 2 Extend (s)	0.0											

Baseline  
 Synchro 10 Report  
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Lanes, Volumes, Timings  
8: Cross Ave & Lyons Lane

Lanes, Volumes, Timings  
4: Trafalgar Road & Argus Road/QEW EB On Ramp

2030 Background AM Peak Hour.syn  
05-04-2022

2025 Total PM Peak Hour.syn  
05-04-2022



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Protected Phases	4	4	4	8	8	8	2	2	2	6	6	6
Permitted Phases	4	4	4	8	8	8	2	2	2	6	6	6
Detector Phase	4	4	4	8	8	8	2	2	2	6	6	6
Switch Phase												
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Minimum Split (s)	22.5	22.5	22.5	22.5	22.5	22.5	22.5	22.5	22.5	22.5	22.5	22.5
Total Split (s)	34.0	34.0	34.0	66.0	66.0	66.0	33.0	33.0	33.0	33.0	33.0	33.0
Total Split (%)	37.8%	37.8%	37.8%	66.7%	66.7%	66.7%	33.3%	33.3%	33.3%	33.3%	33.3%	33.3%
Maximum Green (s)	29.5	29.5	29.5	55.5	55.5	55.5	25.5	25.5	25.5	25.5	25.5	25.5
Yellow Time (s)	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5
Lead/Lag	Lag	Lag	Lag	Lead	Lead	Lead						
Yes	Yes	Yes	Yes									
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	None	None	None	None	None	None	Max	Max	Max	Max	Max	Max
Walk Time (s)	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0
Flash Dont Walk (s)	11.0	11.0	11.0	11.0	11.0	11.0	11.0	11.0	11.0	11.0	11.0	11.0
Pedestrian Calls (#/hr)	0	0	0	0	0	0	0	0	0	0	0	0
Act Effct Green (s)	17.3	17.3	17.3	33.4	33.4	33.4	25.8	25.8	25.8	25.8	25.8	25.8
Actuated g/C Ratio	0.25	0.25	0.25	0.49	0.49	0.49	0.38	0.38	0.38	0.38	0.38	0.38
v/c Ratio	0.43	0.68	0.26	0.59	0.10	0.06	0.08	0.00	0.11	0.20	0.04	0.24
Queue Delay	26.8	27.8	6.1	16.2	9.0	3.0	17.1	17.0	4.9	18.4	16.8	5.4
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	26.8	27.8	6.1	16.2	9.0	3.0	17.1	17.0	4.9	18.4	16.8	5.4
LOS	C	C	A	B	A	A	B	B	A	B	B	A
Approach Delay		24.3			12.4		9.7				11.7	
Approach LOS		C			B		A				B	
Queue Length 50th (m)	13.6	33.3	0.0	18.0	4.9	0.0	3.2	0.3	0.0	8.6	2.2	0.0
Queue Length 95th (m)	21.4	30.4	11.2	30.6	8.7	1.0	10.9	2.2	7.3	15.3	8.6	0.0
Internal Link Dist (m)		257.3			16.1		125.2				17.7	
Turn Bay Length (m)	60.0	60.0	60.0	60.0	60.0	60.0	60.0	60.0	60.0	60.0	60.0	60.0
Base Capacity (vph)	490	1351	689	581	2301	1166	468	632	583	482	632	516
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Spillover Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.25	0.39	0.17	0.41	0.06	0.04	0.08	0.00	0.11	0.20	0.04	0.24

Intersection Summary
Area Type: CBD
Cycle Length: 90
Actuated Cycle Length: 68.3
Natural Cycle: 60
Control Type: Semi Act-Uncoordinated
Maximum v/c Ratio: 0.68
Intersection Signal Delay: 18.1
Intersection Capacity Utilization 44.7%

Intersection Summary
Area Type: Other
Control Type: Unsignalized
Intersection Capacity Utilization 55.9%
Analysis Period (min) 15

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations			↑					↑↑↑		↑	↑↑	↑↑
Traffic Volume (vph)	0	0	56	0	0	0	0	2324	475	0	1832	443
Future Volume (vph)	0	0	56	0	0	0	0	2324	475	0	1832	443
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	70.0	0.0	0.0	0.0
Storage Lanes	0	0	1	0	0	0	0	0	1	0	0	0
Taper Length (m)	7.5	7.5	7.5	7.5	7.5	7.5	7.5	7.5	7.5	7.5	7.5	7.5
Lane Util Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.91	1.00	1.00	0.91	0.91
Ped Bike Factor			0.865						0.850			
Fit												
Fit Protected												
Satd. Flow (prot)	0	0	1522	0	0	0	0	5085	1568	0	4957	0
Fit Permitted												
Satd. Flow (perm)	0	0	1522	0	0	0	0	5085	1568	0	4957	0
Link Speed (k/h)	50	50	50	50	50	50	50	50	50	50	50	50
Link Distance (m)	165.2	165.2	165.2	196.1	196.1	196.1	271.0	271.0	19.5	19.5	50.7	50.7
Travel Time (s)	11.9	11.9	11.9	14.1	14.1	14.1	19.5	19.5	3.7	3.7	3.7	3.7
Confl. Pedis. (#/hr)												24
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	0%	0%	8%	0%	0%	0%	0%	2%	3%	0%	2%	0%
Adj. Flow (vph)	0	0	61	0	0	0	0	2526	516	0	1991	482
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	0	61	0	0	0	0	2526	516	0	2473	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width (m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	3.6	0.0	0.0	3.6	0.0
Link Offset (m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Crosswalk Width (m)	4.8	4.8	4.8	4.8	4.8	4.8	4.8	4.8	4.8	4.8	4.8	4.8
Two way Left Turn Lane												
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (k/h)	25	25	15	25	25	15	25	25	15	25	25	15
Sign Control		Stop		Stop	Stop	Stop	Free	Free	Free	Free	Free	Free

Intersection Summary
Area Type: Other
Control Type: Unsignalized
Intersection Capacity Utilization 55.9%
Analysis Period (min) 15

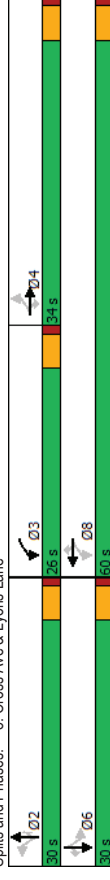


Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	EBL	EBR	NBL	NBT	SBT	SBR
Traffic Volume (vph)	1044	504	0	2324	1771	0
Future Volume (vph)	1044	504	0	2324	1771	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0
Lane Util. Factor	0.97	1.00	0.91	0.91	0.91	0.91
Frbp, ped/bikes	1.00	0.98	1.00	1.00	1.00	1.00
Frbp, ped/bikes	1.00	1.00	1.00	1.00	1.00	1.00
Frt	1.00	0.85	1.00	1.00	1.00	1.00
Flt Protected	0.95	1.00	1.00	1.00	1.00	1.00
Satd. Flow (prot)	3502	1573	5085	5085	5085	5085
Flt Permitted	0.95	1.00	1.00	1.00	1.00	1.00
Satd. Flow (perm)	3502	1573	5085	5085	5085	5085
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	1135	548	0	2526	1925	0
RTOR Reduction (vph)	0	5	0	0	0	0
Lane Group Flow (vph)	1135	543	0	2526	1925	0
Conf. Peds. (#/hr)	2					
Heavy Vehicles (%)	0%	1%	0%	2%	2%	0%
Turn Type	Perm	Perm	NA	NA	NA	NA
Protected Phases				2	6	
Permitted Phases	4	4				
Actuated Green, G (s)	50.9	50.9	75.1	75.1	75.1	
Effective Green, g (s)	53.9	53.9	78.1	78.1	78.1	
Actuated g/C Ratio	0.38	0.38	0.56	0.56	0.56	
Clearance Time (s)	7.0	7.0	7.0	7.0	7.0	
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	
Lane Grp. Cap. (vph)	1348	605	2836	2836	2836	
v/s Ratio Prot			c0.50	0.38		
v/s Ratio Perm	0.32	c0.35				
v/c Ratio	0.84	0.90		0.89	0.68	
Uniform Delay, d1	39.2	40.5		27.2	22.0	
Progression Factor	1.00	1.00		1.64	0.82	
Incremental Delay, d2	4.9	16.0		1.4	0.3	
Delay (s)	44.1	56.4		46.0	18.5	
Level of Service	D	E		D	B	
Approach Delay (s)	48.1			46.0	18.5	
Approach LOS	D			D	B	
<b>Intersection Summary</b>						
HCM 2000 Control Delay			37.9		HCM 2000 Level of Service	D
HCM 2000 Volume to Capacity ratio			0.91			
Actuated Cycle Length (s)			140.0		Sum of lost time (s)	11.0
Intersection Capacity Utilization			106.6%		ICU Level of Service	G
Analysis Period (min)			15			

c Critical Lane Group

Analysis Period (min) 15

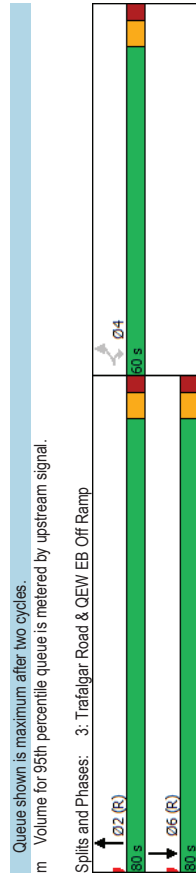
Splits and Phases: 8: Cross Ave & Lyons Lane



HCM Signalized Intersection Capacity Analysis  
 8: Cross Ave & Lyons Lane  
 2030 Background AM Peak Hour.syn  
 05-04-2022

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	←	←	←	←	←	←	←	←	←	←	←	←
Traffic Volume (vph)	84	307	109	221	124	22	35	3	59	61	25	62
Future Volume (vph)	84	307	109	221	124	22	35	3	59	61	25	62
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	0.95	1.00	1.00	1.00	1.00	1.00	1.00
Frb, ped/bikes	1.00	1.00	1.00	1.00	1.00	0.97	1.00	1.00	1.00	1.00	1.00	1.00
Frt	1.00	1.00	0.85	1.00	1.00	1.00	1.00	1.00	0.85	1.00	1.00	1.00
Flt Protected	0.95	1.00	1.00	0.95	1.00	1.00	1.00	1.00	0.95	1.00	1.00	1.00
Satd. Flow (prot)	1614	3094	1425	1593	2801	1414	1593	1676	1425	1612	1676	1163
Flt Permitted	0.66	1.00	1.00	0.26	1.00	1.00	0.74	1.00	1.00	0.76	1.00	1.00
Satd. Flow (perm)	1126	3094	1425	437	2801	1414	1240	1676	1425	1282	1676	1163
Peak-hour factor, PHF	0.69	0.68	0.92	0.92	0.88	0.50	0.92	0.92	0.92	0.62	0.92	0.50
Adj. Flow (vph)	122	529	118	240	141	44	38	3	64	98	27	124
RTOR Reduction (vph)	0	0	88	0	0	22	0	0	40	0	0	77
Lane Group Flow (vph)	122	529	30	240	141	22	38	3	24	98	27	47
Conf. Peds. (#/hr)	4					4					7	
Heavy Vehicles (%)	0%	5%	2%	2%	16%	0%	2%	2%	2%	0%	2%	25%
Turn Type	Perm	NA	Perm	pm+pt	NA	Perm	Perm	NA	Perm	Perm	NA	Perm
Protected Phases	4		4	3	8		2		2		6	
Permitted Phases	4		4	8		8	2		2		6	6
Actuated Green, G (s)	17.3	17.3	17.3	33.4	33.4	33.4	25.8	25.8	25.8	25.8	25.8	25.8
Effective Green, g (s)	17.3	17.3	17.3	33.4	33.4	33.4	25.8	25.8	25.8	25.8	25.8	25.8
Actuated g/C Ratio	0.25	0.25	0.25	0.49	0.49	0.49	0.38	0.38	0.38	0.38	0.38	0.38
Clearance Time (s)	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Lane Grp. Cap (vph)	285	784	361	410	1371	692	469	634	539	484	634	439
v/s Ratio Prot	c0.17			c0.10	0.05		0.00		0.02		0.02	
v/s Ratio Perm	0.11		0.02	0.19	0.02	0.03	0.03	0.02	0.02	c0.08	0.04	0.04
v/c Ratio	0.43	0.67	0.08	0.59	0.10	0.03	0.08	0.00	0.04	0.20	0.04	0.11
Uniform Delay, d1	21.3	22.9	19.4	11.4	9.3	9.0	13.6	13.2	13.4	14.3	13.4	13.7
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	1.0	2.3	0.1	2.1	0.0	0.0	0.3	0.0	0.2	0.9	0.1	0.5
Delay (s)	22.3	25.2	19.5	13.5	9.4	9.0	13.9	13.2	13.6	15.2	13.5	14.2
Level of Service	C	C	B	B	A	A	B	B	B	B	B	B
Approach Delay (s)	23.9			11.7			13.7				14.5	
Approach LOS	C			B			B				B	
Intersection Summary												
HCM 2000 Control Delay	18.3 HCM 2000 Level of Service B											
HCM 2000 Volume to Capacity ratio	0.44											
Actuated Cycle Length (s)	68.2 Sum of lost time (s) 13.5											
Intersection Capacity Utilization	44.7% ICU Level of Service A											
Analysis Period (min)	15											
c Critical Lane Group												

Lanes, Volumes, Timings  
 3: Trafalgar Road & QEW EB Off Ramp  
 2025 Total PM Peak Hour.syn  
 05-04-2022



Lanes, Volumes, Timings  
3: Trafalgar Road & QEW EB Off Ramp

Lanes, Volumes, Timings  
10: Lyons Lane & South Service Road

2025 Total PM Peak Hour.syn  
05-04-2022

2030 Background AM Peak Hour.syn  
05-04-2022



Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Detector Phase	4	4		2	6	6
Switch Phase						
Minimum Initial (s)	10.0	10.0		29.0	29.0	29.0
Minimum Split (s)	38.0	38.0		36.0	36.0	36.0
Minimum Split (%)	60.0	60.0		80.0	80.0	80.0
Total Split (s)	42.9%	42.9%		57.1%	57.1%	57.1%
Total Split (%)	53.0	53.0		73.0	73.0	73.0
Maximum Green (s)	4.0	4.0		4.0	4.0	4.0
Yellow Time (s)	3.0	3.0		3.0	3.0	3.0
All-Red Time (s)	-3.0	-3.0		-3.0	-3.0	-3.0
Lost Time Adjust (s)	4.0	4.0		4.0	4.0	4.0
Total Lost Time (s)						
Lead/Lag						
Lead-Lag Optimize?						
Vehicle Extension (s)	3.0	3.0		3.0	3.0	3.0
Recall Mode	None	None		C-Max	C-Max	C-Max
Walk Time (s)	7.0	7.0		7.0	7.0	7.0
Flash Dont Walk (s)	11.0	11.0		11.0	11.0	11.0
Pedestrian Calls (#/hr)	0	0		0	0	0
Act Effct Green (s)	53.9	53.9		78.1	78.1	78.1
Actuated g/C Ratio	0.38	0.38		0.56	0.56	0.56
v/c Ratio	0.84	0.90		0.89	0.68	0.68
Control Delay	45.8	58.6		47.0	18.9	18.9
Queue Delay	0.0	0.0		0.0	0.0	0.0
Total Delay	45.8	58.6		47.0	18.9	18.9
LOS	D	E		D	B	B
Approach Delay	50.0			47.0	18.9	
Approach LOS	D			D	B	
Queue Length 50th (m)	149.9	143.5		286.8	97.2	
Queue Length 95th (m)	178.9	#211.7		m287.4	m93.1	
Internal Link Dist (m)	47.3			26.7	162.0	
Turn Bay Length (m)						
Base Capacity (vph)	1400	634		2838	2838	
Saturation Cap Reductn	0	0		0	0	
Spillback Cap Reductn	0	0		0	0	
Storage Cap Reductn	0	0		0	0	
Reduced v/c Ratio	0.81	0.86		0.89	0.68	
<b>Intersection Summary</b>						
Area Type:	Other					
Cycle Length:	140					
Actuated Cycle Length:	140					
Offset: 0 (0%), Referenced to phase 2:NBT and 6:SBT, Start of Green						
Natural Cycle:	90					
Control Type:	Actuated-Coordinated					
Maximum w/c Ratio:	0.90					
Intersection Signal Delay:	39.0					
Intersection Capacity Utilization	106.6%					
Analysis Period (min)	15					
# 95th percentile volume exceeds capacity, queue may be longer.						

Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		4	4	8	8	2
Traffic Volume (vph)	0	10	8	8	8	2
Future Volume (vph)	0	10	8	8	8	2
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor						
Ft		0.932			0.963	
Flt Protected						
Satd. Flow (prot)	0	1710	1594	0	1283	0
Flt Permitted						
Satd. Flow (perm)	0	1710	1594	0	1283	0
Link Speed (k/h)		50	50		50	
Link Distance (m)		60.5	89.6		37.6	
Travel Time (s)		4.4	6.5		2.7	
Conf. Peds. (#/hr)	6			6	1	
Peak Hour Factor	0.25	0.50	0.75	0.75	0.38	0.25
Heavy Vehicles (%)	0%	0%	0%	0%	33%	0%
Adj. Flow (vph)	0	20	11	11	21	8
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	20	22	0	29	0
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Left	Left	Right	Left	Right
Median Width(m)		0.0	0.0		3.6	
Link Offset(m)		0.0	0.0		0.0	
Crosswalk Width(m)		4.8	4.8		4.8	
Two way Left Turn Lane						
Headway Factor	1.14	1.14	1.14	1.14	1.14	1.14
Turning Speed (k/h)	24			14	24	14
Sign Control		Free	Free		Stop	
<b>Intersection Summary</b>						
Area Type:	CBD					
Control Type:	Unsignalized					
Intersection Capacity Utilization	15.1%					
Analysis Period (min)	15					
ICU Level of Service A						

HCM Unsignalized Intersection Capacity Analysis 2030 Background AM Peak Hour.syn  
 10: Lyons Lane & South Service Road 05-04-2022

Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	0	10	8	8	8	2
Traffic Volume (veh/h)	0	10	8	8	8	2
Future Volume (Veh/h)	0	10	8	8	8	2
Sign Control	Free	Free	Free	Free	Stop	0%
Grade	0%	0%	0%	0%	0%	0%
Peak Hour Factor	0.25	0.50	0.75	0.75	0.38	0.25
Hourly flow rate (vph)	0	20	11	11	21	8
Pedestrians		1			6	
Lane Width (m)		3.6			3.6	
Walking Speed (m/s)		1.2			1.2	
Percent Blockage		0			1	
Right turn flare (veh)		None			None	
Median type		None			None	
Upstream signal (m)		393			44	22
pX, platoon unblocked		28			44	22
vC, conflicting volume		28			44	22
vC1, stage 1 conf vol		4.1			6.7	6.2
vC2, stage 2 conf vol		2.2			3.8	3.3
tC, single (s)		100			98	99
tC, 2 stage (s)		1591			889	1055
tF (s)						
p0 queue free %						
cM capacity (veh/h)						
Direction_Lane #	EB 1	WB 1	SB 1			
Volume Total	20	22	29			
Volume Left	0	0	21			
Volume Right	0	11	8			
GSH	1591	1700	929			
Volume to Capacity	0.00	0.01	0.03			
Queue Length 95th (m)	0.0	0.0	0.8			
Control Delay (s)	0.0	0.0	9.0			
Lane LOS	A	A	A			
Approach Delay (s)	0.0	0.0	9.0			
Approach LOS	A	A	A			
Intersection Summary						
Average Delay			3.7			
Intersection Capacity Utilization			15.1%		ICU Level of Service	A
Analysis Period (min)			15			

Lanes, Volumes, Timings 2025 Total PM Peak Hour.syn  
 3: Trafalgar Road & QEW EB Off Ramp 05-04-2022

	EBL	EBR	NBL	NBT	SBT	SBR
Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	↑	↑	↑	↑	↑	↑
Traffic Volume (vph)	1044	504	0	2324	1771	0
Future Volume (vph)	1044	504	0	2324	1771	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	0.97	1.00	1.00	0.91	0.91	1.00
Ped Bike Factor		0.98				
Ft		0.850				
Flt Protected		0.950				
Satd. Flow (prot)		3502	1599	0	5085	5085
Flt Permitted		0.950				
Satd. Flow (perm)		3502	1573	0	5085	5085
Right Turn on Red		Yes				Yes
Satd. Flow (RTOR)		8				
Link Speed (k/h)		50		50		
Link Distance (m)		71.3		50.7		186.0
Travel Time (s)		5.1		3.7		13.4
Conf. Peds. (#/h)		2				
Peak Hour Factor		0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)		0%	1%	0%	2%	0%
Adj. Flow (vph)		1135	548	0	2526	1925
Shared Lane Traffic (%)						
Lane Group Flow (vph)		1135	548	0	2526	1925
Enter Blocked Intersection		No	No	No	No	No
Lane Alignment		Left	Left	Left	Left	Right
Median Width (m)		7.2		0.0	0.0	0.0
Link Offset (m)		0.0		0.0	0.0	0.0
Crosswalk Width (m)		4.8		4.8		4.8
Two way Left Turn Lane						
Headway Factor		1.00	1.00	1.00	1.00	1.00
Turning Speed (k/h)		25	15	25		15
Number of Detectors		1		2		2
Detector Template		Left	Right	Thru		Thru
Leading Detector (m)		2.0	2.0	10.0		10.0
Trailing Detector (m)		0.0	0.0	0.0		0.0
Detector 1 Position (m)		0.0	0.0	0.0		0.0
Detector 1 Size (m)		2.0	2.0	0.6		0.6
Detector 1 Type		Ch+Ex	Ch+Ex	Ch+Ex		Ch+Ex
Detector 1 Channel						
Detector 1 Extend (s)		0.0	0.0	0.0		0.0
Detector 1 Queue (s)		0.0	0.0	0.0		0.0
Detector 1 Delay (s)		0.0	0.0	0.0		0.0
Detector 2 Position (m)						9.4
Detector 2 Size (m)						0.6
Detector 2 Type						Ch+Ex
Detector 2 Channel						
Detector 2 Extend (s)						0.0
Turn Type		Perm	Perm	NA		NA
Protected Phases						2
Permitted Phases		4	4			6

HCM Unsignalized Intersection Capacity Analysis  
 2: Trafalgar Road & QEW SB On Ramps

Lanes, Volumes, Timings  
 11: Argus Rd & South Service Rd

2025 Total PM Peak Hour.syn  
 05-04-2022

2030 Background AM Peak Hour.syn  
 05-04-2022

Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations				↑↑↑	↑↑↑	↑
Traffic Volume (veh/h)	0	0	0	3368	1771	1667
Future Volume (Veh/h)	0	0	0	3368	1771	1667
Sign Control	Stop	Free	Free	Free	Free	Free
Grade	0%	0%	0%	0%	0%	0%
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	0	0	0	3661	1925	1812
Pedestrians						
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)						
Median type			None	None	None	
Median storage (veh)						
Upstream signal (m)			186	145		
pX, platoon unblocked	0.29					
vC, conflicting volume	3145	642	1925			
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	0	642	1925			
tC, single (s)	6.8	6.9	4.1			
tC, 2 stage (s)						
tF (s)	3.5	3.3	2.2			
p0 queue free %	100	100	100			
cM capacity (veh/h)	301	417	303			
Direction, Lane #	NB.1	NB.2	NB.3	SB.1	SB.2	SB.3
Volume Total	1220	1220	1220	642	642	1812
Volume Left	0	0	0	0	0	0
Volume Right	0	0	0	0	0	1812
GSH	1700	1700	1700	1700	1700	1700
Volume to Capacity	0.72	0.72	0.72	0.38	0.38	1.07
Queue Length 95th (m)	0.0	0.0	0.0	0.0	0.0	0.0
Control Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0
Lane LOS						
Approach Delay (s)	0.0			0.0		
Approach LOS						
Intersection Summary						
Average Delay	0.0					
Intersection Capacity Utilization	106.6% ICU Level of Service					
Analysis Period (min)	15					

Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	4	1	1	1	5	4
Traffic Volume (vph)	1	6	632	132	5	4
Future Volume (vph)	1	6	632	132	5	4
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor			0.973		0.940	
Flt Protected			0.989		0.973	
Satd. Flow (prot)	0	1384	1625	0	1564	0
Flt Permitted			0.989		0.973	
Satd. Flow (perm)	0	1384	1625	0	1564	0
Link Speed (k/h)		50	50		50	
Link Distance (m)		177.7	165.2		103.5	
Travel Time (s)		12.8	11.9		7.5	
Conf. Peds. (#/hr)	1			1	5	1
Peak Hour Factor	0.25	0.42	0.86	0.72	0.25	0.25
Heavy Vehicles (%)	100%	0%	3%	0%	0%	0%
Adj. Flow (vph)	4	14	735	183	20	16
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	18	918	0	36	0
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Left	Left	Right	Left	Right
Median Width(m)		0.0	0.0		3.6	
Link Offset(m)		0.0	0.0		0.0	
Crosswalk Width(m)		4.8	4.8		4.8	
Two way Left Turn Lane						
Headway Factor	1.14	1.14	1.14	1.14	1.14	1.14
Turning Speed (k/h)	24			14	24	14
Sign Control		Free	Free	Free	Stop	Stop
Intersection Summary						
Area Type:	CBD					
Control Type:	Unsignalized					
Intersection Capacity Utilization	56.2% ICU Level of Service B					
Analysis Period (min)	15					

HCM Unsignalized Intersection Capacity Analysis 2030 Background AM Peak Hour.syn  
 11: Argus Rd & South Service Rd 05-04-2022

Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	4	4	1	1	5	4
Traffic Volume (veh/h)	1	6	632	132	5	4
Future Volume (Veh/h)	1	6	632	132	5	4
Sign Control	Free	Free	Free	Stop	Stop	0%
Grade	0%	0%	0%	0%	0%	0%
Peak Hour Factor	0.25	0.42	0.86	0.72	0.25	0.25
Hourly flow rate (vph)	4	14	735	183	20	16
Pedestrians	1	5			1	
Lane Width (m)	3.6	3.6			3.6	
Walking Speed (m/s)	1.2	1.2			1.2	
Percent Blockage	0	0			0	
Right turn flare (veh)						
Median type	None	None				
Median storage (veh)						
Upstream signal (m)	368					
pX, platoon unblocked					854	828
vC, conflicting volume	919					
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	919				854	828
tC, single (s)	5.1				6.4	6.2
tC, 2 stage (s)						
tF (s)	3.1				3.5	3.3
p0 queue free %	99				94	96
cM capacity (veh/h)	457				327	373
Direction, Lane #	EB 1	WB 1	SB 1			
Volume Total	18	918	36			
Volume Left	4	0	20			
Volume Right	0	183	16			
GSH	457	1700	346			
Volume to Capacity	0.01	0.54	0.10			
Queue Length 95th (m)	0.2	0.0	2.8			
Control Delay (s)	3.0	0.0	16.6			
Lane LOS	A	C	C			
Approach Delay (s)	3.0	0.0	16.6			
Approach LOS	C	C	C			
Intersection Summary						
Average Delay	0.7					
Intersection Capacity Utilization	56.2% ICU Level of Service B					
Analysis Period (min)	15					

Lanes, Volumes, Timings 2025 Total PM Peak Hour.syn  
 2: Trafalgar Road & QEW SB On Ramps 05-04-2022

Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Volume (vph)	0	0	0	3368	1771	1667
Future Volume (vph)	0	0	0	3368	1771	1667
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	0.91	0.91	1.00
FrT						0.850
FrT Protected						
Satd. Flow (prot)	0	0	0	5085	5085	1583
FrT Permitted						
Satd. Flow (perm)	0	0	0	5085	5085	1583
Link Speed (k/h)	50			50	50	
Link Distance (m)	159.6			186.0	145.2	
Travel Time (s)	11.5			13.4	10.5	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	0	0	0	3661	1925	1812
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	0	0	3661	1925	1812
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Left	Left	Right
Median Width (m)	0.0			0.0	0.0	
Link Offset (m)	0.0			0.0	0.0	
Crosswalk Width (m)	4.8			4.8	4.8	
Two way Left Turn Lane						
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (k/h)	25	15	25			15
Sign Control	Stop	Stop	Free	Free	Free	Free
Intersection Summary						
Area Type:	Other					
Control Type:	Unsignalized					
Intersection Capacity Utilization	106.6% ICU Level of Service G					
Analysis Period (min)	15					





1: Trafalgar Road & North Service Road/QEW WB Off Ramp

Analysis Period (min)	ICU Level of Service F
15	

Intersection Capacity Utilization 95.4%

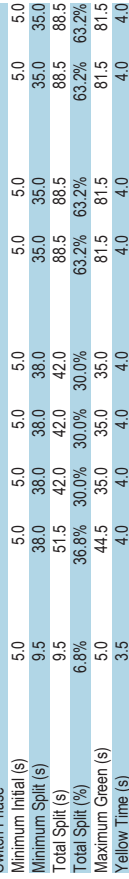
~ Volume exceeds capacity, queue is theoretically infinite.  
 Queue shown is maximum after two cycles.

# 95th percentile volume exceeds capacity, queue may be longer.  
 Queue shown is maximum after two cycles.

m Volume for 95th percentile queue is metered by upstream signal.

1: Trafalgar Road & North Service Road/QEW WB Off Ramp

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Turn Type	Prot	Perm	Perm	Perm	NA	Perm	NA	Perm	NA	Perm	NA	Perm
Protected Phases	7											
Permitted Phases	7	4	8	8	8	8	2	2	2	2	6	6
Detector Phase	7	4	8	8	8	8	2	2	2	2	6	6
Switch Phase												
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Minimum Split (s)	9.5	38.0	38.0	38.0	38.0	38.0	35.0	35.0	35.0	35.0	35.0	35.0
Total Split (s)	9.5	51.5	42.0	42.0	42.0	42.0	88.5	88.5	88.5	88.5	88.5	88.5
Total Split (%)	6.8%	36.8%	30.0%	30.0%	30.0%	30.0%	63.2%	63.2%	63.2%	63.2%	63.2%	63.2%
Maximum Green (s)	5.0	44.5	35.0	35.0	35.0	35.0	81.5	81.5	81.5	81.5	81.5	81.5
Yellow Time (s)	3.5	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
All-Red Time (s)	1.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Lost Time Adjust (s)	-1.0	-3.0	-3.0	-3.0	-3.0	-3.0	-3.0	-3.0	-3.0	-3.0	-3.0	-3.0
Total Lost Time (s)	3.5	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lead/Lag	Lead	Lag	Lag	Lag	Lag	Lag	Lead	Lag	Lag	Lag	Lead	Lag
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	None	None	None	None	None	None	C-Max	C-Max	C-Max	C-Max	C-Max	C-Max
Walk Time (s)		7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0
Flash Dont Walk (s)		11.0	11.0	11.0	11.0	11.0	11.0	11.0	11.0	11.0	11.0	11.0
Pedestrian Calls (#/hr)		0	0	0	0	0	0	0	0	0	0	0
Act Effct Green (s)	6.0	42.4	36.7	36.7	36.7	36.7	89.6	89.6	89.6	89.6	89.6	89.6
Actuated g/C Ratio	0.04	0.30	0.26	0.26	0.26	0.26	0.64	0.64	0.64	0.64	0.64	0.64
v/c Ratio	0.30	0.60	0.87	0.86	0.68	0.87	0.87	0.50	0.72	0.72	0.72	0.72
Control Delay	75.2	41.7	69.6	68.1	44.3	69.6	15.5	4.4	19.8	19.8	19.8	19.8
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.1	0.0	0.0	0.0	0.0	0.0
Total Delay	75.2	41.7	69.6	68.1	44.3	69.6	15.5	4.4	19.8	19.8	19.8	19.8
LOS	E	D	E	E	D	D	B	A	B	A	B	A
Approach Delay	44.0						13.8				19.7	
Approach LOS	D						B				B	
Queue Length 50th (m)	6.6	65.3	111.3	112.8	64.2	112.8	115.3	20.2	145.8	145.8	145.8	145.8
Queue Length 95th (m)	16.7	96.7	#167.3	#168.6	99.2	168.6	211.9	m34.4	166.1	166.1	166.1	166.1
Internal Link Dist (m)		96.4				141.1	121.2				185.2	
Turn Bay Length (m)	50.0						70.0					
Base Capacity (vph)	77	568	456	468	470	468	3253	1064	2697	2697	2697	2697
Starvation Cap Reductn	0	0	0	0	0	0	22	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.30	0.54	0.84	0.83	0.66	0.83	0.88	0.50	0.72	0.72	0.72	0.72



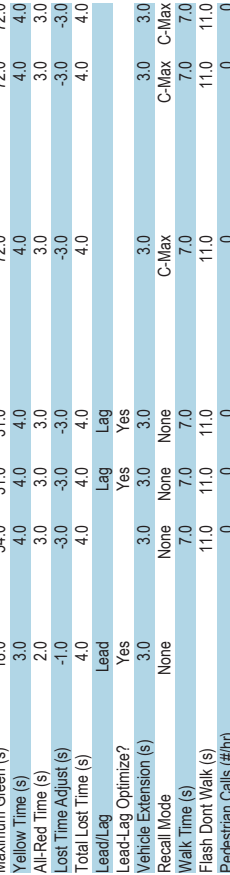
EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Prot	Perm	Perm	Perm	NA	Free	NA	NA	Free	NA	NA	Perm
7	4	4	8	8	Free	2	2	Free	6	6	6

Analysis Period (min)	15
ICU Level of Service	E
Intersection Capacity Utilization	83.3%
# 95th percentile volume exceeds capacity, queue may be longer.	
Queue shown is maximum after two cycles.	
m Volume for 95th percentile queue is metered by upstream signal.	



EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Prot	Perm	Perm	Perm	NA	Free	NA	NA	Free	NA	NA	Perm
7	4	4	8	8	Free	2	2	Free	6	6	6

Analysis Period (min)	15
ICU Level of Service	E
Intersection Capacity Utilization	83.3%
# 95th percentile volume exceeds capacity, queue may be longer.	
Queue shown is maximum after two cycles.	
m Volume for 95th percentile queue is metered by upstream signal.	



Lane Group	EBL			EBT			EBR			WBL			WBT			WBR			NBL			NBT			NBR			SBL			SBT			SBR					
Turn Type	Prot			Perm			Perm			Perm			NA			Free			NA			NA			Free			NA			NA			Perm					
Protected Phases	7			4			4			8			8			Free			2			2			Free			6			6			6					
Detector Phase	7			4			4			8			8			Free			2			2			Free			6			6			6					
Switch Phase																																							
Minimum Initial (s)	5.0			5.0			5.0			5.0			5.0			5.0			5.0			5.0			5.0			5.0			5.0			5.0					
Minimum Split (s)	23.0			38.0			38.0			38.0			38.0			38.0			38.0			38.0			38.0			38.0			38.0			38.0					
Total Split (s)	23.0			61.0			61.0			61.0			61.0			61.0			61.0			61.0			61.0			61.0			61.0			61.0					
Total Split (%)	16.4%			43.6%			27.1%			27.1%			56.4%			56.4%			56.4%			56.4%			56.4%			56.4%			56.4%			56.4%					
Maximum Green (s)	18.0			54.0			31.0			31.0			72.0			72.0			72.0			72.0			72.0			72.0			72.0			72.0					
Yellow Time (s)	3.0			4.0			4.0			4.0			4.0			4.0			4.0			4.0			4.0			4.0			4.0			4.0					
All-Red Time (s)	2.0			3.0			3.0			3.0			3.0			3.0			3.0			3.0			3.0			3.0			3.0			3.0					
Lost Time Adjust (s)	-1.0			-3.0			-3.0			-3.0			-3.0			-3.0			-3.0			-3.0			-3.0			-3.0			-3.0			-3.0					
Total Lost Time (s)	4.0			4.0			4.0			4.0			4.0			4.0			4.0			4.0			4.0			4.0			4.0			4.0					
Lead/Lag	Lead			Lag			Lag			Lag			Lag			Lag			Lag			Lag			Lag			Lag			Lag			Lag					
Lead-Lag Optimize?	Yes			Yes			Yes			Yes			Yes			Yes			Yes			Yes			Yes			Yes			Yes			Yes					
Vehicle Extension (s)	3.0			3.0			3.0			3.0			3.0			3.0			3.0			3.0			3.0			3.0			3.0			3.0					
Recall Mode	None			None			None			None			C-Max			C-Max			C-Max			C-Max			C-Max			C-Max			C-Max								
Walk Time (s)	7.0			7.0			7.0			7.0			7.0			7.0			7.0			7.0			7.0			7.0			7.0			7.0					
Flash Dont Walk (s)	11.0			11.0			11.0			11.0			11.0			11.0			11.0			11.0			11.0			11.0			11.0			11.0					
Pedestrian Calls (#/hr)	0			0			0			0			0			0			0			0			0			0			0			0					
Act Effct Green (s)	8.2			44.2			36.3			140.0			87.8			140.0			87.8			87.8			87.8			87.8			87.8			87.8					
Actuated g/C Ratio	0.06			0.32			0.26			1.00			0.63			1.00			0.63			0.63			0.63			0.63			0.63			0.63					
v/c Ratio	0.21			0.54			0.77			0.27			0.96			0.38			1.10			0.01			0.01			0.01			0.01			0.01					
Control Delay	66.7			37.2			59.9			0.4			26.7			0.3			79.1			0.0			0.0			0.0			0.0			0.0					
Queue Delay	0.0			0.0			0.0			0.0			0.0			0.0			0.0			0.0			0.0			0.0			0.0			0.0					
Total Delay	66.7			37.2			59.9			0.4			26.7			0.3			79.1			0.0			0.0			0.0			0.0			0.0					
LOS	E			D			E			A			C			A			E			A			E			A			E			A					
Approach Delay	39.3			37.4			22.4			78.8			78.8			78.8			78.8			78.8			78.8			78.8			78.8			78.8					
Approach LOS	D			D			C			E			E			E			E			E			E			E			E			E					
Queue Length 50th (m)	6.2			58.2			94.8			98.0			0.0			-346.0			0.0			-370.7			0.0			0.0			0.0			0.0					
Queue Length 95th (m)	15.6			79.6			127.4			130.9			0.0			#401.9			m0.0			#420.6			0.0			0.0			0.0			0.0					
Internal Link Dist (m)	50.0			96.4			141.1			121.2			185.2			185.2			185.2			185.2			185.2			185.2			185.2			185.2					
Turn Bay Length (m)	244			675			454			1563			3187			1556			2643			964			964			964			964			964					
Base Capacity (vph)	0			0			0			0			0			0			0			0			0			0			0			0					
Starvation Cap Reductn	0			0			0			0			0			0			0			0			0			0			0			0					
Spillback Cap Reductn	0			0			0			0			0			0			0			0			0			0			0			0					
Storage Cap Reductn	0			0			0			0			0			0			0			0			0			0			0			0					
Reduced v/c Ratio	0.09			0.43			0.74			0.27			0.96			0.38			1.10			0.01			0.01			0.01			0.01			0.01					

Intersection Summary		
Area Type:	Other	
Cycle Length:	140	
Actuated Cycle Length:	140	
Offset: 0 (0%); Referenced to phase 2:NBT and 6:SBT. Start of Green		
Natural Cycle:	150	
Control Type:	Actuated-Coordinated	
Maximum v/c Ratio:	1.10	
Intersection Signal Delay:	45.8	
Intersection LOS:	D	

HCM Signalized Intersection Capacity Analysis  
 1: Trafalgar Road & North Service Road/QEW WB Off Ramp

Lanes, Volumes, Timings  
 1: Trafalgar Road & North Service Road/QEW WB Off Ramp

2025 Total PM Peak Hour.syn  
 05-04-2022

2030 Background PM Peak Hour.syn  
 05-04-2022

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	21	0	283	569	142	286	0	2614	490	0	1791	10
Traffic Volume (vph)	21	0	283	569	142	286	0	2614	490	0	1791	10
Future Volume (vph)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Ideal Flow (vphpl)	3.5	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Total Lost time (s)	1.00	1.00	0.95	1.00	0.95	1.00	0.91	1.00	1.00	0.91	1.00	1.00
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	0.98	1.00	0.93	1.00	0.93	1.00
Frb. ped/bikes	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frb. ped/bikes	1.00	0.85	1.00	1.00	0.85	1.00	0.85	1.00	0.85	1.00	0.85	1.00
Fit Protected	0.95	1.00	0.95	0.97	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Satd. Flow (prot)	1805	1615	1681	1724	1557	1557	5085	1482	4217	1497	1497	1497
Fit Permitted	0.95	1.00	0.95	0.97	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Satd. Flow (perm)	1805	1615	1681	1724	1557	1557	5085	1482	4217	1497	1497	1497
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	23	0	308	618	154	311	0	2841	533	0	1947	11
RTOR Reduction (vph)	0	0	21	0	0	49	0	0	121	0	0	4
Lane Group Flow (vph)	23	0	287	383	389	262	0	2841	412	0	1947	7
Conf. Peds. (#/hr)	2	0	2	14	2	14	0	2	14	14	14	14
Heavy Vehicles (%)	0%	0%	0%	2%	1%	2%	0%	2%	1%	0%	23%	0%
Turn Type	Prot	Perm	Perm	NA	Perm	NA	Perm	NA	Perm	NA	Perm	NA
Protected Phases	7		4	8	8	8	2	2	2	6	6	6
Permitted Phases	3.0	41.2	33.7	33.7	33.7	33.7	84.8	84.8	84.8	84.8	84.8	84.8
Actuated Green, G (s)	4.0	44.2	36.7	36.7	36.7	36.7	87.8	87.8	87.8	87.8	87.8	87.8
Effective Green, g (s)	0.03	0.32	0.26	0.26	0.26	0.26	0.63	0.63	0.63	0.63	0.63	0.63
Actuated g/C Ratio	4.5	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0
Clearance Time (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Vehicle Extension (s)	51	509	440	451	408	408	3189	929	2644	938	938	938
Lane Grp Cap (vph)	0.01						c0.56		0.46			
v/s Ratio Prot	0.45	0.56	0.87	0.86	0.64	0.64	0.89	0.44	0.74	0.71	0.01	0.01
v/s Ratio Perm	66.9	39.9	49.4	49.2	45.8	45.8	22.1	13.5	18.1	9.8	9.8	9.8
Uniform Delay, d1	1.00	1.00	1.00	1.00	1.00	1.00	0.60	0.65	1.00	1.00	1.00	1.00
Progression Factor	6.2	1.4	16.9	15.5	3.5	3.5	2.5	0.9	1.9	0.0	0.0	0.0
Incremental Delay, d2	73.1	41.3	66.3	64.8	49.3	49.3	15.7	9.7	20.0	9.8	9.8	9.8
Delay (s)	E	D	E	E	D	B	A	B	A	B	A	A
Level of Service	E	D	E	E	D	B	A	B	A	B	A	A
Approach Delay (s)	43.5			60.9		14.7		19.9				
Approach LOS	D			E		B		B				
Intersection Summary												
HCM 2000 Control Delay		25.0		HCM 2000 Level of Service		C						
HCM 2000 Volume to Capacity ratio		0.88										
Actuated Cycle Length (s)		140.0		Sum of lost time (s)		11.5						
Intersection Capacity Utilization		83.3%		ICU Level of Service		E						
Analysis Period (min)		15										
c Critical Lane Group												

HCM Unsignalized Intersection Capacity Analysis  
 12: Site Driveway

Lanes, Volumes, Timings  
 2: Trafalgar Road & QEW SB On Ramps

2025 Total AM Peak Hour.syn  
 05-04-2022

2030 Background PM Peak Hour.syn  
 05-04-2022

Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	3	8	18	4	8	78
Traffic Volume (veh/h)	3	8	18	4	8	78
Future Volume (Veh/h)	3	8	18	4	8	78
Sign Control	Free	Free	Free	Stop	Stop	Stop
Grade	0%	0%	0%	0%	0%	0%
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	3	9	20	4	9	85
Pedestrians						
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)						
Median type	None					
Median storage (veh)						
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume						
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
tC, single (s)						
tC, 2 stage (s)						
tF (s)						
p0 queue free %						
cM capacity (veh/h)						
Direction, Lane #	EB 1	WB 1	NB 1			
Volume Total	12	24	94			
Volume Left	0	20	9			
Volume Right	9	0	85			
GSH	1700	1607	1061			
Volume to Capacity	0.01	0.01	0.09			
Queue Length 95th (m)	0.0	0.3	2.3			
Control Delay (s)	0.0	6.1	8.7			
Lane LOS	A	A	A			
Approach Delay (s)	0.0	6.1	8.7			
Approach LOS	A	A	A			
Intersection Summary						
Average Delay			7.4			
Intersection Capacity Utilization			19.8%			ICU Level of Service A
Analysis Period (min)			15			

Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Volume (vph)	0	0	0	3104	1875	768
Future Volume (vph)	0	0	0	3104	1875	768
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	0.91	0.91	1.00
Flt Protected						0.850
Satd. Flow (prot)	0	0	0	5085	5085	1583
Flt Permitted						
Satd. Flow (perm)	0	0	0	5085	5085	1583
Link Speed (k/h)	50			50		50
Link Distance (m)	159.6			186.0		145.2
Travel Time (s)	11.5			13.4		10.5
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	0	0	0	3374	2038	835
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	0	0	3374	2038	835
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Left	Left	Right
Median Width(m)	0.0			0.0		0.0
Link Offset(m)	0.0			0.0		0.0
Crosswalk Width(m)	4.8			4.8		4.8
Two way Left Turn Lane						
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (k/h)	25	15	25			15
Sign Control	Stop	Stop	Free	Free	Free	Free
Intersection Summary						
Area Type:						Other
Control Type:						Unsignalized
Intersection Capacity Utilization						77.0%
Analysis Period (min)						15
ICU Level of Service D						

HCM Unsignalized Intersection Capacity Analysis 2030 Background PM Peak Hour.syn  
 2: Trafalgar Road & QEW SB On Ramps 05-04-2022

Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations				↑↑↑	↑↑↑	↑
Traffic Volume (veh/h)	0	0	0	3104	1875	768
Future Volume (Veh/h)	0	0	0	3104	1875	768
Sign Control	Stop	Free	Free	Free	Free	Free
Grade	0%			0%	0%	0%
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	0	0	0	3374	2038	835
Pedestrians						
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)				None	None	
Median type						
Median storage (veh)				186	145	
Upstream signal (m)	0.38					
pX, platoon unblocked	3163	679	2038			
vC, conflicting volume						
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	960	679	2038			
tC, single (s)	6.8	6.9	4.1			
tC, 2 stage (s)						
tF (s)	3.5	3.3	2.2			
p0 queue free %	100	100	100			
cM capacity (veh/h)	96	394	273			
Direction, Lane #	NB.1	NB.2	NB.3	SB.1	SB.2	SB.3
Volume Total	1125	1125	1125	679	679	679
Volume Left	0	0	0	0	0	0
Volume Right	0	0	0	0	0	0
GSH	1700	1700	1700	1700	1700	1700
Volume to Capacity	0.66	0.66	0.66	0.40	0.40	0.40
Queue Length 95th (m)	0.0	0.0	0.0	0.0	0.0	0.0
Control Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0
Lane LOS						
Approach Delay (s)	0.0			0.0		
Approach LOS						
Intersection Summary						
Average Delay	0.0					
Intersection Capacity Utilization	77.0%					
Analysis Period (min)	15					
	ICU Level of Service D					

Lanes, Volumes, Timings 2025 Total AM Peak Hour.syn  
 12: Site Driveway 05-04-2022

Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↑			↓	↓	↓
Traffic Volume (vph)	3	8	18	4	8	78
Future Volume (vph)	3	8	18	4	8	78
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt	0.899				0.878	
Flt Protected					0.960	0.995
Satd. Flow (prot)	1675	0	0	1788	1627	0
Flt Permitted					0.960	0.995
Satd. Flow (perm)	1675	0	0	1788	1627	0
Link Speed (k/h)	50			50	50	
Link Distance (m)	159.1			666.7	58.4	
Travel Time (s)	11.5			48.0	4.2	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	3	9	20	4	9	85
Shared Lane Traffic (%)						
Lane Group Flow (vph)	12	0	0	24	94	0
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Left	Left	Right
Median Width(m)	0.0			0.0	3.6	
Link Offset(m)	0.0			0.0	0.0	
Crosswalk Width(m)	4.8			4.8	4.8	
Two way Left Turn Lane						
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (k/h)						
Sign Control	Free	Free	Free	Free	Stop	Stop
Intersection Summary						
Area Type:	Other					
Control Type:	Unsignalized					
Intersection Capacity Utilization	19.8%					
Analysis Period (min)	15					
	ICU Level of Service A					

HCM Unsignalized Intersection Capacity Analysis  
 11: Argus Rd & South Service Rd

2025 Total AM Peak Hour.syn  
 05-04-2022

Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	4	5	607	145	23	62
Traffic Volume (veh/h)	1	5	607	145	23	62
Future Volume (Veh/h)	1	5	607	145	23	62
Sign Control	Free	Free	Free	Free	Stop	Stop
Grade	0%	0%	0%	0%	0%	0%
Peak Hour Factor	0.25	0.42	0.86	0.72	0.25	0.25
Hourly flow rate (vph)	4	12	706	201	92	248
Pedestrians	1	5			1	
Lane Width (m)	3.6	3.6			3.6	
Walking Speed (m/s)	1.2	1.2			1.2	
Percent Blockage	0	0			0	
Right turn flare (veh)						
Median type	None	None				
Upstream signal (m)	358					
pX, platoon unblocked vC, conflicting volume	908				832	808
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCU, unblocked vol	908				832	808
tC, single (s)	5.1				6.4	6.2
tC, 2 stage (s)	3.1				3.5	3.3
tF (s)	99				73	35
p0 queue free %	462				337	383
cM capacity (veh/h)						
Direction_Lane #	EB 1	WB 1	SB 1			
Volume Total	16	907	340			
Volume Left	4	0	92			
Volume Right	0	201	248			
GSH	462	1700	369			
Volume to Capacity	0.01	0.63	0.92			
Queue Length 95th (m)	0.2	0.0	76.8			
Control Delay (s)	3.3	0.0	62.8			
Lane LOS	A	F	F			
Approach Delay (s)	3.3	0.0	62.8			
Approach LOS	F	F	F			
Intersection Summary						
Average Delay		17.0				
Intersection Capacity Utilization		58.0%		ICU Level of Service	B	
Analysis Period (min)		15				

Lanes, Volumes, Timings  
 3: Trafalgar Road & QEW EB Off Ramp

2030 Background PM Peak Hour.syn  
 05-04-2022

Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	↑↑	↑	↑↑↑	↑↑↑	↑↑↑	↑↑↑
Traffic Volume (vph)	1090	548	0	2014	1875	0
Future Volume (vph)	1090	548	0	2014	1875	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	0.97	1.00	1.00	0.91	0.91	1.00
Ped Bike Factor	0.98					
Ft	0.850					
Flt Protected	0.950					
Satd. Flow (prot)	3502	1599	0	5085	5085	0
Flt Permitted	0.950					
Satd. Flow (perm)	3502	1573	0	5085	5085	0
Right Turn on Red	Yes	Yes	Yes	Yes	Yes	Yes
Satd. Flow (RTOR)	3	3	3	3	3	3
Link Speed (k/h)	50			50		
Link Distance (m)	71.3			50.7		186.0
Travel Time (s)	5.1			3.7		13.4
Conf. Peds. (#/h)	2			2		2
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	0%	1%	0%	2%	2%	0%
Adj. Flow (vph)	1185	596	0	2189	2038	0
Shared Lane Traffic (%)						
Lane Group Flow (vph)	1185	596	0	2189	2038	0
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Left	Left	Right
Median Width(m)	7.2			0.0	0.0	0.0
Link Offset(m)	0.0			0.0	0.0	0.0
Crosswalk Width(m)	4.8			4.8		4.8
Two way Left Turn Lane						
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (k/h)	25	15	25	2	2	15
Number of Detectors	1	1	1	2	2	2
Detector Template	Left	Right	Right	Thru	Thru	Thru
Leading Detector (m)	2.0	2.0	2.0	10.0	10.0	10.0
Trailing Detector (m)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Position(m)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Size(m)	2.0	2.0	2.0	0.6	0.6	0.6
Detector 1 Type	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex
Detector 1 Channel						
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 2 Position(m)				9.4	9.4	9.4
Detector 2 Size(m)				0.6	0.6	0.6
Detector 2 Type				Ch+Ex	Ch+Ex	Ch+Ex
Detector 2 Channel						
Detector 2 Extend (s)				0.0	0.0	0.0
Turn Type	Perm	Perm	Perm	NA	NA	NA
Protected Phases				2	6	
Permitted Phases	4	4	4			

Lanes, Volumes, Timings  
3: Trafalgar Road & QEW EB Off Ramp

Lanes, Volumes, Timings  
11: Argus Rd & South Service Rd

2030 Background PM Peak Hour.syn  
05-04-2022

2025 Total AM Peak Hour.syn  
05-04-2022



Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Detector Phase	4	4		2	6	
Switch Phase						
Minimum Initial (s)	10.0	10.0		29.0	29.0	
Minimum Split (s)	38.0	38.0		36.0	36.0	
Total Split (s)	67.0	67.0		73.0	73.0	
Total Split (%)	47.9%	47.9%		52.1%	52.1%	
Maximum Green (s)	60.0	60.0		66.0	66.0	
Yellow Time (s)	4.0	4.0		4.0	4.0	
All-Red Time (s)	3.0	3.0		3.0	3.0	
Lost Time Adjust (s)	-3.0	-3.0		-3.0	-3.0	
Total Lost Time (s)	4.0	4.0		4.0	4.0	
Lead/Lag						
Lead-Lag Optimize?						
Vehicle Extension (s)	3.0	3.0		3.0	3.0	
Recall Mode	None	None		C-Max	C-Max	
Walk Time (s)	7.0	7.0		7.0	7.0	
Flash Dont Walk (s)	11.0	11.0		11.0	11.0	
Pedestrian Calls (#/hr)	0	0		0	0	
Act Effct Green (s)	60.0	60.0		72.0	72.0	
Actuated g/C Ratio	0.43	0.43		0.51	0.51	
v/c Ratio	0.79	0.88		0.84	0.78	
Control Delay	38.8	52.2		43.5	23.9	
Queue Delay	0.0	0.0		0.2	0.0	
Total Delay	38.8	52.2		43.7	23.9	
LOS	D	D		D	C	
Approach Delay	43.3			43.7	23.9	
Approach LOS	D			D	C	
Queue Length 50th (m)	145.7	150.7		240.5	121.6	
Queue Length 95th (m)	173.4	#214.7		m243.0	171.4	
Internal Link Dist (m)	47.3			26.7	162.0	
Turn Bay Length (m)						
Base Capacity (vph)	1575	709		2615	2615	
Starvation Cap Reductn	0	0		0	0	
Spillback Cap Reductn	0	0		56	0	
Storage Cap Reductn	0	0		0	0	
Reduced v/c Ratio	0.75	0.84		0.86	0.78	
<b>Intersection Summary</b>						
Area Type:	Other					
Cycle Length:	140					
Actuated Cycle Length:	140					
Offset: 0 (0%), Referenced to phase 2:NBT and 6:SBT, Start of Green						
Natural Cycle:	80					
Control Type:	Actuated-Coordinated					
Maximum v/c Ratio:	0.88					
Intersection Signal Delay:	36.9					
Intersection Capacity Utilization	77.0%					
Analysis Period (min)	15					
# 95th percentile volume exceeds capacity, queue may be longer.						

Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	1	4	1	1	1	1
Traffic Volume (vph)	5	607	145	145	23	62
Future Volume (vph)	1	5	607	145	23	62
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor			0.970		0.902	
Frt		0.988			0.987	
Flt Protected		0	1352	1621	0	1522
Satd. Flow (prot)		0	1352	1621	0	1522
Flt Permitted		0.988			0.987	
Satd. Flow (perm)		0	1352	1621	0	1522
Link Speed (k/h)		50	50	50	50	50
Link Distance (m)		177.7	165.2	103.5		
Travel Time (s)		12.8	11.9	7.5		
Conf. Peds. (#/hr)		1	1	1	5	1
Peak Hour Factor		0.25	0.42	0.86	0.72	0.25
Heavy Vehicles (%)		100%	0%	3%	0%	0%
Adj. Flow (vph)		4	12	706	201	92
Shared Lane Traffic (%)						
Lane Group Flow (vph)		0	16	907	0	340
Enter Blocked Intersection		No	No	No	No	No
Lane Alignment		Left	Left	Right	Left	Right
Median Width(m)		0.0	0.0	0.0	3.6	
Link Offset(m)		0.0	0.0	0.0	0.0	
Crosswalk Width(m)		4.8	4.8		4.8	
Two way Left Turn Lane						
Headway Factor		1.14	1.14	1.14	1.14	1.14
Turning Speed (k/h)		24		14	24	14
Sign Control		Free	Free	Free	Stop	Stop
<b>Intersection Summary</b>						
Area Type:	CBD					
Control Type:	Unsignalized					
Intersection Capacity Utilization	58.0%					
Analysis Period (min)	15					
ICU Level of Service B						

HCM Unsignalized Intersection Capacity Analysis  
 10: Lyons Lane & South Service Road

2025 Total AM Peak Hour.syn  
 05-04-2022

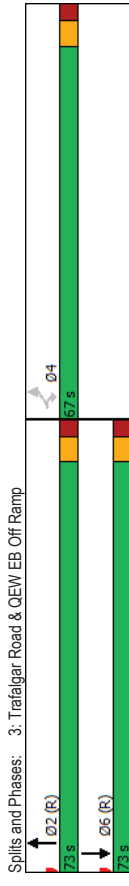


Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	0	4	3	11	11	1
Traffic Volume (veh/h)	0	4	3	11	11	1
Future Volume (Veh/h)	0	4	3	11	11	1
Sign Control	Free	Free	Free	Stop	Stop	0%
Grade	0%	0%	0%	0%	0%	0%
Peak Hour Factor	0.25	0.50	0.75	0.75	0.38	0.25
Hourly flow rate (vph)	0	8	4	15	29	4
Pedestrians						
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)						
Median type						
Median storage (veh)						
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume						
vC1, stage 1 conf/vol						
vC2, stage 2 conf/vol						
vCu, unblocked vol						
tC, single (s)						
tC, 2 stage (s)						
tF (s)						
p0 queue free %						
cM capacity (veh/h)						
Direction_Lane #	EB 1	WB 1	SB 1			
Volume Total	8	19	33			
Volume Left	0	0	29			
Volume Right	0	15	4			
GSH	1595	1700	926			
Volume to Capacity	0.00	0.01	0.04			
Queue Length 95th (m)	0.0	0.0	0.9			
Control Delay (s)	0.0	0.0	9.0			
Lane LOS			A			
Approach Delay (s)	0.0	0.0	9.0			
Approach LOS			A			
Intersection Summary						
Average Delay			5.0			
Intersection Capacity Utilization			15.1%		ICU Level of Service	A
Analysis Period (min)			15			

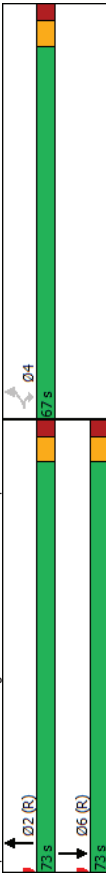
Lanes, Volumes, Timings  
 3: Trafalgar Road & QEW EB Off Ramp

2030 Background PM Peak Hour.syn  
 05-04-2022

Queue shown is maximum after two cycles.  
 m Volume for 95th percentile queue is metered by upstream signal.



Splits and Phases: 3: Trafalgar Road & QEW EB Off Ramp





HCM Signalized Intersection Capacity Analysis  
 3: Trafalgar Road & QEW EB Off Ramp

Lanes, Volumes, Timings  
 10: Lyons Lane & South Service Road

2030 Background PM Peak Hour.syn  
 05-04-2022

2025 Total AM Peak Hour.syn  
 05-04-2022

Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	↔	↔	↔	↔	↔	↔
Traffic Volume (vph)	1090	548	0	2014	1875	0
Future Volume (vph)	1090	548	0	2014	1875	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0
Lane Util. Factor	0.97	1.00	0.91	1.00	0.91	1.00
Frb, ped/bikes	1.00	0.98	1.00	1.00	1.00	1.00
Frb, ped/bikes	1.00	1.00	1.00	1.00	1.00	1.00
Frt	1.00	0.85	1.00	1.00	1.00	1.00
Flt Protected	0.95	1.00	1.00	1.00	1.00	1.00
Satd. Flow (prot)	3502	1573	5085	5085	5085	0
Flt Permitted	0.95	1.00	1.00	1.00	1.00	1.00
Satd. Flow (perm)	3502	1573	5085	5085	5085	0
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	1185	596	0	2189	2038	0
RTOR Reduction (vph)	0	2	0	0	0	0
Lane Group Flow (vph)	1185	594	0	2189	2038	0
Conf. Peds. (#/hr)	2	2	0	2	2	0
Heavy Vehicles (%)	0%	1%	0%	2%	2%	0%
Turn Type	Perm	Perm	NA	NA	NA	NA
Protected Phases						
Permitted Phases	4	4	2	6	6	6
Actuated Green, G (s)	57.0	57.0	69.0	69.0	69.0	69.0
Effective Green, g (s)	60.0	60.0	72.0	72.0	72.0	72.0
Actuated g/C Ratio	0.43	0.43	0.51	0.51	0.51	0.51
Clearance Time (s)	7.0	7.0	7.0	7.0	7.0	7.0
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0
Lane Grp. Cap (vph)	1500	674	2615	2615	2615	2615
v/s Ratio Prot			c0.43	0.40	0.40	
v/s Ratio Perm	0.34	c0.38				
v/c Ratio	0.79	0.88	0.84	0.84	0.78	0.78
Uniform Delay, d1	34.6	36.7	29.0	27.6	27.6	27.6
Progression Factor	1.00	1.00	1.42	0.78	0.78	0.78
Incremental Delay, d2	2.9	12.9	1.0	1.6	1.6	1.6
Delay (s)	37.5	49.7	42.2	23.2	23.2	23.2
Level of Service	D	D	D	C	C	C
Approach Delay (s)	41.6	41.6	42.2	23.2	23.2	23.2
Approach LOS	D	D	D	C	C	C
Intersection Summary						
HCM 2000 Control Delay	35.6		HCM 2000 Level of Service		D	
HCM 2000 Volume to Capacity ratio	0.88					
Actuated Cycle Length (s)	140.0		Sum of lost time (s)		11.0	
Intersection Capacity Utilization	77.0%		ICU Level of Service		D	
Analysis Period (min)	15					
c Critical Lane Group						

Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	↔	↔	↔	↔	↔	↔
Traffic Volume (vph)	0	4	3	11	11	1
Future Volume (vph)	0	4	3	11	11	1
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor			0.893		0.984	
Frt					0.958	
Flt Protected					0.958	
Satd. Flow (prot)					1250	
Flt Permitted					0.958	
Satd. Flow (perm)					1250	
Link Speed (k/h)			50	50	50	50
Link Distance (m)			60.5	89.6	37.6	
Travel Time (s)			4.4	6.5	2.7	
Conf. Peds. (#/hr)			6	6	1	
Peak Hour Factor			0.25	0.75	0.38	0.25
Heavy Vehicles (%)			0%	0%	33%	0%
Adj. Flow (vph)			0	8	29	4
Shared Lane Traffic (%)						
Lane Group Flow (vph)			0	8	19	0
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Left	Left	Right	Left	Right
Median Width(m)			0.0	0.0	3.6	
Link Offset(m)			0.0	0.0	0.0	
Crosswalk Width(m)			4.8	4.8	4.8	
Two way Left Turn Lane						
Headway Factor	1.14	1.14	1.14	1.14	1.14	1.14
Turning Speed (k/h)	24	Free	Free	Free	Stop	Stop
Sign Control						
Intersection Summary						
Area Type:	CBD					
Control Type:	Unsignalized					
Intersection Capacity Utilization	15.1%					
Analysis Period (min)	15					
ICU Level of Service A						

HCM Unsignalized Intersection Capacity Analysis  
 9: Cross Ave & Lyons Lane  
 2025 Total AM Peak Hour.syn  
 05-04-2022

Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	↔	↔	↔	↔	↔	↔
Traffic Volume (veh/h)	25	429	195	13	9	8
Future Volume (Veh/h)	25	429	195	13	9	8
Sign Control	Free	Free	Free	Free	Stop	Stop
Grade	0%	0%	0%	0%	0%	0%
Peak Hour Factor	0.69	0.58	0.88	0.50	0.62	0.50
Hourly flow rate (vph)	36	740	222	26	15	16
Pedestrians						
Lane Width (m)		3.6			3.6	
Walking Speed (m/s)		1.2			1.2	
Percent Blockage			1			0
Right turn flare (veh)			None			
Median type			None			
Median storage (veh)						
Upstream signal (m)			40			
pX, platoon unblocked						
vC, conflicting volume						
vC1, stage 1 conf/vol		252			688	128
vC2, stage 2 conf/vol						
vCu, unblocked vol		252			688	128
tC, single (s)		4.1			6.8	7.4
tC, 2 stage (s)		2.2			3.5	3.5
tF (s)		97			96	98
p0 queue free %		1321			371	827
cM capacity (veh/h)						
Direction_Lane #	EB1	EB2	EB3	WB1	WB2	SB1
Volume Total	36	370	370	148	100	31
Volume Left	36	0	0	0	0	15
Volume Right	0	0	0	0	26	16
GSH	1321	1700	1700	1700	1700	518
Volume to Capacity	0.03	0.22	0.22	0.09	0.06	0.06
Queue Length 95th (m)	0.7	0.0	0.0	0.0	0.0	1.5
Control Delay (s)	7.8	0.0	0.0	0.0	0.0	12.4
Lane LOS	A	A	A	B	B	B
Approach Delay (s)	0.4			0.0		12.4
Approach LOS				B		B
Intersection Summary	Other					
Average Delay	0.6					
Intersection Capacity Utilization	24.0% ICU Level of Service A					
Analysis Period (min)	15					

Lanes, Volumes, Timings  
 4: Trafalgar Road & Argus Road/QEW EB On Ramp  
 2030 Background PM Peak Hour.syn  
 05-04-2022

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔
Traffic Volume (vph)	0	0	49	0	0	0	0	2014	920	0	1984	439
Future Volume (vph)	0	0	49	0	0	0	0	2014	920	0	1984	439
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	70.0	0.0	0.0	0.0	0.0
Storage Lanes	0	0	1	0	0	0	0	1	0	0	0	0
Taper Length (m)	7.5			7.5			7.5			7.5		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.91	1.00	1.00	0.91	0.91
Ped Bike Factor			0.865					0.850				0.973
Fit	Fit Protected											
Satd. Flow (prot)	0	0	1522	0	0	0	0	5085	1568	0	4966	0
Fit Permitted	Fit Permitted											
Satd. Flow (perm)	0	0	1522	0	0	0	0	5085	1568	0	4966	0
Link Speed (k/h)			50			50					50	
Link Distance (m)			165.2			196.1					271.0	
Travel Time (s)			111.9			14.1					19.5	
Confl. Peds. (#/hr)												3.7
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	0%	0%	8%	0%	2%	0%	0%	2%	3%	0%	2%	0%
Adj. Flow (vph)	0	0	53	0	0	0	0	2189	1000	0	2157	477
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	0	53	0	0	0	0	2189	1000	0	2634	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(m)	0.0	0.0	0.0	0.0	0.0	0.0	3.6	0.0	0.0	0.0	3.6	0.0
Link Offset(m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Crosswalk Width(m)			4.8			4.8					4.8	
Two way Left Turn Lane												
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (k/h)	25	15	25	25	25	15	25	15	25	25	15	25
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
Intersection Summary	Other											
Area Type:	Unsignalized											
Control Type:	ICU Level of Service B											
Intersection Capacity Utilization	60.3%											
Analysis Period (min)	15											

HCM Unsignalized Intersection Capacity Analysis 2030 Background PM Peak Hour.syn  
 4: Trafalgar Road & Argus Road/QEW EB On Ramp

05-04-2022

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	0	0	49	0	0	0	0	2014	920	0	1984	439
Future Volume (Veh/h)	0	0	49	0	0	0	0	2014	920	0	1984	439
Sign Control	Stop	Stop	Stop	0%	0%	0%	Free	0%	0%	Free	0%	0%
Grade	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Peak Hour Factor	0	0	53	0	0	0	0	2189	1000	0	2157	477
Hourly flow rate (vph)												
Pedestrians	24											
Lane Width (m)	3.6											
Walking Speed (m/s)	1.2											
Percent Blockage	2											
Right turn flare (veh)							None	None	None	None	None	None
Median type												
Median storage (veh)												
Upstream signal (m)							271					51
pX, platoon unblocked	0.79	0.79	0.67	0.79	0.79	0.76	0.67					0.76
vC, conflicting volume	3149	4608	982	2908	4847	730	2658					2189
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	780	2627	0	475	2929	0	1761					1478
tC, single (s)	7.5	6.5	7.1	7.5	6.5	6.9	4.1					4.1
tC, 2 stage (s)												
tF (s)	3.5	4.0	3.4	3.5	4.0	3.3	2.2					2.2
p0 queue free %	100	100	92	100	100	100	100					100
cM capacity (veh/h)	220	19	702	343	12	834	237					353
Direction_Lane #	EB.1	NB.1	NB.2	NB.3	NB.4	SB.1	SB.2	SB.3				
Volume Total	53	730	730	730	1000	863	863	908				
Volume Left	0	0	0	0	0	0	0	0				
Volume Right	53	0	0	0	1000	0	0	477				
GSH	702	1700	1700	1700	1700	1700	1700	1700				
Volume to Capacity	0.08	0.43	0.43	0.43	0.59	0.51	0.51	0.53				
Queue Length 95th (m)	2.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0				
Control Delay (s)	10.5	0.0	0.0	0.0	0.0	0.0	0.0	0.0				
Lane LOS	B	B	B	B	B	B	B	B				
Approach Delay (s)	10.5	0.0				0.0						
Approach LOS	B	B				B						
Intersection Summary												
Average Delay	0.1											
Intersection Capacity Utilization	60.3%											ICU Level of Service
Analysis Period (min)	15											B

Lanes, Volumes, Timings 2025 Total AM Peak Hour.syn  
 9: Cross Ave & Lyons Lane

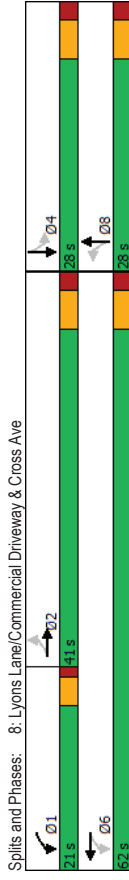
05-04-2022

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	25	429	195	13	9	8						
Future Volume (vph)	25	429	195	13	9	8						
Ideal Flow (vehpl)	1900	1900	1900	1900	1900	1900						
Storage Length (m)	5.0											
Storage Lanes	1											
Taper Length (m)	7.5											
Lane Util. Factor	1.00	0.95	0.95	0.95	0.95	1.00						
Ped Bike Factor												
Fit				0.984		0.930						
Fit Protected	0.950					0.976						
Satd. Flow (prot)	1824	3094	2796	0	1375	0						
Fit Permitted	0.950					0.976						
Satd. Flow (perm)	1824	3094	2796	0	1375	0						
Link Speed (k/h)				50	50	50						
Link Distance (m)				281.3	40.1	41.7						
Travel Time (s)				20.3	2.9	3.0						
Confl. Peds. (#/hr)	4			4	4	7						
Peak Hour Factor	0.69	0.58	0.88	0.50	0.62	0.50						
Heavy Vehicles (%)	0%	5%	16%	0%	0%	25%						
Adj. Flow (vph)	36	740	222	26	15	16						
Shared Lane Traffic (%)												
Lane Group Flow (vph)	36	740	248	0	31	0						
Enter Blocked Intersection	No	No	No	No	No	No						
Lane Alignment	Left	Left	Left	Right	Left	Right						
Median Width(m)				3.6	3.6	3.6						
Link Offset(m)				0.0	0.0	0.0						
Crosswalk Width(m)				4.8	4.8	4.8						
Two way Left Turn Lane												
Headway Factor	1.14	1.14	1.14	1.14	1.14	1.14						
Turning Speed (k/h)	25											
Sign Control				Free	Free	Stop						
Intersection Summary												
Area Type:	CBD											
Control Type:	Unsignalized											
Intersection Capacity Utilization	24.0%											ICU Level of Service A
Analysis Period (min)	15											



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Turn Type	Prot	NA	Perm	Prot	NA	pm+ov	pm+pt	NA	pm+ov	pm+pt	NA	Perm
Protected Phases	7	4	4	3	8	8	1	5	2	3	1	6
Permitted Phases	7	4	4	3	8	8	1	5	2	3	1	6
Switch Phase	7	4	4	3	8	8	1	5	2	3	1	6
Minimum Initial (s)	10.0	5.0	5.0	5.0	10.0	7.0	7.0	27.0	5.0	7.0	27.0	27.0
Minimum Split (s)	17.0	25.0	25.0	9.5	25.0	11.5	34.0	9.5	11.5	34.0	34.0	34.0
Total Split (s)	52.0	56.6	56.6	20.4	25.0	14.4	15.4	48.6	20.4	14.4	47.6	47.6
Total Split (%)	37.1%	40.4%	40.4%	14.6%	17.9%	10.3%	11.0%	34.7%	14.6%	10.3%	34.0%	34.0%
Maximum Green (s)	45.0	49.6	49.6	16.4	18.0	10.4	11.4	41.6	16.4	10.4	40.6	40.6
Yellow Time (s)	4.0	4.0	4.0	3.0	4.0	3.0	3.0	4.0	3.0	3.0	4.0	4.0
All-Red Time (s)	3.0	3.0	3.0	1.0	3.0	1.0	1.0	3.0	1.0	1.0	3.0	3.0
Lost Time Adjust (s)	-3.0	-3.0	0.0	-3.0	0.0	0.0	-3.0	0.0	-3.0	0.0	-3.0	-3.0
Total Lost Time (s)	4.0	4.0	7.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lead/Lag	Lead	Lag	Lag	Lead	Lag	Lead	Lead	Lag	Lead	Lead	Lag	Lag
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	None	None	None	None	None	None	None	C-Max	None	None	C-Max	C-Max
Walk Time (s)	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0
Flash Dont Walk (s)	11.0	11.0	11.0	11.0	11.0	11.0	11.0	11.0	11.0	11.0	11.0	11.0
Pedestrian Calls (#/hr)	0	0	0	0	0	0	0	0	0	0	0	0
Act Effct Green (s)	48.0	51.9	48.9	14.6	18.4	34.1	59.3	45.9	60.5	55.3	43.6	43.6
Actuated g/C Ratio	0.34	0.37	0.35	0.10	0.13	0.24	0.42	0.33	0.43	0.40	0.31	0.31
v/c Ratio	1.05	0.22	0.31	0.69	0.58	0.80	0.87	0.94	0.19	0.80	0.97	0.76
Control Delay	83.9	30.7	5.7	70.8	62.1	47.4	55.7	52.8	12.2	47.9	51.7	16.8
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	83.9	30.7	5.7	70.8	62.1	47.4	55.7	52.8	12.2	47.9	51.7	16.8
LOS	F	C	A	E	E	D	E	D	B	D	D	B
Approach Delay	66.6			58.2			50.4				43.6	
Approach LOS	E			E			D				D	
Queue Length 50th (m)	~198.6	28.3	0.4	36.0	38.3	74.2	49.0	166.9	6.0	25.1	171.6	72.3
Queue Length 95th (m)	#242.2	39.1	17.2	50.6	52.7	116.1	m#53.5	m132.1	m7.1	m#50.6	#202.4	m119.5
Internal Link Dist (m)	219.7			214.9			242.5				247.0	
Turn Bay Length (m)	25.0	60.0	25.0	60.0	25.0	60.0	50.0	60.0	25.0	60.0	25.0	60.0
Base Capacity (vph)	1165	1256	620	402	520	477	225	1683	692	208	1599	654
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	1.05	0.21	0.30	0.61	0.51	0.80	0.87	0.94	0.19	0.80	0.97	0.76
Intersection Summary												
Area Type:	Other											
Cycle Length:	140											
Actuated Cycle Length:	140											
Offset:	54.5 (39%), Referenced to phase 2:NBTL and 6:SBTL, Start of Green											
Natural Cycle:	150											
Control Type:	Actuated-Coordinated											
Maximum v/c Ratio:	1.05											
Intersection Signal Delay:	53.2											

Intersection Capacity Utilization 77.6%  
Analysis Period (min) 15  
ICU Level of Service D



Lanes, Volumes, Timings  
8: Lyons Lane/Commercial Driveway & Cross Ave

2025 Total AM Peak Hour.syn  
05-04-2022

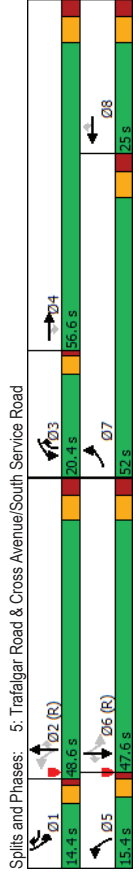
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Detector 2 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Turn Type	Perm	NA	pm-pt	NA	Perm	NA	Perm	NA	Perm	NA	Perm	NA
Protected Phases	2	2	6	1	6	8	8	8	8	4	4	4
Permitted Phases	2	2	6	1	6	8	8	8	8	4	4	4
Detector Phase	2	2	6	1	6	8	8	8	8	4	4	4
Switch Phase	2	2	6	1	6	8	8	8	8	4	4	4
Minimum Initial (s)	35.0	35.0	8.0	35.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0
Minimum Split (s)	41.0	41.0	12.5	41.0	28.0	28.0	28.0	28.0	28.0	28.0	28.0	28.0
Total Split (s)	41.0	41.0	21.0	62.0	28.0	28.0	28.0	28.0	28.0	28.0	28.0	28.0
Total Split (%)	45.6%	45.6%	23.3%	68.9%	31.1%	31.1%	31.1%	31.1%	31.1%	31.1%	31.1%	31.1%
Maximum Green (s)	35.0	35.0	17.0	56.0	22.0	22.0	22.0	22.0	22.0	22.0	22.0	22.0
Yellow Time (s)	4.0	4.0	3.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
All-Red Time (s)	2.0	2.0	1.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
Lost Time Adjust (s)	-2.0	-2.0	0.0	-2.0	-2.0	-2.0	-2.0	-2.0	-2.0	-2.0	-2.0	-2.0
Total Lost Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lead/Lag	Lag	Lag	Lead	Lag	Lead	Lag	Lead	Lag	Lead	Lag	Lead	Lag
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Vehicle Extension (s)	5.0	5.0	2.5	5.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Recall Mode	Min	Min	Min	Min	Min	Min	Min	Min	Min	Min	Min	Min
Walk Time (s)	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0
Flash Dont Walk (s)	15.0	15.0	15.0	15.0	15.0	15.0	15.0	15.0	15.0	15.0	15.0	15.0
Pedestrian Calls (#/hr)	0	0	0	0	0	0	0	0	0	0	0	0
Act Effct Green (s)	37.0	37.0	51.9	51.9	12.3	12.3	12.3	12.3	12.3	12.3	12.3	12.3
Actuated g/C Ratio	0.51	0.51	0.72	0.72	0.17	0.17	0.17	0.17	0.17	0.17	0.17	0.17
v/c Ratio	0.14	0.32	0.57	0.09	0.12	0.10	0.14	0.25	0.14	0.25	0.15	0.16
Control Delay	11.1	5.2	7.7	3.0	28.1	14.1	28.5	15.6	28.5	15.6	15.6	15.6
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	11.1	5.2	7.7	3.0	28.1	14.1	28.5	15.6	28.5	15.6	15.6	15.6
LOS	B	A	A	A	C	B	C	B	C	B	C	B
Approach Delay	6.0	6.1	20.5	6.1	20.5	6.1	20.5	6.1	20.5	6.1	20.5	6.1
Approach LOS	A	A	C	A	C	A	C	A	C	A	C	A
Queue Length 50th (m)	5.2	8.2	13.8	2.8	2.8	0.5	3.3	3.2	3.3	3.2	3.2	3.2
Queue Length 95th (m)	12.0	15.3	16.4	4.8	8.4	5.4	7.2	14.7	7.2	14.7	14.7	14.7
Internal Link Dist (m)	16.1	16.1	350.0	16.1	350.0	16.1	350.0	16.1	350.0	16.1	350.0	16.1
Turn Bay Length (m)	528	1575	683	2195	387	504	378	530	378	530	378	530
Base Capacity (vph)	0	0	0	0	0	0	0	0	0	0	0	0
Stevation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Spillover Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.14	0.32	0.51	0.08	0.06	0.05	0.07	0.14	0.06	0.05	0.07	0.14

Intersection Summary	
Area Type:	CBD
Cycle Length:	90
Actuated Cycle Length:	72.2
Natural Cycle:	85
Control Type:	Semi Act-Uncoord
Maximum v/c Ratio:	0.57
Intersection Signal Delay:	7.7
Intersection LOS: A	

Lanes, Volumes, Timings  
5: Trafalgar Road & Cross Avenue/South Service Road

2030 Background PM Peak Hour.syn  
05-04-2022

Intersection Summary	
Area Type:	CBD
Cycle Length:	90
Actuated Cycle Length:	72.2
Natural Cycle:	85
Control Type:	Semi Act-Uncoord
Maximum v/c Ratio:	0.57
Intersection Signal Delay:	7.7
Intersection LOS: A	



Intersection Summary	
Area Type:	CBD
Cycle Length:	90
Actuated Cycle Length:	72.2
Natural Cycle:	85
Control Type:	Semi Act-Uncoord
Maximum v/c Ratio:	0.57
Intersection Signal Delay:	7.7
Intersection LOS: A	





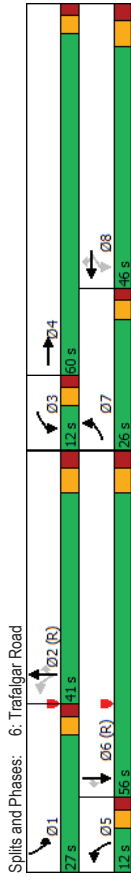


	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Turn Type	Prot	NA	pm+pt	NA	pm+pt	NA	pm+pt	NA	Perm	Prot	NA	Perm
Protected Phases	7	4	3	8	8	8	5	2	2	1	6	6
Permitted Phases	7	4	3	8	8	8	5	2	2	1	6	6
Detector Phase	7	4	3	8	8	8	5	2	2	1	6	6
Switch Phase	7	4	3	8	8	8	5	2	2	1	6	6
Minimum Initial (s)	7.0	10.0	7.0	10.0	10.0	7.0	7.0	20.0	20.0	7.0	20.0	20.0
Minimum Split (s)	12.0	37.0	12.0	37.0	37.0	12.0	39.0	39.0	39.0	12.0	39.0	39.0
Total Split (s)	26.0	60.0	26.0	46.0	46.0	12.0	41.0	41.0	27.0	27.0	56.0	56.0
Total Split (%)	18.6%	42.9%	8.6%	32.9%	32.9%	8.6%	29.3%	29.3%	19.3%	19.3%	40.0%	40.0%
Maximum Green (s)	21.0	55.0	7.0	39.0	39.0	7.0	34.0	34.0	22.0	49.0	49.0	49.0
Yellow Time (s)	3.0	3.0	4.0	4.0	4.0	3.0	4.0	4.0	4.0	3.0	4.0	4.0
All-Red Time (s)	2.0	2.0	3.0	3.0	3.0	2.0	3.0	3.0	3.0	2.0	3.0	3.0
Lost Time Adjust (s)	-1.0	-3.0	-1.0	-3.0	-3.0	-1.0	-3.0	-3.0	-1.0	-3.0	-3.0	-3.0
Total Lost Time (s)	4.0	2.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lead/Lag	Lead	Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lag	Lead	Lag	Lag
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	None	None	None	None	None	None	C-Max	C-Max	C-Max	None	C-Max	C-Max
Walk Time (s)	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0
Flash Dont Walk (s)	11.0	11.0	11.0	11.0	11.0	11.0	11.0	11.0	11.0	11.0	11.0	11.0
Pedestrian Calls (#/hr)	0	0	0	0	0	0	0	0	0	0	0	0
Act Effct Green (s)	22.0	60.4	50.0	42.0	42.0	45.0	37.0	37.0	37.0	23.0	52.0	52.0
Actuated g/C Ratio	0.16	0.43	0.36	0.30	0.30	0.32	0.26	0.26	0.26	0.16	0.37	0.37
v/c Ratio	1.12	0.65	0.29	0.91	1.16	1.04	0.55	0.09	1.11	1.05	0.82	0.82
Control Delay	127.7	33.2	24.3	59.9	116.1	114.0	46.9	0.3	130.8	89.7	42.6	42.6
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.9
Total Delay	127.7	33.2	24.3	59.9	116.1	114.0	46.9	0.3	130.8	89.7	43.5	43.5
LOS	F	C	C	E	F	F	D	A	F	D	F	D
Approach Delay	70.2	82.7	82.7	59.0	59.0	88.4	88.4	88.4	88.4	88.4	88.4	88.4
Approach LOS	E	F	F	E	E	F	F	F	F	F	F	F
Queue Length 50th (m)	~104.5	112.3	9.7	143.7	~201.9	~33.4	69.5	0.0	~110.6	~225.1	105.1	105.1
Queue Length 95th (m)	#143.5	137.2	18.5	#182.3	#282.9	#82.7	88.9	0.0	m#123.8	m#252.7	m117.4	m117.4
Internal Link Dist (m)	74.1	74.1	292.9	292.9	147.0	242.5	242.5	242.5	242.5	242.5	242.5	242.5
Turn Bay Length (m)	80.0	80.0	80.0	25.0	25.0	80.0	80.0	80.0	80.0	80.0	80.0	80.0
Base Capacity (vph)	544	1458	219	1072	670	159	954	549	563	705	746	746
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	27
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	1.12	0.65	0.29	0.91	1.16	1.04	0.55	0.09	1.11	1.05	0.85	0.85
Intersection Summary												
Area Type:	Other											
Cycle Length:	140											
Actuated Cycle Length:	140											
Offset:	54.5 (39%), Referenced to phase 2:NBTL and 6:SBT, Start of Green											
Natural Cycle:	150											
Control Type:	Actuated-Coordinated											
Maximum v/c Ratio:	1.16											
Intersection Signal Delay:	78.5											

	GO1	GO2	GO3	GO4	GO5	GO6	GO7	GO8
Analysis Period (min)	15	15	15	15	15	15	15	15
Intersection Capacity Utilization	81.6%							
ICU Level of Service	D							
# 95th percentile volume exceeds capacity, queue may be longer.								
Queue shown is maximum after two cycles.								
Splits and Phases:	7: GO Bus Terminal/Argus Rd & Cross Ave							
GO1	12.6 s	45.4 s	32 s	32 s	32 s	32 s	32 s	32 s
GO2	58 s	58 s	58 s	58 s	58 s	58 s	58 s	58 s
GO3	58 s	58 s	58 s	58 s	58 s	58 s	58 s	58 s
GO4	58 s	58 s	58 s	58 s	58 s	58 s	58 s	58 s
GO5	58 s	58 s	58 s	58 s	58 s	58 s	58 s	58 s
GO6	58 s	58 s	58 s	58 s	58 s	58 s	58 s	58 s
GO7	58 s	58 s	58 s	58 s	58 s	58 s	58 s	58 s
GO8	58 s	58 s	58 s	58 s	58 s	58 s	58 s	58 s

Intersection Capacity Utilization 98.4%  
 Analysis Period (min) 15  
 ~ Volume exceeds capacity, queue is theoretically infinite.  
 Queue shown is maximum after two cycles.  
 # 95th percentile volume exceeds capacity, queue may be longer.  
 Queue shown is maximum after two cycles.  
 m Volume for 95th percentile queue is metered by upstream signal.

ICU Level of Service F  
 ~ Volume exceeds capacity, queue is theoretically infinite.  
 Queue shown is maximum after two cycles.  
 # 95th percentile volume exceeds capacity, queue may be longer.  
 Queue shown is maximum after two cycles.  
 m Volume for 95th percentile queue is metered by upstream signal.



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Detector 2 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Turn Type	Perm	NA	pm+pt	NA	NA	NA	Perm	NA	NA	Perm	NA	NA
Protected Phases			1	6			8					4
Permitted Phases	2	2	6	6	8	8	8	8	8	4	4	4
Detector Phase	2	2	1	6	6	8	8	8	8	4	4	4
Switch Phase												
Minimum Initial (s)	22.0	22.0	8.0	22.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0
Minimum Split (s)	45.0	45.0	12.5	29.0	29.0	29.0	29.0	29.0	29.0	29.0	29.0	29.0
Total Split (s)	45.4	45.4	12.6	58.0	32.0	32.0	32.0	32.0	32.0	32.0	32.0	32.0
Total Split (%)	50.4%	50.4%	14.0%	64.4%	35.6%	35.6%	35.6%	35.6%	35.6%	35.6%	35.6%	35.6%
Maximum Green (s)	39.4	39.4	8.6	52.0	26.0	26.0	26.0	26.0	26.0	26.0	26.0	26.0
Yellow Time (s)	4.0	4.0	3.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
All-Red Time (s)	2.0	2.0	1.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
Lost Time Adjust (s)	-2.0	-2.0	0.0	-2.0	-2.0	-2.0	-2.0	-2.0	-2.0	-2.0	-2.0	-2.0
Total Lost Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lead/Lag	Lag	Lag	Lead	Lead	Lead	Lead	Lead	Lead	Lead	Lead	Lead	Lead
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Vehicle Extension (s)	5.0	5.0	2.5	5.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Recall Mode	Min	Min	Min	Min	Min	Min	Min	Min	Min	Min	Min	Min
Walk Time (s)	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0
Flash Dont Walk (s)	16.0	16.0	16.0	16.0	16.0	16.0	16.0	16.0	16.0	16.0	16.0	16.0
Pedestrian Calls (#/hr)	0	0	0	0	0	0	0	0	0	0	0	0
Act Effct Green (s)	24.6	24.6	36.8	28.0	28.0	28.0	28.0	28.0	28.0	28.0	28.0	28.0
Actuated g/C Ratio	0.34	0.34	0.51	0.38	0.38	0.38	0.38	0.38	0.38	0.38	0.38	0.38
v/c Ratio	0.31	0.47	0.24	0.46	0.87	0.15	0.35	0.91	0.35	0.91	0.35	0.91
Control Delay	22.8	20.5	12.5	12.6	130.2	0.6	19.1	33.8	19.1	33.8	19.1	33.8
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	22.8	20.5	12.5	12.6	130.2	0.6	19.1	33.8	19.1	33.8	19.1	33.8
LOS	C	C	B	B	F	A	A	B	A	B	C	C
Approach Delay	20.7	20.7	12.6	12.6	46.5	30.9	30.9	30.9	30.9	30.9	30.9	30.9
Approach LOS	C	C	B	B	D	C	C	C	C	C	C	C
Queue Length 50th (m)	6.6	28.0	3.5	33.6	5.1	0.0	14.7	52.3	5.1	14.7	52.3	52.3
Queue Length 95th (m)	8.7	39.8	8.3	45.4	#11.7	0.0	26.2	44.9	#11.7	26.2	44.9	44.9
Internal Link Dist (m)	350.0	350.0	219.7	219.7	57.9	156.7	156.7	156.7	156.7	156.7	156.7	156.7
Turn Bay Length (m)	20.0	20.0	20.0	20.0	46	500	434	674	46	500	434	674
Base Capacity (vph)	346	1737	207	2368	0	0	0	0	0	0	0	0
Stevation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.18	0.28	0.24	0.31	0.87	0.15	0.35	0.91	0.87	0.15	0.35	0.91

Intersection Summary

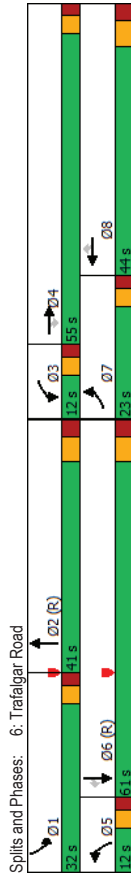
Area Type:	CBD
Cycle Length:	90
Actuated Cycle Length:	72.8
Natural Cycle:	100
Control Type:	Semi Act-Uncoord
Maximum v/c Ratio:	0.91
Intersection Signal Delay:	22.6





	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Detector 2 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Turn Type	Perm	NA	Perm	pm-pt	NA	Perm	pm-pt	NA	Perm	Perm	NA	NA
Protected Phases				1	6	6	3	8				4
Permitted Phases	2	2	2	6	6	6	8	8	8	4	4	4
Detector Phase	2	2	2	1	6	6	3	8	8	4	4	4
Switch Phase												
Minimum Initial (s)	22.0	22.0	8.0	22.0	22.0	22.0	5.0	10.0	10.0	10.0	10.0	10.0
Minimum Split (s)	45.0	45.0	45.0	12.5	29.0	29.0	9.5	29.0	29.0	29.0	29.0	29.0
Total Split (s)	46.5	46.5	46.5	17.0	63.5	63.5	13.0	42.0	42.0	29.0	29.0	29.0
Total Split (%)	44.1%	44.1%	44.1%	16.1%	60.2%	60.2%	12.3%	39.8%	39.8%	27.5%	27.5%	27.5%
Maximum Green (s)	40.5	40.5	40.5	13.0	57.5	57.5	8.5	36.0	36.0	23.0	23.0	23.0
Yellow Time (s)	4.0	4.0	4.0	3.0	4.0	4.0	3.5	4.0	4.0	4.0	4.0	4.0
All-Red Time (s)	2.0	2.0	2.0	1.0	2.0	2.0	1.0	2.0	2.0	2.0	2.0	2.0
Lost Time Adjust (s)	-2.0	-2.0	-2.0	0.0	-2.0	-2.0	-2.0	-2.0	0.0	-2.0	-2.0	-2.0
Total Lost Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	2.5	4.0	6.0	4.0	4.0	4.0
Lead/Lag	Lag	Lag	Lag	Lead	Lead	Lead	Lead	Lag	Lag	Lag	Lag	Lag
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Vehicle Extension (s)	5.0	5.0	5.0	2.5	5.0	5.0	3.0	4.0	4.0	4.0	4.0	4.0
Recall Mode	Min	Min	Min	Min	Min	Min	None	Min	Min	Min	Min	Min
Walk Time (s)	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0
Flash Dont Walk (s)	16.0	16.0	16.0	16.0	16.0	16.0	16.0	16.0	16.0	16.0	16.0	16.0
Pedestrian Calls (#/hr)	0	0	0	0	0	0	0	0	0	0	0	0
Act Effct Green (s)	42.6	42.6	42.6	59.6	59.6	59.6	37.1	35.6	33.6	22.6	22.6	22.6
Actuated g/C Ratio	0.41	0.41	0.41	0.58	0.58	0.58	0.36	0.34	0.33	0.22	0.22	0.22
v/c Ratio	0.14	0.94	0.16	0.93	0.25	0.11	0.94	0.03	0.47	0.77	0.57	0.57
Control Delay	21.3	44.8	1.0	88.7	11.6	2.6	92.1	22.5	9.3	57.8	11.5	11.5
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	21.3	44.8	1.0	88.7	11.6	2.6	92.1	22.5	9.3	57.8	11.5	11.5
LOS	C	D	A	F	B	A	F	C	A	E	B	B
Approach Delay		42.2			24.2			46.2			31.3	
Approach LOS		D			C			D			C	
Queue Length 50th (m)	5.9	138.1	0.0	18.9	26.0	0.0	21.0	1.1	0.0	40.4	4.7	4.7
Queue Length 95th (m)	9.9	144.8	0.0	#57.0	35.4	5.3	#35.9	1.3	4.3	#65.4	13.9	13.9
Internal Link Dist (m)		350.0			219.7			57.9			156.7	
Turn Bay Length (m)	20.0	60.0	20.0	60.0	60.0	60.0	140	315	346	293	502	502
Base Capacity (vph)	324	1339	361	134	1876	844	140	315	346	293	502	502
Stevation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.14	0.94	0.16	0.93	0.25	0.11	0.94	0.03	0.46	0.69	0.54	0.54
Intersection Summary												
Area Type:	CBD											
Cycle Length:	105.5											
Actuated Cycle Length:	103.2											
Natural Cycle:	100											
Control Type:	Semi Act-Uncoord											
Maximum v/c Ratio:	0.94											
Intersection Signal Delay:	36.4											
Intersection LOS: D												

Intersection Capacity Utilization 81.2% ICU Level of Service D  
 Analysis Period (min) 15  
 ~ Volume exceeds capacity, queue is theoretically infinite.  
 Queue shown is maximum after two cycles.  
 # 95th percentile volume exceeds capacity, queue may be longer.  
 Queue shown is maximum after two cycles.  
 m Volume for 95th percentile queue is metered by upstream signal.



Lanes, Volumes, Timings  
6: Trafalgar Road

2025 Total AM Peak Hour.syn  
05-04-2022

	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Turn Type	Prot	NA	Perm	Prot	NA	Perm	Prot	NA	Prot	NA	Prot	NA
Protected Phases	7	4	4	3	8	8	5	2				
Permitted Phases	7	4	4	3	8	8	5	2	1	6		
Detector Phase												
Switch Phase												
Minimum Initial (s)	7.0	10.0	10.0	7.0	10.0	10.0	7.0	20.0	7.0	20.0	20.0	20.0
Minimum Split (s)	12.0	37.0	37.0	12.0	37.0	37.0	12.0	39.0	12.0	39.0	39.0	39.0
Total Split (s)	23.0	55.0	55.0	12.0	44.0	44.0	12.0	41.0	32.0	61.0	61.0	61.0
Total Split (%)	16.4%	39.3%	39.3%	8.6%	31.4%	31.4%	8.6%	29.3%	22.9%	43.6%	43.6%	43.6%
Maximum Green (s)	18.0	50.0	50.0	7.0	37.0	37.0	7.0	34.0	27.0	54.0	54.0	54.0
Yellow Time (s)	3.0	3.0	3.0	3.0	4.0	4.0	3.0	4.0	3.0	4.0	4.0	4.0
All-Red Time (s)	2.0	2.0	2.0	2.0	3.0	3.0	2.0	3.0	2.0	3.0	3.0	3.0
Lost Time Adjust (s)	-1.0	-3.0	-3.0	-1.0	-3.0	-3.0	-1.0	-3.0	-1.0	-3.0	-3.0	-3.0
Total Lost Time (s)	4.0	2.0	2.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lead/Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	Lead	Lag	Lag	Lag
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	None	None	None	None	None	None	None	C-Max	None	C-Max	C-Max	C-Max
Walk Time (s)	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0
Flash Dont Walk (s)	11.0	11.0	11.0	11.0	11.0	11.0	11.0	11.0	11.0	11.0	11.0	11.0
Pedestrian Calls (#/hr)	0	0	0	0	0	0	0	0	0	0	0	0
Act Effct Green (s)	19.0	57.8	57.8	8.0	40.0	40.0	8.0	37.0	28.0	59.4	59.4	59.4
Actuated g/C Ratio	0.14	0.41	0.41	0.06	0.29	0.29	0.06	0.26	0.20	0.42	0.42	0.42
v/c Ratio	0.94	0.41	0.13	0.24	0.48	1.01	0.60	0.50	0.99	0.77	0.50	0.50
Control Delay	89.5	31.1	0.7	69.4	43.4	57.3	88.8	44.8	102.9	37.5	12.2	12.2
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	89.5	31.1	0.7	69.4	43.4	57.3	88.8	44.8	102.9	37.5	12.2	12.2
LOS	F	C	A	E	D	E	F	D	F	D	D	B
Approach Delay	51.5			52.0			49.9			57.3		
Approach LOS	D			D			D			E		
Queue Length 50th (m)	66.2	67.1	0.0	6.5	61.4	~117.8	17.7	59.3	106.6	90.4	26.0	26.0
Queue Length 95th (m)	#99.7	84.7	1.1	16.5	79.4	#206.0	#37.8	77.5	m#142.2	m128.8	m48.1	m48.1
Internal Link Dist (m)	70.0			292.9			147.0			242.5		
Turn Bay Length (m)	80.0	30.0	80.0			25.0			80.0			
Base Capacity (vph)	465	1433	715	97	1001	708	101	929	673	790	809	809
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.94	0.41	0.13	0.24	0.48	1.01	0.60	0.50	0.99	0.77	0.50	0.50

Intersection Summary

Area Type: Other  
Cycle Length: 140  
Actuated Cycle Length: 140  
Offset: 54.5 (39%), Referenced to phase 2:NBT and 6:SBT, Start of Green  
Natural Cycle: 120  
Control Type: Actuated-Coordinated  
Maximum v/c Ratio: 1.01  
Intersection Signal Delay: 53.6

Intersection LOS: D

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Lanes, Volumes, Timings  
7: GO Bus Terminal/Argus Rd & Cross Ave

2030 Background PM Peak Hour.syn  
05-04-2022

	GO1	GO2	GO3	GO4	GO5	GO6
Analysis Period (min)	15	15	15	15	15	15
Intersection Capacity Utilization	76.4%					
# 95th percentile volume exceeds capacity, queue may be longer.						
Queue shown is maximum after two cycles.						
Splits and Phases:	7: GO Bus Terminal/Argus Rd & Cross Ave					
GO1	17 s	46.5 s	13 s	29 s	52 s	53.5 s
GO2						
GO3						
GO4						
GO5						
GO6						

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Syncho 10 Report  
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HCM Signalized Intersection Capacity Analysis  
 5: Trafalgar Road & Cross Avenue/South Service Road

Lanes, Volumes, Timings  
 8: Cross Ave & Lyons Lane

2025 Total AM Peak Hour.syn  
 05-04-2022

2030 Background PM Peak Hour.syn  
 05-04-2022

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	492	94	125	47	94	134	131	1302	59	287	1506	461
Traffic Volume (vph)	492	94	125	47	94	134	131	1302	59	287	1506	461
Future Volume (vph)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Ideal Flow (vphpl)	4.0	4.0	1.5	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Total Lost time (s)	0.97	1.00	1.00	1.00	1.00	0.91	1.00	0.91	1.00	0.91	1.00	0.91
Lane Util. Factor	1.00	0.99	1.00	1.00	0.99	1.00	0.99	1.00	0.99	1.00	0.99	1.00
Frb. ped/bikes	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frb. ped/bikes	1.00	0.91	1.00	1.00	0.85	1.00	0.99	1.00	0.99	1.00	0.96	1.00
Flt Protected	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00
Satd. Flow (prot)	3213	1568	1745	1759	1531	1641	4962	1752	4808	1752	4808	1752
Flt Permitted	0.95	1.00	0.61	1.00	1.00	0.07	1.00	1.00	0.07	1.00	1.00	1.00
Satd. Flow (perm)	3213	1568	1122	1759	1531	124	4962	126	4808	126	4808	126
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	535	102	136	51	102	146	142	1415	64	312	1637	501
RTOR Reduction (vph)	0	34	0	0	0	128	0	3	0	0	37	0
Lane Group Flow (vph)	535	204	0	51	102	18	142	1476	0	312	2101	0
Conf. Peds. (#/hr)	1	4	4	4	4	1	10	3	0	52	52	10
Heavy Vehicles (%)	9%	6%	12%	3%	8%	4%	10%	3%	0%	3%	3%	4%
Turn Type	Prot	NA	pm+pt	NA	Perm	pm+pt	NA	pm+pt	NA	pm+pt	NA	NA
Protected Phases	7	4	3	3	8	8	5	2	2	1	6	6
Permitted Phases	21.0	28.4	24.1	14.5	14.5	67.2	55.6	83.5	67.9	83.5	67.9	83.5
Actuated Green, G (s)	24.0	31.4	30.1	17.5	17.5	67.2	58.6	83.5	67.9	83.5	67.9	83.5
Effective Green, g (s)	0.17	0.22	0.22	0.12	0.12	0.48	0.42	0.60	0.51	0.60	0.51	0.60
Actuated g/C Ratio	7.0	7.0	4.5	7.0	7.0	4.0	7.0	4.0	7.0	4.0	7.0	4.0
Clearance Time (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Vehicle Extension (s)	550	351	297	219	191	185	2076	352	2434	352	2434	352
Lane Grp Cap (vph)	c0.17	c0.13	0.02	0.06	0.06	0.30	c0.15	c0.44	c0.15	c0.44	c0.15	c0.44
v/s Ratio Prot	0.97	0.58	0.17	0.47	0.10	0.77	0.71	0.89	0.86	0.89	0.86	0.86
v/s Ratio Perm	57.7	48.4	44.4	56.9	54.2	32.5	33.7	42.9	30.3	42.9	30.3	42.9
Uniform Delay, d1	1.00	1.00	1.00	1.00	1.00	0.88	1.21	1.19	0.65	1.19	0.65	1.19
Progression Factor	31.3	2.4	0.3	1.6	0.2	9.8	1.1	2.8	0.4	2.8	0.4	2.8
Incremental Delay, d2	89.0	50.9	44.7	58.5	54.5	38.3	42.0	53.9	20.2	53.9	20.2	53.9
Delay (s)	F	D	D	E	D	D	D	D	D	D	D	C
Level of Service	77.2	E	54.2	D	D	D	D	41.7	D	D	D	24.5
Approach Delay (s)	E											
Approach LOS												
Intersection Summary												
HCM 2000 Control Delay												D
HCM 2000 Volume to Capacity ratio												16.0
Actuated Cycle Length (s)												E
Intersection Capacity Utilization												82.9%
Analysis Period (min)												15
c Critical Lane Group												

Baseline

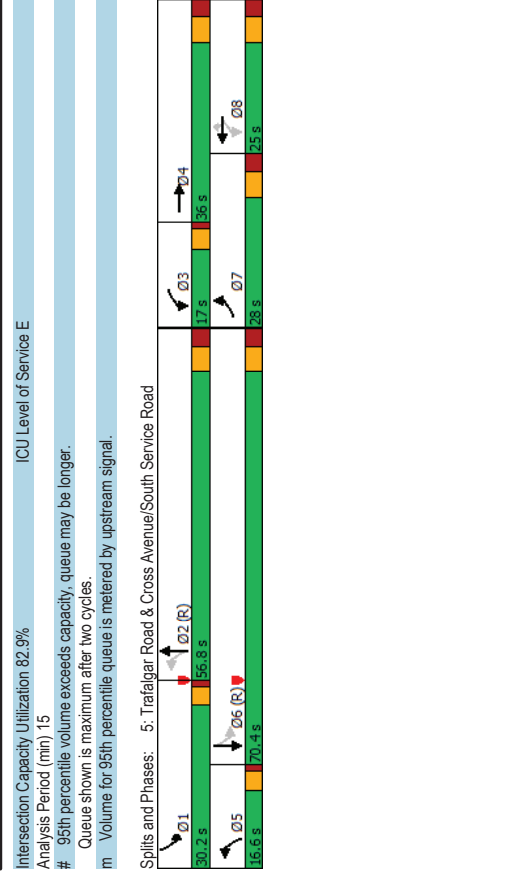
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Baseline

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Turn Type	Perm	NA	Perm	Perm	NA	Perm	Perm	NA	Perm	Perm	NA	Perm
Protected Phases	4		4	8	8	8	2	2	2	6	6	6
Detector Phase	4	4	4	8	8	8	2	2	2	6	6	6
Switch Phase												
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Minimum Split (s)	22.5	22.5	22.5	22.5	22.5	22.5	22.5	22.5	22.5	22.5	22.5	22.5
Total Split (s)	49.0	49.0	49.0	49.0	49.0	49.0	41.0	41.0	41.0	41.0	41.0	41.0
Total Split (%)	54.4%	54.4%	54.4%	54.4%	54.4%	54.4%	45.6%	45.6%	45.6%	45.6%	45.6%	45.6%
Maximum Green (s)	44.5	44.5	44.5	44.5	44.5	44.5	36.5	36.5	36.5	36.5	36.5	36.5
Yellow Time (s)	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5



Lead-Lag Optimize?

Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	None	None	None	None	None	None	Max	Max	Max	Max	Max
Walk Time (s)	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0
Flash Dont Walk (s)	11.0	11.0	11.0	11.0	11.0	11.0	11.0	11.0	11.0	11.0	11.0
Pedestrian Calls (#/hr)	0	0	0	0	0	0	0	0	0	0	0
Act Effct Green (s)	18.6	18.6	18.6	18.6	18.6	18.6	36.7	36.7	36.7	36.7	36.7
Actuated g/C Ratio	0.29	0.29	0.29	0.29	0.29	0.29	0.57	0.57	0.57	0.57	0.57
v/c Ratio	0.61	0.25	0.02	0.08	0.71	0.19	0.24	0.00	0.27	0.15	0.00
Control Delay	43.5	17.8	4.3	16.6	24.9	5.2	9.0	8.0	2.2	8.4	7.5
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	43.5	17.8	4.3	16.6	24.9	5.2	9.0	8.0	2.2	8.4	7.5
LOS	D	B	A	B	C	A	A	A	A	A	A
Approach Delay	23.7			22.2			5.0			6.3	
Approach LOS	C			C			A			A	
Queue Length 50th (m)	7.5	10.4	0.0	2.3	37.4	0.0	9.7	0.1	0.0	5.8	0.1
Queue Length 95th (m)	9.4	17.8	1.7	7.2	48.8	2.3	23.9	0.8	9.8	7.0	1.1
Internal Link Dist (m)	257.3			16.1			148.6			17.7	
Turn Bay Length (m)	60.0	60.0	60.0	60.0	60.0	60.0	60.0	60.0	60.0	60.0	60.0
Base Capacity (vph)	277	2063	995	722	2192	1016	723	955	918	728	955
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.25	0.10	0.01	0.03	0.29	0.09	0.24	0.00	0.27	0.15	0.00

Intersection Summary

Area Type:	CBD
Cycle Length: 90	
Actuated Cycle Length: 64.3	
Natural Cycle: 45	
Control Type: Semi Act-Uncoordinated	
Maximum v/c Ratio: 0.71	
Intersection Signal Delay: 16.4	Intersection LOS: B
Intersection Capacity Utilization 48.5%	ICU Level of Service A

Lanes, Volumes, Timings  
5: Trafalgar Road & Cross Avenue/South Service Road

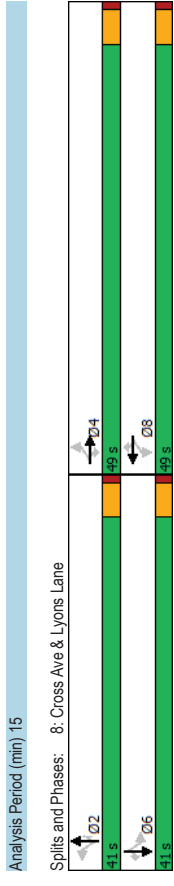
Lanes, Volumes, Timings  
8: Cross Ave & Lyons Lane

2030 Background PM Peak Hour.syn  
05-04-2022

2025 Total AM Peak Hour.syn  
05-04-2022

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Turn Type	Prot	NA	NA	pm-apt	NA	Perm	pm-apt	NA	pm-apt	NA	pm-apt	NA
Protected Phases	7	4	3	8	8	8	5	2	2	1	6	6
Permitted Phases	7	4	8	8	8	8	5	2	2	1	6	6
Detector Phase	7	4	3	8	8	8	5	2	2	1	6	6
Switch Phase												
Minimum Initial (s)	10.0	5.0	12.0	10.0	10.0	10.0	7.0	27.0	7.0	7.0	27.0	27.0
Minimum Split (s)	17.0	25.0	17.0	25.0	25.0	25.0	34.0	11.5	34.0	11.5	34.0	34.0
Total Split (s)	28.0	36.0	17.0	25.0	25.0	25.0	16.6	56.8	30.2	30.2	70.4	70.4
Total Split (%)	20.0%	25.7%	12.1%	17.9%	17.9%	11.9%	40.6%	21.6%	50.3%	21.6%	50.3%	50.3%
Maximum Green (s)	21.0	29.0	12.5	18.0	18.0	12.6	49.8	26.2	63.4	26.2	63.4	63.4
Yellow Time (s)	4.0	4.0	3.5	4.0	4.0	4.0	3.0	4.0	3.0	4.0	3.0	4.0
All-Red Time (s)	3.0	3.0	1.0	3.0	3.0	3.0	1.0	3.0	1.0	3.0	1.0	3.0
Lost Time Adjust (s)	-3.0	-3.0	-3.0	-3.0	-3.0	-3.0	-3.0	-3.0	-3.0	-3.0	-3.0	-3.0
Total Lost Time (s)	4.0	4.0	1.5	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lead/Lag	Lead	Lag	Lead	Lag	Lag	Lead	Lag	Lead	Lag	Lead	Lag	Lead
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	None	None	None	None	None	None	C-Max	None	C-Max	None	C-Max	None
Walk Time (s)	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0
Flash Dont Walk (s)	11.0	11.0	11.0	11.0	11.0	11.0	11.0	11.0	11.0	11.0	11.0	11.0
Pedestrian Calls (#/hr)	0	0	0	0	0	0	0	0	0	0	0	0
Act Effct Green (s)	24.0	31.4	34.1	16.6	16.6	16.6	71.1	59.5	87.4	71.8	71.8	71.8
Actuated g/C Ratio	0.17	0.22	0.24	0.12	0.12	0.12	0.51	0.42	0.62	0.51	0.51	0.51
v/c Ratio	0.97	0.62	0.15	0.49	0.43	0.76	0.70	0.88	0.88	0.85	0.85	0.85
Control Delay	89.5	47.9	32.4	65.2	6.8	43.1	42.5	49.1	19.9	49.1	19.9	19.9
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	89.5	47.9	32.4	65.2	6.8	43.1	42.5	49.1	19.9	49.1	19.9	19.9
LOS	F	D	C	E	A	D	D	D	D	D	D	B
Approach Delay	76.7	31.1	31.1	42.5	42.5	42.5	42.5	42.5	42.5	42.5	42.5	23.6
Approach LOS	E	C	C	D	D	D	D	D	D	D	D	C
Queue Length 50th (m)	80.8	53.2	10.2	28.3	0.0	26.5	115.2	63.4	55.0	63.4	55.0	55.0
Queue Length 95th (m)	#118.3	81.0	19.4	46.3	10.1	m34.8	m142.4	m59.6	m46.1	m59.6	m46.1	m46.1
Internal Link Dist (m)	219.7	214.9	214.9	242.5	242.5	242.5	242.5	242.5	242.5	242.5	242.5	247.0
Turn Bay Length (m)	25.0	25.0	25.0	50.0	50.0	50.0	50.0	50.0	50.0	50.0	50.0	25.0
Base Capacity (vph)	550	413	346	263	385	206	2113	388	2501	388	2501	2501
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.97	0.58	0.15	0.39	0.38	0.69	0.70	0.70	0.80	0.80	0.85	0.85
Intersection Summary	Intersection LOS: D											
Area Type:	Other											
Cycle Length:	140											
Actuated Cycle Length:	140											
Offset:	54.5 (39%), Referenced to phase 2:NBTL and 6:SBTL, Start of Green											
Natural Cycle:	100											
Control Type:	Actuated-Coordinated											
Maximum v/c Ratio:	0.97											
Intersection Signal Delay:	38.0											

Baseline Synchro 10 Report Page 14



Baseline Synchro 10 Report Page 27

HCM Signalized Intersection Capacity Analysis  
8: Cross Ave & Lyons Lane

2030 Background PM Peak Hour.syn  
05-04-2022

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	[Diagram showing lane configurations with arrows]											
Traffic Volume (vph)	35	193	7	23	542	54	158	1	226	48	2	26
Future Volume (vph)	35	193	7	23	542	54	158	1	226	48	2	26
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5
Lane Util. Factor	1.00	0.95	1.00	1.00	0.95	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frpb, ped/bikes	1.00	1.00	1.00	1.00	1.00	0.98	1.00	1.00	1.00	1.00	1.00	1.00
Flt	1.00	1.00	0.85	1.00	1.00	0.85	1.00	1.00	0.85	1.00	1.00	0.85
Flt Protected	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00
Satd. Flow (prot)	1388	2954	1425	1593	3154	1422	1593	1676	1425	1609	1676	1398
Flt Permitted	0.27	1.00	1.00	0.62	1.00	1.00	0.76	1.00	1.00	0.76	1.00	1.00
Satd. Flow (perm)	399	2954	1425	1040	3154	1422	1268	1676	1425	1282	1676	1398
Peak-hour factor, PHF	0.50	0.92	0.92	0.92	0.84	0.58	0.92	0.92	0.92	0.44	0.92	0.41
Adj. Flow (vph)	70	210	8	25	645	93	172	1	246	109	2	63
RTOR Reduction (vph)	0	0	6	0	0	66	0	0	106	0	0	27
Lane Group Flow (vph)	70	210	2	25	645	27	172	1	140	109	2	36
Conf. Peds. (#/hr)	1											
Heavy Vehicles (%)	17%	10%	2%	2%	3%	0%	2%	2%	2%	0%	2%	4%
Turn Type	Perm	NA	Perm	Perm	NA	Perm	Perm	NA	Perm	Perm	NA	Perm
Protected Phases	4		4	8		8		2		6		6
Permitted Phases	4		4	8		8		2		6		6
Actuated Green, G (s)	18.6	18.6	18.6	18.6	18.6	18.6	36.7	36.7	36.7	36.7	36.7	36.7
Effective Green, g (s)	18.6	18.6	18.6	18.6	18.6	18.6	36.7	36.7	36.7	36.7	36.7	36.7
Actuated g/C Ratio	0.29	0.29	0.29	0.29	0.29	0.29	0.57	0.57	0.57	0.57	0.57	0.57
Clearance Time (s)	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Lane Grp Cap (vph)	115	854	412	300	912	411	723	956	813	731	956	797
v/s Ratio Prot					c0.20							0.00
v/s Ratio Perm	0.18	0.01	0.02	0.02	0.02	c0.14	0.02	0.10	0.17	0.08	0.15	0.03
v/c Ratio	0.61	0.25	0.01	0.08	0.71	0.07	0.24	0.00	0.17	0.15	0.00	0.05
Uniform Delay, d1	19.7	17.5	16.3	16.6	20.4	16.6	6.9	5.9	6.6	6.5	5.9	6.1
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	8.8	0.2	0.0	0.1	2.5	0.1	0.8	0.0	0.5	0.4	0.0	0.1
Delay (s)	28.5	17.6	16.3	16.8	22.9	16.6	7.6	5.9	7.0	6.9	5.9	6.2
Level of Service	C	B	B	B	C	B	A	A	A	A	A	A
Approach Delay (s)	20.2				22.0			7.3			6.6	
Approach LOS	C				C			A			A	
Intersection Summary	[Summary table with columns for delay, LOS, etc.]											
HCM 2000 Control Delay				16.3			HCM 2000 Level of Service				B	
HCM 2000 Volume to Capacity ratio			0.40									
Actuated Cycle Length (s)			64.3				Sum of lost time (s)				9.0	
Intersection Capacity Utilization			48.5%				ICU Level of Service				A	
Analysis Period (min)			15									
C Critical Lane Group												

Lanes, Volumes, Timings  
5: Trafalgar Road & Cross Avenue/South Service Road

2025 Total AM Peak Hour.syn  
05-04-2022

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	[Diagram showing lane configurations with arrows]											
Traffic Volume (vph)	492	94	125	47	94	134	131	1302	59	287	1506	461
Future Volume (vph)	492	94	125	47	94	134	131	1302	59	287	1506	461
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (m)	25.0	0.0	0.0	25.0	0.0	0.0	50.0	0.0	0.0	25.0	0.0	0.0
Storage Lanes	2											
Taper Length (m)	7.5			7.5			7.5			7.5		
Lane Util. Factor	0.97	1.00	1.00	1.00	1.00	1.00	1.00	0.91	0.91	1.00	0.91	0.91
Ped Bike Factor	1.00	0.99		0.99		0.99		0.99		0.99		0.99
Flt	0.914					0.850		0.994		0.965		
Flt Protected	0.950			0.950		0.950		0.950		0.950		
Satd. Flow (prot)	3213	1588	0	1752	1759	1553	1641	4965	0	1752	4809	0
Flt Permitted	0.950			0.611		0.072		0.072		0.068		
Satd. Flow (perm)	3206	1588	0	1120	1759	1531	124	4965	0	125	4809	0
Right Turn on Red	Yes		Yes		Yes		Yes	Yes		Yes		Yes
Satd. Flow (RTOR)	44				183		6			75		
Link Speed (k/h)	50				50		50			50		
Link Distance (m)	243.7				238.9		266.5			271.0		
Travel Time (s)	17.5				17.2		19.2			19.5		
Conf. Peds. (#/hr)	1			4			1	10		52		10
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	9%	6%	12%	3%	8%	4%	10%	3%	0%	3%	3%	4%
Adj. Flow (vph)	535	102	136	51	102	146	142	1415	64	312	1637	501
Shared Lane Traffic (%)												
Lane Group Flow (vph)	535	238	0	51	102	146	142	1479	0	312	2138	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Right	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width (m)	7.2			7.2			3.6			3.6		
Link Offset (m)	0.0			0.0			0.0			0.0		
Crosswalk Width (m)	4.8			4.8			4.8			4.8		
Two way Left Turn Lane												
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (k/h)	25		15		25		15		25		15	
Number of Detectors	1		2		1		1		2		1	
Detector Template	Left	Thru	Thru	Left	Thru	Right	Left	Thru	Left	Thru	Thru	
Leading Detector (m)	2.0	10.0	2.0	10.0	2.0	10.0	2.0	10.0	2.0	10.0	2.0	10.0
Trailing Detector (m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Position (m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Size (m)	2.0	0.6		2.0	0.6		2.0	0.6		2.0	0.6	
Detector 1 Type	Ch+Ex	Ch+Ex		Ch+Ex	Ch+Ex		Ch+Ex	Ch+Ex		Ch+Ex	Ch+Ex	
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Queue (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Delay (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 2 Position (m)				9.4					9.4			9.4
Detector 2 Size (m)				0.6					0.6			0.6
Detector 2 Type				Ch+Ex					Ch+Ex			Ch+Ex
Detector 2 Channel												
Detector 2 Extend (s)				0.0			0.0		0.0		0.0	

HCM Unsignalized Intersection Capacity Analysis  
 4: Trafalgar Road & Argus Road/QEW EB On Ramp

2025 Total AM Peak Hour.syn  
 05-04-2022

	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	0	0	37	0	0	0	0	1311	629	0	2341	775
Traffic Volume (veh/h)	0	0	37	0	0	0	0	1311	629	0	2341	775
Future Volume (Veh/h)	0	0	37	0	0	0	0	1311	629	0	2341	775
Sign Control	Stop											
Grade	0%											
Peak Hour Factor	0.92											
Hourly flow rate (vph)	0	0	40	0	0	0	0	1425	684	0	2545	842
Pedestrians	24											
Lane Width (m)	3.6											
Walking Speed (m/s)	1.2											
Percent Blockage	2											
Right turn flare (veh)	None											
Median type	None											
Upstream signal (m)	271											
pX, platoon unblocked	0.58											
vC, conflicting volume	3465											
vC1, stage 1 conf/vol	4415											
vC2, stage 2 conf/vol	2273											
vCu, unblocked vol	4836											
tC, single (s)	1030	2681	0	0	3413	0	2340	0	914			
tC, 2 stage (s)	7.5	6.5	7.1	7.5	6.5	6.9	4.1	0	4.1			
tF (s)	3.5	4.0	3.4	3.5	4.0	3.3	2.2	0	2.2			
p0 queue free %	100	100	92	100	100	100	100	100	100			
cM capacity (veh/h)	106	13	526	538	4	936	106	647				

Direction	Lane #	EB1	NB1	NB2	NB3	NB4	SB1	SB2	SB3
Volume Total		40	475	475	475	684	1018	1351	
Volume Left		0	0	0	0	0	0	0	
Volume Right		40	0	0	0	684	0	0	
GSV		526	1700	1700	1700	1700	1700	1700	
Volume to Capacity		0.08	0.28	0.28	0.28	0.40	0.60	0.79	
Queue Length 95th (m)		2.0	0.0	0.0	0.0	0.0	0.0	0.0	
Control Delay (s)		12.4	0.0	0.0	0.0	0.0	0.0	0.0	
Lane LOS		B	B	B	B	B	B	B	
Approach Delay (s)		12.4	0.0			0.0			
Approach LOS		B	B			B			

Intersection Summary	
Average Delay	0.1
Intersection Capacity Utilization	73.1%
Analysis Period (min)	15
ICU Level of Service D	

Lanes, Volumes, Timings  
 10: Lyons Lane & South Service Road

2030 Background PM Peak Hour.syn  
 05-04-2022

	EBL	EBT	EBR	WBL	WBT	WBR	SBL	SBR	
Lane Configurations	4	6	1	6	15	6			
Traffic Volume (vph)	4	6	1	6	15	6			
Future Volume (vph)	4	6	1	6	15	6			
Ideal Flow (vehpl)	1900	1900	1900	1900	1900	1900			
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00			
Ped Bike Factor	0.904								
Fit	0.974								
Fit Protected	0								
Satd. Flow (prot)	1666								
Fit Permitted	0.974								
Satd. Flow (perm)	1666								
Link Speed (k/h)	50								
Link Distance (m)	60.5								
Travel Time (s)	4.4								
Conf. Peds. (#/hr)	7								
Peak Hour Factor	0.38	0.62	0.25	0.62	0.50	0.42			
Heavy Vehicles (%)	0%	0%	0%	22%	0%	0%			
Adj. Flow (vph)	11	10	4	10	30	14			

Shared Lane Traffic (%)	Lane Group Flow (vph)	Enter Blocked Intersection	Lane Alignment	Median Width(m)	Link Offset(m)	Crosswalk Width(m)	Two way Left Turn Lane	Headway Factor	Turning Speed (k/h)	Sign Control
0	21	No	No	No	No	No	0	1.14	1.14	1.14
No	0	Left	Left	0.0	0.0	3.6	1.14	1.14	1.14	1.14
Left	0	Left	Left	0.0	0.0	4.8	Free	Free	Free	Free
Right	0	Right	Right	0.0	0.0	4.8	Free	Free	Free	Free
0.0	0.0	0.0	0.0	0.0	0.0	0.0	Free	Free	Free	Free
4.8	4.8	4.8	4.8	4.8	4.8	4.8	Free	Free	Free	Free
1.14	1.14	1.14	1.14	1.14	1.14	1.14	14	14	14	14
24	24	24	24	24	24	24	Stop	Stop	Stop	Stop

Intersection Summary	
Area Type:	CBD
Control Type:	Unsignalized
Intersection Capacity Utilization	15.4%
Analysis Period (min)	15
ICU Level of Service A	

HCM Unsignalized Intersection Capacity Analysis  
 10: Lyons Lane & South Service Road

2030 Background PM Peak Hour.syn  
 05-04-2022



Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	4	6	1	6	15	6
Traffic Volume (veh/h)	4	6	1	6	15	6
Future Volume (Veh/h)	4	6	1	6	15	6
Sign Control	Free	Free	0%	Stop	0%	0%
Grade	0%	0%	0%	0%	0%	0%
Peak Hour Factor	0.38	0.62	0.25	0.62	0.50	0.42
Hourly flow rate (vph)	11	10	4	10	30	14
Pedestrians					7	
Lane Width (m)					3.6	
Walking Speed (m/s)					1.2	
Percent Blockage					1	
Right turn flare (veh)					None	
Median type					None	
Median storage (veh)					393	
Upstream signal (m)					393	
pX, platoon unblocked						
vC, conflicting volume	21				48	16
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCU, unblocked vol	21				48	16
tC, single (s)	4.1				6.4	6.2
tC, 2 stage (s)						
tF (s)	2.2				3.5	3.3
p0 queue free %	99				97	99
cM capacity (veh/h)	1599				954	1063
Direction_Lane #	EB 1	WB 1	SB 1			
Volume Total	21	14	44			
Volume Left	11	0	30			
Volume Right	0	10	14			
GSH	1599	1700	986			
Volume to Capacity	0.01	0.01	0.04			
Queue Length 95th (m)	0.2	0.0	1.1			
Control Delay (s)	3.8	0.0	8.8			
Lane LOS	A	A	A			
Approach Delay (s)	3.8	0.0	8.8			
Approach LOS	A	A	A			
Intersection Summary						
Average Delay	5.9					
Intersection Capacity Utilization	15.4%					
ICU Level of Service	A					
Analysis Period (min)	15					

Lanes, Volumes, Timings  
 4: Trafalgar Road & Argus Road/QEW EB On Ramp

2025 Total AM Peak Hour.syn  
 05-04-2022



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	0	0	37	0	0	0	0	1311	629	0	2341	775
Traffic Volume (vph)	0	0	37	0	0	0	0	1311	629	0	2341	775
Future Volume (vph)	0	0	37	0	0	0	0	1311	629	0	2341	775
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	70.0	0.0	0.0	0.0	0.0
Storage Lanes	0	0	1	0	0	0	0	1	0	0	0	0
Taper Length (m)	7.5			7.5			7.5			7.5		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.91	1.00	1.00	0.91	0.91
Ped Bike Factor			0.865					0.850				0.963
Fit			Protected					Protected				Protected
Satd. Flow (prot)	0	0	1522	0	0	0	0	5085	1568	0	4921	0
Fit Permitted			Protected					Protected				Protected
Satd. Flow (perm)	0	0	1522	0	0	0	0	5085	1568	0	4921	0
Link Speed (k/h)			50					50				50
Link Distance (m)			165.2					271.0				50.7
Travel Time (s)			11.9					19.5				3.7
Confl. Peds. (#/hr)												24
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	0%	0%	8%	0%	2%	0%	0%	2%	3%	0%	2%	0%
Adj. Flow (vph)	0	0	40	0	0	0	0	1425	684	0	2545	842
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	0	40	0	0	0	0	1425	684	0	3387	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	3.6	0.0	0.0	3.6	0.0
Link Offset(m)								0.0			0.0	0.0
Crosswalk Width(m)			4.8					4.8			4.8	
Two way Left Turn Lane												
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (k/h)	25	15	25	15	25	15	25	15	25	15	25	15
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
Intersection Summary												
Area Type:	Other											
Control Type:	Unsignalized											
Intersection Capacity Utilization	73.1%											
ICU Level of Service	D											
Analysis Period (min)	15											



Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	↔	↔	↔	↔	↔	↔
Traffic Volume (vph)	963	707	0	1311	2409	0
Future Volume (vph)	963	707	0	1311	2409	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0
Lane Util. Factor	0.97	1.00	0.91	0.91	1.00	0.91
Frt	1.00	0.85	1.00	1.00	1.00	1.00
Flt Protected	0.95	1.00	1.00	1.00	1.00	1.00
Satd. Flow (prot)	3400	1599	4893	4988	4988	4988
Flt Permitted	0.95	1.00	1.00	1.00	1.00	1.00
Satd. Flow (perm)	3400	1599	4893	4988	4988	4988
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	1047	768	0	1425	2618	0
RTOR Reduction (vph)	0	1	0	0	0	0
Lane Group Flow (vph)	1047	767	0	1425	2618	0
Heavy Vehicles (%)	3%	1%	0%	6%	4%	0%
Turn Type	Perm	Perm	NA	NA	NA	NA
Protected Phases			2	6	6	
Permitted Phases	4	4				
Actuated Green, G (s)	59.0	59.0	67.0	67.0	67.0	
Effective Green, g (s)	62.0	62.0	70.0	70.0	70.0	
Actuated g/C Ratio	0.44	0.44	0.50	0.50	0.50	
Clearance Time (s)	7.0	7.0	7.0	7.0	7.0	
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	
Lane Grp Cap (vph)	1505	708	2446	2494	2494	
v/s Ratio Prot			0.29	c0.52		
v/s Ratio Perm	0.31	c0.48				
v/c Ratio	0.70	1.08	0.58	1.05	1.05	
Uniform Delay, d1	31.4	39.0	24.7	35.0	35.0	
Progression Factor	1.00	1.00	1.83	0.89	0.89	
Incremental Delay, d2	1.4	58.9	0.7	23.7	23.7	
Delay (s)	32.8	97.9	45.7	54.7	54.7	
Level of Service	C	F	D	D	D	
Approach Delay (s)	60.3		45.7	54.7	54.7	
Approach LOS	E		D	D	D	

**Intersection Summary**

HCM 2000 Control Delay	54.3	HCM 2000 Level of Service	D
HCM 2000 Volume to Capacity ratio	1.09		
Actuated Cycle Length (s)	140.0	Sum of lost time (s)	11.0
Intersection Capacity Utilization	118.3%	ICU Level of Service	H
Analysis Period (min)	15		

C Critical Lane Group

Lane Group	EBL	WBT	WBR	SBL	SBR
Lane Configurations	↔	↔	↔	↔	↔
Traffic Volume (vph)	12	9	327	96	15
Future Volume (vph)	12	9	327	96	15
Ideal Flow (vphpl)	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor			0.963		0.925
Frt			0.971		0.978
Flt Protected	0	1578	1535	0	1547
Satd. Flow (prot)	0	1578	1535	0	1547
Flt Permitted	0	1578	1535	0	1547
Satd. Flow (perm)	0	1578	1535	0	1547
Link Speed (k/h)	50	50	50	50	50
Link Distance (m)	177.7	165.2	103.5		
Travel Time (s)	12.8	11.9	7.5		
Conf. Peds. (#/hr)			5		
Peak Hour Factor	0.45	0.87	0.68	0.75	0.65
Heavy Vehicles (%)	0%	13%	10%	0%	0%
Adj. Flow (vph)	27	18	376	141	20
Shared Lane Traffic (%)					
Lane Group Flow (vph)	0	45	517	0	45
Enter Blocked Intersection	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Right
Median Width(m)	0.0	0.0	3.6		
Link Offset(m)	0.0	0.0	0.0		
Crosswalk Width(m)	4.8	4.8	4.8		
Two way Left Turn Lane					
Headway Factor	1.14	1.14	1.14	1.14	1.14
Turning Speed (k/h)	24		14	24	14
Sign Control	Free	Free	Free	Stop	Stop

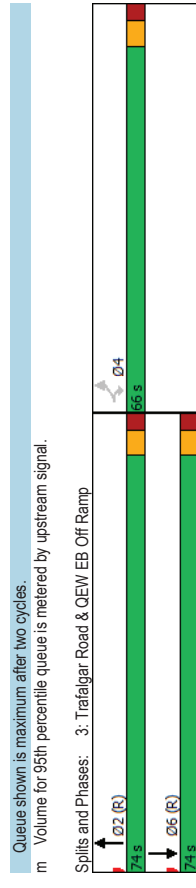
**Intersection Summary**

Area Type: CBD  
 Control Type: Unsignalized  
 Intersection Capacity Utilization 35.6%  
 Analysis Period (min) 15  
 ICU Level of Service A

HCM Unsignalized Intersection Capacity Analysis 2030 Background PM Peak Hour.syn  
 11: Argus Rd & South Service Rd 05-04-2022

Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	4	1	1	1	1	1
Traffic Volume (veh/h)	12	9	327	96	15	16
Future Volume (Veh/h)	12	9	327	96	15	16
Sign Control	Free	Free	Free	Stop	Stop	0%
Grade	0%	0%	0%	0%	0%	0%
Peak Hour Factor	0.45	0.50	0.87	0.68	0.75	0.65
Hourly flow rate (vph)	27	18	376	141	20	25
Pedestrians			5			
Lane Width (m)			3.6			
Walking Speed (m/s)			1.2			
Percent Blockage			0			
Right turn flare (veh)			None			
Median type			None			
Median storage (veh)			358			
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume			517		524	446
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol			517		524	446
tC, single (s)			4.1		6.4	6.2
tC, 2 stage (s)						
tF (s)			2.2		3.5	3.3
p0 queue free %			97		96	96
cM capacity (veh/h)			1059		502	616
Direction_Lane #	EB 1	WB 1	SB 1			
Volume Total	45	517	45			
Volume Left	27	0	20			
Volume Right	0	141	25			
GSH	1059	1700	560			
Volume to Capacity	0.03	0.30	0.08			
Queue Length 95th (m)	0.6	0.0	2.1			
Control Delay (s)	5.2	0.0	12.0			
Lane LOS	A	B	B			
Approach Delay (s)	5.2	0.0	12.0			
Approach LOS	B	B	B			
Intersection Summary						
Average Delay			1.3			
Intersection Capacity Utilization			35.6%		ICU Level of Service	A
Analysis Period (min)			15			

Lanes, Volumes, Timings 2025 Total AM Peak Hour.syn  
 3: Trafalgar Road & QEW EB Off Ramp 05-04-2022



Queue shown is maximum after two cycles.  
 m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 3: Trafalgar Road & QEW EB Off Ramp

Lanes, Volumes, Timings  
3: Trafalgar Road & QEW EB Off Ramp

Lanes, Volumes, Timings  
1: Trafalgar Road & North Service Road/QEW WB Off Ramp

2025 Total AM Peak Hour.syn  
05-04-2022

Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Minimum Initial (s)	10.0	10.0	29.0	29.0	29.0	29.0
Minimum Split (s)	38.0	38.0	36.0	36.0	36.0	36.0
Total Split (s)	66.0	66.0	74.0	74.0	74.0	74.0
Total Split (%)	47.1%	47.1%	52.9%	52.9%	52.9%	52.9%
Maximum Green (s)	59.0	59.0	67.0	67.0	67.0	67.0
Yellow Time (s)	4.0	4.0	4.0	4.0	4.0	4.0
All-Red Time (s)	3.0	3.0	3.0	3.0	3.0	3.0
Lost Time Adjust (s)	-3.0	-3.0	-3.0	-3.0	-3.0	-3.0
Total Lost Time (s)	4.0	4.0	4.0	4.0	4.0	4.0
Lead/Lag						
Lead-Lag Optimize?						
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	None	None	C-Max	C-Max	C-Max	C-Max
Walk Time (s)	7.0	7.0	7.0	7.0	7.0	7.0
Flash Dont Walk (s)	11.0	11.0	11.0	11.0	11.0	11.0
Pedestrian Calls (#/hr)	0	0	0	0	0	0
Act Effct Green (s)	62.0	62.0	70.0	70.0	70.0	70.0
Actuated g/C Ratio	0.44	0.44	0.50	0.50	0.50	0.50
v/c Ratio	0.70	1.08	0.58	0.58	1.05	1.05
Control Delay	34.4	96.4	46.1	46.1	54.6	54.6
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	34.4	96.4	46.1	46.1	54.6	54.6
LOS	C	F	D	D	D	D
Approach	C	F	D	D	D	D
Approach LOS	E	D	D	D	D	D
Queue Length 50th (m)	124.8	~260.5	159.3	~302.7	159.3	~302.7
Queue Length 95th (m)	150.0	#330.8	m173.0	m196.9	m173.0	m196.9
Internal Link Dist (m)	47.3		26.7	162.0	26.7	162.0
Turn Bay Length (m)						
Base Capacity (vph)	1505	708	2446	2494	2446	2494
Starvation Cap Reductn	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	0.70	1.08	0.58	0.58	1.05	1.05
Intersection Summary						
Area Type:	Other					
Cycle Length:	140					
Actuated Cycle Length:	140					
Offset: 0 (0%):	Referenced to phase 2:NBT and 6:SBT, Start of Green					
Natural Cycle:	110					
Control Type:	Actuated-Coordinated					
Maximum v/c Ratio:	1.08					
Intersection Signal Delay:	54.4					
Intersection Capacity Utilization:	118.3%					
Analysis Period (min):	15					
~ Volume exceeds capacity. queue is theoretically infinite.						
# 95th percentile volume exceeds capacity. queue may be longer.						

2035 Background AM Peak Hour.syn  
05-04-2022

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	1	0	1	1	1	1	1	1	1	1	1	1
Traffic Volume (vph)	1	0	196	577	42	249	0	1956	515	0	2266	8
Future Volume (vph)	1	0	196	577	42	249	0	1956	515	0	2266	8
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (m)	50.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	70.0	0.0	0.0	0.0
Storage Lanes	1	1	1	1	1	1	0	1	1	0	1	1
Taper Length (m)	7.5			7.5			7.5			7.5		
Lane Util. Factor	1.00	1.00	1.00	1.00	0.95	0.95	1.00	0.91	1.00	1.00	0.91	1.00
Ped Bike Factor									0.96			0.95
Flt			0.850		0.892		0.850		0.850		0.850	
Flt Protected			0.950		0.950		0.950		0.950		0.950	
Satd. Flow (prot)	1805	0	1568	1719	1453	1447	0	4940	1538	0	5036	1615
Flt Permitted	0.950			0.950								
Satd. Flow (perm)	1805	0	1568	1719	1453	1447	0	4940	1478	0	5036	1533
Right Turn on Red	Yes		Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Satd. Flow (RTOR)	31		98	154				50	323		50	70
Link Speed (k/h)	50		50					145.2			209.2	
Link Distance (m)	120.4		165.1					10.5			15.1	
Travel Time (s)	8.7		11.9					8			5	
Confli. Peds. (#/hr)									5			8
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	0%	0%	3%	5%	23%	6%	0%	5%	5%	0%	3%	0%
Adj. Flow (vph)	1	0	213	627	46	271	0	2126	560	0	2463	9
Shared Lane Traffic (%)							43%					
Lane Group Flow (vph)	1	0	213	627	163	154	0	2126	560	0	2463	9
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width (m)	3.6		3.6		3.6		0.0	0.0	0.0	0.0	0.0	0.0
Link Offset (m)	0.0		0.0		0.0		0.0	0.0	0.0	0.0	0.0	0.0
Crosswalk Width (m)	4.8		4.8		4.8		4.8		4.8		4.8	
Two way Left Turn Lane												
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (k/h)	25	15	25	15	25	15	25	15	25	15	25	15
Number of Detectors	1	1	1	2	1	2	1	2	1	2	1	1
Detector Template	Left	Right	Left	Thru	Right	Thru	Right	Thru	Right	Thru	Right	Right
Leading Detector (m)	2.0	2.0	2.0	10.0	2.0	10.0	2.0	10.0	2.0	10.0	2.0	2.0
Trailing Detector (m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Position (m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Size (m)	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
Detector 1 Type	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 2 Position (m)				9.4			9.4				9.4	
Detector 2 Size (m)				0.6			0.6				0.6	
Detector 2 Type				Ch+Ex			Ch+Ex				Ch+Ex	
Detector 2 Channel												
Detector 2 Extend (s)				0.0			0.0				0.0	



Lanes, Volumes, Timings  
1: Trafalgar Road & North Service Road/QEW WB Off Ramp

Lanes, Volumes, Timings  
3: Trafalgar Road & QEW EB Off Ramp

2035 Background AM Peak Hour.syn  
05-04-2022

2025 Total AM Peak Hour.syn  
05-04-2022

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Turn Type	7											
Protected Phases												
Permitted Phases	7			8			2					
Detector Phase	7	4	8	8	8	8	2	2	2	2	6	6
Switch Phase												
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Minimum Split (s)	23.0	38.0	38.0	38.0	38.0	38.0	35.0	35.0	35.0	35.0	35.0	35.0
Total Split (s)	23.0	74.0	51.0	51.0	51.0	66.0	66.0	66.0	66.0	66.0	66.0	66.0
Total Split (%)	16.4%	52.9%	36.4%	36.4%	36.4%	47.1%	47.1%	47.1%	47.1%	47.1%	47.1%	47.1%
Maximum Green (s)	18.0	67.0	44.0	44.0	44.0	59.0	59.0	59.0	59.0	59.0	59.0	59.0
Yellow Time (s)	3.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
All-Red Time (s)	2.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Lost Time Adjust (s)	-1.0	-3.0	-3.0	-3.0	-3.0	-3.0	-3.0	-3.0	-3.0	-3.0	-3.0	-3.0
Total Lost Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lead/Lag	Lead	Lag	Lag	Lag	Lag	Lag	Lag	Lag	Lag	Lag	Lag	Lag
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	None	None	None	None	None	None	C-Max	C-Max	C-Max	C-Max	C-Max	C-Max
Walk Time (s)		7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0
Flash Dont Walk (s)		11.0	11.0	11.0	11.0	11.0	11.0	11.0	11.0	11.0	11.0	11.0
Pedestrian Calls (#/hr)		0	0	0	0	0	0	0	0	0	0	0
Act Effct Green (s)	6.7	69.1	66.9	66.9	66.9	62.9	62.9	62.9	62.9	62.9	62.9	62.9
Actuated g/C Ratio	0.05	0.49	0.48	0.48	0.48	0.45	0.45	0.45	0.45	0.45	0.45	0.45
v/c Ratio	0.01	0.27	0.76	0.22	0.20	0.96	0.67	1.09	0.01	1.09	0.01	0.01
Control Delay	64.0	18.6	38.2	10.1	4.0	35.5	12.1	85.1	0.0	85.1	0.0	0.0
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	64.0	18.6	38.2	10.1	4.0	35.5	12.1	85.1	0.0	85.1	0.0	0.0
LOS	E	B	D	B	A	D	B	D	B	F	A	A
Approach Delay	18.8			27.8			30.6			84.8		
Approach LOS	B			C			C			F		
Queue Length 50th (m)	0.3	30.5	145.0	10.5	0.0	122.2	31.3	~290.1	0.0	~290.1	0.0	0.0
Queue Length 95th (m)	2.3	47.2	#236.1	28.7	14.5	#254.6	61.6	#326.4	0.0	#326.4	0.0	0.0
Internal Link Dist (m)		96.4		141.1		121.2		185.2		185.2		
Turn Bay Length (m)	50.0					70.0						
Base Capacity (vph)	244	799	821	745	771	2218	841	2261	726	2261	726	726
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.00	0.27	0.76	0.22	0.20	0.96	0.67	1.09	0.01	1.09	0.01	0.01
Intersection Summary												
Area Type:	Other											
Actuated Cycle Length:	140											
Offset: 0 (0%):	Referenced to phase 2:NBT and 6:SBT, Start of Green											
Natural Cycle:	150											
Control Type:	Actuated-Coordinated											
Maximum v/c Ratio:	1.09											
Intersection Signal Delay:	51.0											
Intersection LOS:	D											

Baseline  
Syncho 10 Report  
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Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	EBL	EBR	NBL	NBT	SBT	SBR
Traffic Volume (vph)	963	707	0	1311	2409	0
Future Volume (vph)	963	707	0	1311	2409	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	0.97	1.00	1.00	0.91	0.91	1.00
Flt Protected	0.950					
Satd. Flow (prot)	3400	1599	0	4893	4988	0
Flt Permitted	0.950					
Satd. Flow (perm)	3400	1599	0	4893	4988	0
Right Turn on Red	Yes					Yes
Satd. Flow (RTOR)	50	1		50	50	
Link Speed (k/h)	71.3			50.7	186.0	
Travel Time (s)	5.1			3.7	13.4	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	3%	1%	0%	6%	4%	0%
Adj. Flow (vph)	1047	768	0	1425	2618	0
Shared Lane Traffic (%)						
Lane Group Flow (vph)	1047	768	0	1425	2618	0
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Left	Left	Right
Median Width (m)	7.2			0.0	0.0	
Link Offset (m)	0.0	0.0	0.0	0.0	0.0	
Crosswalk Width (m)	4.8			4.8	4.8	
Two way Left Turn Lane						
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (k/h)	25	15	25	2	2	15
Number of Detectors	1	1				
Detector Template	Left	Right	Thru	Thru	Thru	
Leading Detector (m)	2.0	2.0	10.0	10.0	10.0	
Trailing Detector (m)	0.0	0.0	0.0	0.0	0.0	
Detector 1 Position (m)	0.0	0.0	0.0	0.0	0.0	
Detector 1 Size (m)	2.0	2.0	0.6	0.6	0.6	
Detector 1 Type	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	
Detector 1 Channel						
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	
Detector 2 Position (m)				9.4	9.4	
Detector 2 Size (m)				0.6	0.6	
Detector 2 Type				Ch+Ex	Ch+Ex	
Detector 2 Channel						
Detector 2 Extend (s)				0.0	0.0	
Turn Type	Perm	Perm	Perm	NA	NA	NA
Protected Phases				2	6	
Permitted Phases	4	4				
Detector Phase	4	4				
Switch Phase						

Baseline  
Syncho 10 Report  
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HCM Unsignalized Intersection Capacity Analysis  
 2: Trafalgar Road & QEW SB On Ramps

Lanes, Volumes, Timings  
 1: Trafalgar Road & North Service Road/QEW WB Off Ramp

2025 Total AM Peak Hour.syn  
 05-04-2022

2035 Background AM Peak Hour.syn  
 05-04-2022



Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations				↑↑↑	↑↑↑	↑
Traffic Volume (veh/h)	0	0	0	2274	2409	1856
Future Volume (Veh/h)	0	0	0	2274	2409	1856
Sign Control	Stop	Free	Free	Free	Free	Free
Grade	0%	0%	0%	0%	0%	0%
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	0	0	0	2472	2618	2017
Pedestrians						
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)				None	None	
Median type						
Median storage (veh)				186	145	
Upstream signal (m)						
pX, platoon unblocked	0.57	0.37	0.37			
vC, conflicting volume	3442	873	2618			
vC1, stage 1 conf/vol						
vC2, stage 2 conf/vol						
vCu, unblocked vol	0	0	0			
tC, single (s)	6.8	6.9	4.1			
tC, 2 stage (s)						
tF (s)	3.5	3.3	2.2			
p0 queue free %	100	100	100			
cM capacity (veh/h)	585	406	608			
Direction_Lane #	NB1	NB2	NB3	SB1	SB2	SB3
Volume Total	824	824	824	873	873	873
Volume Left	0	0	0	0	0	0
Volume Right	0	0	0	0	0	0
GSH	1700	1700	1700	1700	1700	1700
Volume to Capacity	0.48	0.48	0.48	0.51	0.51	0.51
Queue Length 95th (m)	0.0	0.0	0.0	0.0	0.0	0.0
Control Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0
Lane LOS						
Approach Delay (s)	0.0			0.0		
Approach LOS						
Intersection Summary						
Average Delay	0.0					
Intersection Capacity Utilization	118.3%					
ICU Level of Service	H					
Analysis Period (min)	15					

Intersection Capacity Utilization 97.9%

Analysis Period (min) 15

ICU Level of Service F

~ Volume exceeds capacity, queue is theoretically infinite.

Queue shown is maximum after two cycles.

# 95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

Spills and Phases: 1: Trafalgar Road & North Service Road/QEW WB Off Ramp



HCM Signalized Intersection Capacity Analysis  
 1: Trafalgar Road & North Service Road/QEW WB Off Ramp

2035 Background AM Peak Hour.syn  
 05-04-2022

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	1	0	196	577	42	249	0	1956	515	0	2266	8
Traffic Volume (vph)	1	0	196	577	42	249	0	1956	515	0	2266	8
Future Volume (vph)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Ideal Flow (vphpl)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Total Lost time (s)	1.00	1.00	1.00	0.95	0.95	0.95	1.00	0.91	1.00	0.91	1.00	1.00
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.96	1.00	0.95	1.00
Flt Protected	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Flt Permitted	0.95	1.00	0.89	0.89	0.85	1.00	0.85	1.00	0.85	1.00	0.85	1.00
Satd. Flow (prot)	1805	1568	1719	1454	1447	4940	1478	5036	1533	5036	1533	1533
Flt Permitted	0.95	1.00	0.95	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Satd. Flow (perm)	1805	1568	1719	1454	1447	4940	1478	5036	1533	5036	1533	1533
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	1	0	213	627	46	271	0	2126	560	0	2463	9
RTOR Reduction (vph)	0	0	15	0	51	80	0	187	0	0	0	5
Lane Group Flow (vph)	1	0	198	627	112	74	0	2126	373	0	2463	4
Conf. Peds. (#/hr)	0%	0%	3%	5%	23%	6%	0%	5%	5%	0%	3%	0%
Heavy Vehicles (%)	Prot	Perm	Perm	NA	Perm	NA	Perm	NA	Perm	NA	Perm	NA
Turn Type	7	0	4	8	8	8	2	2	2	2	6	6
Permitted Phases	1.2	70.1	63.9	63.9	63.9	63.9	55.9	55.9	55.9	55.9	55.9	55.9
Actuated Green, G (s)	2.2	73.1	66.9	66.9	66.9	66.9	58.9	58.9	58.9	58.9	58.9	58.9
Effective Green, g (s)	0.02	0.52	0.48	0.48	0.48	0.48	0.42	0.42	0.42	0.42	0.42	0.42
Actuated g/C Ratio	5.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0
Clearance Time (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Vehicle Extension (s)	28	818	821	694	691	2078	621	2118	644	2118	644	644
Lane Grp. Cap (vph)	v/s Ratio	Prot	Perm	Perm	NA	Perm	NA	Perm	NA	Perm	NA	Perm
v/s Ratio Prot	0.00	0.13	c0.36	0.05	0.05	0.43	0.43	c0.49	0.43	c0.49	0.43	c0.49
v/s Ratio Perm	0.04	0.24	0.76	0.16	0.11	1.02	0.60	1.16	0.60	1.16	0.60	1.16
Uniform Delay, d1	67.9	18.3	30.1	20.7	20.1	40.5	31.4	40.5	31.4	40.5	23.5	23.5
Progression Factor	1.00	1.00	1.00	1.00	1.00	0.71	0.77	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	0.5	0.2	4.3	0.1	0.1	22.4	2.9	78.9	0.0	78.9	0.0	0.0
Delay (s)	68.4	18.5	34.3	20.8	20.2	51.1	27.0	119.5	23.6	119.5	23.6	23.6
Level of Service	E	B	C	C	C	D	C	F	C	F	C	C
Approach Delay (s)	18.7	29.7	29.7	29.7	29.7	46.0	29.7	119.1	29.7	119.1	29.7	29.7
Approach LOS	B	B	C	C	C	D	D	F	D	F	D	F

Intersection Summary  
 HCM 2000 Control Delay 71.3 HCM 2000 Level of Service E  
 HCM 2000 Volume to Capacity ratio 0.94  
 Actuated Cycle Length (s) 140.0 Sum of lost time (s) 12.0  
 Intersection Capacity Utilization 97.9% ICU Level of Service F  
 Analysis Period (min) 15

c Critical Lane Group

Baseline

Lanes, Volumes, Timings  
 2: Trafalgar Road & QEW SB On Ramps

2025 Total AM Peak Hour.syn  
 05-04-2022

Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	0	0	0	2274	2409	1856
Traffic Volume (vph)	0	0	0	2274	2409	1856
Future Volume (vph)	0	0	0	2274	2409	1856
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0
Lane Util. Factor	1.00	1.00	1.00	0.91	0.91	1.00
Flt Protected	0.850	0.850	0.850	0.850	0.850	0.850
Satd. Flow (prot)	0	0	0	5085	5085	1583
Flt Permitted	0	0	0	5085	5085	1583
Satd. Flow (perm)	0	0	0	5085	5085	1583
Link Speed (k/h)	50	50	50	50	50	50
Link Distance (m)	159.6	159.6	159.6	186.0	145.2	145.2
Travel Time (s)	11.5	11.5	11.5	13.4	10.5	10.5
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	0	0	0	2472	2618	2017
Shared Lane Traffic (%)	0	0	0	2472	2618	2017
Lane Group Flow (vph)	0	0	0	2472	2618	2017
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Left	Left	Right
Median Width (m)	0.0	0.0	0.0	0.0	0.0	0.0
Link Offset (m)	0.0	0.0	0.0	0.0	0.0	0.0
Crosswalk Width (m)	4.8	4.8	4.8	4.8	4.8	4.8
Two way Left Turn Lane	1.00	1.00	1.00	1.00	1.00	1.00
Headway Factor	25	15	25	25	15	15
Turning Speed (k/h)	25	15	25	25	15	15
Sign Control	Stop	Stop	Free	Free	Free	Free

Intersection Summary  
 Area Type: Other  
 Control Type: Unsignalized  
 Intersection Capacity Utilization 118.3% ICU Level of Service H  
 Analysis Period (min) 15

Baseline

HCM Signalized Intersection Capacity Analysis  
 1: Trafalgar Road & North Service Road/QEW WB Off Ramp

Lanes, Volumes, Timings  
 2: Trafalgar Road & QEW SB On Ramps

2025 Total AM Peak Hour.syn  
 05-04-2022

2035 Background AM Peak Hour.syn  
 05-04-2022

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	1	0	212	429	35	265	0	1930	344	0	3624	6
Traffic Volume (vph)	1	0	212	429	35	265	0	1930	344	0	3624	6
Future Volume (vph)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Ideal Flow (vphpl)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Total Lost time (s)	1.00	1.00	0.95	0.95	1.00	1.00	0.91	1.00	0.91	1.00	0.91	1.00
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frb, ped/bikes	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frb, ped/bikes	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Flt Protected	0.95	1.00	0.95	0.96	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Satd. Flow (prot)	1805	1568	1633	1607	1524	4940	1478	5036	1533	5036	1533	1533
Flt Permitted	0.95	1.00	0.95	0.96	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Satd. Flow (perm)	1805	1568	1633	1607	1524	4940	1478	5036	1533	5036	1533	1533
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	1	0	230	466	38	288	0	2098	374	0	3939	7
RTOR Reduction (vph)	0	0	23	0	0	172	0	84	0	0	2	2
Lane Group Flow (vph)	1	0	207	252	252	116	0	2098	290	0	3939	5
Conf. Peds. (#/hr)	0%	0%	3%	5%	23%	6%	0%	5%	5%	0%	3%	0%
Heavy Vehicles (%)	Prot	Perm	Perm	NA	Perm	NA	Perm	NA	Perm	NA	Perm	NA
Turn Type	7	8	8	8	8	8	2	2	2	6	6	6
Protected Phases	Permitted Phases	Actuated Green, G (s)	1.2	33.8	27.6	27.6	27.6	92.2	92.2	92.2	92.2	92.2
Effective Green, g (s)	2.2	36.8	30.6	30.6	30.6	30.6	95.2	95.2	95.2	95.2	95.2	95.2
Actuated g/C Ratio	0.02	0.26	0.22	0.22	0.22	0.22	0.68	0.68	0.68	0.68	0.68	0.68
Clearance Time (s)	5.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Lane Grp. Cap (vph)	28	412	356	351	333	333	3359	1005	3424	1042	1042	1042
v/s Ratio Prot	0.00						0.42		c0.78			
v/s Ratio Perm	0.04	0.50	0.71	0.72	0.35	0.62	0.29	0.20	1.15	0.00	0.00	0.00
Uniform Delay, d1	67.9	43.8	50.6	50.7	46.3	12.5	8.9	22.4	7.2	22.4	7.2	7.2
Progression Factor	1.00	1.00	1.00	1.00	1.00	0.45	0.51	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	0.5	1.0	6.3	6.9	0.6	0.7	0.6	71.5	0.0	71.5	0.0	0.0
Delay (s)	68.4	44.8	56.9	57.6	46.9	6.3	5.1	93.9	7.2	93.9	7.2	7.2
Level of Service	E	D	E	E	D	A	A	A	F	A	F	A
Approach Delay (s)	44.9						6.1		93.7			
Approach LOS	D						A		F			F
Intersection Summary												
HCM 2000 Control Delay	58.8											
HCM 2000 Volume to Capacity ratio	1.04											
Actuated Cycle Length (s)	140.0											
Intersection Capacity Utilization	106.0%											
Analysis Period (min)	15											
c Critical Lane Group												

Lane Group	EBL	EBR	NBL	NBT	SBL	SBR
Lane Configurations	0	0	0	2473	2081	958
Traffic Volume (vph)	0	0	0	2473	2081	958
Future Volume (vph)	1900	1900	1900	1900	1900	1900
Ideal Flow (vphpl)	1.00	1.00	1.00	0.91	0.91	1.00
Lane Util. Factor	1.00	1.00	1.00	0.91	0.91	1.00
Flt Protected	0	0	0	5085	5085	1583
Satd. Flow (prot)	0	0	0	5085	5085	1583
Flt Permitted	0	0	0	5085	5085	1583
Satd. Flow (perm)	0	0	0	5085	5085	1583
Link Speed (k/h)	50	50	50	50	50	50
Link Distance (m)	159.6	159.6	186.0	145.2	145.2	145.2
Travel Time (s)	11.5	11.5	13.4	10.5	10.5	10.5
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	0	0	0	2688	2262	1041
Shared Lane Traffic (%)	0	0	0	2688	2262	1041
Lane Group Flow (vph)	No	No	No	No	No	No
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Left	Left	Right
Median Width (m)	0.0	0.0	0.0	0.0	0.0	0.0
Link Offset (m)	0.0	0.0	0.0	0.0	0.0	0.0
Crosswalk Width (m)	4.8	4.8	4.8	4.8	4.8	4.8
Two way Left Turn Lane	1.00	1.00	1.00	1.00	1.00	1.00
Headway Factor	25	15	25	15	15	15
Turning Speed (k/h)	Stop	Stop	Free	Free	Free	Free
Sign Control	Stop	Stop	Free	Free	Free	Free
Intersection Summary						
Area Type:	Other					
Control Type:	Unsignalized					
Intersection Capacity Utilization	115.8%					
Analysis Period (min)	15					
ICU Level of Service H						

HCM Unsignalized Intersection Capacity Analysis 2035 Background AM Peak Hour.syn  
 2: Trafalgar Road & QEW SB On Ramps 05-04-2022

	EBL	EBR	NBL	NBT	SBT	SBR
Movement						
Lane Configurations				↑↑↑	↑↑↑	↑
Traffic Volume (veh/h)	0	0	0	2473	2081	958
Future Volume (Veh/h)	0	0	0	2473	2081	958
Sign Control	Stop	Free	Free	Free	Free	Free
Grade	0%	0%	0%	0%	0%	0%
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	0	0	0	2688	2262	1041
Pedestrians						
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)				None	None	
Median type				None	None	
Median storage (veh)				186	145	
Upstream signal (m)				0.75	0.60	
pX, platoon unblocked				3158	754	
vC, conflicting volume					2262	
vC1, stage 1 conf/vol						
vC2, stage 2 conf/vol						
vCu, unblocked vol				0	796	
tC, single (s)				6.8	6.9	4.1
tC, 2 stage (s)				3.5	3.3	2.2
tF (s)				100	100	100
p0 queue free %				763	655	496
cM capacity (veh/h)						
Direction_Lane #	NB.1	NB.2	NB.3	SB.1	SB.2	SB.3 SB.4
Volume Total	896	896	896	754	754	754 1041
Volume Left	0	0	0	0	0	0
Volume Right	0	0	0	0	0	1041
gSH	1700	1700	1700	1700	1700	1700
Volume to Capacity	0.53	0.53	0.53	0.44	0.44	0.61
Queue Length 95th (m)	0.0	0.0	0.0	0.0	0.0	0.0
Control Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0
Lane LOS						
Approach Delay (s)	0.0			0.0		
Approach LOS						
Intersection Summary						
Average Delay	0.0					
Intersection Capacity Utilization	115.8% ICU Level of Service H					
Analysis Period (min)	15					

Lanes, Volumes, Timings 2025 Total AM Peak Hour.syn  
 1: Trafalgar Road & North Service Road/QEW WB Off Ramp 05-04-2022

	ICU Level of Service G
Intersection Capacity Utilization	106.0%
Analysis Period (min)	15
~ Volume exceeds capacity, queue is theoretically infinite.	
# 95th percentile volume exceeds capacity, queue may be longer.	
Queue shown is maximum after two cycles.	

Splits and Phases: 1: Trafalgar Road & North Service Road/QEW WB Off Ramp	
Phase 1 (R)	79 s
Phase 2 (R)	79 s
Phase 3 (R)	73 s
Phase 4 (R)	38 s



Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	↔	↔	↔	↔	↔	↔
Traffic Volume (vph)	1123	1113	0	1350	2081	0
Future Volume (vph)	1123	1113	0	1350	2081	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	0.97	1.00	1.00	0.91	0.91	1.00
Flt	0.850					
Flt Protected	0.950					
Satd. Flow (prot)	3400 1599 0 4893 4988 0					
Flt Permitted	0.950					
Satd. Flow (perm)	3400 1599 0 4893 4988 0					
Right Turn on Red	Yes					
Satd. Flow (RTOR)	50					
Link Speed (k/h)	71.3					
Link Distance (m)	50.7					
Travel Time (s)	5.1					
Peak Hour Factor	0.92					
Heavy Vehicles (%)	3%					
Adj. Flow (vph)	1221					
Shared Lane Traffic (%)	1221					
Lane Group Flow (vph)	1221					
Enter Blocked Intersection	No					
Lane Alignment	Left					
Median Width (m)	7.2					
Link Offset (m)	0.0					
Crosswalk Width (m)	4.8					
Two way Left Turn Lane	No					
Headway Factor	1.00					
Turning Speed (k/h)	25					
Number of Detectors	1					
Detector Template	Left					
Leading Detector (m)	2.0					
Trailing Detector (m)	0.0					
Detector 1 Position (m)	0.0					
Detector 1 Size (m)	2.0					
Detector 1 Type	Ch+Ex					
Detector 1 Channel	Ch+Ex					
Detector 1 Extend (s)	0.0					
Detector 1 Queue (s)	0.0					
Detector 1 Delay (s)	0.0					
Detector 2 Position (m)	9.4					
Detector 2 Size (m)	0.6					
Detector 2 Type	Ch+Ex					
Detector 2 Channel	Ch+Ex					
Detector 2 Extend (s)	0.0					
Turn Type	Perm					
Protected Phases	4					
Permitted Phases	4					
Detector Phase	4					
Switch Phase	4					

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Turn Type	7											
Protected Phases												
Permitted Phases	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA
Detector Phase	6	6	6	6	6	6	6	6	6	6	6	6
Switch Phase												
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Minimum Split (s)	23.0	38.0	38.0	38.0	38.0	38.0	35.0	35.0	35.0	35.0	35.0	35.0
Total Split (s)	23.0	61.0	38.0	38.0	38.0	38.0	79.0	79.0	79.0	79.0	79.0	79.0
Total Split (%)	16.4%	43.6%	27.1%	27.1%	27.1%	27.1%	56.4%	56.4%	56.4%	56.4%	56.4%	56.4%
Maximum Green (s)	18.0	54.0	31.0	31.0	31.0	31.0	72.0	72.0	72.0	72.0	72.0	72.0
Yellow Time (s)	3.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
All-Red Time (s)	2.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Lost Time Adjust (s)	-1.0	-3.0	-3.0	-3.0	-3.0	-3.0	-3.0	-3.0	-3.0	-3.0	-3.0	-3.0
Total Lost Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lead/Lag	Lead	Lag	Lag	Lag	Lag	Lag	Lead	Lag	Lag	Lag	Lag	Lag
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	None	None	None	None	None	None	C-Max	C-Max	C-Max	C-Max	C-Max	C-Max
Walk Time (s)	11.0	11.0	11.0	11.0	11.0	11.0	11.0	11.0	11.0	11.0	11.0	11.0
Flash Dont Walk (s)	0	0	0	0	0	0	0	0	0	0	0	0
Pedestrian Calls (#/hr)	6.7	32.8	30.6	30.6	30.6	30.6	99.2	99.2	99.2	99.2	99.2	99.2
Act Effct Green (s)	0.05	0.23	0.22	0.22	0.22	0.22	0.71	0.71	0.71	0.71	0.71	0.71
v/c Ratio	0.01	0.59	0.71	0.72	0.57	0.60	0.60	0.33	1.10	0.01	0.01	0.01
Control Delay	64.0	45.5	60.9	61.8	16.1	16.1	6.0	2.2	73.2	0.0	0.0	0.0
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	64.0	45.5	60.9	61.8	16.1	16.1	6.0	2.2	73.2	0.0	0.0	0.0
LOS	E	D	E	E	B	B	A	A	A	E	E	A
Approach Delay	45.6			44.9			5.4			73.1		
Approach LOS	D			D			A			E		
Queue Length 50th (m)	0.3	53.0	71.8	72.1	16.3	16.3	60.1	6.8	~469.7	0.0		
Queue Length 95th (m)	2.3	66.9	96.7	97.1	43.0	43.0	104.4	25.4	#566.6	0.0		
Internal Link Dist (m)		96.4		141.1			121.2			185.2		
Turn Bay Length (m)	50.0						70.0					
Base Capacity (vph)	244	656	410	404	547	547	3501	1123	3569	1106		
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0		
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0		
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0		
Reduced v/c Ratio	0.00	0.35	0.61	0.62	0.53	0.53	0.60	0.33	1.10	0.01		
Intersection Summary	Intersection LOS: D											
Area Type:	Other											
Cycle Length:	140											
Actuated Cycle Length:	140											
Offset: 0 (0%):	Referenced to phase 2:NBT and 6:SBT, Start of Green											
Natural Cycle:	150											
Control Type:	Actuated-Coordinated											
Maximum v/c Ratio:	1.10											
Intersection Signal Delay:	46.7											



Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Minimum Initial (s)	10.0	10.0		29.0	29.0	
Minimum Split (s)	38.0	38.0		36.0	36.0	
Total Split (s)	83.0	83.0		57.0	57.0	
Total Split (%)	59.3%	59.3%		40.7%	40.7%	
Maximum Green (s)	76.0	76.0		50.0	50.0	
Yellow Time (s)	4.0	4.0		4.0	4.0	
All-Red Time (s)	3.0	3.0		3.0	3.0	
Lost Time Adjust (s)	-3.0	-3.0		-3.0	-3.0	
Total Lost Time (s)	4.0	4.0		4.0	4.0	
Lead/Lag						
Lead-Lag Optimize?						
Vehicle Extension (s)	3.0	3.0		3.0	3.0	
Recall Mode	None	None		C-Max	C-Max	
Walk Time (s)	7.0	7.0		7.0	7.0	
Flash Dont Walk (s)	11.0	11.0		11.0	11.0	
Pedestrian Calls (#/hr)	0	0		0	0	
Act Effct Green (s)	79.0	79.0		53.0	53.0	
Actuated g/C Ratio	0.56	0.56		0.38	0.38	
v/c Ratio	0.64	1.34		0.79	1.20	
Control Delay	22.6	189.5		52.2	131.1	
Queue Delay	0.0	0.0		0.0	0.0	
Total Delay	22.6	189.5		52.2	131.1	
LOS	C	F		D	F	
Approach Delay	105.7			52.2	131.1	
Approach LOS	F			D	F	
Queue Length 50th (m)	120.0	~458.9		165.2	~288.6	
Queue Length 95th (m)	143.0	#643.5		m180.5	m#264.9	
Internal Link Dist (m)	47.3			26.7	162.0	
Turn Bay Length (m)						
Base Capacity (vph)	1918	902		1852	1888	
Starvation Cap Reductn	0	0		0	0	
Spillback Cap Reductn	0	0		0	0	
Storage Cap Reductn	0	0		0	0	
Reduced v/c Ratio	0.64	1.34		0.79	1.20	
Intersection Summary						
Area Type:	Other					
Cycle Length:	140					
Actuated Cycle Length:	140					
Offset: 0 (0%):	Referenced to phase 2:NBT and 6:SBT, Start of Green					
Natural Cycle:	75					
Control Type:	Actuated-Coordinated					
Maximum v/c Ratio:	1.34					
Intersection Signal Delay:	102.3					
Intersection Capacity Utilization:	115.8%					
Analysis Period (min):	15					
~ Volume exceeds capacity. queue is theoretically infinite.						
# 95th percentile volume exceeds capacity. queue may be longer.						



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	1	1	1	4	4	4	1	1	1	1	1	1
Traffic Volume (vph)	1	0	212	429	35	265	0	1930	344	0	3624	6
Future Volume (vph)	1	0	212	429	35	265	0	1930	344	0	3624	6
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (m)	50.0	0.0	0.0	0.0	0.0	0.0	0.0	70.0	0.0	0.0	0.0	0.0
Storage Lanes	1		1	1	1	1	1	0	1	0	0	1
Taper Length (m)	7.5		7.5	7.5	7.5	7.5	7.5	0	7.5	0	7.5	0
Lane Util. Factor	1.00	1.00	1.00	0.95	0.95	1.00	1.00	0.91	1.00	1.00	0.91	1.00
Ped Bike Factor			0.850			0.850		0.96			0.96	0.95
Flt			0.950			0.959		0.850			0.850	0.850
Flt Protected			0.950			0.959		0.850			0.850	0.850
Satd. Flow (prot)	1805	0	1568	1633	1607	1524	0	4940	1538	0	5036	1615
Flt Permitted	0.950		0.950		0.959			0.91			0.91	0.91
Satd. Flow (perm)	1805	0	1568	1633	1607	1524	0	4940	1478	0	5036	1533
Right Turn on Red	Yes		Yes		Yes			Yes			Yes	Yes
Satd. Flow (RTOR)	31		31		220			263			263	70
Link Speed (k/h)		50		50				50			50	50
Link Distance (m)		120.4		165.1				145.2			209.2	209.2
Travel Time (s)		8.7		11.9				10.5			15.1	15.1
Confli. Peds. (#/hr)						8			5			8
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	0%	3%	5%	23%	6%	0%	0%	5%	5%	0%	3%	0%
Adj. Flow (vph)	1	0	230	466	38	288	0	2098	374	0	3939	7
Shared Lane Traffic (%)			46%									
Lane Group Flow (vph)	1	0	230	252	288	0	2098	374	0	3939	7	7
Enter Blocked Intersecion	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Right	Right	Left	Left	Right	Left	Left	Right
Median Width(m)	3.6		3.6		3.6			0.0			0.0	0.0
Link Offset(m)		0.0		0.0		0.0		0.0			0.0	0.0
Crosswalk Width(m)		4.8		4.8				4.8			4.8	4.8
Two way Left Turn Lane												
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (k/h)	25	15	25	15	25	15	25	15	25	15	25	15
Number of Detectors	1	1	1	2	1	2	2	1	2	1	2	1
Detector Template	Left	Right	Left	Thru	Right	Right	Thru	Right	Thru	Right	Thru	Right
Leading Detector (m)	2.0	2.0	2.0	10.0	2.0	10.0	2.0	10.0	2.0	10.0	2.0	10.0
Trailing Detector (m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Position(m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Size(m)	2.0	2.0	2.0	0.6	2.0	0.6	2.0	0.6	2.0	0.6	2.0	0.6
Detector 1 Type	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 2 Position(m)				9.4		9.4		9.4			9.4	
Detector 2 Size(m)				0.6		0.6		0.6			0.6	
Detector 2 Type				Ch+Ex		Ch+Ex		Ch+Ex			Ch+Ex	
Detector 2 Channel												
Detector 2 Extend (s)				0.0		0.0		0.0			0.0	

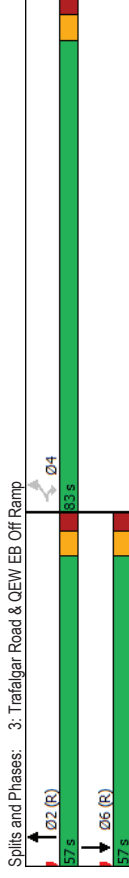
HCM Unsignalized Intersection Capacity Analysis 2035 Background PM Peak Hour.syn  
 11: Argus Rd & South Service Rd 05-04-2022



Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	13	10	361	106	17	18
Traffic Volume (veh/h)	13	10	361	106	17	18
Future Volume (Veh/h)	0%	0%	0%	0%	0%	0%
Sign Control	Free	Free	Free	Free	Stop	Stop
Grade	0%	0%	0%	0%	0%	0%
Peak Hour Factor	0.45	0.50	0.87	0.68	0.75	0.65
Hourly flow rate (vph)	29	20	415	156	23	28
Pedestrians			5			
Lane Width (m)			3.6			
Walking Speed (m/s)			1.2			
Percent Blockage			0			
Right turn flare (veh)			None			
Median type			None			
Median storage (veh)			358			
Upstream signal (m)			571			576
pX, platoon unblocked vC, conflicting volume						493
vC1, stage 1 conf vol						576
vC2, stage 2 conf vol						493
vCu, unblocked vol			4.1			6.4
tC, single (s)			2.2			3.5
tC, 2 stage (s)			97			95
tF (s)			1012			467
p0 queue free %						95
cM capacity (veh/h)						580
Direction_Lane #	EB1	WB1	SB1			
Volume Total	49	571	51			
Volume Left	29	0	23			
Volume Right	0	156	28			
GSH	1012	1700	523			
Volume to Capacity	0.03	0.34	0.10			
Queue Length 95th (m)	0.7	0.0	2.6			
Control Delay (s)	5.2	0.0	12.6			
Lane LOS	A	B	B			
Approach Delay (s)	5.2	0.0	12.6			
Approach LOS	B	B	B			
Intersection Summary						
Average Delay			1.3			
Intersection Capacity Utilization			38.3%		ICU Level of Service	A
Analysis Period (min)			15			

Lanes, Volumes, Timings 2035 Background AM Peak Hour.syn  
 3: Trafalgar Road & QEW EB Off Ramp 05-04-2022

Queue shown is maximum after two cycles.  
 m Volume for 95th percentile queue is metered by upstream signal.





HCM Signalized Intersection Capacity Analysis  
 3: Trafalgar Road & QEW EB Off Ramp

Lanes, Volumes, Timings  
 11: Argus Rd & South Service Rd

2035 Background AM Peak Hour.syn  
 05-04-2022

2035 Background PM Peak Hour.syn  
 05-04-2022

Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	↔	↔	↔	↔	↔	↔
Traffic Volume (vph)	1123	1113	0	1350	2081	0
Future Volume (vph)	1123	1113	0	1350	2081	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0
Lane Util. Factor	0.97	1.00	0.91	0.91	0.91	0.91
Frt	1.00	0.85	1.00	1.00	1.00	1.00
Flt Protected	0.95	1.00	1.00	1.00	1.00	1.00
Satd. Flow (prot)	3400	1599	4893	4988	4988	4988
Flt Permitted	0.95	1.00	1.00	1.00	1.00	1.00
Satd. Flow (perm)	3400	1599	4893	4988	4988	4988
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	1221	1210	0	1467	2262	0
RTOR Reduction (vph)	0	0	0	0	0	0
Lane Group Flow (vph)	1221	1210	0	1467	2262	0
Heavy Vehicles (%)	3%	1%	0%	6%	4%	0%
Turn Type	Perm	Perm	NA	NA	NA	NA
Protected Phases			2		6	
Permitted Phases	4	4				
Actuated Green, G (s)	76.0	76.0	50.0	50.0	50.0	50.0
Effective Green, g (s)	79.0	79.0	53.0	53.0	53.0	53.0
Actuated g/C Ratio	0.56	0.56	0.38	0.38	0.38	0.38
Clearance Time (s)	7.0	7.0	7.0	7.0	7.0	7.0
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0
Lane Gp Cap (vph)	1918	902	1852	1888	1888	1888
v/s Ratio Prot			0.30		0.45	
v/s Ratio Perm	0.36	0.76				
v/c Ratio	0.64	1.34	0.79	1.20	1.20	1.20
Uniform Delay, d1	20.7	30.5	38.6	43.5	43.5	43.5
Progression Factor	1.00	1.00	1.29	1.06	1.06	1.06
Incremental Delay, d2	0.7	161.1	1.9	90.7	90.7	90.7
Delay (s)	21.4	191.6	51.8	136.8	136.8	136.8
Level of Service	C	F	D	F	F	F
Approach Delay (s)	106.2		51.8	136.8		
Approach LOS	F		D	F		
Intersection Summary						
HCM 2000 Control Delay	104.5		HCM 2000 Level of Service		F	
HCM 2000 Volume to Capacity ratio	1.31					
Actuated Cycle Length (s)	140.0		Sum of lost time (s)		11.0	
Intersection Capacity Utilization	115.8%		ICU Level of Service		H	
Analysis Period (min)	15					
c Critical Lane Group						

Lane Group	EBL	WBT	WBR	SBL	SBR
Lane Configurations	↔	↔	↔	↔	↔
Traffic Volume (vph)	13	10	361	106	17
Future Volume (vph)	13	10	361	106	17
Ideal Flow (vphpl)	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor					
Frt		0.963		0.926	
Flt Protected		0.971		0.978	
Satd. Flow (prot)	0	1577	1535	0	1549
Flt Permitted		0.971		0.978	
Satd. Flow (perm)	0	1577	1535	0	1549
Link Speed (k/h)		50	50	50	50
Link Distance (m)		177.7	165.2	103.5	
Travel Time (s)		12.8	11.9	7.5	
Conf. Peds. (#/hr)				5	
Peak Hour Factor	0.45	0.50	0.87	0.68	0.75
Heavy Vehicles (%)	0%	13%	10%	0%	0%
Adj. Flow (vph)	29	20	415	156	23
Shared Lane Traffic (%)					
Lane Group Flow (vph)	0	49	571	0	51
Enter Blocked Intersection	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Right
Median Width(m)		0.0	0.0	3.6	
Link Offset(m)		0.0	0.0	0.0	
Crosswalk Width(m)		4.8	4.8	4.8	
Two way Left Turn Lane					
Headway Factor	1.14	1.14	1.14	1.14	1.14
Turning Speed (k/h)	24		14	24	14
Sign Control		Free	Free	Stop	
Intersection Summary					
Area Type:	CBD				
Control Type:	Unsignalized				
Intersection Capacity Utilization	38.3%				
Analysis Period (min)	15				
ICU Level of Service A					

HCM Unsignalized Intersection Capacity Analysis 2035 Background PM Peak Hour.syn  
10: Lyons Lane & South Service Road 05-04-2022

Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	4	6	1	6	17	6
Traffic Volume (veh/h)	4	6	1	6	17	6
Future Volume (Veh/h)	4	6	1	6	17	6
Sign Control	Free	Free	0%	Free	Stop	0%
Grade	0%	0%	0%	0%	0%	0%
Peak Hour Factor	0.38	0.62	0.25	0.62	0.50	0.42
Hourly flow rate (vph)	11	10	4	10	34	14
Pedestrians					7	
Lane Width (m)					3.6	
Walking Speed (m/s)					1.2	
Percent Blockage					1	
Right turn flare (veh)					None	
Median type					None	
Median storage (veh)					393	
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume						
vC1, stage 1 conf/vol					48	16
vC2, stage 2 conf/vol						
vCu, unblocked vol					48	16
tC, single (s)					6.4	6.2
tC, 2 stage (s)						
tF (s)					3.5	3.3
p0 queue free %					96	99
cM capacity (veh/h)					954	1063
Direction, Lane #	EB 1	WB 1	SB 1			
Volume Total	21	14	48			
Volume Left	11	0	34			
Volume Right	0	10	14			
GSH	1599	1700	984			
Volume to Capacity	0.01	0.01	0.05			
Queue Length 95th (m)	0.2	0.0	1.2			
Control Delay (s)	3.8	0.0	8.8			
Lane LOS	A	A	A			
Approach Delay (s)	3.8	0.0	8.8			
Approach LOS	A	A	A			
Intersection Summary	Other					
Average Delay	6.1					
Intersection Capacity Utilization	15.4% ICU Level of Service A					
Analysis Period (min)	15					

Lanes, Volumes, Timings 2035 Background AM Peak Hour.syn  
4: Trafalgar Road & Argus Road/QEW EB On Ramp 05-04-2022

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	0	0	22	0	0	0	0	1350	578	0	2272	922
Future Volume (vph)	0	0	22	0	0	0	0	1350	578	0	2272	922
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	70.0	0.0	0.0	0.0	0.0
Storage Lanes	0	0	1	0	0	0	0	1	0	0	0	0
Taper Length (m)	7.5			7.5			7.5			7.5		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.91	1.00	1.00	0.91	0.91
Ped Bike Factor			0.865					0.850				0.957
Fit			Fit Protected					Fit Protected				Fit Protected
Satd. Flow (prot)	0	0	1522	0	0	0	0	5085	1568	0	4894	0
Fit Permitted												
Satd. Flow (perm)	0	0	1522	0	0	0	0	5085	1568	0	4894	0
Link Speed (k/h)			50					50				50
Link Distance (m)			165.2					271.0				50.7
Travel Time (s)			11.9					19.5				3.7
Confl. Peds. (#/hr)												24
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	0%	0%	8%	0%	2%	0%	0%	2%	3%	0%	2%	0%
Adj. Flow (vph)	0	0	24	0	0	0	0	1467	628	0	2470	1002
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	0	24	0	0	0	0	1467	628	0	3472	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(m)			0.0			0.0		3.6			3.6	
Link Offset(m)			0.0			0.0		0.0			0.0	
Crosswalk Width(m)			4.8			4.8		4.8			4.8	
Two way Left Turn Lane												
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (k/h)	25	15	25	15	25	15	25	15	25	15	25	15
Sign Control			Stop			Stop		Free			Free	
Intersection Summary	Other											
Area Type:	Unsignalized											
Control Type:	ICU Level of Service D											
Intersection Capacity Utilization	75.2%											
Analysis Period (min)	15											

HCM Unsignalized Intersection Capacity Analysis  
 4: Trafalgar Road & Argus Road/QEW EB On Ramp

2035 Background AM Peak Hour.syn  
 05-04-2022

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	0	0	22	0	0	0	0	1350	578	0	2272	922
Future Volume (Veh/h)	0	0	22	0	0	0	0	1350	578	0	2272	922
Sign Control	Stop			Stop			Free			Free		
Grade	0%											
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	0	0	24	0	0	0	0	1467	628	0	2470	1002
Pedestrians	24											
Lane Width (m)	3.6											
Walking Speed (m/s)	1.2											
Percent Blockage	2											
Right turn flare (veh)	None											
Median type	None											
Median storage (veh)	None											
Upstream signal (m)	271											
pX, platoon unblocked	0.70	0.70	0.63	0.70	0.70	0.85	0.63			0.85		51
vC, conflicting volume	3484	4462	1348	2290	4963	489	3496			1467		
vC1, stage 1 conf/vol												
vC2, stage 2 conf/vol												
vCu, unblocked vol	1601	2996	0	0	3711	0	2895			930		
tC, single (s)	7.5	6.5	7.1	7.5	6.5	6.9	4.1			4.1		
tC, 2 stage (s)												
tF (s)	3.5	4.0	3.4	3.5	4.0	3.3	2.2			2.2		
p0 queue free %	100	100	96	100	100	100	100			100		
cM capacity (veh/h)	49	10	653	684	3	927	79			632		
Direction_Lane #	EB 1	NB 1	NB 2	NB 3	NB 4	SB 1	SB 2	SB 3				
Volume Total	24	489	489	489	628	988	988	1496				
Volume Left	0	0	0	0	0	0	0	0				
Volume Right	24	0	0	0	628	0	0	1002				
GSH	653	1700	1700	1700	1700	1700	1700	1700				
Volume to Capacity	0.04	0.29	0.29	0.29	0.37	0.58	0.58	0.88				
Queue Length 95th (m)	0.9	0.0	0.0	0.0	0.0	0.0	0.0	0.0				
Control Delay (s)	10.7	0.0	0.0	0.0	0.0	0.0	0.0	0.0				
Lane LOS	B	B	B	B	B	B	B	B				
Approach Delay (s)	10.7	0.0				0.0						
Approach LOS	B	B										
Intersection Summary												
Average Delay	0.0											
Intersection Capacity Utilization	75.2%											
ICU Level of Service	D											
Analysis Period (min)	15											

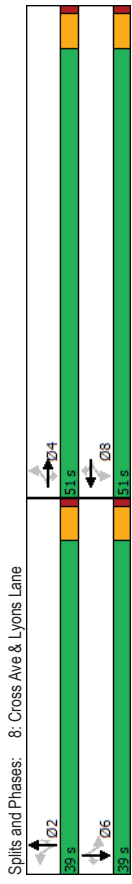
Lanes, Volumes, Timings  
 10: Lyons Lane & South Service Road

2035 Background PM Peak Hour.syn  
 05-04-2022

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	4	6	1	6	17	6						
Future Volume (vph)	4	6	1	6	17	6						
Ideal Flow (vehpl)	1900	1900	1900	1900	1900	1900						
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00						
Ped Bike Factor	0.904											
Ft Protected	0.974											
Satd. Flow (prot)	0	1666	1336	0	1587	0						
Ft Permitted	0.974											
Satd. Flow (perm)	0	1666	1336	0	1587	0						
Link Speed (k/h)	50											
Link Distance (m)	60.5											
Travel Time (s)	4.4											
Conf. Peds. (#/hr)	7											
Peak Hour Factor	0.38	0.62	0.25	0.62	0.50	0.42						
Heavy Vehicles (%)	0%											
Adj. Flow (vph)	11	10	4	10	34	14						
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	21	14	0	48	0						
Enter Blocked Intersection	No	No	No	No	No	No						
Lane Alignment	Left	Left	Left	Right	Left	Right						
Median Width(m)	0.0											
Link Offset(m)	0.0											
Crosswalk Width(m)	4.8											
Two way Left Turn Lane												
Headway Factor	1.14	1.14	1.14	1.14	1.14	1.14						
Turning Speed (k/h)	24											
Sign Control	Free			Free			Free			Stop		
Intersection Summary												
Area Type:	CBD											
Control Type:	Unsignalized											
Intersection Capacity Utilization	15.4%											
ICU Level of Service A												
Analysis Period (min)	15											

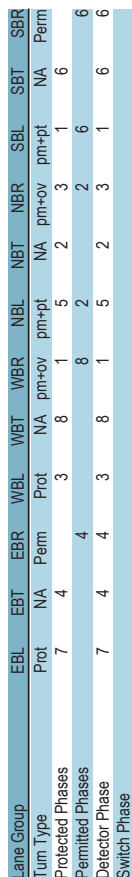


Analysis Period (min) 15



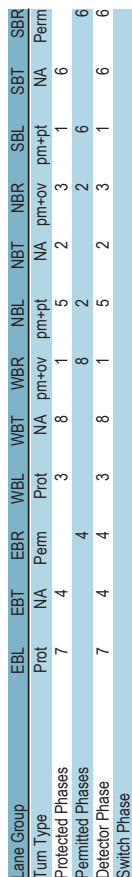
Splits and Phases: 8: Cross Ave & Lyons Lane

Analysis Period (min) 15



Splits and Phases: 8: Cross Ave & Lyons Lane

Analysis Period (min) 15

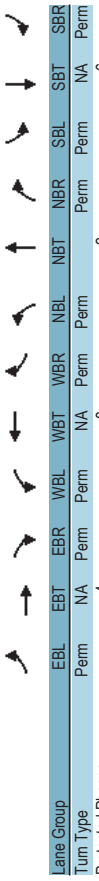


Splits and Phases: 5: Trafalgar Road & Cross Avenue/South Service Road

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Turn Type	Prot	NA	Perm	Prot	NA	Perm+ov	pm+pt	NA	pm+ov	pm+pt	NA	Perm
Protected Phases	7	4	4	3	8	8	1	5	2	3	1	6
Permitted Phases	7	4	4	3	8	8	1	5	2	3	1	6
Detector Phase												
Switch Phase												
Minimum Initial (s)	10.0	5.0	5.0	12.0	10.0	7.0	7.0	27.0	12.0	7.0	7.0	27.0
Minimum Split (s)	17.0	25.0	25.0	17.0	25.0	11.5	11.5	34.0	17.0	11.5	34.0	34.0
Total Split (s)	39.0	33.0	33.0	31.0	25.0	36.0	18.2	40.0	31.0	36.0	57.8	57.8
Total Split (%)	27.9%	23.6%	23.6%	22.1%	17.9%	25.7%	13.0%	28.6%	22.1%	25.7%	41.3%	41.3%
Maximum Green (s)	32.0	26.0	26.0	27.0	18.0	32.0	14.2	33.0	27.0	32.0	50.8	50.8
Yellow Time (s)	4.0	4.0	4.0	3.0	4.0	3.0	3.0	4.0	3.0	4.0	3.0	4.0
All-Red Time (s)	3.0	3.0	3.0	1.0	3.0	1.0	1.0	3.0	1.0	1.0	3.0	3.0
Lost Time Adjust (s)	-3.0	-3.0	-3.0	0.0	-3.0	0.0	0.0	-3.0	0.0	0.0	-3.0	-3.0
Total Lost Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lead/Lag	Lead	Lag	Lag	Lead	Lag	Lead	Lead	Lag	Lead	Lead	Lag	Lag
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	None	None	None	None	None	None	C-Max	None	None	None	C-Max	C-Max
Walk Time (s)	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0
Flash Dont Walk (s)	11.0	11.0	11.0	11.0	11.0	11.0	11.0	11.0	11.0	11.0	11.0	11.0
Pedestrian Calls (#/hr)	0	0	0	0	0	0	0	0	0	0	0	0
Act Effct Green (s)	34.9	37.0	37.0	18.6	20.7	51.5	50.5	37.6	56.2	72.4	55.5	55.5
Actuated g/C Ratio	0.25	0.26	0.26	0.13	0.15	0.37	0.36	0.27	0.40	0.52	0.40	0.40
v/c Ratio	0.93	0.48	0.37	0.21	0.80	0.30	0.78	0.85	0.88	0.96	0.67	0.79
Control Delay	69.8	46.7	46.7	8.2	53.2	70.7	12.4	35.5	53.5	40.9	46.4	23.9
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	69.8	46.7	46.7	8.2	53.2	70.7	12.4	35.5	53.5	41.0	46.4	23.9
LOS	E	D	A	D	E	B	D	D	D	D	D	C
Approach Delay	54.0			51.8			48.0					21.9
Approach LOS	D			D			D					C
Queue Length 50th (m)	109.5	55.4	0.0	12.9	59.5	14.1	33.3	100.1	104.8	91.0	110.6	29.2
Queue Length 95th (m)	#146.9	79.3	21.2	20.3	#80.0	32.1	m34.5	m98.7	m78.0	m59.8	m76.7	m11.7
Internal Link Dist (m)	219.7			214.9			242.5					247.0
Turn Bay Length (m)	25.0	60.0	25.0	60.0	50.0	60.0	50.0	60.0	25.0	60.0	25.0	60.0
Base Capacity (vph)	803	900	511	655	501	654	231	1352	767	455	1996	910
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.93	0.48	0.37	0.15	0.79	0.30	0.72	0.85	0.78	0.93	0.67	0.79
Intersection Summary												
Area Type:	Other											
Cycle Length:	140											
Actuated Cycle Length:	140											
Offset:	54.5 (39%), Referenced to phase 2:NBTL and 6:SBTL - Start of Green											
Natural Cycle:	110											
Control Type:	Actuated-Coordinated											
Maximum v/c Ratio:	0.96											
Intersection Signal Delay:	39.7											
Intersection LOS:	D											

Lanes, Volumes, Timings  
 8: Cross Ave & Lyons Lane

Lanes, Volumes, Timings  
 5: Trafalgar Road & Cross Avenue/South Service Road



Intersection Capacity Utilization 87.1%  
 Analysis Period (min) 15  
 # 95th percentile volume exceeds capacity, queue may be longer.  
 Queue shown is maximum after two cycles.  
 m Volume for 95th percentile queue is metered by upstream signal.

Spills and Phases: 5: Trafalgar Road & Cross Avenue/South Service Road

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Turn Type	NA	NA	NA	Perm	Perm	Perm	Perm	Perm	Perm	Perm	Perm	Perm
Protected Phases	4	4	4	8	8	8	2	2	2	6	6	6
Detector Phase	4	4	4	8	8	8	2	2	2	6	6	6
Switch Phase	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Minimum Initial (s)	22.5	22.5	22.5	22.5	22.5	22.5	22.5	22.5	22.5	22.5	22.5	22.5
Minimum Split (s)	51.0	51.0	51.0	51.0	51.0	51.0	39.0	39.0	39.0	39.0	39.0	39.0
Total Split (s)	56.7%	56.7%	56.7%	56.7%	56.7%	56.7%	43.3%	43.3%	43.3%	43.3%	43.3%	43.3%
Total Split (%)	46.5	46.5	46.5	46.5	46.5	46.5	34.5	34.5	34.5	34.5	34.5	34.5
Maximum Green (s)	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5
Yellow Time (s)	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0
All-Red Time (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Lost Time Adjust (s)	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5
Total Lost Time (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Lead-Lag Optimize?	None	None	None	None	None	None	Max	Max	Max	Max	Max	Max
Vehicle Extension (s)	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0
Recall Mode	11.0	11.0	11.0	11.0	11.0	11.0	11.0	11.0	11.0	11.0	11.0	11.0
Flash Dont Walk (s)	0	0	0	0	0	0	0	0	0	0	0	0
Pedestrian Calls (#/hr)	20.4	20.4	20.4	20.4	20.4	20.4	34.7	34.7	34.7	34.7	34.7	34.7
Act Effct Green (s)	0.32	0.32	0.32	0.32	0.32	0.32	0.54	0.54	0.54	0.54	0.54	0.54
Actuated g/C Ratio	0.71	0.26	0.02	0.11	0.71	0.28	0.27	0.29	0.21	0.00	0.11	0.11
v/c Ratio	53.7	16.6	4.9	15.6	23.4	4.3	10.5	34.5	0.7	10.0	9.0	2.9
Control Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Queue Delay	53.7	16.6	4.9	15.6	23.4	4.3	10.5	34.5	0.7	10.0	9.0	2.9
Total Delay	D	B	A	B	C	A	B	A	A	A	A	A
LOS	25.3	C	19.9	B	4.4	A	4.4	7.4	7.4	A	A	A
Approach Delay	8.8	11.7	0.0	3.0	40.5	0.0	11.0	0.0	8.5	0.1	0.0	0.0
Approach LOS	10.6	19.4	2.1	8.6	52.0	1.6	28.0	0.0	9.8	0.8	0.0	0.0
Queue Length 50th (m)	257.3	60.0	60.0	60.0	60.0	60.0	148.6	60.0	60.0	60.0	60.0	60.0
Queue Length 95th (m)	265	2154	1044	734	2301	1078	687	1044	691	907	796	796
Internal Link Dist (m)	0	0	0	0	0	0	0	0	0	0	0	0
Base Capacity (vph)	0	0	0	0	0	0	0	0	0	0	0	0
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.31	0.11	0.01	0.05	0.31	0.14	0.27	0.29	0.21	0.00	0.11	0.11
Intersection Summary	Intersection LOS: B ICU Level of Service A											

Intersection Summary

Area Type: CBD

Cycle Length: 90

Actuated Cycle Length: 64.1

Natural Cycle: 45

Control Type: Semi Act-Uncoord

Maximum v/c Ratio: 0.71

Intersection Signal Delay: 15.5

Intersection Capacity Utilization 50.9%

Intersection Summary

Area Type: CBD

Cycle Length: 90

Actuated Cycle Length: 64.1

Natural Cycle: 45

Control Type: Semi Act-Uncoord

Maximum v/c Ratio: 0.71

Intersection Signal Delay: 15.5

Intersection Capacity Utilization 50.9%

Baselene

Baselene





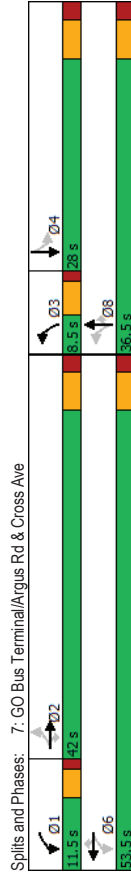


	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Turn Type	Prot	NA	4	Prot	NA	8	Prot	NA	2	Prot	NA	6
Protected Phases	7	4	→	3	8	←	5	2	←	1	→	↔
Permitted Phases	7	4	↔	3	8	↔	5	2	↔	1	↔	↔
Detector Phase	7	4	↔	3	8	↔	5	2	↔	1	↔	↔
Switch Phase	7	4	↔	3	8	↔	5	2	↔	1	↔	↔
Minimum Initial (s)	7.0	10.0	7.0	10.0	10.0	7.0	20.0	20.0	20.0	7.0	20.0	20.0
Minimum Split (s)	12.0	37.0	12.0	37.0	37.0	12.0	39.0	39.0	39.0	12.0	39.0	39.0
Total Split (s)	28.0	64.0	28.0	48.0	48.0	12.0	40.0	40.0	24.0	52.0	52.0	52.0
Total Split (%)	20.0%	45.7%	8.6%	34.3%	34.3%	8.6%	28.6%	28.6%	17.1%	37.1%	37.1%	37.1%
Maximum Green (s)	23.0	59.0	7.0	41.0	41.0	7.0	33.0	33.0	19.0	45.0	45.0	45.0
Yellow Time (s)	3.0	3.0	3.0	4.0	4.0	3.0	4.0	4.0	4.0	3.0	4.0	4.0
All-Red Time (s)	2.0	2.0	2.0	3.0	3.0	2.0	3.0	3.0	3.0	2.0	3.0	3.0
Lost Time Adjust (s)	-1.0	-3.0	-1.0	-3.0	-3.0	-1.0	-3.0	0.0	-1.0	-3.0	-3.0	-3.0
Total Lost Time (s)	4.0	2.0	4.0	4.0	4.0	4.0	4.0	4.0	7.0	4.0	4.0	4.0
Lead/Lag	Lead	Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lag	Lead	Lag	Lag
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	None	None	None	None	None	None	C-Max	C-Max	None	C-Max	C-Max	C-Max
Walk Time (s)	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0
Flash Dont Walk (s)	11.0	11.0	11.0	11.0	11.0	11.0	11.0	11.0	11.0	11.0	11.0	11.0
Pedestrian Calls (#/hr)	0	0	0	0	0	0	0	0	0	0	0	0
Act Effct Green (s)	24.0	69.2	8.0	44.0	44.0	8.0	36.0	36.0	33.0	20.0	48.0	48.0
Actuated g/C Ratio	0.17	0.49	0.06	0.31	0.31	0.06	0.26	0.24	0.14	0.34	0.34	0.34
v/c Ratio	1.14	0.51	0.15	0.57	1.17	0.87	0.53	0.14	1.16	0.86	0.67	0.67
Control Delay	131.7	25.6	66.6	42.7	121.3	124.2	47.2	0.6	167.3	30.9	10.8	10.8
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	131.7	25.6	66.6	42.7	121.3	124.2	47.2	0.6	167.3	30.9	10.8	10.8
LOS	F	C	E	D	F	F	D	A	F	C	B	B
Approach Delay	72.1	85.3	52.8	52.8	52.8	52.8	52.8	52.8	52.8	52.8	72.2	72.2
Approach LOS	E	F	D	D	D	D	D	D	D	D	E	E
Queue Length 50th (m)	~116.4	78.8	4.3	81.3	~205.8	26.0	64.1	0.0	~101.4	38.5	8.1	8.1
Queue Length 95th (m)	#156.3	116.0	12.1	101.9	#286.1	#59.5	82.8	0.0	#140.6	#200.7	39.5	39.5
Internal Link Dist (m)	70.0	292.9	292.9	292.9	292.9	292.9	147.0	147.0	147.0	242.5	242.5	242.5
Turn Bay Length (m)	80.0	80.0	80.0	25.0	25.0	25.0	60.0	60.0	80.0	80.0	80.0	80.0
Base Capacity (vph)	588	1683	97	1101	646	101	919	503	481	638	734	734
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	1.14	0.51	0.15	0.57	1.17	0.87	0.53	0.14	1.16	0.86	0.67	0.67

Intersection Summary

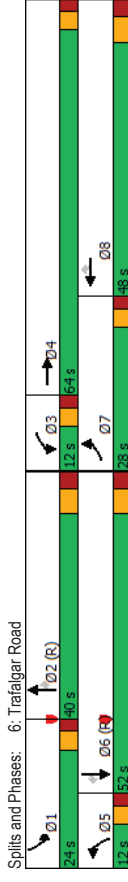
Area Type	Other
Cycle Length	140
Actuated Cycle Length	140
Offset: 54.5 (93%), Referenced to phase 2:NBT and 6:SBT. Start of Green	
Natural Cycle	150
Control Type	Actuated-Coordinated
Maximum v/c Ratio	1.17
Intersection Signal Delay	73.3
Intersection LOS: E	

	ICU Level of Service F
Intersection Capacity Utilization	91.4%
Analysis Period (min)	15
# 95th percentile volume exceeds capacity, queue may be longer.	
Queue shown is maximum after two cycles.	
# 95th percentile volume exceeds capacity, queue may be longer.	
Queue shown is maximum after two cycles.	



	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Detector 2 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Turn Type	Perm	NA	Perm	pm-pt	NA	Perm	pm-pt	NA	Perm	Perm	NA	NA
Permitted Phases	2	2	2	1	6	6	3	8	8	8	4	4
Detector Phase	2	2	2	1	6	6	3	8	8	8	4	4
Switch Phase												
Minimum Initial (s)	22.0	22.0	8.0	22.0	22.0	22.0	5.0	10.0	10.0	10.0	10.0	10.0
Minimum Split (s)	45.0	45.0	12.5	29.0	29.0	29.0	9.5	29.0	29.0	29.0	29.0	29.0
Total Split (s)	42.0	42.0	11.5	53.5	53.5	53.5	8.5	36.5	36.5	36.5	28.0	28.0
Total Split (%)	46.7%	46.7%	12.8%	59.4%	59.4%	59.4%	9.4%	40.6%	40.6%	40.6%	31.1%	31.1%
Maximum Green (s)	36.0	36.0	36.0	7.5	47.5	47.5	4.0	30.5	30.5	30.5	22.0	22.0
Yellow Time (s)	4.0	4.0	4.0	3.0	4.0	4.0	3.5	4.0	4.0	4.0	4.0	4.0
All-Red Time (s)	2.0	2.0	2.0	1.0	2.0	2.0	1.0	2.0	2.0	2.0	2.0	2.0
Lost Time Adjust (s)	-2.0	-2.0	-2.0	0.0	-2.0	-2.0	-2.0	-2.0	0.0	-2.0	-2.0	-2.0
Total Lost Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	2.5	4.0	6.0	4.0	4.0	4.0
Lead/Lag	Lag	Lag	Lag	Lead	Lead	Lead	Lead	Lag	Lag	Lag	Lag	Lag
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Vehicle Extension (s)	5.0	5.0	5.0	2.5	5.0	5.0	3.0	4.0	4.0	4.0	4.0	4.0
Recall Mode	Min	Min	Min	Min	Min	Min	None	Min	Min	Min	Min	Min
Walk Time (s)	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0
Flash Dont Walk (s)	16.0	16.0	16.0	16.0	16.0	16.0	16.0	16.0	16.0	16.0	16.0	16.0
Pedestrian Calls (#/hr)	0	0	0	0	0	0	0	0	0	0	0	0
Act Effct Green (s)	38.0	38.0	38.0	49.5	49.5	49.5	32.0	30.5	28.5	22.0	22.0	22.0
Actuated g/C Ratio	0.43	0.43	0.43	0.56	0.56	0.56	0.36	0.35	0.32	0.25	0.25	0.25
v/c Ratio	0.21	1.03	0.25	1.94	0.31	0.16	2.21	0.04	0.78	0.78	0.67	0.67
Control Delay	18.9	59.9	2.7	479.7	11.1	2.3	591.6	19.3	30.9	49.9	15.0	15.0
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	18.9	59.9	2.7	479.7	11.1	2.3	591.6	19.3	30.9	49.9	15.0	15.0
LOS	B	E	A	F	B	A	F	B	C	C	D	B
Approach Delay	54.8											
Approach LOS	D											
Queue Length 50th (m)	7.6	-154.0	0.0	-53.2	27.4	0.0	-69.6	1.4	17.8	38.9	12.6	12.6
Queue Length 95th (m)	11.9	#163.0	0.8	#98.2	37.8	5.8	#85.3	1.4	24.4	#67.9	23.4	23.4
Internal Link Dist (m)	350.0											
Turn Bay Length (m)	20.0	60.0	20.0	60.0	20.0	60.0	60.0	315	330	330	552	552
Base Capacity (vph)	312	1403	384	108	1828	845	107	315	330	330	552	552
Stevaton Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.21	1.03	0.25	1.94	0.31	0.16	2.21	0.04	0.74	0.71	0.64	0.64
Intersection Summary	Intersection LOS: F											
Area Type:	CBD											
Cycle Length:	90											
Actuated Cycle Length:	88											
Natural Cycle:	140											
Control Type:	Semi Act-Uncoord											
Maximum v/c Ratio:	2.21											
Intersection Signal Delay:	100.2											

Intersection Capacity Utilization 89.7%  
Analysis Period (min) 15  
ICU Level of Service E  
~ Volume exceeds capacity, queue is theoretically infinite.  
Queue shown is maximum after two cycles.  
# 95th percentile volume exceeds capacity, queue may be longer.  
Queue shown is maximum after two cycles.



HCM Signalized Intersection Capacity Analysis  
6: Trafalgar Road

2035 Background AM Peak Hour.syn  
05-04-2022

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	615	680	109	14	582	697	81	445	63	512	502	455
Traffic Volume (vph)	615	680	109	14	582	697	81	445	63	512	502	455
Future Volume (vph)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Ideal Flow (vphpl)	4.0	2.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Total Lost time (s)	0.97	0.95	1.00	0.95	1.00	1.00	0.95	1.00	0.97	1.00	1.00	1.00
Lane Util. Factor	1.00	0.99	1.00	0.93	1.00	0.93	1.00	0.95	1.00	0.95	1.00	0.97
Frpb, ped/bikes	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Flpb, ped/bikes	1.00	0.98	1.00	1.00	0.85	1.00	1.00	0.85	1.00	0.85	1.00	0.85
Flt	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00
Flt Protected	3433	3391	1703	3505	1460	1770	3574	1529	3367	1863	1474	1474
Satd. Flow (prot)	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00
Flt Permitted	3433	3391	1703	3505	1460	1770	3574	1529	3367	1863	1474	1474
Satd. Flow (perm)	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Peak-hour factor, PHF	668	739	118	15	633	758	88	484	68	557	546	495
Adj. Flow (vph)	0	8	0	0	0	181	0	53	0	0	0	237
RTOR Reduction (vph)	668	849	0	15	633	577	88	484	15	557	546	258
Lane Group Flow (vph)	25	8	7	7	7	25	9	9	18	18	18	9
Confl. Peds. (#/hr)	2%	4%	1%	6%	3%	3%	2%	1%	0%	4%	2%	6%
Heavy Vehicles (%)	Prot	NA	Prot	NA	Perm	Prot	NA	Perm	Prot	NA	Perm	NA
Turn Type	7	4	3	3	8	8	5	2	2	1	1	6
Protected Phases												
Permitted Phases	23.0	66.2	2.8	44.0	44.0	7.0	30.0	30.0	19.0	42.0	42.0	6
Actuated Green, G (s)	24.0	69.2	3.8	47.0	47.0	8.0	33.0	30.0	20.0	45.0	45.0	45.0
Effective Green, g (s)	0.17	0.49	0.03	0.34	0.34	0.06	0.24	0.21	0.14	0.32	0.32	0.32
Actuated g/C Ratio	5.0	5.0	5.0	7.0	7.0	5.0	7.0	7.0	5.0	7.0	7.0	7.0
Clearance Time (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Vehicle Extension (s)	588	1676	46	1176	490	101	842	327	481	598	473	
Lane Grp Cap (vph)	e0.19	0.25	0.01	0.18	0.05	0.14			c0.17	c0.29		
v/s Ratio Prot												
v/s Ratio Perm	1.14	0.51	0.33	0.54	1.18	0.87	0.57	0.04	1.16	0.91	0.55	
Uniform Delay, d1	58.0	23.9	66.8	37.7	46.5	65.5	47.3	43.6	60.0	45.6	39.1	
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.63	0.43	0.62	
Incremental Delay, d2	80.6	0.2	4.1	0.5	99.2	50.9	2.8	0.3	88.7	17.5	3.6	
Delay (s)	138.6	24.1	71.0	38.2	145.7	116.4	50.1	43.9	186.3	37.2	27.8	
Level of Service	F	C	E	D	F	F	D	D	D	F	D	C
Approach Delay (s)	74.3			96.5			58.6				86.3	
Approach LOS	E			F			E				F	
Intersection Summary												
HCM 2000 Control Delay	82.1											F
HCM 2000 Volume to Capacity ratio	1.11											16.0
Actuated Cycle Length (s)	140.0											E
Intersection Capacity Utilization	89.7%											
Analysis Period (min)	15											
C Critical Lane Group												

Lanes, Volumes, Timings  
7: GO Bus Terminal/Argus Rd & Cross Ave

2035 Background PM Peak Hour.syn  
05-04-2022

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	44	1175	71	197	534	108	166	3	164	202	24	249
Traffic Volume (vph)	44	1175	71	197	534	108	166	3	164	202	24	249
Future Volume (vph)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Ideal Flow (vphpl)	3.3	3.6	3.6	3.6	3.6	3.6	3.3	3.6	3.6	3.3	3.6	3.6
Lane Width (m)	20.0	20.0	60.0	20.0	60.0	20.0	60.0	0.0	0.0	15.0	0.0	0.0
Storage Lanes	1	1	1	1	1	1	1	1	1	1	1	1
Taper Length (m)	7.5	7.5	7.5	7.5	7.5	7.5	7.5	7.5	7.5	7.5	7.5	7.5
Lane Util. Factor	1.00	0.95	1.00	1.00	0.95	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor	0.99	0.97	0.97	0.97	0.96	0.99	0.96	0.96	0.96	0.98	0.98	0.98
Flt	0.850	0.850	0.850	0.850	0.850	0.850	0.850	0.850	0.850	0.864	0.864	0.864
Flt Protected	0.950	0.950	0.950	0.950	0.950	0.950	0.950	0.950	0.950	0.950	0.950	0.950
Satd. Flow (prot)	1570	3249	727	797	3249	1454	785	855	727	1570	1322	0
Flt Permitted	0.441	0.100	0.100	0.225	0.225	0.225	0.225	0.225	0.225	0.750	0.750	0
Satd. Flow (perm)	723	3249	707	84	3249	1399	185	855	701	1211	1322	0
Right Turn on Red	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Satd. Flow (RTOR)	139	139	133	133	133	133	133	133	133	263	263	50
Link Speed (k/h)	374.0	243.7	50	50	17.5	50	81.9	5.9	20	180.7	180.7	180.7
Travel Time (s)	9	26.9	4	4	17.5	9	12	5.9	20	130.0	130.0	12
Confl. Peds. (#/hr)	0.66	0.81	0.75	0.94	0.95	0.81	0.70	0.25	0.67	0.86	0.75	0.78
Peak Hour Factor	0%	0%	100%	97%	0%	0%	100%	100%	100%	0%	100%	0%
Heavy Vehicles (%)	67	1451	95	210	562	133	237	12	245	235	32	319
Adj. Flow (vph)	67	1451	95	210	562	133	237	12	245	235	32	319
Shared Lane Traffic (%)												
Lane Group Flow (vph)	No	No	No	No	No	No	No	No	No	No	No	No
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Left	7.2	7.2	7.2	7.2	7.2	7.2	7.2	7.2	7.2	7.2	7.2	7.2
Median Width (m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Link Offset (m)	4.8	4.8	4.8	4.8	4.8	4.8	4.8	4.8	4.8	4.8	4.8	4.8
Crosswalk Width (m)	1.14	1.14	1.14	1.14	1.14	1.14	1.14	1.14	1.14	1.14	1.14	1.14
Two way Left Turn Lane	24	24	24	24	24	24	24	24	24	24	24	24
Headway Factor	1	1	1	1	1	1	1	1	1	1	1	1
Turning Speed (k/h)	1	2	1	1	2	1	1	2	1	1	2	1
Number of Detectors	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Detector Template	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Leading Detector (m)	2.0	10.0	2.0	2.0	10.0	2.0	2.0	10.0	2.0	2.0	10.0	2.0
Trailing Detector (m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Position (m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Size (m)	0.0	0.6	2.0	0.6	2.0	0.6	2.0	0.6	2.0	0.6	2.0	0.6
Detector 1 Type	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex
Detector 1 Channel	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 2 Position (m)	9.4	9.4	9.4	9.4	9.4	9.4	9.4	9.4	9.4	9.4	9.4	9.4
Detector 2 Size (m)	0.6	0.6	0.6	0.6	0.6	0.6	0.6	0.6	0.6	0.6	0.6	0.6
Detector 2 Type	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex
Detector 2 Channel	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0

HCM Signalized Intersection Capacity Analysis  
6: Trafalgar Road

2035 Background PM Peak Hour.syn  
05-04-2022

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Traffic Volume (vph)	621	781	239	66	1047	794	170	480	49	643	697	627
Future Volume (vph)	621	781	239	66	1047	794	170	480	49	643	697	627
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	2.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lane Util. Factor	0.97	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.97	1.00	1.00
Frpb, ped/bikes	1.00	0.98	1.00	1.00	0.94	1.00	1.00	0.97	1.00	1.00	1.00	0.95
Flpb, ped/bikes	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt	1.00	0.96	1.00	1.00	0.85	1.00	1.00	0.85	1.00	0.85	1.00	0.85
Flt Protected	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	1.00	0.95
Satd. Flow (prot)	3467	3339	1805	3574	1503	1770	3610	1559	3433	1900	1503	1503
Flt Permitted	0.95	1.00	0.13	1.00	0.12	1.00	1.00	0.12	1.00	0.95	1.00	1.00
Satd. Flow (perm)	3467	3339	241	3574	1503	219	3610	1559	3433	1900	1503	1503
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	675	849	260	72	1138	863	185	522	53	699	758	682
RTOR Reduction (vph)	0	20	0	0	217	0	0	39	0	0	186	496
Lane Group Flow (vph)	675	1089	0	72	1138	646	185	522	14	699	758	496
Conf. Peds. (#/hr)	21	14	14	0%	1%	2%	0%	2%	0%	2%	0%	2%
Heavy Vehicles (%)	1%	3%	1%	0%	1%	1%	2%	0%	0%	2%	0%	2%
Turn Type	Prot	NA	pm-pt	NA	Perm	pm-pt	NA	Perm	Prot	NA	Perm	NA
Protected Phases	7	4	3	3	8	8	5	2	2	1	6	6
Permitted Phases	21.0	57.4	45.6	40.0	40.0	40.0	33.0	33.0	22.0	48.0	48.0	48.0
Actuated Green, G (s)	22.0	60.4	47.6	43.0	43.0	42.0	36.0	36.0	23.0	51.0	51.0	51.0
Effective Green, g (s)	0.16	0.43	0.34	0.31	0.31	0.30	0.26	0.26	0.16	0.36	0.36	0.36
Actuated g/C Ratio	5.0	5.0	5.0	7.0	7.0	7.0	7.0	7.0	5.0	7.0	7.0	7.0
Clearance Time (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Vehicle Extension (s)	544	1440	155	1087	461	154	928	400	583	692	547	547
Lane Grp Cap (vph)	e0.19	0.33	0.02	0.32	0.07	0.14	0.29	0.01	c0.20	c0.40	0.33	0.33
v/s Ratio Prot	1.24	0.76	0.46	1.04	1.40	1.20	0.56	0.03	1.24	1.10	0.91	0.91
v/s Ratio Perm	59.0	33.6	33.6	48.5	48.5	44.4	45.2	39.0	58.5	44.5	42.3	42.3
Uniform Delay, d1	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.32	1.24	1.82	1.82
Progression Factor	123.3	2.3	2.2	37.3	193.4	138.8	2.5	0.2	114.4	51.9	9.5	9.5
Incremental Delay, d2	182.3	35.9	35.8	85.8	241.9	181.1	47.6	39.1	191.7	107.0	86.5	86.5
Delay (s)	F	D	F	D	F	F	D	D	F	F	F	F
Level of Service	F	D	F	D	F	F	D	D	F	F	F	F
Approach Delay (s)	91.3		149.1		79.5				128.1			
Approach LOS	F		F		E				F		F	F
Intersection Summary												
HCM 2000 Control Delay	119.4				HCM 2000 Level of Service				F			
HCM 2000 Volume to Capacity ratio	1.28								16.0			
Actuated Cycle Length (s)	140.0				Sum of lost time (s)				G			
Intersection Capacity Utilization	106.1%				ICU Level of Service							
Analysis Period (min)	15											
C Critical Lane Group												

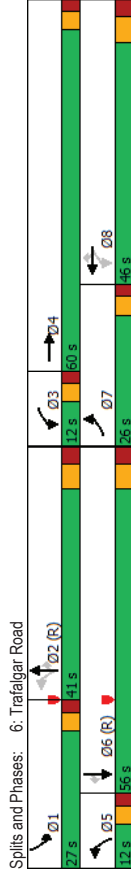
Lanes, Volumes, Timings  
7: GO Bus Terminal/Argus Rd & Cross Ave

2035 Background AM Peak Hour.syn  
05-04-2022

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Traffic Volume (vph)	45	698	81	210	899	50	94	0	237	124	21	648
Future Volume (vph)	45	698	81	210	899	50	94	0	237	124	21	648
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (m)	3.3	3.6	3.6	3.3	3.6	3.6	3.3	3.6	3.6	3.3	3.6	3.6
Storage Length (m)	20.0	60.0	20.0	60.0	20.0	60.0	0.0	0.0	15.0	15.0	0.0	0.0
Storage Lanes	1	1	1	1	1	1	1	1	1	1	1	1
Taper Length (m)	7.5	7.5	7.5	7.5	7.5	7.5	7.5	7.5	7.5	7.5	7.5	7.5
Lane Util. Factor	1.00	0.95	1.00	1.00	0.95	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor	1.00	0.97	1.00	0.98	1.00	0.98	1.00	1.00	0.96	0.98	0.99	0.99
Frt	0.850	0.850	0.850	0.850	0.850	0.850	0.850	0.850	0.850	0.850	0.850	0.850
Flt Protected	0.950	0.950	0.950	0.950	0.950	0.950	0.950	0.950	0.950	0.950	0.950	0.950
Satd. Flow (prot)	1570	3217	727	818	3217	1384	805	1710	761	1570	1386	0
Flt Permitted	0.265	0.187	0.187	0.150	0.150	0.150	0.150	0.150	0.150	0.265	0.757	0
Satd. Flow (perm)	438	3217	708	161	3217	1354	127	1710	734	1222	1386	0
Right Turn on Red	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Satd. Flow (RTOR)	139			91					202		96	
Link Speed (k/h)	50			50			50		50		50	
Link Distance (m)	374.0			243.7			81.9		202		180.7	
Travel Time (s)	1	26.9		17.5			5.9		20		13.0	
Conf. Peds. (#/hr)	1	3	3	3	3	1	3	3	20	20	78	3
Peak Hour Factor	0.52	0.87	0.65	0.84	0.88	0.79	0.53	0.25	0.70	0.78	0.62	0.89
Heavy Vehicles (%)	0%	1%	100%	92%	1%	5%	95%	0%	91%	0%	93%	0%
Adj. Flow (vph)	87	802	125	250	1022	63	177	0	339	159	34	728
Shared Lane Traffic (%)												
Lane Group Flow (vph)	87	802	125	250	1022	63	177	0	339	159	762	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(m)	7.2	7.2	7.2	7.2	7.2	7.2	3.3	3.3	3.3	3.3	3.3	3.3
Link Offset(m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Crosswalk Width(m)	4.8			4.8			4.8		4.8		4.8	
Two way Left Turn Lane												
Headway Factor	1.19	1.14	1.14	1.19	1.14	1.14	1.19	1.14	1.14	1.19	1.14	1.14
Turning Speed (k/h)	24	14	24	14	24	14	24	14	24	14	24	14
Number of Detectors	1	2	1	1	2	1	1	2	1	1	2	1
Detector Template	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Leading Detector (m)	0.0	10.0	2.0	2.0	10.0	2.0	2.0	10.0	2.0	2.0	10.0	10.0
Trailing Detector (m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Position(m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Size(m)	2.0	0.6	2.0	2.0	0.6	2.0	0.6	2.0	0.6	2.0	0.6	2.0
Detector 1 Type	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 2 Position(m)	9.4			9.4			9.4		9.4		9.4	
Detector 2 Size(m)	0.6			0.6			0.6		0.6		0.6	
Detector 2 Type	Ch+Ex			Ch+Ex			Ch+Ex		Ch+Ex		Ch+Ex	
Detector 2 Channel												

	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Detector 2 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Turn Type	Perm	NA	Perm	pm-pt	NA	Perm	pm-pt	3	8	Perm	Perm	NA
Protected Phases		2	2	1	6	6	8	8	8	4	4	
Permitted Phases	2	2	2	1	6	6	3	8	8	4	4	4
Detector Phase	2	2	2	1	6	6	3	8	8	4	4	4
Switch Phase												
Minimum initial (s)	22.0	22.0	8.0	22.0	22.0	22.0	5.0	10.0	10.0	10.0	10.0	10.0
Minimum Split (s)	45.0	45.0	12.5	29.0	29.0	29.0	9.5	29.0	29.0	29.0	29.0	29.0
Total Split (s)	42.0	42.0	11.5	53.5	53.5	53.5	8.5	36.5	36.5	28.0	28.0	28.0
Total Split (%)	46.7%	46.7%	12.8%	59.4%	59.4%	59.4%	9.4%	40.6%	40.6%	31.1%	31.1%	31.1%
Maximum Green (s)	36.0	36.0	36.0	7.5	47.5	47.5	4.0	30.5	30.5	22.0	22.0	22.0
Yellow Time (s)	4.0	4.0	4.0	3.0	4.0	4.0	3.5	4.0	4.0	4.0	4.0	4.0
All-Red Time (s)	2.0	2.0	2.0	1.0	2.0	2.0	1.0	2.0	2.0	2.0	2.0	2.0
Lost Time Adjust (s)	-2.0	-2.0	-2.0	0.0	-2.0	-2.0	-2.0	0.0	0.0	-2.0	-2.0	-2.0
Total Lost Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	2.5	4.0	6.0	4.0	4.0	4.0
Lead/Lag	Lag	Lag	Lag	Lead	Lead	Lead	Lead	Lag	Lag	Lag	Lag	Lag
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Vehicle Extension (s)	5.0	5.0	5.0	2.5	5.0	5.0	3.0	4.0	4.0	4.0	4.0	4.0
Recall Mode	Min	Min	Min	Min	Min	Min	None	Min	Min	Min	Min	Min
Walk Time (s)	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0
Flash Dont Walk (s)	16.0	16.0	16.0	16.0	16.0	16.0	16.0	16.0	16.0	16.0	16.0	16.0
Pedestrian Calls (#/hr)	0	0	0	0	0	0	0	0	0	0	0	0
Act Effct Green (s)	32.5	32.5	32.5	44.0	44.0	44.0	34.1	30.6	24.1	24.1	24.1	24.1
Actuated g/C Ratio	0.38	0.38	0.38	0.52	0.52	0.52	0.40	0.36	0.28	0.28	0.28	0.28
v/c Ratio	0.52	0.65	0.35	1.76	0.61	0.08	1.79	0.86	0.46	1.65	1.65	1.65
Control Delay	32.5	24.0	5.5	389.8	16.0	1.3	412.0	34.7	31.6	323.6	323.6	323.6
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	32.5	24.0	5.5	389.8	16.0	1.3	412.0	34.7	31.6	323.6	323.6	323.6
LOS	C	C	A	F	B	A	F	C	C	C	C	F
Approach Delay	22.4			85.3			164.1			273.2		
Approach LOS	C			F			F			F		F
Queue Length 50th (m)	11.1	57.4	0.0	-52.9	60.5	0.0	-39.7	23.2	22.9	-184.8		
Queue Length 95th (m)	12.6	73.0	1.9	#105.7	76.4	1.9	#38.8	35.6	37.0	#152.2		
Internal Link Dist (m)	350.0			219.7			57.9			156.7		
Turn Bay Length (m)	20.0	60.0	20.0	60.0	60.0	60.0	60.0	15.0	15.0	15.0	15.0	15.0
Base Capacity (vph)	197	1449	395	142	1887	832	99	394	347	463	463	463
Stevation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.44	0.55	0.32	1.76	0.54	0.08	1.79	0.86	0.46	1.65	1.65	1.65
Intersection Summary												
Area Type:	CBD											
Cycle Length:	90											
Actuated Cycle Length:	84.7											
Natural Cycle:	140											
Control Type:	Semi Act-Uncoord											
Maximum v/c Ratio:	1.79											
Intersection Signal Delay:	124.9											
Intersection LOS:	F											

Intersection Capacity Utilization	106.1%	ICU Level of Service	G
Analysis Period (min)	15		
~ Volume exceeds capacity, queue is theoretically infinite.			
Queue shown is maximum after two cycles.			
# 95th percentile volume exceeds capacity, queue may be longer.			
Queue shown is maximum after two cycles.			
m Volume for 95th percentile queue is metered by upstream signal.			



	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Turn Type	Prot	NA	pm-apt	NA	pm-apt	NA	pm-apt	NA	Perm	Prot	NA	Perm
Protected Phases	7	4	3	8	8	8	5	2	2	1	6	
Permitted Phases	7	4	3	8	8	8	5	2	2	1	6	6
Detector Phase	7	4	3	8	8	8	5	2	2	1	6	6
Switch Phase	7	4	3	8	8	8	5	2	2	1	6	6
Minimum Initial (s)	7.0	10.0	7.0	10.0	10.0	7.0	10.0	20.0	20.0	7.0	20.0	20.0
Minimum Split (s)	12.0	37.0	12.0	37.0	37.0	12.0	39.0	39.0	39.0	12.0	39.0	39.0
Total Split (s)	26.0	60.0	26.0	46.0	46.0	12.0	41.0	41.0	27.0	27.0	56.0	56.0
Total Split (%)	18.6%	42.9%	8.6%	32.9%	32.9%	8.6%	29.3%	29.3%	19.3%	40.0%	40.0%	40.0%
Maximum Green (s)	21.0	55.0	7.0	39.0	39.0	7.0	34.0	34.0	22.0	49.0	49.0	49.0
Yellow Time (s)	3.0	3.0	4.0	4.0	4.0	3.0	4.0	4.0	4.0	3.0	4.0	4.0
All-Red Time (s)	2.0	2.0	2.0	3.0	3.0	2.0	3.0	3.0	3.0	2.0	3.0	3.0
Lost Time Adjust (s)	-1.0	-3.0	-1.0	-3.0	-3.0	-1.0	-3.0	-3.0	-1.0	-3.0	-3.0	-3.0
Total Lost Time (s)	4.0	2.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lead/Lag	Lead	Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lag	Lead	Lag	Lag
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	None	None	None	None	None	None	C-Max	C-Max	None	C-Max	C-Max	C-Max
Walk Time (s)	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0
Flash Dont Walk (s)	11.0	11.0	11.0	11.0	11.0	11.0	11.0	11.0	11.0	11.0	11.0	11.0
Pedestrian Calls (#/hr)	0	0	0	0	0	0	0	0	0	0	0	0
Act Effct Green (s)	22.0	60.4	50.0	42.0	42.0	45.0	37.0	37.0	37.0	23.0	52.0	52.0
Actuated g/C Ratio	0.16	0.43	0.36	0.30	0.30	0.32	0.26	0.26	0.26	0.16	0.37	0.37
v/c Ratio	1.24	0.76	0.41	1.06	1.29	1.16	0.55	0.10	1.24	1.08	0.92	0.92
Control Delay	170.9	37.3	28.5	91.9	166.8	152.6	46.9	0.3	173.6	94.3	50.4	50.4
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	170.9	37.3	28.5	91.9	166.8	152.6	46.9	0.3	173.6	94.3	50.4	50.4
LOS	F	D	C	F	F	F	D	A	F	F	D	D
Approach Delay	87.9		120.9			69.4				106.2		
Approach LOS	F		F			E				F		
Queue Length 50th (m)	~125.4	142.6	11.0	~191.5	~252.8	~45.2	69.2	0.0	~133.5	~235.0	127.9	
Queue Length 95th (m)	#165.3	171.8	20.4	#235.7	#335.0	#96.8	88.5	0.0	m#134.3	m#235.6	m125.1	
Internal Link Dist (m)	74.1		292.9			147.0				242.5		
Turn Bay Length (m)	80.0		80.0			25.0				80.0		
Base Capacity (vph)	544	1460	175	1072	670	159	954	549	563	705	741	
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	
Reduced v/c Ratio	1.24	0.76	0.41	1.06	1.29	1.16	0.55	0.10	1.24	1.08	0.92	
Intersection Summary												
Area Type: Other												
Cycle Length: 140												
Actuated Cycle Length: 140												
Offset: 54.5 (99%), Referenced to phase 2:NBTL and 6:SBT, Start of Green												
Natural Cycle: 150												
Control Type: Actuated-Coordinated												
Maximum v/c Ratio: 1.29												
Intersection Signal Delay: 101.7												

	ICU Level of Service H
Intersection Capacity Utilization	111.2%
Analysis Period (min)	15
~ Volume exceeds capacity, queue is theoretically infinite.	
Queue shown is maximum after two cycles.	
# 95th percentile volume exceeds capacity, queue may be longer.	
Queue shown is maximum after two cycles.	
Splits and Phases: 7: GO Bus Terminal/Argus Rd & Cross Ave	

HCM Signalized Intersection Capacity Analysis  
 7: GO Bus Terminal/Argus Rd & Cross Ave

2035 Background AM Peak Hour.syn  
 05-04-2022

Lanes, Volumes, Timings  
 6: Trafalgar Road

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations	45	698	81	210	899	50	94	0	237	124	21	648	
Traffic Volume (vph)	45	698	81	210	899	50	94	0	237	124	21	648	
Future Volume (vph)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	
Ideal Flow (veh/pl)	3.3	3.6	3.6	3.3	3.6	3.3	3.6	3.6	3.3	3.6	3.6	3.6	
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	2.5	6.0	4.0	4.0	4.0	4.0	4.0	
Lane Util. Factor	1.00	0.95	1.00	1.00	0.95	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Frpb, ped/bikes	1.00	1.00	1.00	1.00	1.00	1.00	0.98	1.00	0.97	1.00	0.99	1.00	
Flpb, ped/bikes	1.00	1.00	1.00	1.00	1.00	1.00	0.85	1.00	0.85	1.00	0.86	1.00	
Fit Protected	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	0.95	1.00	0.95	1.00	
Satd. Flow (prot)	1570	3217	708	818	3217	1354	805	735	1536	1386	1386	1386	
Fit Permitted	0.26	1.00	1.00	0.19	1.00	1.00	0.15	1.00	0.76	1.00	0.76	1.00	
Satd. Flow (perm)	438	3217	708	161	3217	1354	127	735	1224	1386	1386	1386	
Peak-hour factor, PHF	0.52	0.87	0.65	0.84	0.88	0.79	0.53	0.25	0.70	0.78	0.62	0.89	
Adj. Flow (vph)	87	802	125	250	1022	63	177	0	339	159	34	728	
RTOR Reduction (vph)	0	0	77	0	0	30	0	0	129	0	69	0	
Lane Group Flow (vph)	87	802	48	250	1022	33	177	0	210	159	693	0	
Conf. Peds. (#/hr)	1	3	3	3	3	1	3	3	20	20	20	3	
Heavy Vehicles (%)	0%	1%	100%	92%	1%	5%	95%	0%	91%	0%	93%	0%	
Turn Type	Perm	NA	Perm	pm+pt	NA	Perm	pm+pt	Perm	pm+pt	Perm	Perm	NA	
Protected Phases	2	2	2	6	6	6	8	8	8	8	4	4	
Permitted Phases	2	2	2	6	6	6	8	8	8	8	4	4	
Actuated Green, G (s)	30.5	30.5	30.5	42.0	42.0	42.0	30.6	30.6	30.6	22.1	22.1	22.1	
Effective Green, g (s)	32.5	32.5	32.5	42.0	44.0	44.0	32.6	32.6	30.6	24.1	24.1	24.1	
Actuated g/C Ratio	0.38	0.38	0.38	0.50	0.52	0.52	0.39	0.36	0.36	0.28	0.28	0.28	
Clearance Time (s)	6.0	6.0	6.0	4.0	6.0	6.0	4.5	6.0	6.0	6.0	6.0	6.0	
Vehicle Extension (s)	5.0	5.0	5.0	2.5	5.0	5.0	3.0	4.0	4.0	4.0	4.0	4.0	
Lane Grp Cap (vph)	168	1235	271	138	1673	704	97	265	348	394	394	394	
v/s Ratio Prot	0.25	c0.16	0.32	c0.13	c0.13	c0.13	0.50	0.50	0.29	0.13	0.50	0.50	
v/s Ratio Perm	0.20	0.07	c0.74	0.02	c0.57	0.02	c0.57	0.29	0.13	0.13	0.13	0.13	
v/c Ratio	0.52	0.65	0.18	1.81	0.61	0.61	1.82	0.79	0.46	1.76	1.76	1.76	
Uniform Delay, d1	20.0	21.4	17.2	17.8	14.3	10.0	24.0	24.2	24.9	30.2	30.2	30.2	
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Incremental Delay, d2	5.3	1.6	0.7	392.3	1.0	0.1	408.4	15.7	1.3	352.2	352.2	352.2	
Level of Service	C	C	B	F	B	B	F	B	D	C	F	F	
Approach Delay (s)	22.6	C	B	F	88.9	F	174.5	F	C	F	320.9	F	
Approach LOS	C	C	B	F	F	F	F	F	F	F	F	F	
Intersection Summary	139.3	HCM 2000 Level of Service											F
HCM 2000 Control Delay	1.90	HCM 2000 Volume to Capacity ratio											F
HCM 2000 Volume to Capacity ratio	84.6	Sum of lost time (s)											14.5
Actuated Cycle Length (s)	111.2%	ICU Level of Service											H
Intersection Capacity Utilization	15												0.6
Analysis Period (min)													Critical Lane Group
c Critical Lane Group													C+Ex

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	2	7.5	7.5	7.5	7.5	7.5	7.5	7.5	7.5	7.5	7.5	7.5
Traffic Volume (vph)	621	781	239	66	1047	794	170	480	49	643	697	627
Future Volume (vph)	621	781	239	66	1047	794	170	480	49	643	697	627
Ideal Flow (veh/pl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (m)	80.0	0.0	80.0	0.0	80.0	0.0	25.0	0.0	80.0	0.0	80.0	0.0
Storage Lanes	2	0	0	1	1	1	1	1	1	1	1	1
Taper Length (m)	7.5	7.5	7.5	7.5	7.5	7.5	7.5	7.5	7.5	7.5	7.5	7.5
Lane Util. Factor	0.97	0.95	0.95	1.00	0.95	1.00	1.00	0.95	1.00	0.97	1.00	1.00
Ped Bike Factor	0.99	0.98	0.98	0.94	0.94	0.94	0.94	0.94	0.97	0.98	0.95	0.95
Fit	0.965	0.965	0.965	0.850	0.850	0.850	0.850	0.850	0.97	0.96	0.850	0.850
Fit Protected	0.950	0.950	0.950	0.950	0.950	0.950	0.950	0.950	0.950	0.950	0.950	0.950
Satd. Flow (prot)	3467	3339	0	1805	3574	1599	1770	3610	1615	3433	1900	1583
Fit Permitted	0.950	0.950	0.950	0.127	0.127	0.118	0.118	0.118	0.118	0.118	0.118	0.118
Satd. Flow (perm)	3432	3339	0	241	3574	1503	220	3610	1559	3368	1900	1503
Right Turn on Red	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Satd. Flow (RTOR)	35	35	0	313	313	313	313	313	187	187	292	292
Link Speed (k/h)	50	50	50	50	50	50	50	50	50	50	50	50
Link Distance (m)	98.1	98.1	98.1	316.9	316.9	316.9	316.9	316.9	171.0	171.0	266.5	266.5
Travel Time (s)	7.1	7.1	7.1	22.8	22.8	22.8	22.8	22.8	12.3	12.3	19.2	19.2
Conf. Peds. (#/hr)	21	14	14	14	14	14	17	17	10	10	10	17
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	1%	3%	1%	0%	1%	1%	2%	2%	0%	0%	2%	2%
Adj. Flow (vph)	675	849	260	72	1138	863	185	522	53	699	758	682
Shared Lane Traffic (%)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Lane Group Flow (vph)	675	1109	0	72	1138	863	185	522	53	699	758	682
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Right	Right	Left	Left	Right	Left	Left	Left	Left	Left	Right
Median Width (m)	7.2	7.2	7.2	7.2	7.2	7.2	7.2	7.2	7.2	7.2	7.2	7.2
Link Offset (m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Crosswalk Width (m)	4.8	4.8	4.8	4.8	4.8	4.8	4.8	4.8	4.8	4.8	4.8	4.8
Two way Left Turn Lane	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Headway Factor	25	15	25	25	25	25	25	25	15	25	15	25
Turning Speed (k/h)	1	2	1	2	1	2	1	2	1	2	1	2
Number of Detectors	1	2	1	2	1	2	1	2	1	2	1	2
Detector Template	Left Thru	Left Thru	Right Thru	Left Thru	Left Thru	Right Thru	Left Thru	Left Thru	Right Thru	Left Thru	Right Thru	Right Thru
Leading Detector (m)	2.0	10.0	2.0	10.0	2.0	10.0	2.0	10.0	2.0	10.0	2.0	10.0
Trailing Detector (m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Position (m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Size (m)	2.0	0.6	2.0	0.6	2.0	0.6	2.0	0.6	2.0	0.6	2.0	0.6
Detector 1 Type	C+Ex	C+Ex	C+Ex	C+Ex	C+Ex	C+Ex	C+Ex	C+Ex	C+Ex	C+Ex	C+Ex	C+Ex
Detector 1 Channel	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 2 Position (m)	9.4	9.4	9.4	9.4	9.4	9.4	9.4	9.4	9.4	9.4	9.4	9.4
Detector 2 Size (m)	0.6	0.6	0.6	0.6	0.6	0.6	0.6	0.6	0.6	0.6	0.6	0.6
Detector 2 Type	C+Ex	C+Ex	C+Ex	C+Ex	C+Ex	C+Ex	C+Ex	C+Ex	C+Ex	C+Ex	C+Ex	C+Ex
Detector 2 Channel	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 2 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0

HCM Signalized Intersection Capacity Analysis  
 5: Trafalgar Road & Cross Avenue/South Service Road  
 2035 Background PM Peak Hour.syn  
 05-04-2022

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBR	Diagram
Lane Configurations	↑↑	↑	↑	↑↑	↑	↑	↑↑	↑	↑	↑	↑	Diagram 1
Traffic Volume (vph)	1240	405	205	294	331	413	224	1474	197	172	1470	584
Future Volume (vph)	1240	405	205	294	331	413	224	1474	197	172	1470	584
Ideal Flow (veh/pl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	7.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lane Util. Factor	0.97	0.95	1.00	0.97	1.00	0.95	1.00	0.91	1.00	0.91	1.00	1.00
Frb. ped/bikes	1.00	1.00	0.95	1.00	1.00	1.00	1.00	1.00	0.88	1.00	1.00	0.95
Flpb. ped/bikes	1.00	1.00	0.85	1.00	1.00	0.85	1.00	1.00	0.85	1.00	1.00	0.85
Flt Protected	0.95	1.00	1.00	0.95	1.00	0.95	1.00	1.00	0.85	1.00	1.00	0.85
Satd. Flow (prot)	3400	3343	1413	3433	3471	1599	1719	5136	1422	1805	5136	1485
Flt Permitted	0.95	1.00	1.00	0.95	1.00	0.95	1.00	1.00	0.11	1.00	1.00	1.00
Satd. Flow (perm)	3400	3343	1413	3433	3471	1599	181	5136	1422	200	5136	1485
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	1348	440	223	320	360	449	243	1602	214	187	1598	635
RTOR Reduction (vph)	0	0	124	0	0	93	0	85	0	0	238	0
Lane Group Flow (vph)	1348	440	99	320	360	356	243	1602	129	187	1598	397
Conf. Peds. (#/hr)	3%	8%	15%	15%	15%	4%	1%	5%	1%	0%	0%	3%
Heavy Vehicles (%)	3%	8%	15%	15%	15%	4%	1%	5%	1%	0%	0%	3%
Turn Type	Prot	NA	Perm	Prot	NA	pm+ov	pm-pt	NA	pm+ov	pm-pt	NA	Perm
Protected Phases	7	4		3	8	1	5	2	3	1	6	
Permitted Phases			4			8	2		2	6		6
Actuated Green, G (s)	46.0	48.7	17.6	17.3	28.9	53.8	40.1	57.7	49.6	38.0	38.0	41.0
Effective Green, g (s)	49.0	51.7	48.7	17.6	20.3	28.9	53.8	43.1	57.7	49.6	41.0	41.0
Actuated g/C Ratio	0.35	0.37	0.35	0.13	0.15	0.21	0.38	0.31	0.41	0.35	0.29	0.29
Clearance Time (s)	7.0	7.0	4.0	7.0	4.0	7.0	4.0	7.0	4.0	7.0	7.0	7.0
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Lane Grp Cap (vph)	1190	1234	491	431	503	330	220	1581	586	203	1504	434
v/s Ratio Prot	c0.40	0.13		0.09	0.10	c0.09	c0.11	0.31	0.03	0.08	0.31	
v/s Ratio Perm			0.07			0.13	c0.32		0.06	0.25		0.27
vic Ratio	1.13	0.36	0.20	0.74	0.72	1.08	1.10	1.01	0.22	0.92	1.06	0.92
Uniform Delay, d1	45.5	32.1	32.0	59.0	57.1	55.5	40.8	48.4	26.6	37.1	49.5	47.8
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.34	1.07	2.19	1.03	0.73	0.48
Incremental Delay, d2	70.7	0.2	0.2	6.8	4.8	72.3	53.8	10.4	0.0	25.3	35.8	15.3
Delay (s)	116.2	32.2	32.2	65.8	61.9	127.9	108.7	62.1	58.3	63.6	72.1	38.3
Level of Service	F	C	C	E	E	F	F	E	E	E	E	D
Approach Delay (s)		88.5		89.2		67.2		67.2			62.6	
Approach LOS		F		F		E		E			E	
Intersection Summary												
HCM 2000 Control Delay												E
HCM 2000 Volume to Capacity ratio												1.08
Actuated Cycle Length (s)												140.0
Intersection Capacity Utilization												99.4%
Analysis Period (min)												15
c Critical Lane Group												E

Lanes, Volumes, Timings  
 8: Cross Ave & Lyons Lane  
 2035 Background AM Peak Hour.syn  
 05-04-2022

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBR	Diagram
Lane Configurations	↑↑	↑	↑	↑↑	↑	↑	↑↑	↑	↑	↑	↑	Diagram 2
Traffic Volume (vph)	93	327	133	247	112	32	54	1	107	99	28	77
Future Volume (vph)	93	327	133	247	112	32	54	1	107	99	28	77
Ideal Flow (veh/pl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (m)	60.0	60.0	60.0	60.0	60.0	60.0	60.0	60.0	60.0	60.0	60.0	60.0
Storage Lanes	1	1	1	1	1	1	1	1	1	1	1	1
Taper Length (m)	7.5		7.5		7.5		7.5		7.5		7.5	7.5
Lane Util. Factor	1.00	0.95	1.00	1.00	0.95	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor	0.99				0.97					0.99		
Flt Protected	0.950		0.850		0.950		0.950		0.850		0.850	0.850
Satd. Flow (prot)	1624	3094	1425	1593	2801	1454	1593	1676	1425	1624	1676	1163
Flt Permitted	0.671		0.244		0.738		0.738		0.671		0.671	0.671
Satd. Flow (perm)	1138	3094	1425	409	2801	1410	1237	1676	1425	1281	1676	1163
Right Turn on Red		Yes	Yes		Yes		Yes		Yes		Yes	Yes
Satd. Flow (RTOR)		50	145		50	64		50	116		50	154
Link Speed (k/h)		281.3		40.1		149.2		10.7			41.7	
Link Distance (m)		20.3		2.9		107.7		3.0			3.0	
Travel Time (s)		4		4		7		7			7	
Conf. Peds. (#/hr)		0.69	0.68	0.92	0.88	0.50	0.92	0.92	0.92	0.62	0.92	0.50
Peak Hour Factor		0%	5%	2%	2%	16%	0%	2%	2%	0%	2%	25%
Heavy Vehicles (%)		135	564	145	268	127	64	59	1	116	160	30
Adj. Flow (vph)		135	564	145	268	127	64	59	1	116	160	30
Shared Lane Traffic (%)												
Lane Group Flow (vph)		135	564	145	268	127	64	59	1	116	160	30
Enter Blocked Intersection		No	No	No	No	No	No	No	No	No	No	No
Lane Alignment		Left	Right	Left	Right	Left	Left	Right	Left	Right	Left	Right
Median Width (m)		3.6		3.6		3.6		3.6		3.6		3.6
Link Offset (m)		0.0		0.0		0.0		0.0		0.0		0.0
Crosswalk Width (m)		4.8		4.8		4.8		4.8		4.8		4.8
Two way Left Turn Lane												
Headway Factor	1.14	1.14	1.14	1.14	1.14	1.14	1.14	1.14	1.14	1.14	1.14	1.14
Turning Speed (k/h)	25	15	25	15	25	15	25	15	25	15	25	15
Number of Detectors	1	2	1	1	2	1	1	2	1	1	2	1
Detector Template	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Leading Detector (m)	2.0	10.0	2.0	2.0	10.0	2.0	2.0	10.0	2.0	2.0	10.0	2.0
Trailing Detector (m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Position (m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Size (m)	2.0	0.6	2.0	2.0	0.6	2.0	0.6	2.0	0.6	2.0	0.6	2.0
Detector 1 Type	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 2 Position (m)		9.4		9.4		9.4		9.4		9.4		9.4
Detector 2 Size (m)		0.6		0.6		0.6		0.6		0.6		0.6
Detector 2 Type		Ch+Ex		Ch+Ex		Ch+Ex		Ch+Ex		Ch+Ex		Ch+Ex
Detector 2 Channel												
Detector 2 Extend (s)		0.0		0.0		0.0		0.0		0.0		0.0



Lanes, Volumes, Timings  
8: Cross Ave & Lyons Lane

2035 Background AM Peak Hour.syn  
05-04-2022

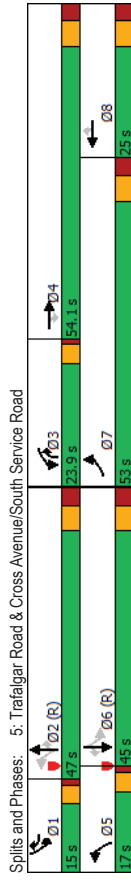
	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Perm	NA	Perm	pm+pt	NA	Perm	NA	Perm	NA	Perm	NA	Perm	NA
Protected Phases	4	4	4	8	8	8	2	2	2	6	6	6
Detector Phase	4	4	4	3	8	8	2	2	2	6	6	6
Switch Phase												
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Minimum Split (s)	22.5	22.5	22.5	9.5	22.5	22.5	22.5	22.5	22.5	22.5	22.5	22.5
Total Split (s)	32.0	32.0	32.0	26.0	58.0	58.0	32.0	32.0	32.0	32.0	32.0	32.0
Total Split (%)	35.6%	35.6%	35.6%	28.9%	64.4%	64.4%	35.6%	35.6%	35.6%	35.6%	35.6%	35.6%
Maximum Green (s)	27.5	27.5	27.5	21.5	53.5	53.5	27.5	27.5	27.5	27.5	27.5	27.5
Yellow Time (s)	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5
Lead/Lag	Lag	Lag	Lag	Lead	Lead	Lead	Lag	Lag	Lag	Lag	Lag	Lag
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	None	None	None	None	None	None	Max	Max	Max	Max	Max	Max
Walk Time (s)	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0
Flash Dont Walk (s)	11.0	11.0	11.0	11.0	11.0	11.0	11.0	11.0	11.0	11.0	11.0	11.0
Pedestrian Calls (#/hr)	0	0	0	0	0	0	0	0	0	0	0	0
Act Effct Green (s)	19.4	19.4	19.4	36.8	36.8	36.8	27.8	27.8	27.8	27.8	27.8	27.8
Actuated g/C Ratio	0.26	0.26	0.26	0.50	0.50	0.50	0.38	0.38	0.38	0.38	0.38	0.38
v/c Ratio	0.45	0.69	0.30	0.65	0.09	0.09	0.13	0.00	0.19	0.33	0.05	0.29
Control Delay	28.3	28.5	5.9	18.7	9.2	2.7	19.0	19.0	5.2	21.3	18.2	5.4
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	28.3	29.5	5.9	18.7	9.2	2.7	19.0	19.0	5.2	21.3	18.2	5.4
LOS	C	C	A	B	A	A	B	B	A	C	B	A
Approach Delay		25.3			13.8			9.9			13.9	
Approach LOS		C			B			A			B	
Queue Length 50th (m)	16.3	38.5	0.0	21.8	4.7	0.0	5.4	0.1	0.0	16.0	2.7	0.0
Queue Length 95th (m)	24.9	34.3	12.8	36.0	8.4	0.8	16.6	1.3	11.4	24.9	9.7	0.0
Internal Link Dist (m)		257.3			16.1		125.2				17.7	
Turn Bay Length (m)	60.0	60.0	60.0	60.0	60.0	60.0	60.0	60.0	60.0	60.0	60.0	60.0
Base Capacity (vph)	429	1168	628	553	2058	1053	467	632	610	483	632	534
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.31	0.48	0.23	0.48	0.06	0.06	0.13	0.00	0.19	0.33	0.05	0.29

Intersection Summary	
Area Type:	CBD
Cycle Length:	90
Actual Cycle Length:	73.7
Natural Cycle:	60
Control Type:	Semi Act-Uncoordinated
Maximum v/c Ratio:	0.69
Intersection Signal Delay:	18.8
Intersection Capacity Utilization:	49.3%

Lanes, Volumes, Timings  
5: Trafalgar Road & Cross Avenue/South Service Road

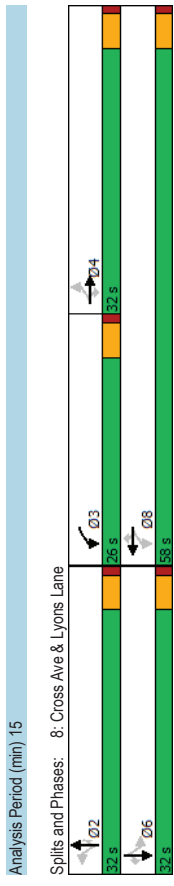
2035 Background PM Peak Hour.syn  
05-04-2022

Intersection Summary	
Area Type:	CBD
Cycle Length:	90
Actual Cycle Length:	73.7
Natural Cycle:	60
Control Type:	Semi Act-Uncoordinated
Maximum v/c Ratio:	0.69
Intersection Signal Delay:	18.8
Intersection Capacity Utilization:	49.3%



Splits and Phases: 5: Trafalgar Road & Cross Avenue/South Service Road

- ~ Volume exceeds capacity, queue is theoretically infinite. Queue shown is maximum after two cycles.
- # 95th percentile volume exceeds capacity, queue may be longer.
- ~ Queue shown is maximum after two cycles.
- m Volume for 95th percentile queue is metered by upstream signal.



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Turn Type	Prot	NA	Perm	Prot	NA	pm+ov	pm+pt	NA	pm+ov	pm+pt	NA	Perm
Protected Phases	7	4	4	3	8	8	2	2	3	1	6	6
Permitted Phases	7	4	4	3	8	1	5	2	3	1	6	6
Detector Phase												
Switch Phase												
Minimum Initial (s)	10.0	5.0	5.0	10.0	10.0	7.0	7.0	27.0	5.0	7.0	27.0	27.0
Minimum Split (s)	17.0	25.0	25.0	9.5	25.0	11.5	11.5	34.0	9.5	11.5	34.0	34.0
Total Split (s)	53.0	54.1	54.1	23.9	25.0	15.0	17.0	47.0	23.9	15.0	45.0	45.0
Total Split (%)	37.9%	38.6%	38.6%	17.1%	17.9%	10.7%	12.1%	33.6%	17.1%	10.7%	32.1%	32.1%
Maximum Green (s)	46.0	47.1	47.1	19.9	18.0	11.0	13.0	40.0	19.9	11.0	38.0	38.0
Yellow Time (s)	4.0	4.0	4.0	3.0	4.0	3.0	4.0	3.0	4.0	3.0	4.0	4.0
All-Red Time (s)	3.0	3.0	3.0	1.0	3.0	1.0	1.0	3.0	1.0	1.0	3.0	3.0
Lost Time Adjust (s)	-3.0	-3.0	-3.0	0.0	-3.0	0.0	0.0	-3.0	0.0	0.0	-3.0	-3.0
Total Lost Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lead/Lag	Lead	Lag	Lag	Lead	Lag	Lead	Lead	Lag	Lead	Lead	Lag	Lag
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	None	None	None	None	None	None	C-Max	None	None	None	C-Max	C-Max
Walk Time (s)	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0
Flash Dont Walk (s)	11.0	11.0	11.0	11.0	11.0	11.0	11.0	11.0	11.0	11.0	11.0	11.0
Pedestrian Calls (#/hr)	0	0	0	0	0	0	0	0	0	0	0	0
Act Effct Green (s)	49.0	51.7	48.7	17.6	20.3	35.9	58.8	43.1	60.7	52.6	41.0	41.0
Actuated g/C Ratio	0.35	0.37	0.35	0.13	0.14	0.26	0.41	0.31	0.43	0.38	0.29	0.29
v/c Ratio	1.13	0.36	0.36	0.74	0.72	0.91	1.08	1.01	0.31	0.90	1.06	0.94
Control Delay	112.2	33.3	8.7	69.9	65.7	59.8	90.1	60.8	16.5	58.8	71.5	26.5
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	112.2	33.3	8.7	69.9	65.7	59.8	90.1	60.8	16.5	58.8	71.5	26.5
LOS	F	C	A	E	E	E	F	E	B	E	E	C
Approach Delay	83.5			64.6			59.7				58.7	
Approach LOS	F			E			E				E	
Queue Length 50th (m)	~234.6	48.7	6.4	46.8	53.0	98.4	-66.9	-183.4	18.5	33.6	-190.6	112.3
Queue Length 95th (m)	#278.6	65.1	27.2	62.9	71.1	#163.4	m#63.8	m124.1	m15.1	m#49.2	m#219.6	m#161.5
Internal Link Dist (m)	219.7			214.9			242.5				247.0	
Turn Bay Length (m)	25.0			60.0			50.0				60.0	
Base Capacity (vph)	1190	1235	615	487	520	496	224	1582	721	207	1504	672
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	1.13	0.36	0.36	0.66	0.69	0.91	1.08	1.01	0.30	0.90	1.06	0.94

Intersection Summary	
Area Type:	Other
Cycle Length:	140
Actuated Cycle Length:	140
Offset:	54.5 (39%), Referenced to phase 2:NBTL and 6:SBTL, Start of Green
Natural Cycle:	140
Control Type:	Actuated-Coordinated
Maximum v/c Ratio:	1.13
Intersection Signal Delay:	66.4
Intersection LOS:	E

HCM Signalized Intersection Capacity Analysis  
 8: Cross Ave & Lyons Lane

2035 Background AM Peak Hour.syn

05-04-2022

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations	←	←	←	←	←	←	←	←	←	←	←	←	
Traffic Volume (vph)	93	327	133	247	112	32	54	1	107	99	28	77	
Future Volume (vph)	93	327	133	247	112	32	54	1	107	99	28	77	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	
Total Lost time (s)	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Frb. ped/bikes	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Frb. ped/bikes	0.99	1.00	0.85	1.00	1.00	0.85	1.00	1.00	0.85	1.00	1.00	0.85	
Frt	1.00	1.00	0.85	1.00	1.00	0.85	1.00	1.00	0.85	1.00	1.00	0.85	
Flt Protected	0.95	1.00	1.00	0.95	1.00	1.00	1.00	1.00	0.95	1.00	1.00	1.00	
Satd. Flow (prot)	1613	3094	1425	1593	2801	1413	1593	1676	1425	1611	1676	1163	
Flt Permitted	0.67	1.00	1.00	0.24	1.00	1.00	0.74	1.00	1.00	0.76	1.00	1.00	
Satd. Flow (perm)	1140	3094	1425	410	2801	1413	1237	1676	1425	1284	1676	1163	
Peak-hour factor, PHF	0.69	0.68	0.92	0.92	0.88	0.50	0.92	0.92	0.92	0.82	0.92	0.50	
Adj. Flow (vph)	135	564	145	268	127	64	59	1	116	160	30	154	
RTOR Reduction (vph)	0	0	107	0	0	32	0	0	72	0	0	96	
Lane Group Flow (vph)	135	564	38	268	127	32	59	1	44	160	30	58	
Conf. Peds. (#/hr)	4					4						7	
Heavy Vehicles (%)	0%	5%	2%	2%	16%	0%	2%	2%	2%	0%	2%	25%	
Turn Type	Perm	NA	Perm	pm+pt	NA	Perm	Perm	NA	Perm	Perm	NA	Perm	
Protected Phases	4		4	8	3	8	2	2				6	
Permitted Phases	4		4	8	3	8	2	2				6	
Actuated Green, G (s)	19.5	19.5	19.5	36.8	36.8	36.8	27.8	27.8	27.8	27.8	27.8	27.8	
Effective Green, g (s)	19.5	19.5	19.5	36.8	36.8	36.8	27.8	27.8	27.8	27.8	27.8	27.8	
Actuated g/C Ratio	0.26	0.26	0.26	0.50	0.50	0.50	0.38	0.38	0.38	0.38	0.38	0.38	
Clearance Time (s)	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5	
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	
Lane Grp Cap (vph)	302	819	377	410	1400	706	467	633	538	484	633	439	
v/s Ratio Prot	0.18		c0.11	0.05		0.00		0.00			0.02		
v/s Ratio Perm	0.12		0.03	c0.21	0.02	0.05		0.03	c0.12		0.05		
vic Ratio	0.45	0.69	0.10	0.65	0.09	0.05	0.13	0.00	0.08	0.33	0.05	0.13	
Uniform Delay, d1	22.6	24.3	20.4	12.3	9.6	9.4	15.0	14.3	14.7	16.3	14.5	15.0	
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Incremental Delay, d2	1.1	2.4	0.1	3.7	0.0	0.0	0.6	0.0	0.3	1.8	0.1	0.6	
Delay (s)	C	C	C	B	A	A	B	B	B	B	B	B	
Level of Service	C	C	C	B	A	A	B	B	B	B	B	B	
Approach Delay (s)	25.2			13.3			15.2				16.7		
Approach LOS	C			B			B				B		
<b>Intersection Summary</b>													
HCM 2000 Control Delay	19.6					HCM 2000 Level of Service			B				
HCM 2000 Volume to Capacity ratio	0.54												
Actuated Cycle Length (s)	73.6												
Sum of lost time (s)	13.5												
Intersection Capacity Utilization	49.3%												
ICU Level of Service	A												
Analysis Period (min)	15												
c Critical Lane Group													

Lanes, Volumes, Timings  
 5: Trafalgar Road & Cross Avenue/South Service Road

05-04-2022

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	←	←	←	←	←	←	←	←	←	←	←	←
Traffic Volume (vph)	1240	405	205	294	331	413	224	1474	197	172	1470	584
Future Volume (vph)	1240	405	205	294	331	413	224	1474	197	172	1470	584
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (m)	25.0		60.0	25.0		60.0	50.0		60.0	25.0		60.0
Storage Lanes	2		1	2		1	1		1	1		1
Taper Length (m)	7.5		7.5		7.5		7.5		7.5		7.5	
Lane Util. Factor	0.97	0.95	1.00	0.97	0.95	1.00	1.00	0.91	1.00	1.00	0.91	1.00
Ped Bike Factor		0.95	0.98		0.98			0.83		0.83		0.95
Flt		0.850		0.850		0.850		0.850		0.850		0.850
Flt Protected		0.950		0.950		0.950		0.950		0.950		0.950
Satd. Flow (prot)	3400	3343	1482	3433	3471	1599	1719	5136	1615	1805	5136	1568
Flt Permitted	0.950			0.950			0.100			0.105		
Satd. Flow (perm)	3400	3343	1413	3368	3471	1599	181	5136	1337	200	5136	1485
Right Turn on Red		Yes		Yes		Yes		Yes		Yes		Yes
Satd. Flow (RTOR)		190		117		145		50		145		336
Link Speed (k/h)		50		50		50		266.5		19.2		271.0
Link Distance (m)		243.7		238.9		17.2		19.2		19.2		19.5
Travel Time (s)		17.5		15		15		18		70		70
Conf. Peds. (#/hr)		0.92		0.92		0.92		0.92		0.92		0.92
Peak Hour Factor		3%		8%		9%		2%		4%		1%
Heavy Vehicles (%)		3%		9%		2%		5%		1%		0%
Adj. Flow (vph)		1348		440		223		320		360		214
Shared Lane Traffic (%)		1348		440		223		320		360		214
Lane Group Flow (vph)		1348		440		223		320		360		214
Enter Blocked Intersection		No		No		No		No		No		No
Lane Alignment		Left		Right		Left		Left		Right		Left
Median Width (m)		7.2		7.2		7.2		3.6		3.6		3.6
Link Offset (m)		0.0		0.0		0.0		0.0		0.0		0.0
Crosswalk Width (m)		4.8		4.8		4.8		4.8		4.8		4.8
<b>Two way Left Turn Lane</b>												
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (k/h)	25	15	25	15	25	15	25	15	25	15	25	15
Number of Detectors	1	2	1	1	2	1	1	1	2	1	1	2
Detector Template	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Leading Detector (m)	2.0	10.0	2.0	2.0	10.0	2.0	2.0	10.0	2.0	2.0	10.0	2.0
Trailing Detector (m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Position (m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Size (m)	2.0	0.6	2.0	2.0	0.6	2.0	2.0	0.6	2.0	2.0	0.6	2.0
Detector 1 Type	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 2 Position (m)	9.4			9.4				9.4				9.4
Detector 2 Size (m)	0.6			0.6				0.6				0.6
Detector 2 Type	Ch+Ex			Ch+Ex				Ch+Ex				Ch+Ex
Detector 2 Channel												
Detector 2 Extend (s)	0.0			0.0				0.0				0.0

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations			↔				↔	↔	↔	↔	↔	↔
Traffic Volume (veh/h)	0	0	54	0	0	0	0	2187	940	0	2165	485
Future Volume (Veh/h)	0	0	54	0	0	0	0	2187	940	0	2165	485
Sign Control	Stop	Stop	Stop	Stop	0%	0%	Free	Free	Free	Free	Free	Free
Grade	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	0	0	59	0	0	0	0	2377	1022	0	2353	527
Pedestrians		24										
Lane Width (m)		3.6										
Walking Speed (m/s)		1.2										
Percent Blockage		2										
Right turn flare (veh)							None	None	None	None	None	None
Median type												
Median storage (veh)												
Upstream signal (m)							271				51	
pX, platoon unblocked	0.73	0.73	0.61	0.73	0.73	0.75	0.61			0.75		
vC, conflicting volume	3433	5018	1072	3161	5281	792	2904			2377		
vC1, stage 1 conf/vol												
vC2, stage 2 conf/vol												
vCu, unblocked vol	783	2939	0	413	3297	0	1891			1682		
tC, single (s)	7.5	6.5	7.1	7.5	6.5	6.9	4.1			4.1		
tC, 2 stage (s)												
tF (s)	3.5	4.0	3.4	3.5	4.0	3.3	2.2			2.2		
p0 queue free %	100	100	91	100	100	100	100			100		
cM capacity (veh/h)	204	11	638	347	6	822	192			291		
Direction_Lane #	EB 1	NB 1	NB 2	NB 3	NB 4	SB 1	SB 2	SB 3				
Volume Total	59	792	792	792	1022	941	941	998				
Volume Left	0	0	0	0	0	0	0	0				
Volume Right	59	0	0	0	1022	0	0	527				
GSH	638	1700	1700	1700	1700	1700	1700	1700				
Volume to Capacity	0.09	0.47	0.47	0.47	0.60	0.55	0.55	0.59				
Queue Length 95th (m)	2.4	0.0	0.0	0.0	0.0	0.0	0.0	0.0				
Control Delay (s)	11.2	0.0	0.0	0.0	0.0	0.0	0.0	0.0				
Lane LOS	B	B	B	B	B	B	B	B				
Approach Delay (s)	11.2	0.0				0.0						
Approach LOS	B	B				B						
Intersection Summary												
Average Delay	0.1											
Intersection Capacity Utilization	63.1%											
Analysis Period (min)	15											
ICU Level of Service	B											

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations			↔									
Traffic Volume (vph)	0	10	8	8	8	8	8	8	8	2		
Future Volume (vph)	0	10	8	8	8	8	8	8	8	2		
Ideal Flow (veh/pl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00		
Ped Bike Factor												
Fit		0.932								0.963		
Fit Protected										0.965		
Satd. Flow (prot)		0	1710	1594	0	1283	0			1283	0	
Fit Permitted										0.965		
Satd. Flow (perm)		0	1710	1594	0	1283	0			1283	0	
Link Speed (k/h)			50	50	50	50	50			50		
Link Distance (m)			60.5	89.6		37.6				37.6		
Travel Time (s)			4.4	6.5		2.7				2.7		
Conf. Peds. (#/hr)		6				6				1		
Peak Hour Factor		0.25	0.50	0.75	0.75	0.38	0.25			0.38	0.25	
Heavy Vehicles (%)		0%	0%	0%	0%	0%	0%			33%	0%	
Adj. Flow (vph)		0	20	11	11	21	8			21	8	
Shared Lane Traffic (%)												
Lane Group Flow (vph)		0	20	22	0	29	0			29	0	
Enter Blocked Intersection		No	No	No	No	No	No			No	No	
Lane Alignment		Left	Left	Left	Right	Left	Right			Left	Right	
Median Width(m)		0.0	0.0	0.0	0.0	3.6	0.0			3.6	0.0	
Link Offset(m)		0.0	0.0	0.0	0.0	0.0	0.0			0.0	0.0	
Crosswalk Width(m)		4.8	4.8	4.8	4.8	4.8	4.8			4.8	4.8	
Two way Left Turn Lane												
Headway Factor		1.14	1.14	1.14	1.14	1.14	1.14			1.14	1.14	
Turning Speed (k/h)		24				14	24			14	24	
Sign Control		Free	Free	Free	Free	Free	Free			Stop	Stop	
Intersection Summary												
Area Type:	CBD											
Control Type:	Unsignalized											
Intersection Capacity Utilization	15.1%											
Analysis Period (min)	15											
ICU Level of Service	A											

HCM Unsignalized Intersection Capacity Analysis  
 10: Lyons Lane & South Service Road

2035 Background AM Peak Hour.syn  
 05-04-2022



Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	0	10	8	8	8	2
Traffic Volume (veh/h)	0	10	8	8	8	2
Future Volume (Veh/h)	0	10	8	8	8	2
Sign Control	Free	Free	Free	Free	Stop	Stop
Grade	0%	0%	0%	0%	0%	0%
Peak Hour Factor	0.25	0.50	0.75	0.75	0.38	0.25
Hourly flow rate (vph)	0	20	11	11	21	8
Pedestrians						
Lane Width (m)		3.6			3.6	
Walking Speed (m/s)		1.2			1.2	
Percent Blockage		0			1	
Right turn flare (veh)		None			None	
Median type		None			None	
Median storage (veh)		393				
Upstream signal (m)		28			44	22
pX, platoon unblocked						
vC, conflicting volume						
vC1, stage 1 conf/vol						
vC2, stage 2 conf/vol						
vCU, unblocked vol		28			44	22
tC, single (s)		4.1			6.7	6.2
tC, 2 stage (s)						
tF (s)		2.2			3.8	3.3
p0 queue free %		100			98	99
cM capacity (veh/h)		1591			889	1055
Direction, Lane #	EB 1	WB 1	SB 1	SB 1		
Volume Total	20	22	29			
Volume Left	0	0	21			
Volume Right	0	11	8			
GSH	1591	1700	929			
Volume to Capacity	0.00	0.01	0.03			
Queue Length 95th (m)	0.0	0.0	0.8			
Control Delay (s)	0.0	0.0	9.0			
Lane LOS			A			
Approach Delay (s)	0.0	0.0	9.0			
Approach LOS			A			
<b>Intersection Summary</b>						
Average Delay	3.7			ICU Level of Service		
Intersection Capacity Utilization	15.1%			A		
Analysis Period (min)	15					

Lanes, Volumes, Timings  
 4: Trafalgar Road & Argus Road/QEW EB On Ramp

2035 Background PM Peak Hour.syn  
 05-04-2022



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	0	0	54	0	0	0	0	2187	940	0	2165	485
Traffic Volume (vph)	0	0	54	0	0	0	0	2187	940	0	2165	485
Future Volume (vph)	0	0	54	0	0	0	0	2187	940	0	2165	485
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	70.0	0.0	0.0	0.0	0.0
Storage Lanes	0	0	1	0	0	0	0	1	0	0	0	0
Taper Length (m)	7.5		7.5				7.5			7.5		7.5
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.91	1.00	1.00	0.91	0.91
Ped Bike Factor			0.865						0.850			0.973
Fit			Protected						Protected			Protected
Satd. Flow (prot)	0	0	1522	0	0	0	0	5085	1568	0	4966	0
Fit Permitted			Permitted						Permitted			Permitted
Satd. Flow (perm)	0	0	1522	0	0	0	0	5085	1568	0	4966	0
Link Speed (k/h)			50					50			50	
Link Distance (m)			165.2					271.0			50.7	
Travel Time (s)			11.9					19.5			3.7	
Confl. Peds. (#/hr)												24
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	0%	0%	8%	0%	2%	0%	0%	2%	3%	0%	2%	0%
Adj. Flow (vph)	0	0	59	0	0	0	0	2377	1022	0	2353	527
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	0	59	0	0	0	0	2377	1022	0	2880	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(m)			0.0			0.0		3.6			3.6	
Link Offset(m)			0.0			0.0		0.0			0.0	
Crosswalk Width(m)			4.8			4.8		4.8			4.8	
Two way Left Turn Lane												
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (k/h)	25	15	25	15	25	15	25	15	25	15	25	15
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
<b>Intersection Summary</b>												
Area Type:	Other											
Control Type:	Unsignalized											
Intersection Capacity Utilization	63.1%											
Analysis Period (min)	15											

HCM Signalized Intersection Capacity Analysis  
 3: Trafalgar Road & QEW EB Off Ramp

2035 Background PM Peak Hour.syn  
 05-04-2022

Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	↔	↔	↔	↔	↔	↔
Traffic Volume (vph)	1210	610	0	2187	2040	0
Future Volume (vph)	1210	610	0	2187	2040	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0
Lane Util. Factor	0.97	1.00	0.91	0.91	0.91	0.91
Frbp, ped/bikes	1.00	0.98	1.00	1.00	1.00	1.00
Frbp, ped/bikes	1.00	1.00	1.00	1.00	1.00	1.00
Frt	1.00	0.85	1.00	1.00	1.00	1.00
Flt Protected	0.95	1.00	1.00	1.00	1.00	1.00
Satd. Flow (prot)	3502	1573	5085	5085	5085	5085
Flt Permitted	0.95	1.00	1.00	1.00	1.00	1.00
Satd. Flow (perm)	3502	1573	5085	5085	5085	5085
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	1315	663	0	2377	2217	0
RTOR Reduction (vph)	0	1	0	0	0	0
Lane Group Flow (vph)	1315	662	0	2377	2217	0
Conf. Peds. (#/hr)	2					
Heavy Vehicles (%)	0%	1%	0%	2%	2%	0%
Turn Type	Perm	Perm	NA	NA	NA	NA
Protected Phases				2	6	
Permitted Phases	4	4				
Actuated Green, G (s)	58.6	58.6	67.4	67.4	67.4	
Effective Green, g (s)	61.6	61.6	70.4	70.4	70.4	
Actuated g/C Ratio	0.44	0.44	0.50	0.50	0.50	
Clearance Time (s)	7.0	7.0	7.0	7.0	7.0	
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	
Lane Grp. Cap (vph)	1540	692	2557	2557	2557	
v/s Ratio Prot			c0.47	0.44		
v/s Ratio Perm	0.38	c0.42				
v/c Ratio	0.85	0.96		0.93	0.87	
Uniform Delay, d1	35.2	37.9	32.5	30.7	30.7	
Progression Factor	1.00	1.00	1.40	0.85	0.85	
Incremental Delay, d2	4.8	23.8	0.8	2.3	2.3	
Delay (s)	40.0	61.7	46.3	28.3	28.3	
Level of Service	D	E	D	D	C	
Approach Delay (s)	47.3		46.3	28.3		
Approach LOS	D		D	D	C	
Intersection Summary						
HCM 2000 Control Delay	40.5		HCM 2000 Level of Service		D	
HCM 2000 Volume to Capacity ratio	0.96					
Actuated Cycle Length (s)	140.0		Sum of lost time (s)		11.0	
Intersection Capacity Utilization	84.1%		ICU Level of Service		E	
Analysis Period (min)	15					
c Critical Lane Group						

Lanes, Volumes, Timings  
 11: Argus Rd & South Service Rd

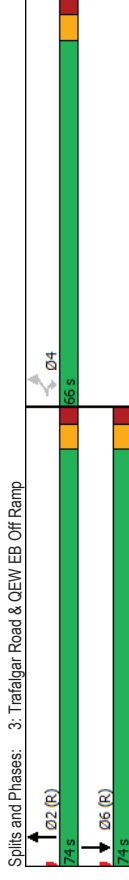
2035 Background AM Peak Hour.syn  
 05-04-2022

Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	↔	↔	↔	↔	↔	↔
Traffic Volume (vph)	1	6	740	155	5	4
Future Volume (vph)	1	6	740	155	5	4
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor			0.973		0.940	
Frt						
Flt Protected			0.989		0.973	
Satd. Flow (prot)	0	1384	1625	0	1564	0
Flt Permitted			0.989		0.973	
Satd. Flow (perm)	0	1384	1625	0	1564	0
Link Speed (k/h)		50	50		50	
Link Distance (m)		177.7	165.2		103.5	
Travel Time (s)		12.8	11.9		7.5	
Conf. Peds. (#/hr)	1			1	5	1
Peak Hour Factor	0.25	0.42	0.86	0.72	0.25	0.25
Heavy Vehicles (%)	100%	0%	3%	0%	0%	0%
Adj. Flow (vph)	4	14	860	215	20	16
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	18	1075	0	36	0
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Left	Left	Right	Left	Right
Median Width(m)		0.0	0.0		3.6	
Link Offset(m)		0.0	0.0		0.0	
Crosswalk Width(m)		4.8	4.8		4.8	
Two way Left Turn Lane						
Headway Factor	1.14	1.14	1.14	1.14	1.14	1.14
Turning Speed (k/h)	24		Free		14	24
Sign Control			Free		Stop	
Intersection Summary						
Area Type:	CBD					
Control Type:	Unsignalized					
Intersection Capacity Utilization	64.1%					
Analysis Period (min)	15					
ICU Level of Service	C					



Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	1	4	1	1	5	4
Traffic Volume (veh/h)	6	740	155	5	4	4
Future Volume (Veh/h)	6	740	155	5	4	4
Sign Control	Free	Free	Stop	Stop	0%	0%
Grade	0%	0%	0%	0%	0%	0%
Peak Hour Factor	0.25	0.42	0.86	0.72	0.25	0.25
Hourly flow rate (vph)	4	14	860	215	20	16
Pedestrians	1	5	1	1	1	1
Lane Width (m)	3.6	3.6	3.6	3.6	3.6	3.6
Walking Speed (m/s)	1.2	1.2	1.2	1.2	1.2	1.2
Percent Blockage	0	0	0	0	0	0
Right turn flare (veh)						
Median type	None	None	None	None	None	None
Median storage (veh)						
Upstream signal (m)		358				
pX, platoon unblocked						
vC, conflicting volume		1076		996	970	970
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol		1076		996	970	970
tC, single (s)		5.1		6.4	6.2	6.2
tC, 2 stage (s)						
tF (s)		3.1		3.5	3.3	3.3
p0 queue free %		99		93	95	95
cM capacity (veh/h)		388		269	310	310
Direction_Lane #	EB 1	WB 1	SB 1			
Volume Total	18	1075	36			
Volume Left	4	0	20			
Volume Right	0	215	16			
GSH	388	1700	286			
Volume to Capacity	0.01	0.63	0.13			
Queue Length 95th (m)	0.3	0.0	3.4			
Control Delay (s)	3.3	0.0	19.4			
Lane LOS	A	C	C			
Approach Delay (s)	3.3	0.0	19.4			
Approach LOS	C	C	C			
Intersection Summary						
Average Delay			0.7			
Intersection Capacity Utilization			64.1%		ICU Level of Service	C
Analysis Period (min)			15			

Queue shown is maximum after two cycles.  
 m Volume for 95th percentile queue is metered by upstream signal.



Splits and Phases: 3: Trafalgar Road & QEW EB Off Ramp



Lanes, Volumes, Timings  
1: Trafalgar Road & North Service Road/QEW WB Off Ramp

Lanes, Volumes, Timings  
3: Trafalgar Road & QEW EB Off Ramp

2035 Background PM Peak Hour.syn  
05-04-2022

2035 Background PM Peak Hour.syn  
05-04-2022

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	1	1	1	1	1	1	1	1	1	1	1	1
Traffic Volume (vph)	24	0	314	658	158	330	0	2846	551	0	2027	11
Future Volume (vph)	24	0	314	658	158	330	0	2846	551	0	2027	11
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (m)	50.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Storage Lanes	1	1	1	1	1	1	1	1	1	1	1	1
Taper Length (m)	7.5	1.00	1.00	0.95	0.95	1.00	1.00	0.91	1.00	1.00	0.91	1.00
Lane Util. Factor	1.00	1.00	1.00	0.95	0.95	1.00	1.00	0.91	1.00	1.00	0.91	1.00
Ped Bike Factor	1.00	0.850	0.850	0.850	0.850	0.850	0.850	0.850	0.850	0.850	0.850	0.850
Flt Protected	0.950	0.950	0.950	0.970	0.970	0.950	0.970	0.950	0.970	0.950	0.970	0.950
Satd. Flow (prot)	1805	0	1615	1681	1723	1583	0	5085	1599	0	4217	1615
Flt Permitted	0.950	0.950	0.950	0.970	0.970	0.950	0.970	0.950	0.970	0.950	0.970	0.950
Satd. Flow (perm)	1800	0	1615	1681	1723	1557	0	5085	1482	0	4217	1497
Right Turn on Red	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Satd. Flow (RTOR)	31	31	31	66	66	66	66	347	347	66	66	66
Link Speed (k/h)	50	50	50	50	50	50	50	50	50	50	50	50
Link Distance (m)	120.4	120.4	120.4	165.1	165.1	165.1	165.1	145.2	145.2	165.1	145.2	209.2
Travel Time (s)	8.7	8.7	8.7	11.9	11.9	11.9	11.9	10.5	10.5	11.9	10.5	15.1
Confl. Peds. (#/hr)	2	2	2	14	14	14	14	14	14	14	14	14
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	0%	0%	0%	2%	2%	2%	2%	0%	2%	0%	2%	0%
Adj. Flow (vph)	26	0	341	715	172	359	0	3093	599	0	2203	12
Shared Lane Traffic (%)	39%	39%	39%	39%	39%	39%	39%	39%	39%	39%	39%	39%
Lane Group Flow (vph)	26	0	341	436	451	359	0	3093	599	0	2203	12
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(m)	3.6	3.6	3.6	3.6	3.6	3.6	3.6	0.0	0.0	0.0	0.0	0.0
Link Offset(m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Crosswalk Width(m)	4.8	4.8	4.8	4.8	4.8	4.8	4.8	4.8	4.8	4.8	4.8	4.8
Two way Left Turn Lane	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Headway Factor	25	15	25	25	25	15	25	15	25	15	25	15
Turning Speed (k/h)	1	1	1	1	1	1	1	1	1	1	1	1
Number of Detectors	Left	Right	Thru	Right	Thru	Right	Thru	Right	Thru	Right	Thru	Right
Detector Template	2.0	2.0	2.0	10.0	2.0	10.0	2.0	10.0	2.0	10.0	2.0	10.0
Leading Detector (m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Trailing Detector (m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Position(m)	2.0	2.0	2.0	0.6	2.0	0.6	2.0	0.6	2.0	0.6	2.0	0.6
Detector 1 Size(m)	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex
Detector 1 Type	Detector 1 Channel	Detector 1 Channel	Detector 1 Channel	Detector 1 Channel	Detector 1 Channel	Detector 1 Channel	Detector 1 Channel	Detector 1 Channel	Detector 1 Channel	Detector 1 Channel	Detector 1 Channel	Detector 1 Channel
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 2 Position(m)	9.4	9.4	9.4	9.4	9.4	9.4	9.4	9.4	9.4	9.4	9.4	9.4
Detector 2 Size(m)	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex
Detector 2 Type	Detector 2 Channel	Detector 2 Channel	Detector 2 Channel	Detector 2 Channel	Detector 2 Channel	Detector 2 Channel	Detector 2 Channel	Detector 2 Channel	Detector 2 Channel	Detector 2 Channel	Detector 2 Channel	Detector 2 Channel
Detector 2 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0

Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Detector Phase	4	4	4	2	2	6
Switch Phase	10.0	10.0	29.0	29.0	29.0	29.0
Minimum Initial (s)	38.0	38.0	36.0	36.0	36.0	36.0
Minimum Split (s)	66.0	66.0	74.0	74.0	74.0	74.0
Total Split (s)	47.1%	47.1%	52.9%	52.9%	52.9%	52.9%
Maximum Green (s)	59.0	59.0	67.0	67.0	67.0	67.0
Yellow Time (s)	4.0	4.0	4.0	4.0	4.0	4.0
All-Red Time (s)	3.0	3.0	3.0	3.0	3.0	3.0
All-Red Adjust (s)	-3.0	-3.0	-3.0	-3.0	-3.0	-3.0
Total Lost Time (s)	4.0	4.0	4.0	4.0	4.0	4.0
Lead/Lag						
Lead-Lag Optimize?	3.0	3.0	3.0	3.0	3.0	3.0
Vehicle Extension (s)	None	None	C-Max	C-Max	C-Max	C-Max
Recall Mode	7.0	7.0	7.0	7.0	7.0	7.0
Flash Dont Walk (s)	11.0	11.0	11.0	11.0	11.0	11.0
Pedestrian Calls (#/hr)	0	0	0	0	0	0
Act Effct Green (s)	61.6	61.6	70.4	70.4	70.4	70.4
Actuated g/C Ratio	0.44	0.44	0.50	0.50	0.50	0.50
v/c Ratio	0.85	0.96	0.93	0.87	0.87	0.87
Control Delay	41.8	63.2	46.5	28.8	28.8	28.8
Queue Delay	0.0	0.0	3.7	0.0	0.0	0.0
Total Delay	41.8	63.2	50.2	28.8	28.8	28.8
LOS	D	E	D	D	C	C
Approach Delay	48.9	50.2	50.2	28.8	28.8	28.8
Approach LOS	D	D	D	C	C	C
Queue Length 50th (m)	173.6	182.9	262.2	171.4	171.4	171.4
Queue Length 95th (m)	205.4	#267.2	m245.6	215.9	215.9	215.9
Internal Link Dist (m)	47.3	47.3	26.7	162.0	162.0	162.0
Turn Bay Length (m)	1550	687	2556	2556	2556	2556
Base Capacity (vph)	0	0	0	0	0	0
Stevartion Cap Reductn	0	0	129	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	0.85	0.95	0.98	0.87	0.87	0.87
Intersection Summary	Other					
Area Type:	Other					
Cycle Length: 140	Cycle Length: 140					
Offset: 0 (0%), Referenced to phase 2:NBT and 6:SBT, Start of Green	Offset: 0 (0%), Referenced to phase 2:NBT and 6:SBT, Start of Green					
Natural Cycle: 90	Natural Cycle: 90					
Control Type: Actuated-Coordinated	Control Type: Actuated-Coordinated					
Maximum v/c Ratio: 0.96	Maximum v/c Ratio: 0.96					
Intersection Signal Delay: 42.6	Intersection Signal Delay: 42.6					
Intersection Capacity Utilization 84.1%	Intersection Capacity Utilization 84.1%					
ICU Level of Service E	ICU Level of Service E					
Analysis Period (min) 15	Analysis Period (min) 15					
# 95th percentile volume exceeds capacity, queue may be longer.	# 95th percentile volume exceeds capacity, queue may be longer.					



Lanes, Volumes, Timings  
1: Trafalgar Road & North Service Road/QEW/WB Off Ramp

2035 Background PM Peak Hour.syn  
05-04-2022

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Turn Type	7											
Protected Phases	7											
Permitted Phases												
Detector Phase	7	4	8	8	8	8	2	2	2	2	6	6
Switch Phase												
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Minimum Split (s)	9.5	38.0	38.0	38.0	38.0	38.0	35.0	35.0	35.0	35.0	35.0	35.0
Total Split (%)	6.8%	35.4%	28.6%	28.6%	28.6%	28.6%	64.6%	64.6%	64.6%	64.6%	64.6%	64.6%
Maximum Green (s)	5.0	42.5	33.0	33.0	33.0	33.0	83.5	83.5	83.5	83.5	83.5	83.5
Yellow Time (s)	3.5	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
All-Red Time (s)	1.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Lost Time Adjust (s)	-1.0	-3.0	-3.0	-3.0	-3.0	-3.0	-3.0	-3.0	-3.0	-3.0	-3.0	-3.0
Total Lost Time (s)	3.5	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
LeadLag	Lead	Lag	Lag	Lag	Lag	Lag						
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes						
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	None	None	None	None	None	None	C-Max	C-Max	C-Max	C-Max	C-Max	C-Max
Walk Time (s)		7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0
Flash Dont Walk (s)		11.0	11.0	11.0	11.0	11.0	11.0	11.0	11.0	11.0	11.0	11.0
Pedestrian Calls (#/hr)		0	0	0	0	0	0	0	0	0	0	0
Act Effct Green (s)	6.0	45.5	39.8	39.8	39.8	39.8	86.5	86.5	86.5	86.5	86.5	86.5
Actuated g/C Ratio	0.04	0.32	0.28	0.28	0.28	0.28	0.62	0.62	0.62	0.62	0.62	0.62
v/c Ratio	0.34	0.63	0.91	0.92	0.73	0.73	0.98	0.98	0.57	0.85	0.85	0.01
Control Delay	77.2	42.1	73.4	74.3	47.6	47.6	24.4	24.4	4.5	25.4	25.4	0.0
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	2.0	2.0	0.0	0.0	0.0	0.0
Total Delay	77.2	42.1	73.4	74.3	47.6	47.6	26.4	26.4	4.5	25.4	25.4	0.0
LOS	E	D	E	E	D	D	C	A	A	C	C	A
Approach Delay	44.6						22.8			25.3		
Approach LOS	D						C			C		
Queue Length 50th (m)	7.5	76.7	~136.5	~146.1	81.8	186.9	21.5	178.5	178.5	178.5	178.5	0.0
Queue Length 95th (m)	18.3	112.2	#211.8	#218.6	#130.2	#358.4	m24.8	202.7	202.7	202.7	202.7	0.0
Internal Link Dist (m)		96.4		141.1		121.2		185.2				
Turn Bay Length (m)	50.0						70.0					
Base Capacity (vph)	77	545	478	489	489	489	3141	1048	2605	2605	950	
Starvation Cap Reductn	0	0	0	0	0	0	35	0	0	0	0	
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	
Reduced v/c Ratio	0.34	0.63	0.91	0.92	0.73	0.73	1.00	0.57	0.85	0.85	0.01	
Intersection Summary												
Area Type:	Other											
Cycle Length:	140											
Actuated Cycle Length:	140											
Offset: 0 (0%):	Referenced to phase 2:NBT and 6:SBT, Start of Green											
Natural Cycle:	125											
Control Type:	Actuated-Coordinated											
Maximum v/c Ratio:	0.98											
Intersection Signal Delay:	31.8											
Intersection LOS:	C											

Lanes, Volumes, Timings  
3: Trafalgar Road & QEW/EB Off Ramp

2035 Background PM Peak Hour.syn  
05-04-2022

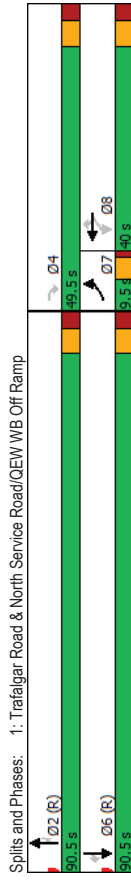
Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	EBL	EBR	NBL	NBT	SBT	SBR
Traffic Volume (vph)	1210	610	0	2187	2040	0
Future Volume (vph)	1210	610	0	2187	2040	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	0.97	1.00	1.00	0.91	0.91	1.00
Ped Bike Factor	0.98					
Ft	0.850					
Flt Protected						
Satd. Flow (prot)	3502	1599	0	5085	5085	0
Flt Permitted	0.950					
Satd. Flow (perm)	3502	1573	0	5085	5085	0
Right Turn on Red	Yes					Yes
Satd. Flow (RTOR)	2					
Link Speed (k/h)	50		50		50	
Link Distance (m)	71.3		50.7		186.0	
Travel Time (s)	5.1		3.7		13.4	
Conf. Peds. (#/hr)	2					
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	0%	1%	0%	2%	2%	0%
Adj. Flow (vph)	1315	663	0	2377	2217	0
Shared Lane Traffic (%)						
Lane Group Flow (vph)	1315	663	0	2377	2217	0
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Left	Left	Right
Median Width (m)	7.2		0.0	0.0	0.0	0.0
Link Offset (m)	0.0		0.0	0.0	0.0	0.0
Crosswalk Width (m)	4.8		4.8		4.8	
Two way Left Turn Lane						
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (k/h)	25	15	25		15	
Number of Detectors	1	1	2	2	2	
Detector Template	Left	Right	Thru	Thru	Thru	
Leading Detector (m)	2.0	2.0	10.0	10.0	10.0	
Trailing Detector (m)	0.0	0.0	0.0	0.0	0.0	
Detector 1 Position (m)	0.0	0.0	0.0	0.0	0.0	
Detector 1 Size (m)	2.0	2.0	0.6	0.6	0.6	
Detector 1 Type	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	
Detector 1 Channel						
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	
Detector 2 Position (m)			9.4	9.4	9.4	
Detector 2 Size (m)			0.6	0.6	0.6	
Detector 2 Type			Ch+Ex	Ch+Ex	Ch+Ex	
Detector 2 Channel						
Detector 2 Extend (s)			0.0	0.0	0.0	
Turn Type	Perm	Perm	NA	NA	NA	
Protected Phases	4	4	2	6		
Permitted Phases						

HCM Unsignalized Intersection Capacity Analysis 2035 Background PM Peak Hour.syn  
 2: Trafalgar Road & QEW SB On Ramps 05-04-2022

	EBL	EBR	NBL	NBT	SBT	SBR
Movement						
Lane Configurations				↑↑↑	↑↑↑	↑
Traffic Volume (veh/h)	0	0	0	3397	2040	959
Future Volume (Veh/h)	0	0	0	3397	2040	959
Sign Control	Stop	Free	Free	Free	Free	Free
Grade	0%	0%	0%	0%	0%	0%
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	0	0	0	3692	2217	1042
Pedestrians						
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)				None	None	
Median type				None	None	
Median storage (veh)				186	145	
Upstream signal (m)						
pX, platoon unblocked	0.26					
vC, conflicting volume	3448	739	2217			
vC1, stage 1 conf vol						
vC2, stage 2 conf vol	422	739	2217			
vCu, unblocked vol	6.8	6.9	4.1			
tC, single (s)	3.5	3.3	2.2			
tC, 2 stage (s)						
pf queue free %	100	100	100			
cM capacity (veh/h)	144	360	232			
Direction_Lane #	NB1	NB2	NB3	SB1	SB2	SB3
Volume Total	1231	1231	1231	739	739	739
Volume Left	0	0	0	0	0	0
Volume Right	0	0	0	0	0	0
GSH	1700	1700	1700	1700	1700	1700
Volume to Capacity	0.72	0.72	0.72	0.43	0.43	0.43
Queue Length 95th (m)	0.0	0.0	0.0	0.0	0.0	0.0
Control Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0
Lane LOS						
Approach Delay (s)	0.0			0.0		
Approach LOS						
Intersection Summary						
Average Delay	0.0					
Intersection Capacity Utilization	84.1%					
ICU Level of Service	E					
Analysis Period (min)	15					

Lanes, Volumes, Timings 2035 Background PM Peak Hour.syn  
 1: Trafalgar Road & North Service Road/QEW WB Off Ramp 05-04-2022

	ICU Level of Service E
Intersection Capacity Utilization	91.0%
Analysis Period (min)	15
Volume exceeds capacity, queue is theoretically infinite.	
Queue shown is maximum after two cycles.	
# 95th percentile volume exceeds capacity, queue may be longer.	
Queue shown is maximum after two cycles.	
m Volume for 95th percentile queue is metered by upstream signal.	



HCM Signalized Intersection Capacity Analysis  
 1: Trafalgar Road & North Service Road/QEW WB Off Ramp

2035 Background PM Peak Hour.syn  
 05-04-2022  
 Lanes, Volumes, Timings  
 2: Trafalgar Road & QEW SB On Ramps

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	24	0	314	658	158	330	0	2846	551	0	2027	11
Traffic Volume (vph)	24	0	314	658	158	330	0	2846	551	0	2027	11
Future Volume (vph)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Ideal Flow (vphpl)	3.5	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Total Lost time (s)	1.00	1.00	0.95	0.95	1.00	0.91	1.00	0.91	1.00	0.91	1.00	1.00
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	0.98	1.00	0.93	1.00	0.93	1.00	0.93
Frb, ped/bikes	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frb, ped/bikes	1.00	0.85	1.00	1.00	1.00	0.85	1.00	0.85	1.00	0.85	1.00	0.85
Flt Protected	0.95	1.00	0.95	0.97	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Satd. Flow (prot)	1805	1615	1681	1723	1557	1557	5085	1482	4217	1497	1497	1497
Flt Permitted	0.95	1.00	0.95	0.97	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Satd. Flow (perm)	1805	1615	1681	1723	1557	1557	5085	1482	4217	1497	1497	1497
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	26	0	341	715	172	359	0	3093	599	0	2203	12
RTOR Reduction (vph)	0	0	21	0	0	47	0	137	0	0	0	5
Lane Group Flow (vph)	26	0	320	436	451	312	0	3093	462	0	2203	7
Conf. Peds. (#/hr)	2	0	0	0	0	2	14	14	14	14	14	14
Heavy Vehicles (%)	0%	0%	0%	2%	1%	2%	0%	2%	1%	0%	23%	0%
Turn Type	Prot	Perm	Perm	NA	Perm	NA	Perm	NA	Perm	NA	Perm	NA
Protected Phases	7			8			2			6		
Permitted Phases			4	8	8	8		2	2		6	6
Actuated Green, G (s)	3.0	44.3	36.8	36.8	36.8	36.8	81.7	81.7	81.7	81.7	81.7	81.7
Effective Green, g (s)	4.0	47.3	39.8	39.8	39.8	39.8	84.7	84.7	84.7	84.7	84.7	84.7
Actuated g/C Ratio	0.03	0.34	0.28	0.28	0.28	0.28	0.61	0.61	0.61	0.61	0.61	0.61
Clearance Time (s)	4.5	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Lane Grp Cap (vph)	51	545	477	489	442	442	3076	896	896	2551	905	905
v/s Ratio Prot	0.01						c0.61			0.52		
v/s Ratio Perm	0.51	0.59	0.91	0.92	0.71	0.71	1.01	0.52	0.86	0.86	0.01	0.01
Uniform Delay, d1	67.0	38.3	48.4	48.6	44.9	44.9	27.6	15.9	22.9	11.0	11.0	11.0
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	0.63	0.60	1.00	1.00	1.00	1.00
Incremental Delay, d2	7.8	1.6	21.9	23.0	5.1	5.1	11.8	0.9	4.2	0.0	4.2	0.0
Delay (s)	74.8	39.9	70.4	71.6	49.9	49.9	29.1	10.4	27.1	11.0	11.0	11.0
Level of Service	E	D	E	E	D	D	C	B	C	B	C	B
Approach Delay (s)	42.4			64.9			26.1		27.0			
Approach LOS	D			E			C		C		C	
Intersection Summary												
HCM 2000 Control Delay	33.6 HCM 2000 Level of Service C											
HCM 2000 Volume to Capacity ratio	0.97											
Actuated Cycle Length (s)	140.0 Sum of lost time (s)											
Intersection Capacity Utilization	91.0% ICU Level of Service E											
Analysis Period (min)	15											
c Critical Lane Group												

Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	0	0	0	3397	2040	959
Traffic Volume (vph)	0	0	0	3397	2040	959
Future Volume (vph)	0	0	0	3397	2040	959
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	0.91	0.91	1.00
Flt Protected						0.850
Satd. Flow (prot)	0	0	0	5085	5085	1583
Flt Permitted						1583
Satd. Flow (perm)	0	0	0	5085	5085	1583
Link Speed (k/h)	50			50		50
Link Distance (m)	159.6			186.0		145.2
Travel Time (s)	11.5			13.4		10.5
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	0	0	0	3692	2217	1042
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	0	0	3692	2217	1042
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Left	Left	Right
Median Width (m)	0.0	0.0	0.0	0.0	0.0	0.0
Link Offset (m)	0.0	0.0	0.0	0.0	0.0	0.0
Crosswalk Width (m)	4.8			4.8		4.8
Two way Left Turn Lane						
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (k/h)	25	15	25			15
Sign Control	Stop			Free	Free	Free
Intersection Summary						
Area Type:	Other					
Control Type:	Unsignalized					
Intersection Capacity Utilization	84.1%					
Analysis Period (min)	15					
ICU Level of Service	E					

# Appendix G

## Lyons Lane Closure Sensitivity Operations Reports



HCM Unsignalized Intersection Capacity Analysis 2035 Total PM Peak Hour (Lyons Closed).syn  
12: Site Driveway

05-04-2022

Movement	EBT	EBR	WBL	WBT	NBL	NBR	
Lane Configurations	↑		4	4	4		
Traffic Volume (veh/h)	69	0	67	62	0	43	
Future Volume (Veh/h)	69	0	67	62	0	43	
Sign Control	Free		Free	Stop			
Grade	0%		0%	0%			
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	
Hourly flow rate (vph)	75	0	73	67	0	47	
Pedestrians							
Lane Width (m)							
Walking Speed (m/s)							
Percent Blockage							
Right turn flare (veh)							
Median type	None						
Median storage (veh)							
Upstream signal (m)	75				288	75	
pX, platoon unblocked							
vC, conflicting volume							
vC1, stage 1 conf vol							
vC2, stage 2 conf vol							
vCu, unblocked vol	75					288	75
tC, single (s)	4.1					6.4	6.2
tC, 2 stage (s)	2.2					3.5	3.3
tF (s)	95					100	95
cM capacity (veh/h)	1524					669	986
Direction, Lane #	EB 1	WB 1	NB 1				
Volume Total	75	140	47				
Volume Left	0	73	0				
Volume Right	0	0	47				
GSH	1700	1524	986				
Volume to Capacity	0.04	0.05	0.05				
Queue Length 95th (m)	0.0	1.2	1.2				
Control Delay (s)	0.0	4.1	8.8				
Lane LOS	A	A	A				
Approach Delay (s)	0.0	4.1	8.8				
Approach LOS	A	A	A				
Intersection Summary							
Average Delay	3.8						A
Intersection Capacity Utilization	23.6%					ICU Level of Service	
Analysis Period (min)	15						

Baseline

Syncho 10 Report  
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2035 Total AM Peak Hour (Lyons Closed).syn  
1: Trafalgar Road & North Service Road/QEW WB Off Ramp

05-04-2022

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	1	0	1	1	1	1	1	1	1	1	1	1
Traffic Volume (vph)	1	0	196	579	42	249	0	1989	535	0	2276	8
Future Volume (vph)	1	0	196	579	42	249	0	1989	535	0	2276	8
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (m)	50.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	70.0	0.0	0.0	0.0
Storage Lanes	1	0	1	1	1	1	0	1	1	0	1	1
Taper Length (m)	7.5	0	0	7.5	0	7.5	0	7.5	0	7.5	0	7.5
Lane Util. Factor	1.00	1.00	1.00	1.00	0.95	0.95	1.00	0.91	1.00	1.00	0.91	1.00
Ped Bike Factor								0.96			0.96	
Fit			0.850		0.892		0.850		0.850		0.850	
Fit Protected		0.950		0.950								
Satd. Flow (prot)	1805	0	1568	1719	1453	1447	0	4940	1538	0	5036	1615
Fit Permitted		0.950		0.950								
Satd. Flow (perm)	1805	0	1568	1719	1453	1447	0	4940	1478	0	5036	1533
Right Turn on Red	Yes		Yes		Yes			Yes	Yes		Yes	Yes
Satd. Flow (RTOR)	31		98		154			331			70	
Link Speed (k/h)	50		50		50		50		50		50	
Link Distance (m)	120.4		165.1		111.9		145.2		209.2		15.1	
Travel Time (s)	8.7		11.9		10.5		15.1		4.8		15.1	
Confl. Peds. (#/hr)						8			5		5	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	0%	0%	3%	5%	23%	6%	0%	5%	5%	0%	3%	0%
Adj. Flow (vph)	1	0	213	629	46	271	0	2162	582	0	2474	9
Shared Lane Traffic (%)						43%						
Lane Group Flow (vph)	1	0	213	629	163	154	0	2162	582	0	2474	9
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Right	Right	Left	Left	Right	Left	Right	Right
Median Width (m)	3.6		3.6		3.6		3.6		3.6		3.6	
Link Offset (m)	0.0		0.0		0.0		0.0		0.0		0.0	
Crosswalk Width (m)	4.8		4.8		4.8		4.8		4.8		4.8	
Two way Left Turn Lane												
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (k/h)	25	15	25	15	25	15	25	15	25	15	25	15
Number of Detectors	1	1	1	2	1	2	2	1	1	2	1	1
Detector Template	Left	Right	Thru	Left	Thru	Right	Thru	Right	Thru	Right	Thru	Right
Leading Detector (m)	2.0	2.0	10.0	2.0	10.0	2.0	10.0	2.0	10.0	2.0	10.0	2.0
Trailing Detector (m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Position (m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Size (m)	2.0	2.0	2.0	0.6	2.0	0.6	2.0	0.6	2.0	0.6	2.0	0.6
Detector 1 Type	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 2 Position (m)				9.4		9.4		9.4		9.4		9.4
Detector 2 Size (m)				0.6		0.6		0.6		0.6		0.6
Detector 2 Type				Ch+Ex		Ch+Ex		Ch+Ex		Ch+Ex		Ch+Ex
Detector 2 Channel												
Detector 2 Extend (s)				0.0		0.0		0.0		0.0		0.0

Baseline

Syncho 10 Report  
Page 1

Lanes, Volumes, Timings  
 1: Trafalgar Road & North Service Road/QEW WB Off Ramp

Lanes, Volumes, Timings  
 12: Site Driveway

2035 Total AM Peak Hour (Lyons Closed).syn  
 05-04-2022

2035 Total PM Peak Hour (Lyons Closed).syn  
 05-04-2022

Lane Group	EBL Prot	EBT Perm	WBL Perm	WBT NA	WBR Perm	NBL NA	NBT Perm	NBR NA	SBL Perm	SBT NA	SBR Perm
Protected Phases	7										
Permitted Phases		4	8	8	8	2	2	2	2	6	6
Detector Phase	7	4	8	8	8	2	2	2	2	6	6
Switch Phase											
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Minimum Split (s)	23.0	38.0	38.0	38.0	38.0	35.0	35.0	35.0	35.0	35.0	35.0
Total Split (s)	23.0	74.0	51.0	51.0	51.0	66.0	66.0	66.0	66.0	66.0	66.0
Total Split (%)	16.4%	52.9%	36.4%	36.4%	36.4%	47.1%	47.1%	47.1%	47.1%	47.1%	47.1%
Maximum Green (s)	18.0	67.0	44.0	44.0	44.0	59.0	59.0	59.0	59.0	59.0	59.0
Yellow Time (s)	3.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
All-Red Time (s)	2.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Lost Time Adjust (s)	-1.0	-3.0	-3.0	-3.0	-3.0	-3.0	-3.0	-3.0	-3.0	-3.0	-3.0
Total Lost Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lead/Lag	Lead	Lag	Lag	Lag	Lag	Lag	Lag	Lag	Lag	Lag	Lag
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	None	None	None	None	None	C-Max	C-Max	C-Max	C-Max	C-Max	C-Max
Walk Time (s)		7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0
Flash Dont Walk (s)		11.0	11.0	11.0	11.0	11.0	11.0	11.0	11.0	11.0	11.0
Pedestrian Calls (#/hr)		0	0	0	0	0	0	0	0	0	0
Act Effct Green (s)	6.7	69.4	67.2	67.2	67.2	62.6	62.6	62.6	62.6	62.6	62.6
Actuated g/C Ratio	0.05	0.50	0.48	0.48	0.48	0.45	0.45	0.45	0.45	0.45	0.45
v/c Ratio	0.01	0.27	0.76	0.22	0.20	0.98	0.69	1.10	0.01	0.01	0.01
Control Delay	64.0	18.4	38.0	10.0	4.0	40.0	13.1	88.9	0.0	0.0	0.0
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	64.0	18.4	38.0	10.0	4.0	40.0	13.1	88.9	0.0	0.0	0.0
LOS	E	B	D	B	A	D	D	B	F	F	A
Approach Delay	18.6		27.6			34.3			88.6		
Approach LOS	B		C			C			F		
Queue Length 50th (m)	0.3	30.3	144.9	10.4	0.0	126.7	31.1	~294.2	0.0		
Queue Length 95th (m)	2.3	47.2	#237.3	28.7	14.5	#262.8	m66.5	#328.9	0.0		
Internal Link Dist (m)		96.4		141.1		121.2		185.2			
Turn Bay Length (m)	50.0										
Base Capacity (vph)	244	799	824	748	774	2208	843	2252	724		
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0		
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0		
Storage Cap Reductn	0	0	0	0	0	0	0	0	0		
Reduced v/c Ratio	0.00	0.27	0.76	0.22	0.20	0.98	0.69	1.10	0.01		
Intersection Summary	Intersection LOS: D										
Area Type:	Other										
Control Type:	Unsignalized										
Intersection Capacity Utilization	23.6%										
Analysis Period (min)	15										

Lane Group	EBT	EBR	WBL	WBT	NBL	NBR	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations												
Traffic Volume (vph)	69	0	67	62	0	43	69	0	67	62	0	43
Future Volume (vph)	69	0	67	62	0	43	69	0	67	62	0	43
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Fit						0.865						
Fit Protected						0.975						
Satd. Flow (prot)	1863	0	0	1816	1611	0	1863	0	0	1816	1611	0
Fit Permitted						0.975						
Satd. Flow (perm)	1863	0	0	1816	1611	0	1863	0	0	1816	1611	0
Link Speed (k/h)	50		50		50		50		50		50	
Link Distance (m)	110.8		715.0		42.7		110.8		715.0		42.7	
Travel Time (s)	8.0		51.5		3.1		8.0		51.5		3.1	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	75	0	73	67	0	47	75	0	73	67	0	47
Shared Lane Traffic (%)												
Lane Group Flow (vph)	75	0	0	140	47	0	75	0	0	140	47	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Left	Left	Right	Left	Right	Left	Left	Left	Right
Median Width (m)	0.0	0.0	0.0	0.0	0.0	3.6	0.0	0.0	0.0	0.0	3.6	0.0
Link Offset (m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Crosswalk Width (m)	4.8		4.8		4.8		4.8		4.8		4.8	
Two way Left Turn Lane												
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (k/h)		15	25		25			15	25		25	15
Sign Control	Free	Free	Free	Free	Free	Stop	Free	Free	Free	Free	Stop	Free
Intersection Summary	Other											
Area Type:	Other											
Control Type:	Unsignalized											
Intersection Capacity Utilization	23.6%											
Analysis Period (min)	15											
ICU Level of Service A												

HCM Unsignalized Intersection Capacity Analysis: 2035 Total PM Peak Hour (L-lyons Closed).syn  
 11: Argus Rd & South Service Rd

05-04-2022



Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	74	10	361	151	51	86
Traffic Volume (veh/h)	74	10	361	151	51	86
Future Volume (Veh/h)	74	10	361	151	51	86
Sign Control	Free	Free	0%	0%	Stop	0%
Grade	0%	0%	0%	0%	0%	0%
Peak Hour Factor	0.45	0.50	0.87	0.68	0.75	0.65
Hourly flow rate (vph)	164	20	415	222	68	132
Pedestrians			5			
Lane Width (m)			3.6			
Walking Speed (m/s)			1.2			
Percent Blockage			0			
Right turn flare (veh)			None			
Median type			None			
Median storage (veh)			None			
Upstream signal (m)			358			
pX, platoon unblocked						
vC, conflicting volume			637		879	526
vC1, stage 1 conf/vol						
vC2, stage 2 conf/vol						
vCu, unblocked vol			637		879	526
tC, single (s)			4.1		6.4	6.2
tC, 2 stage (s)			2.2		3.5	3.3
p0 queue free %			83		74	76
cM capacity (veh/h)			956		265	556
Direction_Lane #	EB 1	WB 1	SB 1			
Volume Total	184	637	200			
Volume Left	164	0	68			
Volume Right	0	222	132			
GSH	956	1700	404			
Volume to Capacity	0.17	0.37	0.49			
Queue Length 95th (m)	4.9	0.0	21.3			
Control Delay (s)	8.7	0.0	22.3			
Lane LOS	A	C	C			
Approach Delay (s)	8.7	0.0	22.3			
Approach LOS	C	C	C			
Intersection Summary						
Average Delay			5.9			
Intersection Capacity Utilization			55.5%		ICU Level of Service	B
Analysis Period (min)			15			

Baseline

Lanes, Volumes, Timings 2035 Total AM Peak Hour (Lyons Closed).syn  
 1: Trafalgar Road & North Service Road/QEW WB Off Ramp

05-04-2022

Intersection Capacity Utilization 98.2% ICU Level of Service F

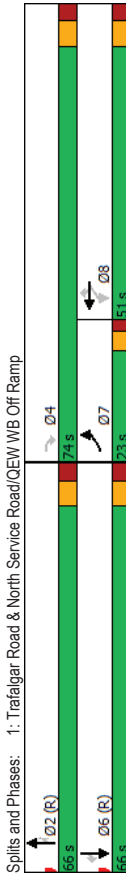
Analysis Period (min) 15

~ Volume exceeds capacity, queue is theoretically infinite.  
 Queue shown is maximum after two cycles.

# 95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

m Volume for 95th percentile queue is metered by upstream signal.



Splits and Phases: 1: Trafalgar Road & North Service Road/QEW WB Off Ramp

Baseline

HCM Signalized Intersection Capacity Analysis 2035 Total AM Peak Hour (Lyons Closed).syn  
 1: Trafalgar Road & North Service Road/QEW WB Off Ramp 05-04-2022

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBR
Lane Configurations	1	0	196	579	42	249	0	1989	535	0	2276
Traffic Volume (vph)	1	0	196	579	42	249	0	1989	535	0	2276
Future Volume (vph)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Ideal Flow (vphpl)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Total Lost time (s)	1.00	1.00	1.00	0.95	0.95	0.91	1.00	0.91	1.00	0.91	1.00
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.96	1.00	0.95
Frb, ped/bikes	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frb, ped/bikes	1.00	0.85	1.00	0.89	0.85	1.00	0.85	1.00	0.85	1.00	0.85
Flt Protected	0.95	1.00	0.95	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Satd. Flow (prot)	1805	1568	1719	1454	1447	4940	1478	5036	1533	5036	1533
Flt Permitted	0.95	1.00	0.95	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Satd. Flow (perm)	1805	1568	1719	1454	1447	4940	1478	5036	1533	5036	1533
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	1	0	213	629	46	271	0	2162	582	0	2474
RTOR Reduction (vph)	0	0	15	0	51	80	0	192	0	0	5
Lane Group Flow (vph)	1	0	198	629	112	74	0	2162	390	0	2474
Conf. Peds. (#/hr)	0%	0%	3%	5%	23%	6%	0%	5%	5%	0%	3%
Heavy Vehicles (%)	Prot	Perm	Perm	NA	Perm	NA	Perm	NA	Perm	NA	Perm
Turn Type	7	8	8	8	2	2	6	6	6	6	6
Protected Phases	7	8	8	8	2	2	6	6	6	6	6
Permitted Phases	1.2	70.4	64.2	64.2	64.2	55.6	55.6	55.6	55.6	55.6	55.6
Actuated Green, G (s)	2.2	73.4	67.2	67.2	67.2	58.6	58.6	58.6	58.6	58.6	58.6
Effective Green, g (s)	0.02	0.52	0.48	0.48	0.48	0.42	0.42	0.42	0.42	0.42	0.42
Actuated g/C Ratio	5.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0
Clearance Time (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Vehicle Extension (s)	28	822	825	697	694	2067	618	2107	641	2107	641
Lane Grp Cap (vph)	0.00	c0.13	c0.37	0.05	0.05	0.44	0.44	c0.49	0.26	0.26	0.00
v/s Ratio Prot	0.04	0.24	0.76	0.16	0.11	1.05	0.63	1.17	0.63	1.17	0.01
v/c Ratio Perm	67.9	18.1	29.9	20.5	19.9	40.7	32.1	40.7	32.1	40.7	23.7
Uniform Delay, d1	1.00	1.00	1.00	1.00	1.00	0.74	0.80	0.74	0.80	1.00	1.00
Progression Factor	0.5	0.2	4.2	0.1	0.1	29.6	3.2	83.8	3.2	83.8	0.0
Incremental Delay, d2	68.4	18.3	34.1	20.6	20.0	59.9	28.8	124.5	23.7	124.5	23.7
Delay (s)	E	B	C	C	C	E	C	F	C	F	C
Level of Service	E	B	C	C	C	E	C	F	C	F	C
Approach Delay (s)	18.5	29.5	29.5	53.3	53.3	124.1	124.1	124.1	124.1	124.1	124.1
Approach LOS	B	C	C	D	D	F	F	F	F	F	F

Lanes, Volumes, Timings 2035 Total PM Peak Hour (Lyons Closed).syn  
 11: Argus Rd & South Service Rd 05-04-2022

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBR
Lane Configurations	74	10	361	151	151	51	86	86	86	86	86
Traffic Volume (vph)	74	10	361	151	151	51	86	86	86	86	86
Future Volume (vph)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Ideal Flow (vphpl)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Total Lost time (s)	0.957	0.957	0.957	0.957	0.957	0.957	0.957	0.957	0.957	0.957	0.957
Lane Util. Factor	0	0	0.1614	0.1630	0	0	0.1531	0	0	0	0
Frb, ped/bikes	0	0	1614	1530	0	1531	0	1531	0	0	0
Flt Permitted	0	0	1614	1530	0	1531	0	1531	0	0	0
Satd. Flow (perm)	0	0	1614	1530	0	1531	0	1531	0	0	0
Link Speed (k/h)	50	50	50	50	50	50	50	50	50	50	50
Link Distance (m)	1777.7	1777.7	1652	103.5	103.5	103.5	103.5	103.5	103.5	103.5	103.5
Travel Time (s)	12.8	11.9	7.5	7.5	7.5	7.5	7.5	7.5	7.5	7.5	7.5
Conf. Peds. (#/hr)	0.45	0.50	0.87	0.68	0.75	0.65	0.65	0.65	0.65	0.65	0.65
Peak Hour Factor	0%	13%	10%	0%	0%	0%	0%	0%	0%	0%	0%
Heavy Vehicles (%)	164	20	415	222	68	132	132	132	132	132	132
Adj. Flow (vph)	0	184	637	0	200	0	200	0	0	0	0
Shared Lane Traffic (%)	No	No	No	No	No	No	No	No	No	No	No
Lane Group Flow (vph)	No	No	No	No	No	No	No	No	No	No	No
Enter Blocked Intersection	Left	Left	Left	Left	Right	Right	Right	Right	Right	Right	Right
Lane Alignment	0.0	0.0	0.0	0.0	3.6	3.6	3.6	3.6	3.6	3.6	3.6
Median Width(m)	0.0	0.0	0.0	0.0	4.8	4.8	4.8	4.8	4.8	4.8	4.8
Link Offset(m)	0.0	0.0	0.0	0.0	4.8	4.8	4.8	4.8	4.8	4.8	4.8
Crosswalk Width(m)	1.14	1.14	1.14	1.14	1.14	1.14	1.14	1.14	1.14	1.14	1.14
Two way Left Turn Lane	24	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop	Stop
Headway Factor	24	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop	Stop
Turning Speed (k/h)	24	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop	Stop
Sign Control	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop	Stop



HCM Signalized Intersection Capacity Analysis 2035 Total PM Peak Hour (Lyons Closed).syn  
 8: Cross Ave & Lyons Lane

05-04-2022

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	33	232	9	32	514	81	188	0	288	46	1	0
Traffic Volume (vph)	33	232	9	32	514	81	188	0	288	46	1	0
Future Volume (vph)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Ideal Flow (vphpl)	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5
Total Lost time (s)	1.00	0.95	1.00	1.00	0.95	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	0.98	1.00	1.00	1.00	1.00	1.00	1.00
Frb, ped/bikes	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.99	1.00	1.00
Frb, ped/bikes	1.00	1.00	0.85	1.00	1.00	0.85	1.00	1.00	0.85	1.00	1.00	1.00
Flt Protected	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	0.95	1.00	0.95	1.00
Satd. Flow (prot)	1388	2954	1425	1593	3154	1422	1593	1425	1610	1676	1041	1041
Flt Permitted	0.30	1.00	1.00	0.60	1.00	1.00	0.76	1.00	0.76	1.00	0.76	1.00
Satd. Flow (perm)	438	2954	1425	999	3154	1422	1269	1425	1283	1676	1041	1041
Peak-hour factor, PHF	0.50	0.92	0.92	0.92	0.84	0.58	0.92	0.92	0.92	0.44	0.92	0.41
Adj. Flow (vph)	66	282	10	35	612	140	204	0	313	105	1	0
RTOR Reduction (vph)	0	0	7	0	0	100	0	0	136	0	0	0
Lane Group Flow (vph)	66	282	3	35	612	40	204	0	177	105	1	0
Conf. Peds. (#/hr)	1	1	1	1	1	1	1	1	1	1	1	1
Heavy Vehicles (%)	17%	10%	2%	2%	3%	0%	2%	2%	2%	0%	2%	4%
Turn Type	Perm	NA	Perm	Perm	NA	Perm	Perm	Perm	Perm	Perm	NA	Perm
Protected Phases	4	4	4	8	8	8	2	2	2	6	6	6
Permitted Phases	4	17.5	17.5	17.5	17.5	17.5	34.6	34.6	34.6	34.6	34.6	34.6
Actuated Green, G (s)	17.5	17.5	17.5	17.5	17.5	17.5	34.6	34.6	34.6	34.6	34.6	34.6
Effective Green, g (s)	0.29	0.29	0.29	0.29	0.29	0.29	0.57	0.57	0.57	0.57	0.57	0.57
Actuated g/C Ratio	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5
Clearance Time (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Vehicle Extension (s)	125	846	408	286	903	407	718	806	726	949	0.00	0.00
Lane Grp. Cap (vph)	c0.19											
v/s Ratio Prot	0.15	0.30	0.01	0.12	0.68	0.10	0.28	0.22	0.14	0.00	0.00	0.00
v/s Ratio Perm	18.3	17.0	15.6	16.1	19.3	16.0	6.8	6.6	6.3	5.8	0.00	0.00
Uniform Delay, d1	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Progression Factor	4.0	0.2	0.0	0.2	2.0	0.1	1.0	0.6	0.4	0.0	0.0	0.0
Incremental Delay, d2	22.3	17.2	15.6	16.3	21.3	16.1	7.8	7.2	6.7	5.8	0.0	0.0
Delay (s)	C	B	B	B	C	B	A	A	A	A	A	A
Level of Service	C	B	B	B	C	B	A	A	A	A	A	A
Approach Delay (s)	18.2	20.2	20.2		7.4	7.4	6.7	6.7	6.7	6.7	6.7	6.7
Approach LOS	B	C	C		A	A	A	A	A	A	A	A
Intersection Summary												
HCM 2000 Control Delay												B
HCM 2000 Volume to Capacity ratio												0.42
Actuated Cycle Length (s)												9.0
Intersection Capacity Utilization												49.4%
Analysis Period (min)												15
c Critical Lane Group												

2035 Total AM Peak Hour (Lyons Closed).syn  
 2: Trafalgar Road & QEW SB On Ramps

05-04-2022

Movement	EBL	EBR	NBL	NBT	SBL	SBT	SBR
Lane Configurations	0	0	0	0	0	0	0
Traffic Volume (vph)	0	0	0	2524	2093	2093	958
Future Volume (vph)	0	0	0	2524	2093	2093	958
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	1.00	1.00	1.00	0.91	0.91	0.91	1.00
Lane Util. Factor	1.00	1.00	1.00	0.91	0.91	0.91	1.00
Flt Protected	0.850						
Satd. Flow (prot)	0	0	0	5085	5085	5085	1583
Flt Permitted	0	0	0	5085	5085	5085	1583
Satd. Flow (perm)	0	0	0	50	50	50	50
Link Speed (k/h)	50						
Link Distance (m)	159.6						
Travel Time (s)	11.5						
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	0	0	0	2743	2275	2275	1041
Shared Lane Traffic (%)							
Lane Group Flow (vph)	0	0	0	2743	2275	2275	1041
Enter Blocked Intersection	No	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Left	Left	Right	Right
Median Width (m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Link Offset (m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Crosswalk Width (m)	4.8						
Two way Left Turn Lane	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Headway Factor	25	15	25	15	15	15	15
Turning Speed (k/h)	25	15	25	15	15	15	15
Sign Control	Stop	Free	Free	Free	Free	Free	Free
Intersection Summary							
Area Type:	Other						
Control Type:	Unsignalized						
Intersection Capacity Utilization	116.4%						
Analysis Period (min)	15						
ICU Level of Service H							

HCM Unsignalized Intersection Capacity Analysis 2035 Total AM Peak Hour (Lyons Closed).syn  
 05-04-2022



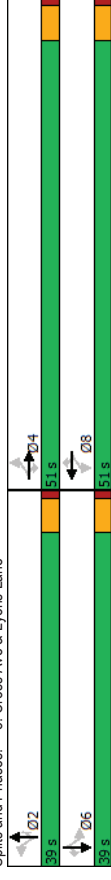
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations				↑↑↑	↑↑↑	↑
Traffic Volume (veh/h)	0	0	0	2524	2093	958
Future Volume (Veh/h)	0	0	0	2524	2093	958
Sign Control	Stop	Free	Free	Free	Free	0%
Grade	0%			0%	0%	0%
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	0	0	0	2743	2275	1041
Pedestrians						
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)				None	None	
Median type						
Median storage (veh)						
Upstream signal (m)				186	145	
pX, platoon unblocked	0.73	0.61	0.61			
vC, conflicting volume	3189	758	2275			
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	0	0	824			
tC, single (s)	6.8	6.9	4.1			
tC, 2 stage (s)						
tF (s)	3.5	3.3	2.2			
p0 queue free %	100	100	100			
cM capacity (veh/h)	748	666	485			

Direction_Lane #	NB.1	NB.2	NB.3	SB.1	SB.2	SB.3	SB.4
Volume Total	914	914	914	758	758	758	1041
Volume Left	0	0	0	0	0	0	0
Volume Right	0	0	0	0	0	0	1041
GSH	1700	1700	1700	1700	1700	1700	1700
Volume to Capacity	0.54	0.54	0.54	0.45	0.45	0.45	0.61
Queue Length 95th (m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Control Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Lane LOS							
Approach Delay (s)	0.0			0.0			
Approach LOS							
<b>Intersection Summary</b>							
Average Delay	0.0						
Intersection Capacity Utilization	116.4% ICU Level of Service H						
Analysis Period (min)	15						

Lanes, Volumes, Timings  
 8: Cross Ave & Lyons Lane  
 2035 Total PM Peak Hour (Lyons Closed).syn  
 05-04-2022

Analysis Period (min) 15

Splits and Phases: 8: Cross Ave & Lyons Lane



Lanes, Volumes, Timings 2035 Total PM Peak Hour (Lyons Closed).syn  
 05-04-2022

	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBR
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBR
Turn Type	Perm	NA	Perm	Perm	NA	Perm	Perm	Perm	Perm	NA	Perm
Protected Phases	4	4	4	8	8	8	2	2	2	6	6
Permitted Phases	4	4	4	8	8	8	2	2	2	6	6
Detector Phase	4	4	4	8	8	8	2	2	2	6	6
Switch Phase											
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Minimum Split (s)	22.5	22.5	22.5	22.5	22.5	22.5	22.5	22.5	22.5	22.5	22.5
Total Split (s)	51.0	51.0	51.0	51.0	51.0	51.0	39.0	39.0	39.0	39.0	39.0
Total Split (%)	56.7%	56.7%	56.7%	56.7%	56.7%	56.7%	43.3%	43.3%	43.3%	43.3%	43.3%
Maximum Green (s)	46.5	46.5	46.5	46.5	46.5	46.5	34.5	34.5	34.5	34.5	34.5
Yellow Time (s)	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5
LeadLag											
Lead-Lag Optimize?											
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	None	None	None	None	None	None	Max	Max	Max	Max	Max
Walk Time (s)	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0
Flash Dont Walk (s)	11.0	11.0	11.0	11.0	11.0	11.0	11.0	11.0	11.0	11.0	11.0
Pedestrian Calls (#/hr)	0	0	0	0	0	0	0	0	0	0	0
Act Effct Green (s)	17.5	17.5	17.5	17.5	17.5	17.5	34.7	34.7	34.7	34.7	34.7
Actuated g/C Ratio	0.29	0.29	0.29	0.29	0.29	0.29	0.57	0.57	0.57	0.57	0.57
v/c Ratio	0.53	0.30	0.02	0.12	0.68	0.28	0.28	0.30	0.15	0.00	0.00
Control Delay	34.7	17.7	5.3	16.6	23.3	4.9	9.2	0.7	8.1	7.0	7.0
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	34.7	17.7	5.3	16.6	23.3	4.9	9.2	0.7	8.1	7.0	7.0
LOS	C	B	A	B	C	A	A	A	A	A	A
Approach Delay	20.7			19.8			4.1			8.1	
Approach LOS	C			B			A			A	
Queue Length 50th (m)	6.5	12.0	0.0	3.0	33.3	0.0	11.1	0.0	5.2	0.1	0.1
Queue Length 95th (m)	8.4	20.3	2.1	8.8	44.3	2.1	27.0	0.0	6.4	0.7	0.7
Internal Link Dist (m)	257.3			16.1			148.6			17.7	
Turn Bay Length (m)	60.0	60.0	60.0	60.0	60.0	60.0	60.0	60.0	60.0	60.0	60.0
Base Capacity (vph)	334	2265	1092	762	2408	1118	718	1060	723	949	949
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.20	0.11	0.01	0.05	0.25	0.13	0.28	0.30	0.15	0.00	0.00
Intersection Summary											
Area Type:	CBD										
Cycle Length:	90										
Actuated Cycle Length:	61.2										
Natural Cycle:	45										
Control Type:	Semi Act-Uncoordinated										
Maximum v/c Ratio:	0.68										
Intersection Signal Delay:	14.6										
Intersection Capacity Utilization:	49.4%										
ICU Level of Service A											

Lanes, Volumes, Timings 2035 Total AM Peak Hour (Lyons Closed).syn  
 05-04-2022

	EBL	EBR	NBL	NBT	SBL	SBR
Lane Group	EBL	EBR	NBL	NBT	SBL	SBR
Lane Configurations	EBL	EBR	NBL	NBT	SBL	SBR
Traffic Volume (vph)	1123	1119	0	1401	2093	0
Future Volume (vph)	1123	1119	0	1401	2093	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	0.97	1.00	1.00	0.91	0.91	1.00
Frt	0.850					
Flt Protected	0.950					
Satd. Flow (prot)	3400	1599	0	4893	4988	0
Flt Permitted	0.950					
Satd. Flow (perm)	3400	1599	0	4893	4988	0
Right Turn on Red	Yes	Yes	Yes	Yes	Yes	Yes
Satd. Flow (RTOR)	50	50	50	50	50	50
Link Speed (k/h)	71.3	50.7	186.0	3.7	13.4	13.4
Travel Time (s)	5.1	0.92	0.92	0.92	0.92	0.92
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	3%	1%	0%	6%	4%	0%
Adj. Flow (vph)	1221	1216	0	1523	2275	0
Shared Lane Traffic (%)						
Lane Group Flow (vph)	1221	1216	0	1523	2275	0
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Left	Left	Right
Median Width (m)	7.2	0.0	0.0	0.0	0.0	0.0
Link Offset (m)	0.0	0.0	0.0	0.0	0.0	0.0
Crosswalk Width (m)	4.8	4.8	4.8	4.8	4.8	4.8
Two way Left Turn Lane						
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (k/h)	25	15	25	2	2	15
Number of Detectors	1	1	1	2	2	2
Detector Template	Left	Right	Thru	Thru	Thru	Thru
Leading Detector (m)	2.0	2.0	10.0	10.0	10.0	10.0
Trailing Detector (m)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Position (m)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Size (m)	2.0	2.0	0.6	0.6	0.6	0.6
Detector 1 Type	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex
Detector 1 Channel	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 2 Position (m)	9.4	9.4	0.6	0.6	0.6	0.6
Detector 2 Size (m)	0.6	0.6	0.6	0.6	0.6	0.6
Detector 2 Type	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex
Detector 2 Channel	0.0	0.0	0.0	0.0	0.0	0.0
Detector 2 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0
Turn Type	Perm	Perm	Perm	NA	NA	NA
Protected Phases	4	4	4	2	6	6
Permitted Phases	4	4	4	2	6	6
Detector Phase	4	4	4	2	6	6
Switch Phase						



Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Minimum Initial (s)	10.0	10.0	29.0	29.0	29.0	29.0
Minimum Split (s)	38.0	38.0	36.0	36.0	36.0	36.0
Total Split (s)	84.0	84.0	56.0	56.0	56.0	56.0
Total Split (%)	60.0%	60.0%	40.0%	40.0%	40.0%	40.0%
Maximum Green (s)	77.0	77.0	49.0	49.0	49.0	49.0
Yellow Time (s)	4.0	4.0	4.0	4.0	4.0	4.0
All-Red Time (s)	3.0	3.0	3.0	3.0	3.0	3.0
Lost Time Adjust (s)	-3.0	-3.0	-3.0	-3.0	-3.0	-3.0
Total Lost Time (s)	4.0	4.0	4.0	4.0	4.0	4.0
Lead/Lag						
Lead-Lag Optimize?						
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	None	None	C-Max	C-Max	C-Max	C-Max
Walk Time (s)	7.0	7.0	7.0	7.0	7.0	7.0
Flash Dont Walk (s)	11.0	11.0	11.0	11.0	11.0	11.0
Pedestrian Calls (#/hr)	0	0	0	0	0	0
Act Effct Green (s)	80.0	80.0	52.0	52.0	52.0	52.0
Actuated g/C Ratio	0.57	0.57	0.37	0.37	0.37	0.37
v/c Ratio	0.63	1.33	0.84	0.84	1.23	1.23
Control Delay	21.9	185.0	51.4	144.6	144.6	144.6
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	21.9	185.0	51.4	144.6	144.6	144.6
LOS	C	F	D	F	F	F
Approach Delay	103.3		51.4	144.6		
Approach LOS	F		D	F		
Queue Length 50th (m)	117.8	~459.0	172.0	~295.4		
Queue Length 95th (m)	140.4	#543.7	m183.6	m268.4		
Internal Link Dist (m)	47.3		26.7	162.0		
Turn Bay Length (m)						
Base Capacity (vph)	1942	913	1817	1852		
Starvation Cap Reductn	0	0	0	0		
Spillback Cap Reductn	0	0	0	0		
Storage Cap Reductn	0	0	0	0		
Reduced v/c Ratio	0.63	1.33	0.84	1.23		
<b>Intersection Summary</b>						
Area Type:	Other					
Cycle Length:	140					
Actuated Cycle Length:	140					
Offset: 0 (0%):	Referenced to phase 2:NBT and 6:SBT, Start of Green					
Natural Cycle:	75					
Control Type:	Actuated-Coordinated					
Maximum v/c Ratio:	1.33					
Intersection Signal Delay:	105.7					
Intersection Capacity Utilization:	116.4%					
Analysis Period (min):	15					
~ Volume exceeds capacity, queue is theoretically infinite.						
# 95th percentile volume exceeds capacity, queue may be longer.						



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	33	232	9	32	514	81	188	0	288	46	1	0
Traffic Volume (vph)	33	232	9	32	514	81	188	0	288	46	1	0
Future Volume (vph)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Ideal Flow (vphpl)	60.0	60.0	60.0	60.0	60.0	60.0	60.0	60.0	60.0	60.0	60.0	60.0
Storage Length (m)	1	1	1	1	1	1	1	1	1	1	1	1
Storage Lanes	7.5	7.5	7.5	7.5	7.5	7.5	7.5	7.5	7.5	7.5	7.5	7.5
Taper Length (m)	1.00	0.95	1.00	1.00	0.95	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Lane Util. Factor	1.00	0.95	1.00	1.00	0.98	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor												
Flt Protected	0.950	0.850	0.950	0.950	0.850	0.950	0.950	0.850	0.950	0.850	0.950	0.950
Satd. Flow (prot)	1388	2954	1425	1593	3154	1454	1593	1676	1425	1624	1676	1644
Flt Permitted	0.300	0.596	0.300	0.596	0.300	0.596	0.300	0.596	0.300	0.596	0.300	0.596
Satd. Flow (perm)	438	2954	1425	999	3154	1421	1269	1676	1425	1277	1676	1644
Right Turn on Red		Yes		Yes		Yes		Yes		Yes		Yes
Satd. Flow (RTOR)		18		140		140		50		584		50
Link Speed (k/h)		50		50		50		50		50		50
Link Distance (m)		281.3		40.1		172.6		172.6		41.7		41.7
Travel Time (s)		20.3		2.9		12.4		12.4		3.0		3.0
Conf. Peds. (#/hr)	1			1		9		9		9		9
Peak Hour Factor	0.50	0.92	0.92	0.84	0.58	0.92	0.92	0.92	0.84	0.44	0.92	0.41
Heavy Vehicles (%)	17%	10%	2%	3%	0%	2%	2%	2%	0%	2%	0%	4%
Adj. Flow (vph)	66	252	10	35	612	140	204	0	313	105	1	0
Shared Lane Traffic (%)												
Lane Group Flow (vph)	66	252	10	35	612	140	204	0	313	105	1	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Right	Left	Right	Left	Right	Left	Right	Left	Right
Median Width (m)	3.6	3.6	3.6	3.6	3.6	3.6	3.6	3.6	3.6	3.6	3.6	3.6
Link Offset (m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Crosswalk Width (m)	4.8	4.8	4.8	4.8	4.8	4.8	4.8	4.8	4.8	4.8	4.8	4.8
Two way Left Turn Lane												
Headway Factor	1.14	1.14	1.14	1.14	1.14	1.14	1.14	1.14	1.14	1.14	1.14	1.14
Turning Speed (k/h)	25	15	25	15	25	15	25	15	25	15	25	15
Number of Detectors	1	2	1	1	2	1	1	2	1	1	2	1
Detector Template	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Leading Detector (m)	2.0	10.0	2.0	2.0	10.0	2.0	2.0	10.0	2.0	2.0	10.0	2.0
Trailing Detector (m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Position (m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Size (m)	2.0	0.6	2.0	2.0	0.6	2.0	2.0	0.6	2.0	2.0	0.6	2.0
Detector 1 Type	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 2 Position (m)	9.4	9.4	9.4	9.4	9.4	9.4	9.4	9.4	9.4	9.4	9.4	9.4
Detector 2 Size (m)	0.6	0.6	0.6	0.6	0.6	0.6	0.6	0.6	0.6	0.6	0.6	0.6
Detector 2 Type	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex
Detector 2 Channel												
Detector 2 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0

HCM Signalized Intersection Capacity Analysis 2035 Total PM Peak Hour (Lyons Closed).syn  
 7: GO Bus Terminal/Argus Rd & Cross Ave 05-04-2022

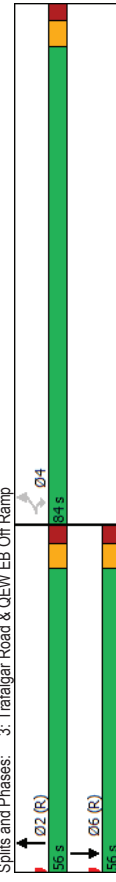
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	58	1156	71	197	525	155	166	3	164	230	24	289
Traffic Volume (vph)	58	1156	71	197	525	155	166	3	164	230	24	289
Future Volume (vph)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Ideal Flow (vphpl)	3.3	3.6	3.6	3.3	3.6	3.6	3.3	3.6	3.6	3.3	3.6	3.6
Lane Width	4.0	4.0	4.0	4.0	4.0	4.0	2.5	4.0	6.0	4.0	4.0	4.0
Total Lost time (s)	1.00	0.95	1.00	1.00	0.95	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Lane Util. Factor	1.00	1.00	0.97	1.00	1.00	0.96	1.00	1.00	0.97	1.00	0.98	1.00
Frpb, ped/bikes	0.99	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.98	1.00	1.00
Flt Protected	0.95	1.00	1.00	0.85	1.00	0.85	1.00	0.85	1.00	0.85	1.00	0.86
Flt Permitted	1.00	1.00	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.95
Satd. Flow (prot)	1558	3249	707	797	3249	1400	785	855	701	1534	1333	1333
Flt Permitted	0.45	1.00	1.00	0.10	1.00	0.10	1.00	0.17	1.00	0.75	1.00	1.00
Satd. Flow (perm)	730	3249	707	84	3249	1400	137	855	701	1211	1333	1333
Peak-hour factor, PHF	0.66	0.81	0.75	0.94	0.95	0.81	0.70	0.25	0.67	0.86	0.75	0.78
Adj. Flow (vph)	88	1427	95	210	553	191	237	12	245	267	32	371
RTOR Reduction (vph)	0	0	54	0	0	85	0	0	89	0	196	0
Lane Group Flow (vph)	88	1427	41	210	553	106	237	12	156	267	207	0
Conf. Peds. (#/hr)	9	4	4	4	9	12	12	20	20	20	20	12
Heavy Vehicles (%)	0%	0%	100%	97%	0%	0%	100%	100%	100%	100%	0%	100%

Turn Type	Perm	NA	Perm	pm-pt	NA	Perm	pm-pt	NA	Perm	Perm	NA
Protected Phases	2	2	6	6	3	8	8	8	4		
Permitted Phases	2	2	6	6	8	8	8	8	4		
Actuated Green, G (s)	36.0	36.0	47.5	47.5	47.5	29.6	29.6	29.6	21.1	21.1	21.1
Effective Green, g (s)	38.0	38.0	47.5	49.5	49.5	31.6	31.6	29.6	23.1	23.1	23.1
Actuated g/C Ratio	0.43	0.43	0.43	0.53	0.56	0.35	0.35	0.33	0.26	0.26	0.26
Clearance Time (s)	6.0	6.0	6.0	6.0	6.0	4.5	6.0	6.0	6.0	6.0	6.0
Vehicle Extension (s)	5.0	5.0	2.5	5.0	5.0	3.0	4.0	4.0	4.0	4.0	4.0
Lane Grp Cap (vph)	311	1385	301	104	1805	777	92	303	232	313	345
v/s Ratio Prot	0.44	0.17	0.17	0.17	0.17	0.01	0.01	0.22	0.22	0.16	0.16
v/s Ratio Perm	0.12	0.06	0.90	0.08	0.74	0.22	0.22	0.22	0.22	0.22	0.22
v/c Ratio	0.28	1.03	0.13	2.02	0.31	0.14	2.58	0.04	0.67	0.85	0.60
Uniform Delay, d1	16.7	25.5	15.5	23.6	10.6	9.5	26.4	18.8	25.6	31.4	28.9
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	1.0	32.3	0.4	490.7	0.2	0.2	739.9	0.1	8.1	20.2	3.3
Delay (s)	17.7	57.9	16.0	514.3	10.8	9.7	766.3	18.9	33.6	51.6	32.2
Level of Service	B	E	B	F	B	A	F	B	C	D	C
Approach Delay (s)	53.2			121.4			384.8				39.9
Approach LOS	D			F			F				D

Intersection Summary	Value	Level of Service
HCM 2000 Control Delay	112.2	HCM 2000 Level of Service
HCM 2000 Volume to Capacity ratio	2.33	F
Actuated Cycle Length (s)	89.1	Sum of lost time (s)
Intersection Capacity Utilization	93.6%	ICU Level of Service
Analysis Period (min)	15	F
c Critical Lane Group		

2035 Total AM Peak Hour (Lyons Closed).syn  
 3: Trafalgar Road & QEW EB Off Ramp 05-04-2022

Queue shown is maximum after two cycles.  
 m Volume for 95th percentile queue is metered by upstream signal.

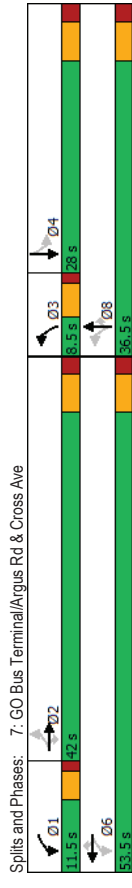


HCM Signalized Intersection Capacity Analysis 2035 Total AM Peak Hour (Lyons Closed).syn  
 3: Trafalgar Road & QEW EB Off Ramp 05-04-2022

Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	↔	↔		↔	↔	↔
Traffic Volume (vph)	1123	1119	0	1401	2093	0
Future Volume (vph)	1123	1119	0	1401	2093	0
Ideal Flow (v/hpl)	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	0.0	4.0	4.0	0.0
Lane Util. Factor	0.97	1.00	0.91	0.91	1.00	0.91
Frt	1.00	0.85	1.00	1.00	1.00	1.00
Flt Protected	0.95	1.00	1.00	1.00	1.00	1.00
Satd. Flow (prot)	3400	1599	4893	4988	4988	4988
Satd. Flow (perm)	3400	1599	4893	4988	4988	4988
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	1221	1216	0	1523	2275	0
RTOR Reduction (vph)	0	0	0	0	0	0
Lane Group Flow (vph)	1221	1216	0	1523	2275	0
Heavy Vehicles (%)	3%	1%	0%	6%	4%	0%
Turn Type	Perm	Perm	NA	NA	NA	NA
Protected Phases			2	6		
Permitted Phases	4	4				
Actuated Green, G (s)	77.0	77.0	49.0	49.0	49.0	
Effective Green, g (s)	80.0	80.0	52.0	52.0	52.0	
Actuated g/C Ratio	0.57	0.57	0.37	0.37	0.37	
Clearance Time (s)	7.0	7.0	7.0	7.0	7.0	
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	
Lane Grp Cap (vph)	1942	913	1817	1852		
v/s Ratio Prot			0.31	0.46		
v/s Ratio Perm	0.36	0.76				
v/c Ratio	0.63	1.33	0.84	1.23		
Uniform Delay, d1	20.1	30.0	40.2	44.0		
Progression Factor	1.00	1.00	1.21	1.08		
Incremental Delay, d2	0.6	156.9	2.4	104.1		
Delay (s)	20.7	186.9	51.0	151.5		
Level of Service	C	F	D	F		
Approach Delay (s)	103.6		51.0	151.5		
Approach LOS	F		D	F		
Intersection Summary						
HCM 2000 Control Delay			108.3		HCM 2000 Level of Service	F
HCM 2000 Volume to Capacity ratio			1.32			
Actuated Cycle Length (s)			140.0		Sum of lost time (s)	11.0
Intersection Capacity Utilization			116.4%		ICU Level of Service	H
Analysis Period (min)			15			
C Critical Lane Group						

Lanes, Volumes, Timings 2035 Total PM Peak Hour (Lyons Closed).syn  
 7: GO Bus Terminal/Argus Rd & Cross Ave 05-04-2022

Intersection Capacity Utilization 93.6%	ICU Level of Service F
Analysis Period (min) 15	
~ Volume exceeds capacity, queue is theoretically infinite.	
Queue shown is maximum after two cycles.	
# 95th percentile volume exceeds capacity, queue may be longer.	
Queue shown is maximum after two cycles.	



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Detector 2 Extend (s)	0.0			0.0			0.0			0.0		
Turn Type	Perm	NA	Perm	pm-pt	NA	pm-pt	NA	Perm	NA	Perm	NA	NA
Protected Phases				1	6	6	3	8				4
Permitted Phases	2	2	2	6	6	6	8	8	8	4	4	
Detector Phase	2	2	2	1	6	6	3	8	8	4	4	
Switch Phase												
Minimum Initial (s)	22.0	22.0	8.0	22.0	22.0	22.0	5.0	10.0	10.0	10.0	10.0	10.0
Minimum Split (s)	45.0	45.0	45.0	12.5	29.0	29.0	9.5	29.0	29.0	29.0	29.0	29.0
Total Split (s)	42.0	42.0	42.0	11.5	53.5	53.5	8.5	36.5	36.5	28.0	28.0	28.0
Total Split (%)	46.7%	46.7%	46.7%	12.8%	59.4%	59.4%	9.4%	40.6%	40.6%	31.1%	31.1%	31.1%
Maximum Green (s)	36.0	36.0	36.0	7.5	47.5	47.5	4.0	30.5	30.5	22.0	22.0	22.0
Yellow Time (s)	4.0	4.0	4.0	3.0	4.0	4.0	3.5	4.0	4.0	4.0	4.0	4.0
All-Red Time (s)	2.0	2.0	2.0	1.0	2.0	2.0	1.0	2.0	2.0	2.0	2.0	2.0
Lost Time Adjust (s)	-2.0	-2.0	-2.0	0.0	-2.0	-2.0	-2.0	0.0	0.0	-2.0	-2.0	-2.0
Total Lost Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	2.5	4.0	6.0	4.0	4.0	4.0
Lead/Lag	Lag	Lag	Lag	Lead	Lead	Lead				Lag	Lag	Lag
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes				Yes	Yes	Yes
Vehicle Extension (s)	5.0	5.0	5.0	2.5	5.0	5.0	3.0	4.0	4.0	4.0	4.0	4.0
Recall Mode	Min	Min	Min	Min	Min	Min	None	Min	Min	Min	Min	Min
Walk Time (s)	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0
Flash Dont Walk (s)	16.0	16.0	16.0	16.0	16.0	16.0	16.0	16.0	16.0	16.0	16.0	16.0
Pedestrian Calls (#/hr)	0	0	0	0	0	0	0	0	0	0	0	0
Act Effct Green (s)	38.0	38.0	38.0	49.5	49.5	49.5	33.1	31.6	29.6	23.1	23.1	23.1
Actuated g/C Ratio	0.43	0.43	0.43	0.56	0.56	0.56	0.37	0.35	0.33	0.26	0.26	0.26
v/c Ratio	0.28	1.03	0.25	1.98	0.31	0.22	2.52	0.04	0.76	0.85	0.74	0.74
Control Delay	20.2	59.0	2.7	491.2	11.3	2.2	737.5	19.3	29.5	57.7	20.1	20.1
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	20.2	59.0	2.7	491.2	11.3	2.2	737.5	19.3	29.5	57.7	20.1	20.1
LOS	C	E	A	F	B	A	F	B	C	E	E	C
Approach Delay												
Approach LOS												
Queue Length 50th (m)	10.2	-149.1	0.0	-53.2	26.9	0.0	-65.3	1.4	17.5	45.7	21.4	21.4
Queue Length 95th (m)	15.1	#158.6	0.8	#98.2	37.1	6.5	#81.4	1.4	24.2	#82.1	34.1	34.1
Internal Link Dist (m)												
Turn Bay Length (m)	20.0			60.0	20.0	60.0				15.0		
Base Capacity (vph)	311	1386	381	106	1805	862	94	311	328	326	552	552
Stallback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.28	1.03	0.25	1.98	0.31	0.22	2.52	0.04	0.75	0.82	0.73	0.73

Intersection Summary

Area Type: CBD

Cycle Length: 90

Actuated Cycle Length: 83.1

Natural Cycle: 130

Control Type: Semi Act-Uncoord

Maximum v/c Ratio: 2.52

Intersection Signal Delay: 107.8

Intersection LOS: F

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	0	0	51	0	0	0	0	1401	584	0	2272	940
Future Volume (vph)	0	0	51	0	0	0	0	1401	584	0	2272	940
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	70.0	0.0	0.0	0.0
Storage Lanes	0	0	1	0	0	0	0	0	1	0	0	0
Taper Length (m)	7.5			7.5			7.5			7.5		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.91	1.00	1.00	0.91	0.91
Ped Bike Factor												
Fit			0.865						0.850			0.956
Fit Protected												
Satd. Flow (prot)	0	0	1522	0	0	0	0	5085	1568	0	4890	0
Fit Permitted												
Satd. Flow (perm)	0	0	1522	0	0	0	0	5085	1568	0	4890	0
Link Speed (k/h)			50			50		50			50	
Link Distance (m)			165.2			196.1		271.0			50.7	
Travel Time (s)			111.9			14.1		19.5			3.7	
Confl. Peds. (#/hr)												24
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	0%	0%	8%	0%	2%	0%	0%	2%	3%	0%	2%	0%
Adj. Flow (vph)	0	0	55	0	0	0	0	1523	635	0	2470	1022
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	0	55	0	0	0	0	1523	635	0	3492	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Right	Left	Left	Left	Right	Left	Left	Right
Median Width (m)			0.0			0.0		3.6			3.6	
Link Offset (m)			0.0			0.0		0.0			0.0	
Crosswalk Width (m)			4.8			4.8		4.8			4.8	
Two way Left Turn Lane												
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (k/h)	25		15	25		15	25		15	25		15
Sign Control		Stop		Stop		Stop		Free		Free		Free
Intersection Summary												
Area Type:	Other											
Control Type:	Unsignalized											
Intersection Capacity Utilization	75.6%											
Analysis Period (min)	15											

Intersection Summary

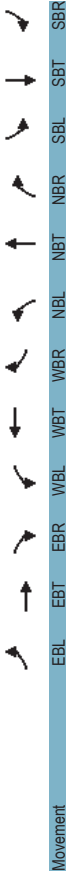
Area Type: Other

Control Type: Unsignalized

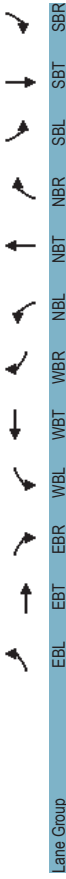
Intersection Capacity Utilization 75.6%

Analysis Period (min) 15

HCM Unsignalized Intersection Capacity Analysis 2035 Total AM Peak Hour (Lyons Closed).syn  
 4: Trafalgar Road & Argus Road/QEW EB On Ramp



Lanes, Volumes, Timings  
 7: GO Bus Terminal/Argus Rd & Cross Ave



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	0	0	51	0	0	0	0	1401	584	0	2272	940
Traffic Volume (veh/h)	0	0	51	0	0	0	0	1401	584	0	2272	940
Future Volume (Veh/h)	0	0	51	0	0	0	0	1401	584	0	2272	940
Sign Control	Stop	Stop	Stop	0%	0%	0%	Free	Free	Free	Free	Free	0%
Grade	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	0	0	55	0	0	0	0	1523	635	0	2470	1022
Pedestrians	0	0	24	0	0	0	0	0	0	0	0	0
Lane Width (m)	3.6	3.6	3.6	3.6	3.6	3.6	3.6	3.6	3.6	3.6	3.6	3.6
Walking Speed (m/s)	1.2	1.2	1.2	1.2	1.2	1.2	1.2	1.2	1.2	1.2	1.2	1.2
Percent Blockage	2	2	2	2	2	2	2	2	2	2	2	2
Right turn flare (veh)	0	0	0	0	0	0	0	0	0	0	0	0
Median type	None	None	None	None	None	None	None	None	None	None	None	None
Median storage (veh)	0	0	0	0	0	0	0	0	0	0	0	0
Upstream signal (m)	0.71	0.71	0.63	0.71	0.71	0.85	0.63	0.71	0.85	0.85	0.85	51
pX, platoon unblocked	3513	4528	1358	2346	5039	508	3516	1523	1523	1523	1523	1523
vC, conflicting volume	1642	3072	0	0	3792	0	2945	980	980	980	980	980
vC1, stage 1 conf vol	7.5	6.5	7.1	7.5	6.5	6.9	4.1	4.1	4.1	4.1	4.1	4.1
vC2, stage 2 conf vol	3.5	4.0	3.4	3.5	4.0	3.3	2.2	2.2	2.2	2.2	2.2	2.2
tC, 2 stage (s)	100	100	92	100	100	100	100	100	100	100	100	100
tF (s)	46	9	660	659	3	923	76	603	603	603	603	603
cM capacity (veh/h)	EB 1	NB 1	NB 2	NB 3	NB 4	SB 1	SB 2	SB 3				
Direction, Lane #	55	508	508	508	635	988	988	1516				
Volume Total	0	0	0	0	0	0	0	0				
Volume Left	0	0	0	0	0	0	0	0				
Volume Right	55	0	0	0	635	0	0	1022				
GSH	660	1700	1700	1700	1700	1700	1700	1700				
Volume to Capacity	0.08	0.30	0.30	0.30	0.37	0.58	0.58	0.89				
Queue Length 95th (m)	2.2	0.0	0.0	0.0	0.0	0.0	0.0	0.0				
Control Delay (s)	10.9	0.0	0.0	0.0	0.0	0.0	0.0	0.0				
Lane LOS	B	B	B	B	B	B	B	B				
Approach Delay (s)	10.9	0.0	0.0	0.0	0.0	0.0	0.0	0.0				
Approach LOS	B	B	B	B	B	B	B	B				
Intersection Summary												
Average Delay	0.1											
Intersection Capacity Utilization	75.6%											
ICU Level of Service	D											
Analysis Period (min)	15											

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑↑	↑↑	↑↑	↑↑	↑↑	↑↑	↑↑	↑↑	↑↑	↑↑	↑↑	↑↑
Traffic Volume (vph)	58	1156	71	197	525	155	166	3	164	230	24	289
Future Volume (vph)	58	1156	71	197	525	155	166	3	164	230	24	289
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (m)	3.3	3.6	3.6	3.3	3.6	3.6	3.3	3.6	3.6	3.3	3.6	3.6
Storage Length (m)	20.0	60.0	20.0	60.0	20.0	60.0	20.0	60.0	20.0	15.0	15.0	0.0
Storage Lanes	1	1	1	1	1	1	1	1	1	1	1	0
Taper Length (m)	7.5	0.95	1.00	1.00	0.95	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Lane Util. Factor	1.00	0.95	1.00	1.00	0.95	1.00	1.00	1.00	1.00	0.98	0.98	0.98
Ped Bike Factor	0.99	0.97	0.850	0.97	0.850	0.96	0.99	0.96	0.98	0.96	0.98	0.98
Frt	0.850	0.850	0.850	0.850	0.850	0.850	0.850	0.850	0.850	0.850	0.862	0.862
Flt Protected	0.950	0.950	0.950	0.950	0.950	0.950	0.950	0.950	0.950	0.950	0.950	0.950
Satd. Flow (prot)	1570	3249	727	797	3249	1454	785	855	727	1570	1333	0
Flt Permitted	0.445	0.100	0.100	0.100	0.165	0.165	0.165	0.165	0.165	0.750	0.750	0
Satd. Flow (perm)	730	3249	707	84	3249	1399	136	855	701	1211	1333	0
Right Turn on Red	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Satd. Flow (RTOR)	139	139	139	139	139	139	139	139	139	139	265	265
Link Speed (k/h)	50	50	50	50	50	50	50	50	50	50	50	50
Link Distance (m)	374.0	243.7	17.5	81.9	180.7	180.7	180.7	180.7	180.7	180.7	180.7	180.7
Travel Time (s)	9	4	4	9	12	20	20	20	20	20	13.0	13.0
Conf. Peds. (#/hr)	0.66	0.81	0.75	0.94	0.95	0.81	0.70	0.25	0.67	0.86	0.75	0.78
Peak Hour Factor	0%	0%	100%	97%	0%	0%	100%	100%	100%	100%	0%	0%
Heavy Vehicles (%)	0%	0%	100%	97%	0%	0%	100%	100%	100%	100%	0%	0%
Adj. Flow (vph)	88	1427	95	210	553	191	237	12	245	267	32	371
Shared Lane Traffic (%)	88	1427	95	210	553	191	237	12	245	267	32	371
Lane Group Flow (vph)	No	No	No	No	No	No	No	No	No	No	No	No
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(m)	7.2	7.2	7.2	7.2	7.2	7.2	7.2	7.2	7.2	7.2	7.2	7.2
Link Offset(m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Crosswalk Width(m)	4.8	4.8	4.8	4.8	4.8	4.8	4.8	4.8	4.8	4.8	4.8	4.8
Two way Left Turn Lane	1.19	1.14	1.14	1.19	1.14	1.14	1.19	1.14	1.14	1.19	1.14	1.14
Headway Factor	24	14	24	14	24	14	24	14	24	14	24	14
Turning Speed (k/h)	1	2	1	1	2	1	1	2	1	1	2	1
Number of Detectors	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Detector Template	2.0	10.0	2.0	10.0	2.0	10.0	2.0	10.0	2.0	2.0	10.0	2.0
Leading Detector (m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Trailing Detector (m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Position(m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Size(m)	2.0	0.6	2.0	2.0	0.6	2.0	0.6	2.0	0.6	2.0	0.6	2.0
Detector 1 Type	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex
Detector 1 Channel	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 2 Position(m)	9.4	9.4	9.4	9.4	9.4	9.4	9.4	9.4	9.4	9.4	9.4	9.4
Detector 2 Size(m)	0.6	0.6	0.6	0.6	0.6	0.6	0.6	0.6	0.6	0.6	0.6	0.6
Detector 2 Type	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex
Detector 2 Channel	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex



HCM Signalized Intersection Capacity Analysis 2035 Total PM Peak Hour (Lyons Closed).syn  
 6: Trafalgar Road

Lanes, Volumes, Timings 2035 Total AM Peak Hour (Lyons Closed).syn  
 5: Trafalgar Road & Cross Avenue/South Service Road

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Traffic Volume (vph)	621	781	239	66	1047	804	170	486	49	650	703	627
Future Volume (vph)	621	781	239	66	1047	804	170	486	49	650	703	627
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	2.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lane Util. Factor	0.97	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.97	1.00	1.00
Frpb, ped/bikes	1.00	0.98	1.00	1.00	0.94	1.00	1.00	0.97	1.00	1.00	1.00	0.95
Fpb, ped/bikes	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt	1.00	0.96	1.00	1.00	0.85	1.00	1.00	0.85	1.00	0.85	1.00	0.85
Flt Protected	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	1.00	0.95
Satd. Flow (prot)	3467	3339	1805	3574	1503	1770	3610	1559	3433	1900	1503	1503
Flt Permitted	0.95	1.00	0.13	1.00	0.12	1.00	0.12	1.00	0.13	1.00	0.95	1.00
Satd. Flow (perm)	3467	3339	241	3574	1503	219	3610	1559	3433	1900	1503	1503
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	675	849	260	72	1138	874	185	528	53	707	764	682
RTOR Reduction (vph)	0	20	0	0	216	0	0	39	0	0	186	496
Lane Group Flow (vph)	675	1089	0	72	1138	658	185	528	14	707	764	496
Conf. Peds. (#/hr)	21	14	14	14	21	21	17	17	10	10	10	17
Heavy Vehicles (%)	1%	3%	1%	0%	1%	1%	2%	0%	0%	2%	0%	2%
Turn Type	Prot	NA	pm+pt	NA	Perm	pm+pt	NA	Perm	Prot	NA	Prot	NA
Protected Phases	7	4	3	8	8	5	2	2	1	6	6	6
Permitted Phases	21.0	57.4	45.6	40.0	40.0	40.0	33.0	33.0	22.0	48.0	48.0	48.0
Actuated Green, G (s)	22.0	60.4	47.6	43.0	43.0	42.0	36.0	36.0	23.0	51.0	51.0	51.0
Effective Green, g (s)	0.16	0.43	0.34	0.31	0.31	0.30	0.26	0.26	0.16	0.36	0.36	0.36
Actuated g/C Ratio	5.0	5.0	7.0	7.0	7.0	7.0	7.0	7.0	5.0	7.0	7.0	7.0
Clearance Time (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Vehicle Extension (s)	544	1440	155	1097	461	154	928	400	563	692	547	547
Lane Grp. Cap (vph)	c0.19	0.33	0.02	0.32	0.07	0.15	0.15	c0.21	c0.40	0.33	0.40	0.33
v/s Ratio Prot	0.14	0.46	0.14	0.44	0.29	0.29	0.01	0.01	0.01	0.26	0.10	0.91
v/s Ratio Perm	0.46	0.76	0.46	1.04	1.43	1.20	0.57	0.03	1.26	1.10	0.91	0.91
Uniform Delay, d1	59.0	33.6	33.6	48.5	48.5	44.4	45.2	39.0	58.5	44.5	42.3	42.3
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.33	1.24	1.85	1.85
Incremental Delay, d2	123.3	2.3	2.2	37.3	204.4	138.8	2.5	0.2	119.9	54.4	8.5	8.5
Delay (s)	182.3	35.9	35.8	85.8	252.9	181.1	47.8	39.1	197.7	109.6	86.6	86.6
Level of Service	F	D	D	F	F	F	D	D	F	F	F	F
Approach Delay (s)	F	91.3	154.2	F	79.4	F	E	E	F	131.2	F	F
Approach LOS	F	F	F	F	F	F	E	E	F	F	F	F
Intersection Summary												
HCM 2000 Control Delay	121.9	HCM 2000 Level of Service										
HCM 2000 Volume to Capacity ratio	1.29	F										
Actuated Cycle Length (s)	140.0	Sum of lost time (s)										
Intersection Capacity Utilization	106.4%	ICU Level of Service										
Analysis Period (min)	15	G										
c Critical Lane Group												

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Traffic Volume (vph)	743	400	163	87	366	179	160	1062	547	393	1254	663
Future Volume (vph)	743	400	163	87	366	179	160	1062	547	393	1254	663
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (m)	25.0	60.0	25.0	60.0	25.0	60.0	50.0	60.0	25.0	60.0	25.0	60.0
Storage Lanes	2	1	2	1	2	1	1	1	1	1	1	1
Taper Length (m)	7.5	7.5	7.5	7.5	7.5	7.5	7.5	7.5	7.5	7.5	7.5	7.5
Lane Util. Factor	0.97	0.95	1.00	0.97	0.95	1.00	0.91	0.91	1.00	1.00	0.91	1.00
Ped Bike Factor	1.00	0.98	0.99	0.99	1.00	0.87	0.87	0.87	0.87	0.87	0.87	0.97
Flt	0.850	0.850	0.850	0.850	0.850	0.850	0.850	0.850	0.850	0.850	0.850	0.850
Flt Protected	0.950	0.950	0.950	0.950	0.950	0.950	0.950	0.950	0.950	0.950	0.950	0.950
Satd. Flow (prot)	3213	3406	1442	3400	3343	1553	1641	5036	1615	1752	5036	1553
Flt Permitted	0.950	0.950	0.950	0.950	0.950	0.950	0.124	0.124	0.106	0.106	0.106	0.106
Satd. Flow (perm)	3208	3406	1411	3382	3343	1531	214	5036	1404	196	5036	1499
Right Turn on Red	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Satd. Flow (RTOR)	50	17.7	62	50	62	94	50	50	94	50	50	509
Link Distance (m)	243.7	17.5	17.2	238.9	17.2	266.5	19.2	266.5	19.2	271.0	19.5	271.0
Travel Time (s)	1	4	4	1	10	10	52	52	3%	3%	4%	4%
Conf. Peds. (#/hr)	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Peak Hour Factor	9%	12%	3%	8%	4%	10%	3%	0%	3%	3%	3%	4%
Heavy Vehicles (%)	808	435	177	95	398	195	174	1154	595	427	1363	721
Adj. Flow (vph)	808	435	177	95	398	195	174	1154	595	427	1363	721
Shared Lane Traffic (%)												
Lane Group Flow (vph)	808	435	177	95	398	195	174	1154	595	427	1363	721
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Left	Right	Left	Left	Left	Right	Left	Left	Right
Median Width (m)	7.2	7.2	7.2	7.2	7.2	7.2	3.6	3.6	3.6	3.6	3.6	3.6
Link Offset (m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Crosswalk Width (m)	4.8	4.8	4.8	4.8	4.8	4.8	4.8	4.8	4.8	4.8	4.8	4.8
Two way Left Turn Lane												
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (k/h)	25	15	25	15	25	15	25	15	25	15	25	15
Number of Detectors	1	2	1	1	2	1	1	2	1	1	2	1
Detector Template	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Leading Detector (m)	2.0	10.0	2.0	2.0	10.0	2.0	2.0	10.0	2.0	2.0	10.0	2.0
Trailing Detector (m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Position (m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Size (m)	2.0	0.6	2.0	2.0	0.6	2.0	2.0	0.6	2.0	2.0	0.6	2.0
Detector 1 Type	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 2 Position (m)	9.4	9.4	9.4	9.4	9.4	9.4	9.4	9.4	9.4	9.4	9.4	9.4
Detector 2 Size (m)	0.6	0.6	0.6	0.6	0.6	0.6	0.6	0.6	0.6	0.6	0.6	0.6
Detector 2 Type	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex
Detector 2 Channel												
Detector 2 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0

Lanes, Volumes, Timings  
5: Trafalgar Road & Cross Avenue/South Service Road

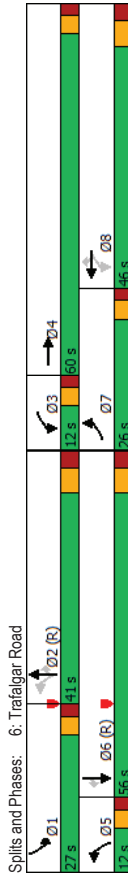
Lanes, Volumes, Timings  
6: Trafalgar Road

2035 Total AM Peak Hour (Lyons Closed).syn  
05-04-2022

2035 Total PM Peak Hour (Lyons Closed).syn  
05-04-2022

	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Turn Type	Prot	NA	Perm	Prot	NA	pm+ov	pm+pt	NA	pm+ov	pm+pt	NA	Perm
Protected Phases	7	4	4	3	8	8	1	5	2	3	1	6
Permitted Phases	7	4	4	3	8	8	1	5	2	3	1	6
Detector Phase	7	4	4	3	8	8	1	5	2	3	1	6
Switch Phase	7	4	4	3	8	8	1	5	2	3	1	6
Minimum Initial (s)	10.0	5.0	5.0	12.0	10.0	7.0	7.0	27.0	12.0	7.0	27.0	27.0
Minimum Split (s)	17.0	25.0	25.0	17.0	25.0	11.5	11.5	34.0	17.0	11.5	34.0	34.0
Total Split (s)	40.0	31.0	31.0	34.0	25.0	35.0	17.9	40.0	34.0	35.0	57.1	57.1
Total Split (%)	28.6%	22.1%	22.1%	24.3%	17.9%	25.0%	12.8%	28.6%	24.3%	25.0%	40.8%	40.8%
Maximum Green (s)	33.0	24.0	24.0	30.0	18.0	31.0	13.9	33.0	30.0	31.0	50.1	50.1
Yellow Time (s)	4.0	4.0	4.0	3.0	4.0	3.0	4.0	3.0	4.0	3.0	4.0	4.0
All-Red Time (s)	3.0	3.0	3.0	1.0	3.0	1.0	1.0	3.0	1.0	1.0	3.0	3.0
Lost Time Adjust (s)	-3.0	-3.0	-3.0	0.0	-3.0	0.0	0.0	-3.0	0.0	0.0	-3.0	-3.0
Total Lost Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lead/Lag	Lead	Lag	Lag	Lead	Lag	Lead	Lead	Lag	Lag	Lead	Lead	Lag
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	None	None	None	None	None	None	None	C-Max	None	None	C-Max	C-Max
Walk Time (s)	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0
Flash Dont Walk (s)	11.0	11.0	11.0	11.0	11.0	11.0	11.0	11.0	11.0	11.0	11.0	11.0
Pedestrian Calls (#/hr)	0	0	0	0	0	0	0	0	0	0	0	0
Act Effct Green (s)	36.0	35.6	35.6	21.1	20.7	51.3	49.9	36.7	57.9	71.3	54.1	54.1
Actuated g/C Ratio	0.26	0.25	0.25	0.15	0.15	0.37	0.36	0.26	0.41	0.51	0.39	0.39
v/c Ratio	0.98	0.50	0.36	0.19	0.80	0.32	0.83	0.87	0.89	0.97	0.70	0.81
Control Delay	77.8	48.5	8.6	50.4	70.9	20.9	40.5	54.3	42.3	48.0	25.4	4.7
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	77.8	48.5	8.6	50.4	70.9	20.9	40.5	54.3	42.3	48.0	25.4	4.7
LOS	E	D	A	D	E	C	D	D	D	D	D	A
Approach Delay	60.2			53.9			49.3				23.3	
Approach LOS	E			D			D				C	
Queue Length 50th (m)	121.1	57.7	0.0	12.5	59.6	25.5	36.7	100.1	148.5	94.3	119.3	42.9
Queue Length 95th (m)	#164.5	81.7	21.0	19.7	#80.4	45.1	m37.7	m98.6	m77.9	m61.4	m80.8	m13.6
Internal Link Dist (m)	219.7			214.9			242.5				247.0	
Turn Bay Length (m)	25.0	60.0	25.0	60.0	50.0	60.0	50.0	60.0	60.0	25.0	60.0	60.0
Base Capacity (vph)	826	866	490	728	501	609	218	1321	763	443	1947	891
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.98	0.50	0.36	0.13	0.79	0.32	0.80	0.87	0.78	0.96	0.70	0.81
Intersection Summary												
Area Type:	Other											
Cycle Length:	140											
Actuated Cycle Length:	140											
Offset:	54.5 (39%), Referenced to phase 2:NBTL and 6:SBTL - Start of Green											
Natural Cycle:	120											
Control Type:	Actuated-Coordinated											
Maximum v/c Ratio:	0.98											
Intersection Signal Delay:	42.2											

Intersection Capacity Utilization 106.4%  
Analysis Period (min) 15  
ICU Level of Service G  
~ Volume exceeds capacity, queue is theoretically infinite.  
Queue shown is maximum after two cycles.  
# 95th percentile volume exceeds capacity, queue may be longer.  
Queue shown is maximum after two cycles.  
m Volume for 95th percentile queue is metered by upstream signal.





HCM Signalized Intersection Capacity Analysis 2035 Total AM Peak Hour (Lyons Closed).syn  
 5: Trafalgar Road & Cross Avenue/South Service Road 05-04-2022

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	→	→	→	←	←	←	←	←	←	←	←	←
Traffic Volume (vph)	743	400	163	87	366	179	160	1062	547	393	1254	663
Future Volume (vph)	743	400	163	87	366	179	160	1062	547	393	1254	663
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lane Util. Factor	0.97	0.95	1.00	0.97	1.00	1.00	0.91	1.00	1.00	0.91	1.00	1.00
Frpb, ped/bikes	1.00	1.00	0.98	1.00	1.00	0.99	1.00	1.00	0.92	1.00	1.00	0.97
Flpb, ped/bikes	1.00	1.00	0.85	1.00	1.00	0.85	1.00	1.00	0.85	1.00	1.00	0.85
Flt	1.00	1.00	0.85	1.00	1.00	0.85	1.00	1.00	0.85	1.00	1.00	0.85
Flt Protected												
Satd. Flow (prot)	3213	3406	1411	3400	3343	1545	1641	5036	1485	1752	5036	1499
Flt Permitted	0.95	1.00	1.00	0.95	1.00	0.95	1.00	1.00	0.95	1.00	1.00	0.95
Satd. Flow (perm)	3213	3406	1411	3400	3343	1545	213	5036	1485	195	5036	1499
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	808	435	177	95	398	195	174	1154	595	427	1363	721
RTOR Reduction (vph)	0	0	132	0	0	41	0	0	57	0	0	312
Lane Group Flow (vph)	808	435	45	95	398	154	174	1154	538	427	1363	409
Conf. Peds. (#/hr)	1	4	4	4	4	4	1	10	52	52	52	10
Heavy Vehicles (%)	9%	6%	12%	3%	8%	4%	10%	3%	0%	3%	3%	4%
Turn Type	Prot	NA	Perm	Prot	NA	pm+ov	pm+pt	NA	pm+ov	pm+pt	NA	Perm
Protected Phases	7	4	3	8	1	5	2	3	1	6		
Permitted Phases			4		8	2	2	6				
Actuated Green, G (s)	33.0	32.6	32.6	21.1	17.7	48.2	46.9	33.8	54.9	68.3	51.2	51.2
Effective Green, g (s)	36.0	35.6	35.6	21.1	20.7	48.2	46.9	36.8	54.9	68.3	54.2	54.2
Actuated g/C Ratio	0.26	0.25	0.25	0.15	0.15	0.34	0.33	0.26	0.39	0.49	0.39	0.39
Clearance Time (s)	7.0	7.0	7.0	4.0	7.0	4.0	7.0	4.0	7.0	4.0	7.0	7.0
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Lane Grp Cap (vph)	826	866	358	512	494	531	204	1323	582	434	1949	580
v/s Ratio Prot	c0.25	0.13	0.03	c0.12	0.06	0.28	0.23	0.14	c0.21	0.27		
v/s Ratio Perm												
vic Ratio	0.98	0.50	0.13	0.19	0.81	0.29	0.85	0.87	0.92	0.98	0.70	0.71
Uniform Delay, d1	51.6	44.6	40.2	51.9	57.7	33.4	35.7	49.4	40.6	43.4	36.1	36.2
Progression Factor	1.00	1.00	1.00	1.00	1.00	0.92	1.05	1.24	0.99	0.69	0.69	0.35
Incremental Delay, d2	25.7	0.5	0.2	0.2	9.3	0.3	8.1	2.1	6.5	9.5	0.2	0.7
Delay (s)	77.3	45.1	40.4	52.1	67.0	33.7	40.8	53.9	56.6	52.5	25.0	13.2
Level of Service	E	D	D	D	E	C	D	D	E	D	C	B
Approach Delay (s)	62.8				55.5		53.6				26.3	
Approach LOS	E				E		D				C	
Intersection Summary												
HCM 2000 Control Delay	45.3	HCM 2000 Level of Service										
HCM 2000 Volume to Capacity ratio	0.95	D										
Actuated Cycle Length (s)	140.0	Sum of lost time (s)										
Intersection Capacity Utilization	89.1%	ICU Level of Service										
Analysis Period (min)	15	E										
c Critical Lane Group												

2035 Total PM Peak Hour (Lyons Closed).syn  
 6: Trafalgar Road 05-04-2022

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	→	→	→	←	←	←	←	←	←	←	←	←
Traffic Volume (vph)	621	781	239	66	1047	804	170	486	49	650	703	627
Future Volume (vph)	621	781	239	66	1047	804	170	486	49	650	703	627
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (m)	80.0	0.0	0.0	80.0	0.0	25.0	0.0	80.0	0.0	80.0	0.0	0.0
Storage Lanes	2	0	0	1	1	1	1	1	1	1	1	1
Taper Length (m)	7.5	7.5	7.5	7.5	7.5	7.5	7.5	7.5	7.5	7.5	7.5	7.5
Lane Util. Factor	0.97	0.95	0.95	1.00	0.95	1.00	1.00	0.95	1.00	0.97	1.00	1.00
Ped Bike Factor	0.99	0.98	0.98	0.94	0.94	0.94	0.94	0.97	0.98	0.97	0.98	0.95
Flt	0.965	0.965	0.965	0.850	0.850	0.850	0.850	0.850	0.850	0.850	0.850	0.850
Flt Protected	0.950	0.950	0.950	0.950	0.950	0.950	0.950	0.950	0.950	0.950	0.950	0.950
Satd. Flow (prot)	3467	3339	0	1805	3574	1599	1770	3610	1615	3433	1900	1583
Flt Permitted	0.950	0.950	0.127	0.127	0.118	0.118	0.118	0.118	0.118	0.118	0.118	0.118
Satd. Flow (perm)	3432	3339	0	241	3574	1503	220	3610	1559	3369	1900	1503
Right Turn on Red	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Satd. Flow (RTOR)	35	35	35	312	312	312	312	312	312	312	312	292
Link Speed (k/h)	50	50	50	50	50	50	50	50	50	50	50	50
Link Distance (m)	98.1	98.1	98.1	316.9	316.9	316.9	171.0	171.0	171.0	171.0	266.5	19.2
Travel Time (s)	7.1	7.1	7.1	22.8	22.8	22.8	12.3	12.3	12.3	12.3	19.2	17
Conf. Peds. (#/hr)	21	14	14	14	14	14	17	17	10	10	10	17
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	1%	3%	1%	0%	1%	1%	2%	2%	0%	2%	0%	2%
Adj. Flow (vph)	675	849	260	72	1138	874	185	528	53	707	764	682
Shared Lane Traffic (%)												
Lane Group Flow (vph)	675	1109	0	72	1138	874	185	528	53	707	764	682
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Right	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width (m)	7.2	7.2	7.2	7.2	7.2	7.2	7.2	7.2	7.2	7.2	7.2	7.2
Link Offset (m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Crosswalk Width (m)	4.8	4.8	4.8	4.8	4.8	4.8	4.8	4.8	4.8	4.8	4.8	4.8
Two way Left Turn Lane												
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (k/h)	25	15	25	25	25	15	25	25	15	25	15	25
Number of Detectors	1	2	1	2	1	1	2	1	2	1	2	1
Detector Template	Left	Thru	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Right
Leading Detector (m)	2.0	10.0	2.0	10.0	2.0	2.0	10.0	2.0	2.0	10.0	2.0	2.0
Trailing Detector (m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Position (m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Size (m)	2.0	0.6	2.0	0.6	2.0	0.6	2.0	0.6	2.0	0.6	2.0	0.6
Detector 1 Type	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 2 Position (m)	9.4	9.4	9.4	9.4	9.4	9.4	9.4	9.4	9.4	9.4	9.4	9.4
Detector 2 Size (m)	0.6	0.6	0.6	0.6	0.6	0.6	0.6	0.6	0.6	0.6	0.6	0.6
Detector 2 Type	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex
Detector 2 Channel												
Detector 2 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0

HCM Signalized Intersection Capacity Analysis 2035 Total PM Peak Hour (Lyons Closed).syn  
5: Trafalgar Road & Cross Avenue/South Service Road 05-04-2022

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	1268	407	184	294	334	413	240	1474	197	172	1504	584
Traffic Volume (vph)	1268	407	184	294	334	413	240	1474	197	172	1504	584
Future Volume (vph)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Ideal Flow (vphpl)	4.0	4.0	7.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Total Lost time (s)	0.97	0.95	1.00	0.97	1.00	0.95	1.00	0.91	1.00	1.00	0.91	1.00
Lane Util. Factor	1.00	1.00	0.95	1.00	1.00	1.00	1.00	1.00	0.88	1.00	1.00	0.95
Frb. ped/bikes	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frb. ped/bikes	1.00	1.00	0.85	1.00	1.00	0.85	1.00	0.85	1.00	1.00	0.85	1.00
Flt	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	0.95	1.00	0.95	1.00
Flt Protected	3400	3343	1413	3433	3471	1599	1719	5136	1422	1805	5136	1485
Satd. Flow (prot)	0.95	1.00	1.00	0.95	1.00	1.00	0.10	1.00	1.00	0.11	1.00	1.00
Flt Permitted	3400	3343	1413	3433	3471	1599	181	5136	1422	201	5136	1485
Satd. Flow (perm)	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Peak-hour factor, PHF	1378	442	200	320	363	449	261	1602	214	187	1635	635
Adj. Flow (vph)	0	0	124	0	0	93	0	85	0	0	232	403
RTOR Reduction (vph)	1378	442	76	320	363	356	261	1602	129	187	1635	403
Lane Group Flow (vph)	3%	8%	9%	2%	4%	1%	5%	1%	0%	0%	1%	3%
Conf. Peds. (#/hr)	Prot	NA	Perm	Prot	NA	pm+ov	pm+pt	NA	pm+ov	pm+pt	NA	Perm
Heavy Vehicles (%)	7	4	3	3	8	1	5	2	3	3	1	6
Turn Type	Protected Phases											
Permitted Phases	4 4											
Actuated Green, G (s)	46.0	48.8	17.6	17.4	28.9	53.8	40.1	57.7	49.4	37.9	37.9	6
Effective Green, g (s)	49.0	51.8	48.8	17.6	20.4	28.9	53.8	43.1	57.7	49.4	40.9	40.9
Actuated g/C Ratio	0.35	0.37	0.35	0.13	0.15	0.21	0.38	0.31	0.41	0.35	0.29	0.29
Clearance Time (s)	7.0	7.0	4.0	7.0	4.0	4.0	7.0	4.0	4.0	4.0	7.0	7.0
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Lane Grp Cap (vph)	1190	1236	492	431	505	330	220	1581	586	202	1500	433
v/s Ratio Prot	c0.41	0.13	0.09	0.10	c0.09	c0.12	0.31	0.03	0.08	0.32		
v/s Ratio Perm												
vic Ratio	1.16	0.36	0.15	0.74	0.72	1.08	1.19	1.01	0.22	0.93	1.09	0.93
Uniform Delay, d1	45.5	32.0	31.4	59.0	57.1	55.5	40.8	48.4	26.6	37.3	49.5	48.2
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.34	1.06	2.18	1.05	0.74	0.47
Incremental Delay, d2	80.8	0.2	0.1	6.8	4.9	72.3	88.3	10.4	0.0	25.5	46.4	16.8
Delay (s)	F	C	C	E	E	F	F	E	E	E	F	D
Level of Service	F	C	C	E	E	F	F	E	E	E	F	D
Approach Delay (s)	96.4											
Approach LOS	F											
Intersection Summary												
HCM 2000 Control Delay	80.4											
HCM 2000 Volume to Capacity ratio	1.12											
Actuated Cycle Length (s)	140.0											
Intersection Capacity Utilization	101.1%											
Analysis Period (min)	15											
c Critical Lane Group												

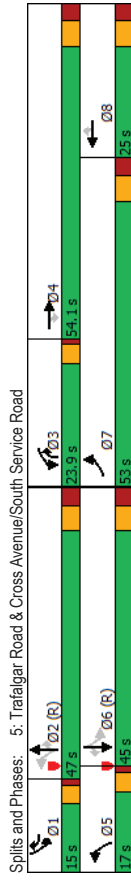
2035 Total AM Peak Hour (Lyons Closed).syn  
6: Trafalgar Road 05-04-2022

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	615	680	109	14	582	701	81	447	63	523	508	455
Traffic Volume (vph)	615	680	109	14	582	701	81	447	63	523	508	455
Future Volume (vph)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Ideal Flow (vphpl)	80.0	80.0	80.0	80.0	80.0	80.0	25.0	80.0	80.0	80.0	80.0	80.0
Storage Length (m)	2	0	0	1	1	1	1	1	1	1	1	1
Storage Length (m)	7.5	7.5	7.5	7.5	7.5	7.5	7.5	7.5	7.5	7.5	7.5	7.5
Lane Util. Factor	0.97	0.95	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.97	1.00
Ped Bike Factor	0.98	0.99	0.99	0.99	0.99	0.99	0.99	0.99	0.95	0.96	0.96	0.97
Flt	0.979	0.979	0.979	0.979	0.979	0.979	0.979	0.979	0.850	0.850	0.850	0.850
Flt Protected	0.950	0.950	0.950	0.950	0.950	0.950	0.950	0.950	0.950	0.950	0.950	0.950
Satd. Flow (prot)	3433	3390	0	1703	3505	1568	1770	3574	1615	3367	1863	1524
Flt Permitted	0.950	0.950	0.950	0.950	0.950	0.950	0.950	0.950	0.950	0.950	0.950	0.950
Satd. Flow (perm)	3350	3390	0	1689	3505	1460	1752	3574	1529	3247	1863	1474
Right Turn on Red	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Satd. Flow (RTOR)	16	16	16	16	16	16	16	16	16	16	16	16
Link Speed (k/h)	50	50	50	50	50	50	50	50	50	50	50	50
Link Distance (m)	94.0	94.0	94.0	94.0	94.0	94.0	94.0	94.0	94.0	94.0	94.0	94.0
Travel Time (s)	6.8	6.8	6.8	6.8	6.8	6.8	6.8	6.8	6.8	6.8	6.8	6.8
Conf. Peds. (#/hr)	25	7	7	7	25	9	9	18	18	18	18	9
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	2%	4%	1%	6%	3%	3%	2%	1%	0%	4%	2%	6%
Adj. Flow (vph)	668	739	118	15	633	762	88	486	68	571	552	495
Shared Lane Traffic (%)	0.00											
Lane Group Flow (vph)	668	867	0	15	633	762	88	486	68	571	552	495
Enter Blocked Intersec	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Left	Right	Left	Left	Left	Right	Left	Left	Right
Median Width(m)	7.2	7.2	7.2	7.2	7.2	7.2	7.2	7.2	7.2	7.2	7.2	7.2
Link Offset(m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Crosswalk Width(m)	4.8	4.8	4.8	4.8	4.8	4.8	4.8	4.8	4.8	4.8	4.8	4.8
Two way Left Turn Lane												
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (k/h)	25	15	25	15	25	15	25	15	25	15	25	15
Number of Detectors	1	2	1	2	1	1	2	1	2	1	2	1
Detector Template	Left	Thru	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Right
Leading Detector (m)	2.0	10.0	2.0	10.0	2.0	2.0	10.0	2.0	2.0	10.0	2.0	2.0
Trailing Detector (m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Position(m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Size(m)	2.0	0.6	2.0	0.6	2.0	0.6	2.0	0.6	2.0	0.6	2.0	0.6
Detector 1 Type	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 2 Position(m)	9.4	9.4	9.4	9.4	9.4	9.4	9.4	9.4	9.4	9.4	9.4	9.4
Detector 2 Size(m)	0.6	0.6	0.6	0.6	0.6	0.6	0.6	0.6	0.6	0.6	0.6	0.6
Detector 2 Type	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex
Detector 2 Channel												
Detector 2 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0

	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Turn Type	Prot	NA	4	Prot	NA	8	Prot	NA	2	Prot	NA	6
Protected Phases	7	4		3	8		5	2		1		6
Permitted Phases												
Detector Phase	7	4		3	8		5	2		1		6
Switch Phase												
Minimum Initial (s)	7.0	10.0		7.0	10.0		7.0	20.0		7.0	20.0	20.0
Minimum Split (s)	12.0	37.0		12.0	37.0		12.0	39.0		12.0	39.0	39.0
Total Split (s)	28.0	64.0		28.0	64.0		28.0	40.0		24.0	52.0	52.0
Total Split (%)	20.0%	45.7%		8.6%	34.3%		8.6%	28.6%		17.1%	37.1%	37.1%
Maximum Green (s)	23.0	59.0		7.0	41.0		7.0	33.0		33.0	19.0	45.0
Yellow Time (s)	3.0	3.0		3.0	4.0		3.0	4.0		4.0	3.0	4.0
All-Red Time (s)	2.0	2.0		2.0	3.0		2.0	3.0		3.0	2.0	3.0
Lost Time Adjust (s)	-1.0	-3.0		-1.0	-3.0		-1.0	-3.0		-1.0	-3.0	-3.0
Total Lost Time (s)	4.0	2.0		4.0	4.0		4.0	4.0		4.0	4.0	4.0
Lead/Lag	Lead	Lag		Lead	Lag		Lead	Lag		Lead	Lag	Lag
Lead-Lag Optimize?	Yes	Yes		Yes	Yes		Yes	Yes		Yes	Yes	Yes
Vehicle Extension (s)	3.0	3.0		3.0	3.0		3.0	3.0		3.0	3.0	3.0
Recall Mode	None	None		None	None		None	C-Max		None	C-Max	C-Max
Walk Time (s)	7.0	7.0		7.0	7.0		7.0	7.0		7.0	7.0	7.0
Flash Dont Walk (s)	11.0	11.0		11.0	11.0		11.0	11.0		11.0	11.0	11.0
Pedestrian Calls (#/hr)	0	0		0	0		0	0		0	0	0
Act Effct Green (s)	24.0	69.2		8.0	44.0		8.0	36.0		33.0	20.0	48.0
Actuated g/C Ratio	0.17	0.49		0.06	0.31		0.06	0.26		0.24	0.14	0.34
v/c Ratio	1.14	0.51		0.15	0.57		0.18	0.87		0.53	0.14	1.19
Control Delay	131.7	25.6		66.6	42.7		123.8	124.2		47.2	0.6	175.9
Queue Delay	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	0.0
Total Delay	131.7	25.6		66.6	42.7		123.8	124.2		47.2	0.6	175.9
LOS	F	C		E	D		F	D		A	F	C
Approach Delay	72.1			86.8			52.8					76.5
Approach LOS	E			F			D					E
Queue Length 50th (m)	~116.4	78.8		4.3	81.3		~208.2	26.0		64.4	0.0	~105.9
Queue Length 95th (m)	#156.3	116.0		12.1	101.9		#288.6	#59.5		83.2	0.0	#145.1
Internal Link Dist (m)	70.0			292.9			147.0					242.5
Turn Bay Length (m)	80.0			80.0			25.0			60.0		80.0
Base Capacity (vph)	588	1683		97	1101		646	101		919	503	481
Starvation Cap Reductn	0	0		0	0		0	0		0	0	0
Spillback Cap Reductn	0	0		0	0		0	0		0	0	0
Storage Cap Reductn	0	0		0	0		0	0		0	0	0
Reduced v/c Ratio	1.14	0.51		0.15	0.57		1.18	0.87		0.53	0.14	1.19

Intersection Summary	
Area Type:	Other
Cycle Length:	140
Actuated Cycle Length:	140
Offset:	54.5 (39%), Referenced to phase 2:NBT and 6:SBT, Start of Green
Natural Cycle:	150
Control Type:	Actuated-Coordinated
Maximum v/c Ratio:	1.19
Intersection Signal Delay:	75.0

Intersection Summary	
Area Type:	Other
Cycle Length:	140
Actuated Cycle Length:	140
Offset:	54.5 (39%), Referenced to phase 2:NBT and 6:SBT, Start of Green
Natural Cycle:	150
Control Type:	Actuated-Coordinated
Maximum v/c Ratio:	1.19
Intersection Signal Delay:	75.0



Lanes, Volumes, Timings  
5: Trafalgar Road & Cross Avenue/South Service Road

Lanes, Volumes, Timings  
6: Trafalgar Road

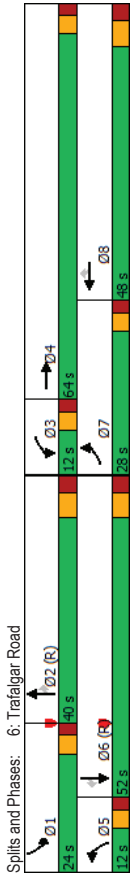
2035 Total PM Peak Hour (Lyons Closed).syn  
05-04-2022

2035 Total AM Peak Hour (Lyons Closed).syn  
05-04-2022

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Turn Type	Prot	NA	Perm	Prot	NA	pm+ov	pm+pt	NA	pm+ov	pm+pt	NA	Perm
Protected Phases	7	4	4	3	8	8	2	2	3	1	6	
Permitted Phases	7	4	4	3	8	8	2	2	3	1	6	6
Detector Phase	7	4	4	3	8	8	2	2	3	1	6	6
Switch Phase												
Minimum Initial (s)	10.0	5.0	5.0	5.0	10.0	7.0	7.0	27.0	5.0	7.0	27.0	27.0
Minimum Split (s)	17.0	25.0	25.0	9.5	25.0	11.5	11.5	34.0	9.5	11.5	34.0	34.0
Total Split (s)	53.0	54.1	54.1	23.9	25.0	15.0	17.0	47.0	23.9	15.0	45.0	45.0
Total Split (%)	37.9%	38.6%	38.6%	17.1%	17.9%	10.7%	12.1%	33.6%	17.1%	10.7%	32.1%	32.1%
Maximum Green (s)	46.0	47.1	47.1	19.9	18.0	11.0	13.0	40.0	19.9	11.0	38.0	38.0
Yellow Time (s)	4.0	4.0	4.0	3.0	4.0	3.0	4.0	3.0	4.0	3.0	4.0	4.0
All-Red Time (s)	3.0	3.0	3.0	1.0	3.0	1.0	1.0	3.0	1.0	1.0	3.0	3.0
Lost Time Adjust (s)	-3.0	-3.0	-3.0	0.0	-3.0	0.0	0.0	-3.0	0.0	0.0	-3.0	-3.0
Total Lost Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lead/Lag	Lead	Lag	Lag	Lead	Lag	Lead	Lead	Lag	Lead	Lead	Lag	Lag
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	None	None	None	None	None	None	None	C-Max	None	None	C-Max	C-Max
Walk Time (s)	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0
Flash Dont Walk (s)	11.0	11.0	11.0	11.0	11.0	11.0	11.0	11.0	11.0	11.0	11.0	11.0
Pedestrian Calls (#/hr)	0	0	0	0	0	0	0	0	0	0	0	0
Act Effct Green (s)	49.0	51.8	48.8	17.6	20.3	35.9	56.8	43.1	60.7	52.5	41.0	41.0
Actuated g/C Ratio	0.35	0.37	0.35	0.13	0.14	0.26	0.41	0.31	0.43	0.38	0.29	0.29
v/c Ratio	1.16	0.36	0.32	0.74	0.72	0.90	1.17	1.01	0.31	0.90	1.09	0.95
Control Delay	121.8	33.3	6.7	69.9	65.9	59.8	122.2	60.9	16.5	58.6	80.8	27.7
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	121.8	33.3	6.7	69.9	65.9	59.8	122.2	60.9	16.5	58.6	80.8	27.7
LOS	F	C	A	E	E	E	F	E	B	E	F	C
Approach Delay	91.0			64.6			64.0				65.4	
Approach LOS	F			E			E				E	
Queue Length 50th (m)	~244.0	49.0	1.9	46.8	53.5	98.4	~77.8	~183.5	18.5	33.8	~200.2	114.0
Queue Length 95th (m)	#287.9	65.2	20.1	62.9	71.8	#163.4	#66.2	m123.3	m14.7	#46.0	#228.5	m#155.6
Internal Link Dist (m)	219.7			214.9			242.5				247.0	
Turn Bay Length (m)	25.0	60.0	25.0	60.0	50.0	60.0	50.0	60.0	25.0	60.0	25.0	60.0
Base Capacity (vph)	1190	1235	616	487	520	497	223	1581	721	207	1504	666
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	1.16	0.36	0.32	0.66	0.70	0.90	1.17	1.01	0.30	0.90	1.09	0.95

Intersection Summary  
Area Type: Other  
Cycle Length: 140  
Actuated Cycle Length: 140  
Offset: 54.5 (39%), Referenced to phase 2:NBTL and 6:SBTL. Start of Green  
Natural Cycle: 150  
Control Type: Actuated-Coordinated  
Maximum v/c Ratio: 1.17  
Intersection Signal Delay: 71.6

Intersection Capacity Utilization 89.9%  
Analysis Period (min) 15  
~ Volume exceeds capacity, queue is theoretically infinite.  
Queue shown is maximum after two cycles.  
# 95th percentile volume exceeds capacity, queue may be longer.  
Queue shown is maximum after two cycles.



HCM Signalized Intersection Capacity Analysis 2035 Total AM Peak Hour (Lyons Closed).syn  
6: Trafalgar Road

2035 Total PM Peak Hour (Lyons Closed).syn  
5: Trafalgar Road & Cross Avenue/South Service Road

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔
Traffic Volume (vph)	615	680	109	14	582	701	81	447	63	523	508	455
Future Volume (vph)	615	680	109	14	582	701	81	447	63	523	508	455
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	2.0	2.0	4.0	4.0	4.0	4.0	4.0	7.0	4.0	4.0	4.0
Lane Util. Factor	0.97	0.95	1.00	0.95	1.00	1.00	0.95	1.00	0.97	1.00	1.00	1.00
Frb. ped/bikes	1.00	0.99	1.00	1.00	0.93	1.00	1.00	1.00	0.95	1.00	1.00	0.97
Frb. ped/bikes	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt	1.00	0.98	1.00	1.00	0.85	1.00	1.00	0.85	1.00	1.00	1.00	0.85
Flt Protected												
Satd. Flow (prot)	3433	3391	1703	3505	1460	1770	3574	1529	3367	1863	1474	1474
Flt Permitted	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	1.00	1.00
Satd. Flow (perm)	3433	3391	1703	3505	1460	1770	3574	1529	3367	1863	1474	1474
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	668	739	118	15	633	762	88	486	68	571	552	495
RTOR Reduction (vph)	0	8	0	0	0	181	0	53	0	0	237	0
Lane Group Flow (vph)	668	849	0	15	633	581	88	486	15	571	552	258
Conf. Peds. (#/hr)	25	25	7	7	6%	3%	2%	1%	0%	4%	2%	6%
Heavy Vehicles (%)	2%	4%	1%	6%	3%	3%	2%	1%	0%	4%	2%	6%
Turn Type	Prot	NA	Prot	NA	Prot	NA	Prot	NA	Prot	NA	Prot	NA
Protected Phases	7	4	3	8	5	2						6
Permitted Phases												
Actuated Green, G (s)	23.0	66.2	2.8	44.0	44.0	7.0	30.0	30.0	30.0	19.0	42.0	42.0
Effective Green, g (s)	24.0	69.2	3.8	47.0	47.0	8.0	33.0	33.0	30.0	20.0	45.0	45.0
Actuated g/C Ratio	0.17	0.49	0.03	0.34	0.34	0.06	0.24	0.21	0.14	0.32	0.32	0.32
Clearance Time (s)	5.0	5.0	5.0	7.0	7.0	5.0	7.0	7.0	5.0	7.0	7.0	7.0
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Lane Grp Cap (vph)	588	1676	46	1176	490	101	842	327	481	598	473	473
v/s Ratio Prot	c0.19	0.25	0.01	0.18	0.05	0.14			c0.17	c0.30		
v/s Ratio Perm												0.18
v/c Ratio	1.14	0.51	0.33	0.54	1.19	0.87	0.58	0.04	1.19	0.92	0.55	0.55
Uniform Delay, d1	58.0	23.9	66.8	37.7	46.5	65.5	47.3	43.6	60.0	45.8	39.1	39.1
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.63	0.45	0.67	0.67
Incremental Delay, d2	80.6	0.2	4.1	0.5	102.4	50.9	2.9	0.3	99.7	18.3	3.5	3.5
Delay (s)	138.6	24.1	71.0	38.2	148.9	116.4	50.2	43.9	197.2	38.9	29.5	29.5
Level of Service	F	C	E	D	F	F	D	D	D	F	D	C
Approach LOS	E	F	F	F	E	E	E	E	F	E	F	F
<b>Intersection Summary</b>												
HCM 2000 Control Delay	84.4									F		
HCM 2000 Volume to Capacity ratio	1.12											
Actuated Cycle Length (s)	140.0									16.0		
Intersection Capacity Utilization	89.9%									E		
Analysis Period (min)	15											
c Critical Lane Group												

Baseline  
Syncho 10 Report Page 20

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔
Traffic Volume (vph)	1268	407	184	294	334	413	240	1474	197	172	1504	584
Future Volume (vph)	1268	407	184	294	334	413	240	1474	197	172	1504	584
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (m)	25.0	60.0	25.0	60.0	25.0	60.0	50.0	60.0	25.0	60.0	25.0	60.0
Storage Length (m)	2	1	2	1	2	1	1	1	1	1	1	1
Taper Length (m)	7.5	7.5	7.5	7.5	7.5	7.5	7.5	7.5	7.5	7.5	7.5	7.5
Lane Util. Factor	0.97	0.95	1.00	0.97	0.95	1.00	1.00	0.91	1.00	1.00	0.91	1.00
Ped Bike Factor	0.95	0.98	0.95	0.98	0.95	0.98	0.83	0.83	0.95	0.95	0.95	0.95
Flt Protected												
Satd. Flow (prot)	3400	3343	1482	3433	3471	1599	1719	5136	1615	1805	5136	1568
Flt Permitted	0.950	0.950	0.950	0.950	0.950	0.950	0.100	0.100	0.100	0.100	0.100	0.100
Satd. Flow (perm)	3400	3343	1413	3368	3471	1599	181	5136	1337	201	5136	1485
Right Turn on Red	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Satd. Flow (RTOR)	190	190	190	117	117	117	50	50	145	145	50	328
Link Speed (k/h)	243.7	243.7	243.7	243.7	243.7	243.7	266.5	266.5	266.5	266.5	271.0	271.0
Link Distance (m)	17.5	17.5	17.5	17.2	17.2	17.2	19.2	19.2	19.2	19.2	19.5	19.5
Travel Time (s)												
Conf. Peds. (#/hr)	15	15	15	15	15	15	18	18	70	70	18	18
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	3%	8%	9%	2%	4%	1%	5%	1%	0%	0%	1%	3%
Adj. Flow (vph)	1378	442	200	320	363	449	261	1602	214	187	1635	635
Shared Lane Traffic (%)												
Lane Group Flow (vph)	1378	442	200	320	363	449	261	1602	214	187	1635	635
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Left	Right	Left	Left	Left	Right	Left	Left	Right
Median Width (m)	7.2	7.2	7.2	7.2	7.2	7.2	3.6	3.6	3.6	3.6	3.6	3.6
Link Offset (m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Crosswalk Width (m)	4.8	4.8	4.8	4.8	4.8	4.8	4.8	4.8	4.8	4.8	4.8	4.8
Two way Left Turn Lane												
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (k/h)	25	15	25	25	15	25	15	25	15	25	15	15
Number of Detectors	1	2	1	1	2	1	1	2	1	1	2	1
Detector Template	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Leading Detector (m)	2.0	10.0	2.0	2.0	10.0	2.0	2.0	10.0	2.0	2.0	10.0	2.0
Trailing Detector (m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Position (m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Size (m)	2.0	0.6	2.0	2.0	0.6	2.0	2.0	0.6	2.0	2.0	0.6	2.0
Detector 1 Type	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 2 Position (m)	9.4								9.4			9.4
Detector 2 Size (m)	0.6								0.6			0.6
Detector 2 Type	Ch+Ex								Ch+Ex			Ch+Ex
Detector 2 Channel												
Detector 2 Extend (s)	0.0								0.0			0.0

Baseline  
Syncho 10 Report Page 13



HCM Unsignalized Intersection Capacity Analysis 2035 Total PM Peak Hour (Lyons Closed).syn  
 4: Trafalgar Road & Argus Road/QEW EB On Ramp

05-04-2022

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	0	0	88	0	0	0	0	2210	944	0	2165	530
Future Volume (Veh/h)	0	0	88	0	0	0	0	2210	944	0	2165	530
Sign Control	Stop	Stop	Stop	0%	0%	0%	Free	Free	Free	Free	Free	0%
Grade	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	0	0	96	0	0	0	0	2402	1026	0	2353	576
Pedestrians	24											
Lane Width (m)	3.6											
Walking Speed (m/s)	1.2											
Percent Blockage	2											
Right turn flare (veh)												
Median type												
Median storage (veh)												
Upstream signal (m)												
pX, platoon unblocked	0.72	0.72	0.59	0.72	0.72	0.75	0.59	271		0.75		51
vC, conflicting volume	3466	5067	1096	3186	5355	801	2953			2402		
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCU, unblocked vol	735	2967	0	346	3368	0	1896			1714		
tC, 1 stage (s)	7.5	6.5	7.1	7.5	6.5	6.9	4.1			4.1		
tC, 2 stage (s)												
tF (s)	3.5	4.0	3.4	3.5	4.0	3.3	2.2			2.2		
p0 queue free %	100	100	85	100	100	100	100			100		
cM capacity (veh/h)	215	10	620	352	5	821	186			282		
Direction_Lane #	EB 1	NB 1	NB 2	NB 3	NB 4	SB 1	SB 2	SB 3				
Volume Total	96	801	801	801	1026	941	941	1047				
Volume Left	0	0	0	0	0	0	0	0				
Volume Right	96	0	0	0	1026	0	0	576				
GSH	620	1700	1700	1700	1700	1700	1700	1700				
Volume to Capacity	0.15	0.47	0.47	0.47	0.60	0.55	0.55	0.62				
Queue Length 95th (m)	4.4	0.0	0.0	0.0	0.0	0.0	0.0	0.0				
Control Delay (s)	11.9	0.0	0.0	0.0	0.0	0.0	0.0	0.0				
Lane LOS	B											
Approach Delay (s)	11.9	0.0				0.0						
Approach LOS	B											
Intersection Summary												
Average Delay	0.2											
Intersection Capacity Utilization	66.2%											
ICU Level of Service	C											
Analysis Period (min)	15											

Baseline

2035 Total AM Peak Hour (Lyons Closed).syn  
 7: GO Bus Terminal/Argus Rd & Cross Ave

05-04-2022

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	33	404	15	210	891	72	94	0	237	183	21	661
Future Volume (vph)	33	404	15	210	891	72	94	0	237	183	21	661
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (m)	3.3	3.6	3.6	3.3	3.6	3.6	3.3	3.6	3.6	3.3	3.6	3.6
Storage Length (m)	20.0	60.0	20.0	60.0	20.0	60.0	0.0	0.0	0.0	15.0	0.0	0.0
Storage Lanes	1	1	1	1	1	1	1	1	1	1	1	0
Taper Length (m)	7.5		7.5		7.5		7.5		7.5		7.5	
Lane Util. Factor	1.00	0.95	1.00	1.00	0.95	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor	1.00	0.97	1.00	0.98	0.98	1.00	0.98	0.98	0.98	0.98	0.98	0.99
Fit	0.850	0.850	0.850	0.850	0.850	0.850	0.850	0.850	0.850	0.850	0.857	0.857
Flt Protected	0.950		0.950		0.950		0.950		0.950		0.950	
Satd. Flow (prot)	1570	3217	727	818	3217	1384	805	1710	761	1570	1387	0
Flt Permitted	0.267		0.354		0.150		0.150		0.757		0.757	
Satd. Flow (perm)	441	3217	708	304	3217	1354	127	1710	734	1222	1387	0
Right Turn on Red	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Satd. Flow (RTOR)	139		139		91		91		342		98	
Link Speed (k/h)	50		50		50		50		50		50	
Link Distance (m)	374.0		243.7		81.9		81.9		180.7		180.7	
Travel Time (s)	26.9		17.5		5.9		5.9		13.0		13.0	
Conf. Peds. (#/hr)	1		3		3		3		20		20	
Peak Hour Factor	0.52	0.87	0.65	0.84	0.88	0.79	0.53	0.25	0.70	0.78	0.62	0.89
Heavy Vehicles (%)	0%	1%	100%	92%	1%	5%	95%	0%	91%	0%	93%	0%
Adj. Flow (vph)	63	464	23	250	1013	91	177	0	339	235	34	743
Shared Lane Traffic (%)												
Lane Group Flow (vph)	63	464	23	250	1013	91	177	0	339	235	777	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(m)	7.2		7.2		3.3		3.3		3.3		3.3	
Link Offset(m)	0.0		0.0		0.0		0.0		0.0		0.0	
Crosswalk Width(m)	4.8		4.8		4.8		4.8		4.8		4.8	
Two way Left Turn Lane												
Headway Factor	1.19	1.14	1.14	1.19	1.14	1.14	1.19	1.14	1.14	1.19	1.14	1.14
Turning Speed (k/h)	24	14	24	14	24	14	24	14	24	14	24	14
Number of Detectors	1	2	1	1	2	1	1	2	1	1	2	1
Detector Template	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Leading Detector (m)	2.0	10.0	2.0	2.0	10.0	2.0	2.0	10.0	2.0	2.0	10.0	2.0
Trailing Detector (m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Position(m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Size(m)	2.0	0.6	2.0	2.0	0.6	2.0	2.0	0.6	2.0	2.0	0.6	2.0
Detector 1 Type	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 2 Position(m)	9.4		9.4		9.4		9.4		9.4		9.4	
Detector 2 Size(m)	0.6		0.6		0.6		0.6		0.6		0.6	
Detector 2 Type	Ch+Ex		Ch+Ex		Ch+Ex		Ch+Ex		Ch+Ex		Ch+Ex	
Detector 2 Channel												

Baseline

Lanes, Volumes, Timings  
7: GO Bus Terminal/Argus Rd & Cross Ave

Lanes, Volumes, Timings  
4: Trafalgar Road & Argus Road/QEW EB On Ramp

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Detector 2 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Turn Type	Perm	NA	Perm	pm-pt	NA	Perm	pm-pt	3	8	Perm	Perm	NA
Protected Phases				1	6	6	8	8	8	4	4	
Permitted Phases	2	2	2	6	6	6	8	8	8	4	4	4
Detector Phase	2	2	2	1	6	6	3	8	8	4	4	4
Switch Phase												
Minimum Initial (s)	22.0	22.0	8.0	22.0	22.0	22.0	5.0	10.0	10.0	10.0	10.0	10.0
Minimum Split (s)	45.0	45.0	12.5	29.0	29.0	29.0	9.5	29.0	29.0	29.0	29.0	29.0
Total Split (s)	42.0	42.0	11.5	53.5	53.5	53.5	8.5	36.5	36.5	28.0	28.0	28.0
Total Split (%)	46.7%	46.7%	12.8%	59.4%	59.4%	59.4%	9.4%	40.6%	40.6%	31.1%	31.1%	31.1%
Maximum Green (s)	36.0	36.0	36.0	7.5	47.5	47.5	4.0	30.5	30.5	22.0	22.0	22.0
Yellow Time (s)	4.0	4.0	4.0	3.0	4.0	4.0	3.5	4.0	4.0	4.0	4.0	4.0
All-Red Time (s)	2.0	2.0	2.0	1.0	2.0	2.0	1.0	2.0	2.0	2.0	2.0	2.0
Lost Time Adjust (s)	-2.0	-2.0	-2.0	0.0	-2.0	-2.0	-2.0	0.0	0.0	-2.0	-2.0	-2.0
Total Lost Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	2.5	4.0	6.0	4.0	4.0	4.0
Lead/Lag	Lag	Lag	Lag	Lead	Lead	Lead	Lead	Lag	Lag	Lag	Lag	Lag
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Vehicle Extension (s)	5.0	5.0	5.0	2.5	5.0	5.0	3.0	4.0	4.0	4.0	4.0	4.0
Recall Mode	Min	Min	Min	Min	Min	Min	None	Min	Min	Min	Min	Min
Walk Time (s)	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0
Flash Dont Walk (s)	16.0	16.0	16.0	16.0	16.0	16.0	16.0	16.0	16.0	16.0	16.0	16.0
Pedestrian Calls (#/hr)	0	0	0	0	0	0	0	0	0	0	0	0
Act Effct Green (s)	28.3	28.3	28.3	39.8	39.8	39.8	34.1	30.6	24.1	24.1	24.1	24.1
Actuated g/C Ratio	0.35	0.35	0.35	0.49	0.49	0.49	0.42	0.38	0.30	0.30	0.30	0.30
v/c Ratio	0.41	0.41	0.07	1.26	0.64	0.13	1.70	0.69	0.64	1.61	1.61	1.61
Control Delay	28.3	20.7	0.4	173.4	16.9	2.8	371.9	11.3	35.8	305.5	305.5	305.5
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	28.3	20.7	0.4	173.4	16.9	2.8	371.9	11.3	35.8	305.5	305.5	305.5
LOS	C	C	A	F	B	A	F	B	D	D	F	F
Approach Delay	20.7	20.7	20.7	44.9	44.9	44.9	135.0	242.9	242.9	242.9	242.9	242.9
Approach LOS	C	C	C	D	D	D	F	F	F	F	F	F
Queue Length 50th (m)	7.5	29.1	0.0	-39.3	59.7	0.0	-33.0	0.0	30.8	-163.9	-163.9	-163.9
Queue Length 95th (m)	9.4	39.5	0.0	#92.9	75.6	4.7	#38.8	3.7	54.7	#156.3	#156.3	#156.3
Internal Link Dist (m)	350.0	350.0	350.0	219.7	219.7	219.7	57.9	57.9	57.9	57.9	57.9	57.9
Turn Bay Length (m)	20.0	60.0	20.0	60.0	60.0	60.0	15.0	15.0	15.0	15.0	15.0	15.0
Base Capacity (vph)	208	1524	408	198	1986	870	104	491	365	483	483	483
Stallion Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.30	0.30	0.06	1.26	0.51	0.10	1.70	0.69	0.64	1.61	1.61	1.61
Intersection Summary												
Area Type:	CBD											
Cycled Length:	90											
Actuated Cycle Length:	80.5											
Natural Cycle:	150											
Control Type:	Semi Act-Uncoord											
Maximum v/c Ratio:	1.70											
Intersection Signal Delay:	112.9											
Intersection LOS: F												

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	0	0	88	0	0	0	0	2210	944	0	2165	530
Future Volume (vph)	0	0	88	0	0	0	0	2210	944	0	2165	530
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	70.0	0.0	0.0	0.0
Storage Lanes	0	0	1	0	0	0	0	0	1	0	0	0
Taper Length (m)	7.5	7.5	7.5	7.5	7.5	7.5	7.5	7.5	7.5	7.5	7.5	7.5
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.91	1.00	1.00	0.91	0.91
Ped Bike Factor												
Fit	0.865	0.865	0.865	0.865	0.865	0.865	0.865	0.865	0.865	0.865	0.865	0.865
Fit Protected												
Satd. Flow (prot)	0	0	1522	0	0	0	0	5085	1568	0	4957	0
Fit Permitted												
Satd. Flow (perm)	0	0	1522	0	0	0	0	5085	1568	0	4957	0
Link Speed (k/h)	50	50	50	50	50	50	50	50	50	50	50	50
Link Distance (m)	165.2	165.2	165.2	196.1	196.1	196.1	271.0	271.0	19.5	19.5	50.7	50.7
Travel Time (s)	11.9	11.9	11.9	14.1	14.1	14.1	19.5	19.5	3.7	3.7	3.7	3.7
Confl. Peds. (#/hr)												24
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	0%	0%	8%	0%	2%	0%	0%	2%	3%	0%	2%	0%
Adj. Flow (vph)	0	0	96	0	0	0	0	2402	1026	0	2353	576
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	0	96	0	0	0	0	2402	1026	0	2929	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Right	Left	Left	Right	Left	Right	Left	Right
Median Width (m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Link Offset (m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Crosswalk Width (m)	4.8	4.8	4.8	4.8	4.8	4.8	4.8	4.8	4.8	4.8	4.8	4.8
Two way Left Turn Lane												
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (k/h)	25	25	25	25	25	25	25	25	25	25	25	25
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
Intersection Summary												
Area Type:	Other											
Control Type:	Unsignalized											
Intersection Capacity Utilization	66.2%											
Analysis Period (min)	15											

HCM Signalized Intersection Capacity Analysis 2035 Total PM Peak Hour (Lyons Closed).syn  
 3: Trafalgar Road & QEW EB Off Ramp

05-04-2022



Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	↔	↔	↔	↔	↔	↔
Traffic Volume (vph)	1210	623	0	2210	2072	0
Future Volume (vph)	1210	623	0	2210	2072	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0
Lane Util. Factor	0.97	1.00	0.91	0.91	0.91	0.91
Frb, ped/bikes	1.00	0.98	1.00	1.00	1.00	1.00
Frb, ped/bikes	1.00	1.00	1.00	1.00	1.00	1.00
Frt	1.00	0.85	1.00	1.00	1.00	1.00
Flt Protected	0.95	1.00	1.00	1.00	1.00	1.00
Satd. Flow (prot)	3502	1573	5085	5085	5085	5085
Flt Permitted	0.95	1.00	1.00	1.00	1.00	1.00
Satd. Flow (perm)	3502	1573	5085	5085	5085	5085
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	1315	677	0	2402	2252	0
RTOR Reduction (vph)	0	1	0	0	0	0
Lane Group Flow (vph)	1315	676	0	2402	2252	0
Conf. Peds. (#/hr)	2	2	2	2	2	2
Heavy Vehicles (%)	0%	1%	0%	2%	2%	0%
Turn Type	Perm	Perm	NA	NA	NA	NA
Protected Phases	4	4	2	2	6	6
Permitted Phases	4	4	2	2	6	6
Actuated Green, G (s)	59.7	59.7	66.3	66.3	66.3	66.3
Effective Green, g (s)	62.7	62.7	69.3	69.3	69.3	69.3
Actuated g/C Ratio	0.45	0.45	0.49	0.49	0.49	0.49
Clearance Time (s)	7.0	7.0	7.0	7.0	7.0	7.0
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0
Lane Grp. Cap (vph)	1568	704	2517	2517	2517	2517
v/s Ratio Prot			c0.47	0.44		
v/s Ratio Perm	0.38	c0.43				
v/c Ratio	0.84	0.96	0.95	0.89	0.89	0.89
Uniform Delay, d1	34.2	37.4	33.8	32.0	32.0	32.0
Progression Factor	1.00	1.00	1.36	0.87	0.87	0.87
Incremental Delay, d2	4.1	24.4	1.3	2.9	2.9	2.9
Delay (s)	38.3	61.8	47.4	30.6	30.6	30.6
Level of Service	D	E	D	C	C	C
Approach Delay (s)	46.3		47.4	30.6	30.6	
Approach LOS	D		D	C	C	
Intersection Summary						
HCM 2000 Control Delay	41.4	HCM 2000 Level of Service				
HCM 2000 Volume to Capacity ratio	0.98	D				
Actuated Cycle Length (s)	140.0	Sum of lost time (s)				
Intersection Capacity Utilization	85.5%	ICU Level of Service				
Analysis Period (min)	15	E				

c Critical Lane Group

Baseline

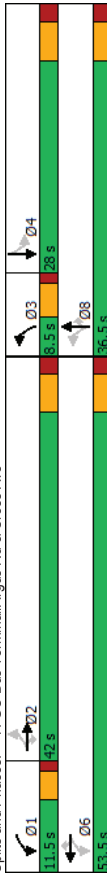
Lanes, Volumes, Timings 2035 Total AM Peak Hour (Lyons Closed).syn  
 7: GO Bus Terminal/Argus Rd & Cross Ave

05-04-2022



Intersection Capacity Utilization	111.8%	ICU Level of Service H
Analysis Period (min)	15	
~ Volume exceeds capacity, queue is theoretically infinite.		
Queue shown is maximum after two cycles.		
# 95th percentile volume exceeds capacity, queue may be longer.		
Queue shown is maximum after two cycles.		

Splits and Phases: 7: GO Bus Terminal/Argus Rd & Cross Ave



Baseline

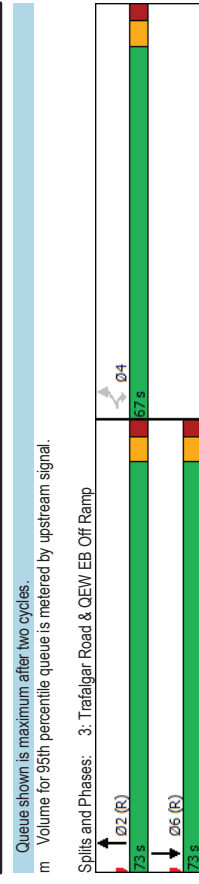
HCM Signalized Intersection Capacity Analysis 2035 Total AM Peak Hour (Lyons Closed).syn  
 7: GO Bus Terminal/Argus Rd & Cross Ave 05-04-2022

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	33	404	15	210	891	72	94	0	237	183	21	661
Traffic Volume (vph)	33	404	15	210	891	72	94	0	237	183	21	661
Future Volume (vph)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Ideal Flow (vphpl)	3.3	3.6	3.6	3.3	3.6	3.6	3.3	3.6	3.6	3.3	3.6	3.6
Lane Width	4.0	4.0	4.0	4.0	4.0	4.0	2.5	6.0	4.0	4.0	4.0	4.0
Total Lost time (s)	1.00	0.95	1.00	1.00	0.95	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	0.98	1.00	0.97	1.00	0.99	1.00
Frbp, ped/bikes	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.98	1.00	1.00
Fipb, ped/bikes	1.00	1.00	1.00	1.00	1.00	1.00	0.85	1.00	0.85	1.00	0.86	1.00
Flt Protected	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	0.95	1.00	0.95	1.00
Satd. Flow (prot)	1570	3217	708	817	3217	1354	805	736	1537	1387	1387	1387
Flt Permitted	0.27	1.00	1.00	0.35	1.00	1.00	0.15	1.00	0.76	1.00	1.00	1.00
Satd. Flow (perm)	441	3217	708	305	3217	1354	127	736	1225	1387	1387	1387
Peak-hour factor, PHF	0.52	0.87	0.65	0.84	0.88	0.79	0.53	0.25	0.70	0.78	0.62	0.89
Adj. Flow (vph)	63	464	23	250	1012	91	177	0	339	235	34	743
RTOR Reduction (vph)	0	0	15	0	0	46	0	0	210	0	69	0
Lane Group Flow (vph)	63	464	8	250	1013	45	177	0	129	235	708	0
Conf. Peds. (#/hr)	1	3	3	3	3	1	3	20	20	20	20	3
Heavy Vehicles (%)	0%	1%	100%	92%	1%	5%	95%	0%	91%	0%	93%	0%

Turn Type	Perm	NA	Perm	pm-pt	NA	Perm	pm-pt	Perm	pm-pt	Perm	pm-pt	NA
Protected Phases	2	2	6	6	3	8	8	8	8	8	8	4
Permitted Phases	2	2	6	6	8	8	8	8	8	8	8	4
Actuated Green, G (s)	26.3	26.3	26.3	37.8	37.8	30.6	30.6	30.6	22.1	22.1	22.1	22.1
Effective Green, g (s)	28.3	28.3	28.3	37.8	39.8	32.6	32.6	30.6	24.1	24.1	24.1	24.1
Actuated g/C Ratio	0.35	0.35	0.35	0.47	0.50	0.41	0.41	0.38	0.30	0.30	0.30	0.30
Clearance Time (s)	6.0	6.0	6.0	4.0	6.0	4.5	6.0	6.0	6.0	6.0	6.0	6.0
Vehicle Extension (s)	5.0	5.0	5.0	2.5	5.0	3.0	3.0	4.0	4.0	4.0	4.0	4.0
Lane Grp Cap (vph)	155	1132	249	191	1592	670	102	280	367	415	415	415
v/s Ratio Prot	0.14	0.14	0.01	0.12	0.31	0.13	0.13	0.18	0.19	0.19	0.19	0.51
v/s Ratio Perm	0.41	0.41	0.03	0.31	0.64	0.07	0.174	0.46	0.64	1.71	1.71	1.71
Uniform Delay, d1	19.7	19.7	17.1	20.0	15.0	10.6	21.9	18.7	24.4	28.2	28.2	28.2
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	3.6	0.5	0.1	171.4	1.2	0.1	368.3	1.6	4.2	328.3	328.3	328.3
Delay (s)	23.3	20.2	17.2	191.3	16.1	10.7	390.2	20.3	28.6	356.4	356.4	356.4
Level of Service	C	C	B	F	B	B	F	C	C	C	F	F
Approach Delay (s)	20.5	20.5	20.5	48.1	147.2	280.3	280.3	280.3	280.3	280.3	280.3	280.3
Approach LOS	C	C	C	D	F	F	F	F	F	F	F	F

Intersection Summary	Value	Level of Service
HCM 2000 Control Delay	127.0	HCM 2000 Level of Service
HCM 2000 Volume to Capacity ratio	1.59	F
Actuated Cycle Length (s)	80.4	Sum of lost time (s)
Intersection Capacity Utilization	111.8%	ICU Level of Service
Analysis Period (min)	15	H
c Critical Lane Group		

2035 Total PM Peak Hour (Lyons Closed).syn  
 3: Trafalgar Road & QEW EB Off Ramp 05-04-2022



Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Detector Phase	4	4		2	2	6
Switch Phase						
Minimum Initial (s)	10.0	10.0		29.0	29.0	29.0
Minimum Split (s)	38.0	38.0		36.0	36.0	36.0
Total Split (s)	67.0	67.0		73.0	73.0	73.0
Total Split (%)	47.9%	47.9%		52.1%	52.1%	52.1%
Maximum Green (s)	60.0	60.0		66.0	66.0	66.0
Yellow Time (s)	4.0	4.0		4.0	4.0	4.0
All-Red Time (s)	3.0	3.0		3.0	3.0	3.0
Lost Time Adjust (s)	-3.0	-3.0		-3.0	-3.0	-3.0
Total Lost Time (s)	4.0	4.0		4.0	4.0	4.0
Lead/Lag						
Lead-Lag Optimize?						
Vehicle Extension (s)	3.0	3.0		3.0	3.0	3.0
Recall Mode	None	None		C-Max	C-Max	C-Max
Walk Time (s)	7.0	7.0		7.0	7.0	7.0
Flash Dont Walk (s)	11.0	11.0		11.0	11.0	11.0
Pedestrian Calls (#/hr)	0	0		0	0	0
Act Effct Green (s)	62.7	62.7		69.3	69.3	69.3
Actuated g/C Ratio	0.45	0.45		0.50	0.50	0.50
v/c Ratio	0.84	0.96		0.95	0.89	0.89
Control Delay	40.1	63.1		47.2	31.0	31.0
Queue Delay	0.0	0.0		8.2	0.0	0.0
Total Delay	40.1	63.1		55.4	31.0	31.0
LOS	D	E		E	C	C
Approach Delay	47.9			55.4	31.0	
Approach LOS	D			E	C	
Queue Length 50th (m)	171.1	187.0		266.1	185.4	
Queue Length 95th (m)	202.5	#272.9		m247.2	228.1	
Internal Link Dist (m)	47.3			26.7	162.0	
Turn Bay Length (m)						
Base Capacity (vph)	1575	708		2517	2517	
Stevartion Cap Reductn	0	0		0	0	
Spillback Cap Reductn	0	0		133	0	
Storage Cap Reductn	0	0		0	0	
Reduced v/c Ratio	0.83	0.96		1.01	0.89	
Intersection Summary						
Area Type:	Other					
Cycle Length:	140					
Offset:	0 (0%), Referenced to phase 2:NBT and 6:SBT, Start of Green					
Natural Cycle:	90					
Control Type:	Actuated-Coordinated					
Maximum v/c Ratio:	0.96					
Intersection Signal Delay:	44.9					
Intersection Capacity Utilization:	85.5%					
Analysis Period (min):	15					
# 95th percentile volume exceeds capacity, queue may be longer.						

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔
Traffic Volume (vph)	63	445	133	247	277	24	54	1	107	93	28	72
Future Volume (vph)	63	445	133	247	277	24	54	1	107	93	28	72
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (m)	60.0	60.0	60.0	60.0	60.0	60.0	60.0	60.0	60.0	60.0	60.0	60.0
Storage Lanes	1	1	1	1	1	1	1	1	1	1	1	1
Taper Length (m)	7.5	7.5	7.5	7.5	7.5	7.5	7.5	7.5	7.5	7.5	7.5	7.5
Lane Util. Factor	1.00	0.95	1.00	1.00	0.95	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor	0.99			0.97		0.97			0.99			
Flt	0.950		0.850	0.950		0.850	0.950		0.850	0.950		0.850
Satd. Flow (prot)	1624	3094	1425	1593	2801	1454	1593	1676	1425	1624	1676	1163
Flt Permitted	0.561		0.159		0.738		0.738		0.561		0.159	0.738
Satd. Flow (perm)	953	3094	1425	267	2801	1410	1237	1676	1425	1281	1676	1163
Right Turn on Red	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Satd. Flow (RTOR)	145		145		48		48		116		116	144
Link Speed (k/h)		50		50		50		50		50		50
Link Distance (m)		281.3		40.1		149.2		10.7		41.7		41.7
Travel Time (s)		20.3		2.9		10.7		0.7		3.0		3.0
Conf. Peds. (#/hr)	4		4		4		4		7		7	7
Peak Hour Factor	0.69	0.68	0.92	0.92	0.88	0.50	0.92	0.92	0.92	0.62	0.92	0.50
Heavy Vehicles (%)	0%	5%	2%	2%	16%	0%	2%	2%	2%	0%	2%	25%
Adj. Flow (vph)	91	767	145	268	315	48	59	1	116	150	30	144
Shared Lane Traffic (%)												
Lane Group Flow (vph)	91	767	145	268	315	48	59	1	116	150	30	144
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Right	Left	Left	Left	Right	Left	Left	Right
Median Width (m)	3.6		3.6		3.6		3.6		3.6		3.6	3.6
Link Offset (m)	0.0		0.0		0.0		0.0		0.0		0.0	0.0
Crosswalk Width (m)	4.8		4.8		4.8		4.8		4.8		4.8	4.8
Two way Left Turn Lane												
Headway Factor	1.14	1.14	1.14	1.14	1.14	1.14	1.14	1.14	1.14	1.14	1.14	1.14
Turning Speed (k/h)	25	15	25	15	25	15	25	15	25	15	25	15
Number of Detectors	1	2	1	1	2	1	1	2	1	1	2	1
Detector Template	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Leading Detector (m)	2.0	10.0	2.0	2.0	10.0	2.0	2.0	10.0	2.0	2.0	10.0	2.0
Trailing Detector (m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Position (m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Size (m)	2.0	0.6	2.0	2.0	0.6	2.0	2.0	0.6	2.0	2.0	0.6	2.0
Detector 1 Type	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 2 Position (m)	9.4		9.4		9.4		9.4		9.4		9.4	9.4
Detector 2 Size (m)	0.6		0.6		0.6		0.6		0.6		0.6	0.6
Detector 2 Type	Ch+Ex		Ch+Ex		Ch+Ex		Ch+Ex		Ch+Ex		Ch+Ex	Ch+Ex
Detector 2 Channel												
Detector 2 Extend (s)	0.0		0.0		0.0		0.0		0.0		0.0	0.0

Lanes, Volumes, Timings  
8: Cross Ave & Lyons Lane

2035 Total AM Peak Hour (Lyons Closed).syn  
05-04-2022

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Turn Type	Perm	NA	Perm	pm+pt	NA	Perm	Perm	NA	Perm	Perm	NA	Perm
Protected Phases	4	4	4	8	8	8	2	2	2	6	6	6
Detector Phase	4	4	4	3	8	8	2	2	2	6	6	6
Switch Phase												
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Minimum Split (s)	22.5	22.5	22.5	9.5	22.5	22.5	22.5	22.5	22.5	22.5	22.5	22.5
Total Split (s)	32.0	32.0	32.0	26.0	58.0	58.0	32.0	32.0	32.0	32.0	32.0	32.0
Total Split (%)	35.6%	35.6%	35.6%	28.9%	64.4%	64.4%	35.6%	35.6%	35.6%	35.6%	35.6%	35.6%
Maximum Green (s)	27.5	27.5	27.5	21.5	53.5	53.5	27.5	27.5	27.5	27.5	27.5	27.5
Yellow Time (s)	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5
LeadLag	Lag	Lag	Lag	Lag	Lead	Lead						
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes						
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	None	None	None	None	None	None	Max	Max	Max	Max	Max	Max
Walk Time (s)	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0
Flash Dont Walk (s)	11.0	11.0	11.0	11.0	11.0	11.0	11.0	11.0	11.0	11.0	11.0	11.0
Pedestrian Calls (#/hr)	0	0	0	0	0	0	0	0	0	0	0	0
Act Effct Green (s)	24.0	24.0	24.0	42.4	42.4	42.4	27.8	27.8	27.8	27.8	27.8	27.8
Actuated g/C Ratio	0.30	0.30	0.30	0.53	0.53	0.53	0.35	0.35	0.35	0.35	0.35	0.35
v/c Ratio	0.32	0.82	0.27	0.72	0.21	0.06	0.14	0.00	0.20	0.33	0.05	0.29
Control Delay	25.8	34.5	5.7	24.7	9.6	2.7	21.7	21.0	5.7	24.1	20.6	5.9
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	25.8	34.5	5.7	24.7	9.6	2.7	21.7	21.0	5.7	24.1	20.6	5.9
LOS	C	C	A	C	A	A	C	C	A	C	C	A
Approach Delay		29.6			15.5		11.1				15.7	
Approach LOS		C			B		B				B	
Queue Length 50th (m)	10.8	57.2	0.0	22.0	12.6	0.0	6.5	0.1	0.0	17.8	3.2	0.0
Queue Length 95th (m)	19.1	50.4	13.2	47.5	18.6	0.8	17.5	1.3	11.9	25.0	10.3	0.0
Internal Link Dist (m)		257.3			16.1		125.2				17.7	
Turn Bay Length (m)	60.0	60.0	60.0	60.0	60.0	60.0	60.0	60.0	60.0	60.0	60.0	60.0
Base Capacity (vph)	334	1085	594	506	1911	977	433	567	575	449	587	501
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.27	0.71	0.24	0.53	0.16	0.05	0.14	0.00	0.20	0.33	0.05	0.29
Intersection Summary												
Area Type:	CBD											
Cycle Length:	90											
Actuated Cycle Length:	79.3											
Natural Cycle:	60											
Control Type:	Semi Act-Uncooord											
Maximum v/c Ratio:	0.82											
Intersection Signal Delay:	21.8											
Intersection Capacity Utilization:	52.5%											
ICU Level of Service:	A											

Lanes, Volumes, Timings  
3: Trafalgar Road & QEW EB Off Ramp

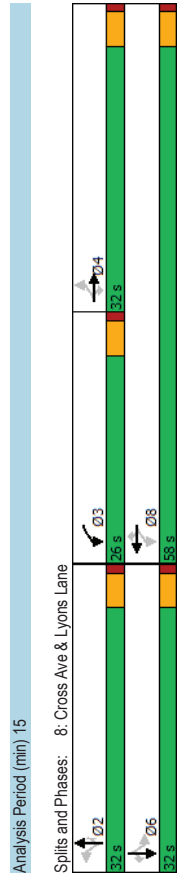
2035 Total PM Peak Hour (Lyons Closed).syn  
05-04-2022

Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	↑↑	↑	↑↑↑	↑↑↑	↑↑↑	↑↑
Traffic Volume (vph)	1210	623	0	2210	2072	0
Future Volume (vph)	1210	623	0	2210	2072	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	0.97	1.00	1.00	0.91	0.91	1.00
Ped Bike Factor						
Fit		0.98				
Fit Protected		0.850				
Satd. Flow (prot)	3502	1599	0	5085	5085	0
Fit Permitted	0.950					
Satd. Flow (perm)	3502	1573	0	5085	5085	0
Right Turn on Red		Yes			Yes	
Satd. Flow (RTOR)		2				
Link Speed (k/h)	50		50		50	
Link Distance (m)	71.3		50.7		186.0	
Travel Time (s)	5.1		3.7		13.4	
Contf. Peds. (#/hr)		2				
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	0%	1%	0%	2%	2%	0%
Adj. Flow (vph)	1315	677	0	2402	2252	0
Shared Lane Traffic (%)						
Lane Group Flow (vph)	1315	677	0	2402	2252	0
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Left	Right	Right
Median Width (m)	7.2		0.0	0.0	0.0	
Link Offset (m)	0.0		0.0	0.0	0.0	
Crosswalk Width (m)	4.8		4.8		4.8	
Two way Left Turn Lane						
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (k/h)	25	15	25		15	
Number of Detectors	1	1	2	2	2	
Detector Template	Left	Right	Thru	Thru	Thru	
Leading Detector (m)	2.0	2.0	10.0	10.0	10.0	
Trailing Detector (m)	0.0	0.0	0.0	0.0	0.0	
Detector 1 Position (m)	0.0	0.0	0.0	0.0	0.0	
Detector 1 Size (m)	2.0	2.0	0.6	0.6	0.6	
Detector 1 Type	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	
Detector 1 Channel						
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	
Detector 2 Position (m)			9.4	9.4	9.4	
Detector 2 Size (m)			0.6	0.6	0.6	
Detector 2 Type			Ch+Ex	Ch+Ex	Ch+Ex	
Detector 2 Channel						
Detector 2 Extend (s)			0.0	0.0	0.0	
Turn Type	Perm	Perm	NA	NA	NA	
Protected Phases			2	2	6	
Permitted Phases	4	4				

HCM Unsignalized Intersection Capacity Analysis: 2035 Total PM Peak Hour (Lyons Closed).syn  
 05-04-2022

Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations				↑↑↑	↑↑↑	↑
Traffic Volume (veh/h)	0	0	0	3420	2072	959
Future Volume (Veh/h)	0	0	0	3420	2072	959
Sign Control	Stop	Free	Free	Free	Free	0%
Grade	0%			0%	0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	0	0	0	3717	2252	1042
Pedestrians						
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)				None	None	
Median type						
Median storage (veh)						
Upstream signal (m)				186	145	
pX, platoon unblocked	0.25					
vC, conflicting volume	3491	751	2252			
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	401	751	2252			
tC, single (s)	6.8	6.9	4.1			
tC, 2 stage (s)						
tF (s)	3.5	3.3	2.2			
p0 queue free %	100	100	100			
cM capacity (veh/h)	142	363	225			
Direction_Lane #	NB1	NB2	NB3	SB1	SB2	SB3
Volume Total	1239	1239	1239	751	751	751
Volume Left	0	0	0	0	0	0
Volume Right	0	0	0	0	0	1042
GSH	1700	1700	1700	1700	1700	1700
Volume to Capacity	0.73	0.73	0.73	0.44	0.44	0.61
Queue Length 95th (m)	0.0	0.0	0.0	0.0	0.0	0.0
Control Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0
Lane LOS						
Approach Delay (s)	0.0			0.0		
Approach LOS						
Intersection Summary						
Average Delay				0.0		
Intersection Capacity Utilization				85.5%		ICU Level of Service
Analysis Period (min)				15		E

2035 Total AM Peak Hour (Lyons Closed).syn  
 05-04-2022



HCM Signalized Intersection Capacity Analysis 2035 Total AM Peak Hour (Lyons Closed).syn  
 8: Cross Ave & Lyons Lane 05-04-2022

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	←	←	←	←	←	←	←	←	←	←	←	←
Traffic Volume (vph)	63	445	133	247	277	24	54	1	107	93	28	72
Future Volume (vph)	63	445	133	247	277	24	54	1	107	93	28	72
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5
Lane Util. Factor	1.00	1.00	1.00	0.95	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frb, ped/bikes	1.00	1.00	1.00	1.00	1.00	0.97	1.00	1.00	1.00	1.00	1.00	1.00
Frt	1.00	1.00	0.85	1.00	1.00	1.00	1.00	1.00	0.85	1.00	1.00	0.85
Flt Protected	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00
Satd. Flow (prot)	1615	3094	1425	1593	2801	1412	1593	1676	1425	1610	1676	1163
Flt Permitted	0.56	1.00	1.00	0.16	1.00	1.00	0.74	1.00	1.00	0.76	1.00	1.00
Satd. Flow (perm)	953	3094	1425	266	2801	1412	1237	1676	1425	1283	1676	1163
Peak-hour factor, PHF	0.69	0.68	0.92	0.92	0.88	0.50	0.92	0.92	0.92	0.62	0.92	0.50
Adj. Flow (vph)	91	767	145	268	315	48	59	1	116	150	30	144
RTOR Reduction (vph)	0	0	101	0	0	22	0	0	75	0	0	93
Lane Group Flow (vph)	91	767	44	268	315	26	59	1	41	150	30	51
Conf. Peds. (#/hr)	4					4				7		
Heavy Vehicles (%)	0%	5%	2%	16%	0%	2%	2%	2%	2%	0%	2%	25%
Turn Type	Perm	NA	Perm	pm+pt	NA	Perm	Perm	NA	Perm	Perm	NA	Perm
Protected Phases	4		4	3	8		2		2		0	6
Permitted Phases	4	24.0	24.0	42.4	42.4	42.4	27.8	27.8	27.8	27.8	27.8	27.8
Actuated Green, G (s)	24.0	24.0	24.0	42.4	42.4	42.4	27.8	27.8	27.8	27.8	27.8	27.8
Effective Green, g (s)	24.0	24.0	24.0	42.4	42.4	42.4	27.8	27.8	27.8	27.8	27.8	27.8
Actuated g/C Ratio	0.30	0.30	0.30	0.54	0.54	0.54	0.35	0.35	0.35	0.35	0.35	0.35
Clearance Time (s)	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Lane Grp. Cap (vph)	288	937	431	375	1499	755	434	588	500	450	588	408
v/s Ratio Prot	c0.25			c0.13	0.11		0.00			0.02		
v/s Ratio Perm	0.10	0.03	0.26	0.02	0.05	0.02	0.05	0.03	0.03	c0.12	0.04	0.04
vic Ratio	0.32	0.82	0.10	0.71	0.21	0.03	0.14	0.00	0.08	0.33	0.05	0.12
Uniform Delay, d1	21.3	25.6	19.8	13.3	9.6	8.7	17.5	16.7	17.2	18.9	17.0	17.4
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	0.6	5.6	0.1	6.3	0.1	0.0	0.7	0.0	0.3	2.0	0.2	0.6
Delay (s)	21.9	31.2	20.0	19.6	9.7	8.7	18.2	16.7	17.5	20.9	17.1	18.1
Level of Service	C	C	B	B	A	A	B	B	B	C	B	B
Approach Delay (s)	28.7			13.8			17.7				19.3	
Approach LOS	C			B			B				B	
Intersection Summary												
HCM 2000 Control Delay	22.0 HCM 2000 Level of Service C											
HCM 2000 Volume to Capacity ratio	0.59											
Actuated Cycle Length (s)	79.2 Sum of lost time (s) 13.5											
Intersection Capacity Utilization	52.5% ICU Level of Service A											
Analysis Period (min)	15											
c Critical Lane Group												

2035 Total PM Peak Hour (Lyons Closed).syn  
 2: Trafalgar Road & QEW SB On Ramps 05-04-2022

Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	←	←	←	←	←	←
Traffic Volume (vph)	0	0	0	3420	2072	959
Future Volume (vph)	0	0	0	3420	2072	959
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	0.91	0.91	1.00
Flt Protected						0.850
Satd. Flow (prot)	0	0	0	5085	5085	1583
Flt Permitted						
Satd. Flow (perm)	0	0	0	5085	5085	1583
Link Speed (k/h)	50			50	50	
Link Distance (m)	159.6			186.0	145.2	
Travel Time (s)	11.5			13.4	10.5	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	0	0	0	3717	2252	1042
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	0	0	3717	2252	1042
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Left	Left	Right
Median Width (m)	0.0	0.0	0.0	0.0	0.0	0.0
Link Offset (m)	0.0	0.0	0.0	0.0	0.0	0.0
Crosswalk Width (m)	4.8			4.8	4.8	
Two way Left Turn Lane						
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (k/h)	25	15	25		15	
Sign Control	Stop			Free	Free	
Intersection Summary						
Area Type:	Other					
Control Type:	Unsignalized					
Intersection Capacity Utilization	85.5% ICU Level of Service E					
Analysis Period (min)	15					



HCM Signalized Intersection Capacity Analysis 2035 Total PM Peak Hour (Lyons Closed).syn  
 1: Trafalgar Road & North Service Road/QEW WB Off Ramp 05-04-2022

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	24	0	317	664	158	330	0	2861	559	0	2050	11
Traffic Volume (vph)	24	0	317	664	158	330	0	2861	559	0	2050	11
Future Volume (vph)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Ideal Flow (vphpl)	3.5	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Total Lost time (s)	1.00	1.00	0.95	0.95	1.00	0.91	1.00	0.91	1.00	0.91	1.00	1.00
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	0.98	1.00	0.93	1.00	0.93	1.00	0.93
Frb, ped/bikes	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt	1.00	0.85	1.00	1.00	0.85	1.00	0.85	1.00	0.85	1.00	0.85	1.00
Flt Protected	0.95	1.00	0.95	0.97	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Satd. Flow (prot)	1805	1615	1681	1723	1557	5085	1482	4217	1497	4217	1497	1497
Flt Permitted	0.95	1.00	0.95	0.97	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Satd. Flow (perm)	1805	1615	1681	1723	1557	5085	1482	4217	1497	4217	1497	1497
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	26	0	345	722	172	359	0	3110	608	0	2228	12
RTOR Reduction (vph)	0	0	21	0	0	47	0	138	0	0	0	5
Lane Group Flow (vph)	26	0	324	440	454	312	0	3110	470	0	2228	7
Conf. Peds. (#/hr)	2	0	0	2	14	2	14	14	14	14	14	14
Heavy Vehicles (%)	0%	0%	0%	2%	1%	2%	0%	2%	1%	0%	23%	0%
Turn Type	Prot	Perm	Perm	NA	Perm	NA	Perm	NA	Perm	NA	Perm	NA
Protected Phases	7			8			2				6	
Permitted Phases			4	8		8		2		2		6
Actuated Green, G (s)	3.0	44.3	36.8	36.8	36.8	36.8	81.7	81.7	81.7	81.7	81.7	81.7
Effective Green, g (s)	4.0	47.3	39.8	39.8	39.8	39.8	84.7	84.7	84.7	84.7	84.7	84.7
Actuated g/C Ratio	0.03	0.34	0.28	0.28	0.28	0.28	0.61	0.61	0.61	0.61	0.61	0.61
Clearance Time (s)	4.5	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Lane Grp. Cap (vph)	51	545	477	489	442	442	3076	896	896	2551	905	905
v/s Ratio Prot	0.01						c0.61			0.53		
v/s Ratio Perm		c0.20	0.26	0.26	0.20	0.20	0.32	0.32	0.32	0.32	0.32	0.00
v/c Ratio	0.51	0.60	0.92	0.93	0.71	0.71	1.01	0.52	0.52	0.87	0.01	0.01
Uniform Delay, d1	67.0	38.4	48.6	48.7	44.9	44.9	27.6	16.0	23.2	23.2	11.0	11.0
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	0.63	0.60	0.60	1.00	1.00	1.00
Incremental Delay, d2	7.8	1.8	23.4	23.9	5.1	5.1	13.0	0.9	0.9	4.5	0.0	0.0
Delay (s)	74.8	40.2	72.0	72.6	49.9	49.9	30.4	10.5	10.5	27.7	11.0	11.0
Level of Service	E	D	E	E	D	D	C	B	B	C	C	B
Approach Delay (s)	42.6			65.9			27.1			27.6		
Approach LOS	D			E			C			C		C
Intersection Summary												
HCM 2000 Control Delay												C
HCM 2000 Volume to Capacity ratio	34.4											C
Actuated Cycle Length (s)	140.0											11.5
Intersection Capacity Utilization	91.8%											F
Analysis Period (min)	15											
c Critical Lane Group												

Lanes, Volumes, Timings 2035 Total AM Peak Hour (Lyons Closed).syn  
 11: Argus Rd & South Service Rd 05-04-2022

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	4	4	4	4	4	4				4	4	4
Traffic Volume (vph)	1	5	604	145	145	23				62	62	62
Future Volume (vph)	1	5	604	145	145	23				62	62	62
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor			0.970			0.987				0.902		
Flt Protected			0.988			0.987				0.987		
Satd. Flow (prot)	0	1352	1621	0	1522	0				1522		0
Flt Permitted			0.988			0.987				0.987		
Satd. Flow (perm)	0	1352	1621	0	1522	0				1522		0
Link Speed (k/h)			50			50				50		
Link Distance (m)			177.7			165.2				103.5		
Travel Time (s)			12.8			11.9				7.5		
Conf. Peds. (#/hr)	1					1				5		1
Peak Hour Factor	0.25	0.42	0.86	0.72	0.25	0.25				0.25		0.25
Heavy Vehicles (%)	100%	0%	3%	0%	0%	0%				0%		0%
Adj. Flow (vph)	4	12	702	201	92	248				92		248
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	16	903	0	340	0				340		0
Enter Blocked Intersection	No	No	No	No	No	No				No		No
Lane Alignment	Left	Left	Left	Right	Left	Right				Left		Right
Median Width(m)			0.0		0.0					3.6		
Link Offset(m)			0.0		0.0					0.0		
Crosswalk Width(m)			4.8		4.8					4.8		
Two way Left Turn Lane												
Headway Factor	1.14	1.14	1.14	1.14	1.14	1.14	1.14	1.14	1.14	1.14	1.14	1.14
Turning Speed (k/h)	24									24		14
Sign Control			Free		Free					Stop		
Intersection Summary												
Area Type:	CBD											
Control Type:	Unsignalized											
Intersection Capacity Utilization	57.8%											
Analysis Period (min)	15											
ICU Level of Service B												

HCM Unsignalized Intersection Capacity Analysis: 2035 Total AM Peak Hour (L-lyons Closed).syn  
 11: Argus Rd & South Service Rd

05-04-2022



Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	1	4	1	1	2	2
Traffic Volume (veh/h)	1	5	604	145	23	62
Future Volume (Veh/h)	1	5	604	145	23	62
Sign Control	Free	Free	Free	Stop	Stop	Stop
Grade	0%	0%	0%	0%	0%	0%
Peak Hour Factor	0.25	0.42	0.86	0.72	0.25	0.25
Hourly flow rate (vph)	4	12	702	201	92	248
Pedestrians	1	5	1	1	1	1
Lane Width (m)	3.6	3.6	3.6	3.6	3.6	3.6
Walking Speed (m/s)	1.2	1.2	1.2	1.2	1.2	1.2
Percent Blockage	0	0	0	0	0	0
Right turn flare (veh)						
Median type	None	None	None	None	None	None
Median storage (veh)						
Upstream signal (m)		358				
pX, platoon unblocked						
vC, conflicting volume		904			828	804
vC1, stage 1 conf vol						
vC2, stage 2 conf vol		904			828	804
vCu, unblocked vol		5.1			6.4	6.2
tC, single (s)		3.1			3.5	3.3
tC, 2 stage (s)		99			73	36
tF (s)		464			339	385
p0 queue free %						
cM capacity (veh/h)						

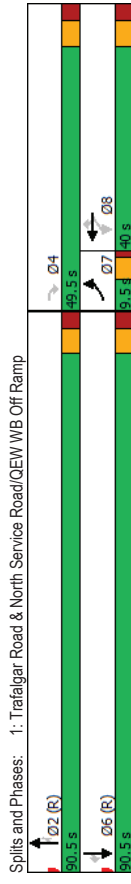
Direction	Lane #	EB 1	WB 1	SB 1
Volume Total		16	903	340
Volume Left		4	0	92
Volume Right		0	201	248
GSH		464	1700	371
Volume to Capacity		0.01	0.63	0.92
Queue Length 95th (m)		0.2	0.0	76.0
Control Delay (s)		3.3	0.0	61.7
Lane LOS		A	F	F
Approach Delay (s)		3.3	0.0	61.7
Approach LOS		F	F	F
<b>Intersection Summary</b>				
Average Delay			16.7	
Intersection Capacity Utilization			57.8%	ICU Level of Service B
Analysis Period (min)			15	

Baseline

Lanes, Volumes, Timings 2035 Total PM Peak Hour (Lyons Closed).syn  
 1: Trafalgar Road & North Service Road/QEW WB Off Ramp

05-04-2022

Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	1	4	1	1	2	2
Traffic Volume (veh/h)	1	5	604	145	23	62
Future Volume (Veh/h)	1	5	604	145	23	62
Sign Control	Free	Free	Free	Stop	Stop	Stop
Grade	0%	0%	0%	0%	0%	0%
Peak Hour Factor	0.25	0.42	0.86	0.72	0.25	0.25
Hourly flow rate (vph)	4	12	702	201	92	248
Pedestrians	1	5	1	1	1	1
Lane Width (m)	3.6	3.6	3.6	3.6	3.6	3.6
Walking Speed (m/s)	1.2	1.2	1.2	1.2	1.2	1.2
Percent Blockage	0	0	0	0	0	0
Right turn flare (veh)						
Median type	None	None	None	None	None	None
Median storage (veh)						
Upstream signal (m)		358				
pX, platoon unblocked						
vC, conflicting volume		904			828	804
vC1, stage 1 conf vol						
vC2, stage 2 conf vol		904			828	804
vCu, unblocked vol		5.1			6.4	6.2
tC, single (s)		3.1			3.5	3.3
tC, 2 stage (s)		99			73	36
tF (s)		464			339	385
p0 queue free %						
cM capacity (veh/h)						



Splits and Phases: 1: Trafalgar Road & North Service Road/QEW WB Off Ramp

Phase	Duration (s)	Direction
Ø2 (R)	90.5	Right
Ø6 (R)	90.5	Right
Ø4	90.5	Through
Ø7	90.5	Through
Ø8	90.5	Through
Ø9	90.5	Through

Intersection Capacity Utilization 91.8%  
 Analysis Period (min) 15  
 ICU Level of Service F

~ Volume exceeds capacity, queue is theoretically infinite.  
 Queue shown is maximum after two cycles.

# 95th percentile volume exceeds capacity, queue may be longer.  
 Queue shown is maximum after two cycles.

m Volume for 95th percentile queue is metered by upstream signal.

Baseline

Lanes, Volumes, Timings  
1: Trafalgar Road & North Service Road/QEW WB Off Ramp

Lanes, Volumes, Timings  
12: Site Driveway

2035 Total PM Peak Hour (Lyons Closed).syn  
05-04-2022

2035 Total AM Peak Hour (Lyons Closed).syn  
05-04-2022

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Turn Type	Prot	Perm	Perm	Perm	Perm	Perm	NA	NA	NA	NA	NA	Perm
Protected Phases	7											
Permitted Phases		4	8	8	8	8	2	2	2	2	6	6
Detector Phase	7	4	8	8	8	8	2	2	2	2	6	6
Switch Phase												
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Minimum Split (s)	9.5	38.0	38.0	38.0	38.0	38.0	35.0	35.0	35.0	35.0	35.0	35.0
Total Split (s)	9.5	49.5	40.0	40.0	40.0	40.0	90.5	90.5	90.5	90.5	90.5	90.5
Total Split (%)	6.8%	35.4%	28.6%	28.6%	28.6%	28.6%	64.6%	64.6%	64.6%	64.6%	64.6%	64.6%
Maximum Green (s)	5.0	42.5	33.0	33.0	33.0	33.0	83.5	83.5	83.5	83.5	83.5	83.5
Yellow Time (s)	3.5	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
All-Red Time (s)	1.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Lost Time Adjust (s)	-1.0	-3.0	-3.0	-3.0	-3.0	-3.0	-3.0	-3.0	-3.0	-3.0	-3.0	-3.0
Total Lost Time (s)	3.5	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lead/Lag	Lead	Lag	Lag	Lag	Lag	Lag	Lag	Lag	Lag	Lag	Lag	Lag
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	None	None	None	None	None	None	C-Max	C-Max	C-Max	C-Max	C-Max	C-Max
Walk Time (s)		7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0
Flash Dont Walk (s)		11.0	11.0	11.0	11.0	11.0	11.0	11.0	11.0	11.0	11.0	11.0
Pedestrian Calls (#/hr)		0	0	0	0	0	0	0	0	0	0	0
Act Effct Green (s)	6.0	45.5	39.8	39.8	39.8	39.8	86.5	86.5	86.5	86.5	86.5	86.5
Actuated g/C Ratio	0.04	0.32	0.28	0.28	0.28	0.28	0.62	0.62	0.62	0.62	0.62	0.62
v/c Ratio	0.34	0.63	0.92	0.93	0.73	0.73	0.99	0.58	0.86	0.01	0.86	0.01
Control Delay	77.2	42.5	74.7	75.3	47.6	47.6	25.3	4.5	26.0	0.0	26.0	0.0
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	2.3	0.0	0.0	0.0	0.0	0.0
Total Delay	77.2	42.5	74.7	75.3	47.6	47.6	27.6	4.5	26.0	0.0	26.0	0.0
LOS	E	D	E	E	D	D	C	A	C	A	C	A
Approach Delay	44.9			67.1			23.8				25.8	
Approach LOS	D			E			C				C	
Queue Length 50th (m)	7.5	78.0	~142.5	~148.0	81.8	81.8	210.5	22.5	22.5	182.8	182.8	0.0
Queue Length 95th (m)	18.3	113.6	#215.0	#221.1	#130.2	#130.2	#362.0	m23.2	m23.2	207.3	207.3	0.0
Internal Link Dist (m)		96.4		141.1			121.2			185.2		
Turn Bay Length (m)	50.0									70.0		
Base Capacity (vph)	77	545	478	489	489	489	3141	1049	2605	950	2605	950
Starvation Cap Reductn	0	0	0	0	0	0	35	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.34	0.63	0.92	0.93	0.73	0.73	1.00	0.58	0.86	0.01	0.86	0.01
Intersection Summary	Intersection LOS: C											
Area Type:	Other											
Actuated Cycle Length: 140	Cycle Length: 140											
Offset: 0 (0%); Referenced to phase 2:NBT and 6:SBT; Start of Green												
Natural Cycle: 135	Natural Cycle: 135											
Control Type: Actuated-Coordinated	Control Type: Actuated-Coordinated											
Maximum v/c Ratio: 0.99	Maximum v/c Ratio: 0.99											
Intersection Signal Delay: 32.6	Intersection Signal Delay: 32.6											

Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	EBT	EBR	WBL	WBT	NBL	NBR
Traffic Volume (vph)	27	0	26	56	0	86
Future Volume (vph)	27	0	26	56	0	86
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt					0.865	
Flt Protected					0.985	
Satd. Flow (prot)	1863	0	0	1835	1611	0
Flt Permitted					0.985	
Satd. Flow (perm)	1863	0	0	1835	1611	0
Link Speed (k/h)	50			50	50	
Link Distance (m)	103.3			722.4	44.4	
Travel Time (s)	7.4			52.0	3.2	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	29	0	28	61	0	93
Shared Lane Traffic (%)						
Lane Group Flow (vph)	29	0	0	89	93	0
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Left	Left	Left	Left	Right
Median Width (m)	0.0	0.0	0.0	0.0	3.6	
Link Offset (m)	0.0	0.0	0.0	0.0	0.0	
Crosswalk Width (m)	4.8			4.8	4.8	
Two way Left Turn Lane						
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (k/h)		15	25	25	25	15
Sign Control	Free	Free	Free	Free	Stop	Stop
Intersection Summary	Other					
Area Type:	Other					
Control Type: Unsignalized	Control Type: Unsignalized					
Intersection Capacity Utilization 23.0%	Intersection Capacity Utilization 23.0%					
Analysis Period (min) 15	Analysis Period (min) 15					
ICU Level of Service A	ICU Level of Service A					

HCM Unsignalized Intersection Capacity Analysis: 2035 Total AM Peak Hour (L-lyons Closed).syn  
 12: Site Driveway

05-04-2022

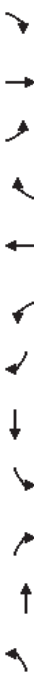


Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	27	0	26	56	0	86
Traffic Volume (veh/h)	27	0	26	56	0	86
Future Volume (Veh/h)	Free	Stop	Free	Stop	0%	0%
Sign Control	0%	0%	0%	0%	0%	0%
Grade	0.92	0.92	0.92	0.92	0.92	0.92
Peak Hour Factor	29	0	28	61	0	93
Hourly flow rate (vph)						
Pedestrians						
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)						
Median type			None			
Median storage (veh)						
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume						
vC1, stage 1 conf/vol			29		146	29
vC2, stage 2 conf/vol						
tC, single (s)			29		146	29
tC, 2 stage (s)			4.1		6.4	6.2
tF (s)			2.2		3.5	3.3
p0 queue free %			98		100	91
cM capacity (veh/h)			1584		831	1046
Direction, Lane #	EB 1	WB 1	NB 1			
Volume Total	29	89	93			
Volume Left	0	28	0			
Volume Right	0	0	93			
GSH	1700	1584	1046			
Volume to Capacity	0.02	0.02	0.09			
Queue Length 95th (m)	0.0	0.4	2.3			
Control Delay (s)	0.0	2.4	8.8			
Lane LOS	A	A	A			
Approach Delay (s)	0.0	2.4	8.8			
Approach LOS	A	A	A			
Intersection Summary						
Average Delay	4.9					A
Intersection Capacity Utilization	23.0%			ICU Level of Service		
Analysis Period (min)	15					

Baseline

Lanes, Volumes, Timings  
 1: Trafalgar Road & North Service Road/QEW WB Off Ramp

2035 Total PM Peak Hour (L-lyons Closed).syn  
 05-04-2022



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	24	0	317	664	158	330	0	2861	559	0	2050	11
Traffic Volume (vph)	24	0	317	664	158	330	0	2861	559	0	2050	11
Future Volume (vph)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Ideal Flow (vphpl)	50.0	0.0	0.0	0.0	0.0	0.0	0.0	70.0	0.0	0.0	0.0	0.0
Storage Length (m)	1	1	1	1	1	1	1	0	1	0	1	1
Storage Lanes	7.5	1.0	1.0	0.95	0.95	1.00	1.00	0.91	1.00	1.00	0.91	1.00
Taper Length (m)	1.00	1.00	1.00	0.98	0.98	0.98	0.98	0.93	0.93	0.93	0.93	0.93
Ped Bike Factor	0.850	0.850	0.850	0.850	0.850	0.850	0.850	0.850	0.850	0.850	0.850	0.850
Fit Protected	0.950	0.950	0.970	0.950	0.970	0.950	0.970	0.950	0.970	0.950	0.970	0.950
Satd. Flow (prot)	1805	0	1615	1681	1723	1583	0	5085	1599	0	4217	1615
Fit Permitted	0.950	0.950	0.970	0.950	0.970	0.950	0.970	0.950	0.970	0.950	0.970	0.950
Satd. Flow (perm)	1801	0	1615	1681	1723	1557	0	5085	1482	0	4217	1497
Right Turn on Red	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Satd. Flow (RTOR)	31	31	31	66	66	66	66	350	350	350	50	66
Link Speed (k/h)	50	50	50	50	50	50	50	50	50	50	50	50
Link Distance (m)	120.4	120.4	120.4	165.1	165.1	165.1	165.1	145.2	145.2	145.2	209.2	165.1
Travel Time (s)	8.7	8.7	8.7	11.9	11.9	11.9	11.9	10.5	10.5	10.5	15.1	11.9
Confl. Peds. (#/hr)	2	2	2	14	14	14	14	14	14	14	14	14
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	0%	0%	0%	2%	2%	2%	2%	1%	2%	1%	0%	23%
Adj. Flow (vph)	26	0	345	722	172	359	0	3110	608	0	2228	12
Shared Lane Traffic (%)				39%	39%	39%	39%					
Lane Group Flow (vph)	26	0	345	440	454	359	0	3110	608	0	2228	12
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width (m)	3.6	3.6	3.6	3.6	3.6	3.6	3.6	3.6	3.6	3.6	3.6	3.6
Link Offset (m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Crosswalk Width (m)	4.8	4.8	4.8	4.8	4.8	4.8	4.8	4.8	4.8	4.8	4.8	4.8
Two way Left Turn Lane												
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (k/h)	25	15	25	15	25	15	25	15	25	15	25	15
Number of Detectors	1	1	1	2	1	2	1	2	1	2	1	1
Detector Template	Left	Right	Left	Thru	Right	Thru	Right	Thru	Right	Thru	Right	Right
Leading Detector (m)	2.0	2.0	2.0	10.0	2.0	10.0	2.0	10.0	2.0	10.0	2.0	2.0
Trailing Detector (m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Position (m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Size (m)	2.0	2.0	2.0	0.6	2.0	0.6	2.0	0.6	2.0	0.6	2.0	2.0
Detector 1 Type	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex	Ch+Ex
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 2 Position (m)				9.4		9.4		9.4		9.4		9.4
Detector 2 Size (m)				0.6		0.6		0.6		0.6		0.6
Detector 2 Type				Ch+Ex		Ch+Ex		Ch+Ex		Ch+Ex		Ch+Ex
Detector 2 Channel												
Detector 2 Extend (s)				0.0		0.0		0.0		0.0		0.0

Baseline

# Appendix H

## Sensitivity Analysis Operations Reports



HCM Unsignalized Intersection Capacity Analysis  
 11: Argus Rd & South Service Rd

Lanes, Volumes, Timings  
 11: Argus Rd & South Service Rd

2035 Sensitivity PM Peak Hour.syn  
 05-04-2022

2035 Sensitivity AM Peak Hour.syn  
 05-04-2022



Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	0	10	361	151	0	74
Traffic Volume (veh/h)	0	10	361	151	0	74
Future Volume (Veh/h)	0	10	361	151	0	74
Sign Control	Free	Free	Free	Free	Stop	0%
Grade	0%	0%	0%	0%	0%	0%
Peak Hour Factor	0.45	0.50	0.87	0.68	0.75	0.65
Hourly flow rate (vph)	0	20	415	222	0	114
Pedestrians						
Lane Width (m)			3.6			
Walking Speed (m/s)			1.2			
Percent Blockage			0			
Right turn flare (veh)			None	None		
Median type			None	None		
Median storage (veh)			358			
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume			637			551
vC1, stage 1 conf/vol						526
vC2, stage 2 conf/vol						
vCu, unblocked vol			637			551
tC, single (s)			4.1			6.4
tC, 2 stage (s)						6.2
tF (s)			2.2			3.5
p0 queue free %			100			100
cM capacity (veh/h)			956			497
Direction, Lane #	EB 1	WB 1	SB 1			
Volume Total	20	637	114			
Volume Left	0	0	0			
Volume Right	0	222	114			
GSH	1700	1700	556			
Volume to Capacity	0.01	0.37	0.21			
Queue Length 95th (m)	0.0	0.0	6.1			
Control Delay (s)	0.0	0.0	13.1			
Lane LOS			B			
Approach Delay (s)	0.0	0.0	13.1			
Approach LOS			B			
Intersection Summary						
Average Delay			1.9			
Intersection Capacity Utilization			43.1%			ICU Level of Service A
Analysis Period (min)			15			



Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	0	6	740	173	0	78
Traffic Volume (vph)	0	6	740	173	0	78
Future Volume (vph)	0	6	740	173	0	78
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor			0.971			0.865
Flt Protected			0	1710	1622	0
Satd. Flow (prot)			0	1710	1622	0
Flt Permitted			0	1710	1622	0
Satd. Flow (perm)			0	1710	1622	0
Link Speed (k/h)			50	50	50	50
Link Distance (m)			177.7	165.2	103.5	
Travel Time (s)			12.8	11.9	7.5	
Conf. Peds. (#/hr)			1	1	5	1
Peak Hour Factor			0.25	0.42	0.86	0.72
Heavy Vehicles (%)			100%	0%	3%	0%
Adj. Flow (vph)			0	14	860	240
Shared Lane Traffic (%)						
Lane Group Flow (vph)			0	14	1100	0
Enter Blocked Intersection			No	No	No	No
Lane Alignment			Left	Left	Right	Right
Median Width(m)			0.0	0.0	0.0	0.0
Link Offset(m)			0.0	0.0	0.0	0.0
Crosswalk Width(m)			4.8	4.8	4.8	4.8
Two way Left Turn Lane			1.14	1.14	1.14	1.14
Headway Factor			24	14	24	14
Turning Speed (k/h)			Free	Free	Stop	Stop
Sign Control			Free	Free	Stop	Stop
Intersection Summary						
Area Type:						CBD
Control Type:						Unsignalized
Intersection Capacity Utilization						ICU Level of Service C
Analysis Period (min)						15

HCM Unsignalized Intersection Capacity Analysis  
 11: Argus Rd & South Service Rd

Lanes, Volumes, Timings  
 11: Argus Rd & South Service Rd

2035 Sensitivity AM Peak Hour.syn  
 05-04-2022

2035 Sensitivity PM Peak Hour.syn  
 05-04-2022



Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations						
Traffic Volume (veh/h)	0	6	740	173	0	78
Future Volume (Veh/h)	0	6	740	173	0	78
Sign Control	Free	Free	Free	Free	Stop	Stop
Grade	0%	0%	0%	0%	0%	0%
Peak Hour Factor	0.25	0.42	0.86	0.72	0.25	0.25
Hourly flow rate (vph)	0	14	860	240	0	312
Pedestrians	1	5			1	
Lane Width (m)	3.6	3.6			3.6	
Walking Speed (m/s)	1.2	1.2			1.2	
Percent Blockage	0	0			0	
Right turn flare (veh)						
Median type	None	None				
Median storage (veh)						
Upstream signal (m)		358				
pX, platoon unblocked						
vC, conflicting volume	1101				1000	982
vC1, stage 1 conf/vol						
vC2, stage 2 conf/vol						
vCu, unblocked vol	1101				1000	982
tC, single (s)	5.1				6.4	6.2
tC, 2 stage (s)						
tF (s)	3.1				3.5	3.3
p0 queue free %	100				100	0
cM capacity (veh/h)	377				270	304
Direction, Lane #	EB 1	WB 1	SB 1			
Volume Total	14	1100	312			
Volume Left	0	0	0			
Volume Right	0	240	312			
GSH	1700	1700	304			
Volume to Capacity	0.01	0.65	1.02			
Queue Length 95th (m)	0.0	0.0	90.4			
Control Delay (s)	0.0	0.0	96.5			
Lane LOS	F	F	F			
Approach Delay (s)	0.0	0.0	96.5			
Approach LOS	F	F	F			
Intersection Summary						
Average Delay	21.1					
Intersection Capacity Utilization	67.4%					
Analysis Period (min)	15					
	ICU Level of Service C					

Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations						
Traffic Volume (vph)	0	10	361	151	0	74
Future Volume (vph)	0	10	361	151	0	74
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor			0.953			0.865
Flt Protected						
Satd. Flow (prot)	0	1513	1530	0	0	1479
Flt Permitted						
Satd. Flow (perm)	0	1513	1530	0	0	1479
Link Speed (k/h)		50	50		50	
Link Distance (m)		177.7	165.2		103.5	
Travel Time (s)		12.8	11.9		7.5	
Conf. Peds. (#/hr)					5	
Peak Hour Factor	0.45	0.50	0.87	0.68	0.75	0.65
Heavy Vehicles (%)	0%	13%	10%	0%	0%	0%
Adj. Flow (vph)	0	20	415	222	0	114
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	20	637	0	0	114
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Left	Left	Right	Left	Right
Median Width(m)	0.0	0.0	0.0	0.0	0.0	0.0
Link Offset(m)	0.0	0.0	0.0	0.0	0.0	0.0
Crosswalk Width(m)	4.8	4.8	4.8	4.8	4.8	4.8
Two way Left Turn Lane						
Headway Factor	1.14	1.14	1.14	1.14	1.14	1.14
Turning Speed (k/h)	24			14	24	14
Sign Control	Free	Free	Free	Free	Stop	Stop
Intersection Summary						
Area Type:	CBD					
Control Type:	Unsignalized					
Intersection Capacity Utilization	43.1%					
Analysis Period (min)	15					
	ICU Level of Service A					